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Note: Addenda information is NOT included with the electronic documents available via electronic file transfer. Only bidder or non-bidder package holders listed with the Caltrans Plans and Bid Documents section as described above will receive addenda information.

STATE OF CALIFORNIA

DEPARTMENT OF TRANSPORTATION

NOTICE TO CONTRACTORS

AND

SPECIAL PROVISIONS

FOR CONSTRUCTION ON STATE HIGHWAY IN

ALAMEDA COUNTY IN HAYWARD FROM 0.1 km WEST OF THE SAN MATEO-HAYWARD BRIDGE TOLL PLAZA TO 0.2 km WEST OF ROUTE 880/92 SEPARATION

DISTRICT 04, ROUTE 92

For Use in Connection with Standard Specifications Dated JULY 1995, Standard Plans Dated JULY 1997, and Labor Surcharge and Equipment Rental Rates.

CONTRACT NO. 04-045034

04-Ala-92-3.7/10.3

Federal Aid Project

ACNH-P092 (033)E

Bids Open: April 11, 2000

Dated: February 14, 2000

QCQA
OSD

****

IMPORTANT SPECIAL NOTICES

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- The bidder's attention is directed to Section 5, containing specifications for "Disputes Review Board," of the Special Provisions, regarding establishing a Disputes Review Board (DRB) for the project.

- Bidder inquiries are to be directed to the District Construction Office at:

  Telephone No.:(510) 286-5209 E-mail: DUTY_SENIOR_DISTRICT04@dot.ca.gov
  FAX No.:(510) 286-5171

- The specifications for this project include Quality Control / Quality Assurance provisions for the contract item "Asphalt Concrete" in the Special Provisions. In addition to the project special provisions for asphalt concrete in Section 10-1 of the Special Provisions, Section 11-1, "Asphalt Concrete," of the Special Provisions replaces Section 39 of the Standard Specifications for the contract item "Asphalt Concrete."

- The specifications for this project require the production of asphalt-rubber binder. The Air Quality Management District must approve the production and placement of asphalt-rubber binder and rubberized asphalt concrete, in addition to the Operating Permits required to produce asphalt concrete. Air Quality Management Districts have stopped production of these products due to failure to obtain necessary approvals.

- The Special Provisions for Federal-aid projects (with and without DBE goals) have been revised to incorporate changes made by new regulations governing the DBE Program (49 CFR Part 26).
  Sections 2 and 5 incorporate the changes. Bidders should read these sections to become familiar with them. Attention is directed to the following significant changes:

  Section 2, "Disadvantaged Business Enterprise (DBE)" revises the counting of participation by DBE primes, and the counting of trucking performed by DBE firms. The section also revises the information that must be submitted to the Department in order to receive credit for trucking.

  Section 2, "Submission of DBE Information" revises the information required to be submitted to the Department to receive credit toward the DBE goal. It also revises the criteria to demonstrate good faith efforts.

  Section 5, "Subcontractor and DBE Records" revises the information required to be reported at the end of the project, and information related to trucking that must be submitted throughout the project.

  Section 5, "DBE Certification Status" adds new reporting requirements related to DBE certification.

  Section 5, "Subcontracting" describes the efforts that must be made in the event a DBE subcontractor is terminated or fails to complete its work for any reason.

  Section 5, "Prompt Progress Payment to Subcontractors" requires prompt payment to all subcontractors.

  Section 5, "Prompt Payment of Withheld Funds to Subcontractors" requires the prompt payment of retention to all subcontractors.

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STANDARD PLANS LIST

The Standard Plan sheets applicable to this contract include, but are not limited to those indicated below. The Revised Standard Plans (RSP) and New Standard Plans (NSP) which apply to this contract are included as individual sheets of the project plans.

A10A	Abbreviations
A10B	Symbols
A20A	Pavement Markers and Traffic Lines - Typical Details
A20B	Pavement Markers and Traffic Lines - Typical Details
A20C	Pavement Markers and Traffic Lines - Typical Details
A20D	Pavement Markers and Traffic Lines - Typical Details
A24B	Pavement Markings - Arrows
A24C	Pavement Markings - Symbols and Numerals
A24D	Pavement Markings - Words
A24E	Pavement Markings - Words and Crosswalks
A35A	Portland Cement Concrete Paving Details
A62C	Limits of Payment for Excavation and Backfill - Bridge
A62D	Excavation and Backfill - Concrete Pipe Culverts
A73C	Delineators, Channelizers and Barricades
RSP A76A	*Concrete Barrier Type 60*
A76B	Concrete Barrier Type 60
A76C	Concrete Barrier Type 60E
RSP A77A	*Metal Beam Guard Railing - Wood Posts and Wood Blocks*
RSP A77B	*Metal Beam Guard Railing - Standard Hardware*
RSP A77C	*Metal Beam Guard Railing - Wood Posts and Wood Blocks*
RSP A77F	*Metal Beam Guard Railing - Typical Embankment Widening for End Treatments*
NSP A77L	*Guard Railing and Barrier Railing End Treatment*
NSP A77M	*Guard Railing and Barrier Railing End Treatment*
NSP A77N	*Guard Railing and Barrier Railing End Treatment*
A78A	Thrie Beam Barrier
RSP A78B	*Thrie Beam Barrier*
A78C	Thrie Beam Barrier - Standard Hardware
RSP A78E	*Thrie Beam Barrier - End Anchors and End Treatments*
NSP A81A	*Crash Cushion, Sand Filled (Unidirectional)*
NSP A81C	*Crash Cushion, Sand Filled (Bidirectional)*
NSP A82	*Crash Cushion (Type CAT)*
A85	Chain Link Fence
A87	Curbs, Dikes and Driveways
D73	Drainage Inlets
D74A	Drainage Inlets
D74B	Drainage Inlets
RSP D77A	*Grate Details*
D78	Gutter Depressions
D87B	Overside Drains
D87C	Underdrains
D98A	Slotted Corrugated Steel Pipe Drain Details
D98B	Slotted Corrugated Steel Pipe Drain Details
H1	Planting and Irrigation - Abbreviations
RSP H2	*Planting and Irrigation - Symbols*
H7	Planting and Irrigation - Details
H8	Planting and Irrigation - Details
NSP T1A	*Temporary Crash Cushion, Sand Filled (Unidirectional)*
NSP T1B	*Temporary Crash Cushion, Sand Filled (Bidirectional)*
RSP T2	*Temporary Crash Cushion, Sand Filled (Shoulder Installations)*
T3	Temporary Railing (Type K)

RSP T7	*Construction Project Information Signs*
T10	Traffic Control System for Lane Closure On Freeways and Expressways
T12	Traffic Control System for Lane Closure On Multilane Conventional Highways
T13	Traffic Control System for Lane Closure On Two Lane Conventional Highways
T14	Traffic Control System for Ramp Closures
B0-1	Bridge Details
B0-3	Bridge Details
B0-5	Bridge Details
B0-13	Bridge Details
B2-6	Pile Details - Class 400C and Class 625C
RSP B3-8	*Retaining Wall Details No. 1*
B3-9	Retaining Wall Details No. 2
B6-10	Utility Openings - T-Beam
B6-21	Joint Seals (Maximum Movement Rating = 50 mm)
B7-1	Box Girder Details
B7-6	Deck Drains - Types D-1 and D-2
B7-11	Utility Details
RSP B11-52	*Chain Link Railing Type 7*
B11-53	Concrete Barrier Type 25
B14-3	Communication and Sprinkler Control Conduits (Conduit Less Than Size 103)
B14-4	Water Supply Line (Bridge) (Pipe Less Than NPS 4)
B14-5	Water Supply Line (Details) (Pipe Less Than NPS 4)
RS1	Roadside Signs - Typical Installation Details No. 1
RS2	Roadside Signs - Wood Post, Typical Installation Details No. 2
RS4	Roadside Signs - Typical Installation Details No. 4
S1	Overhead Signs-Truss - Instructions and Examples
S2	Overhead Signs-Truss - Single Post Type, Post Types II through VII
S3	Overhead Signs-Truss - Two Post Type, Post Types I-S through VII-S
RSP S4	*Overhead Signs-Truss - Single Post Type, Structural Frame Members*
S5	Overhead Signs-Truss - Two Post Type, Structural Frame Members
S6	Overhead Signs-Truss - Structural Frame Details
S7	Overhead Signs-Truss - Frame Juncture Details
S8B	Overhead Signs - Removable Sign Panel Frames, Overhead Formed Panel Mounting Details
S9	Overhead Signs - Walkway Details No. 1
S10	Overhead Signs - Walkway Details No. 2
RSP S11	*Overhead Signs - Walkway Safety Railing Details*
RSP S13	*Overhead Signs-Truss - Pile Foundation*
ES-1A	Signal, Lighting and Electrical Systems - Symbols and Abbreviations
ES-1B	Signal, Lighting and Electrical Systems - Symbols and Abbreviations
ES-2A	Signal, Lighting and Electrical Systems - Service Equipment
ES-2C	Signal, Lighting and Electrical Systems - Service Equipment Notes
ES-2D	Signal, Lighting and Electrical Systems - Service Equipment and Typical Wiring Diagram, Type A
ES-4B	Signal, Lighting and Electrical Systems - Controller Cabinet Details
ES-4D	Irrigation Controller Enclosure Cabinet
ES-6B	Lighting Standards - Types 15, 21 and 22
ES-6D	Lighting Standards - Types 30 and 31
ES-6S	Signal and Lighting Standards - Details No. 1
ES-6T	Signal and Lighting Standards - Details No. 2
ES-7A	Signal, Lighting and Electrical Systems - Electrical Details, Structure Installations
ES-7B	Signal, Lighting and Electrical Systems - Electrical Details, Structure Installations
ES-8	Signal, Lighting and Electrical Systems - Pull Box Details
ES-10	Signal, Lighting and Electrical Systems - Isolux Diagrams
ES-11	Signal, Lighting and Electrical Systems - Foundation Installations
ES-13	Signal, Lighting and Electrical Systems - Splicing Details
ES-14	Signal, Lighting and Electrical Systems - Wiring Details and Fuse Ratings

ES-29	Sign Illumination - Mercury Sign Illumination Equipment
ES-32A	Sign Illumination - Sign Illumination Equipment
ES-32B	Sign Illumination - Sign Illumination Control
RSP ES-34A	*Closed Circuit Television - Pole Details*
ES-34B	Closed Circuit Television - Pole Details, Overhead Sign Mounted

DEPARTMENT OF TRANSPORTATION

NOTICE TO CONTRACTORS

CONTRACT NO. 04-045034

04-Ala-92-3.7/10.3

Sealed proposals for the work shown on the plans entitled:

STATE OF CALIFORNIA; DEPARTMENT OF TRANSPORTATION; PROJECT PLANS FOR CONSTRUCTION ON STATE HIGHWAY IN ALAMEDA COUNTY IN HAYWARD FROM 0.1 km WEST OF THE SAN MATEO-HAYWARD BRIDGE TOLL PLAZA TO 0.2 km WEST OF ROUTE 880/92 SEPARATION

will be received at the Department of Transportation, 1120 N Street, Room 0200, MS #26, Sacramento, CA 95814, until 2 o'clock p.m. on April 11, 2000, at which time they will be publicly opened and read in Room 0100 at the same address.

Proposal forms for this work are included in a separate book entitled:

STATE OF CALIFORNIA; DEPARTMENT OF TRANSPORTATION; PROPOSAL AND CONTRACT FOR CONSTRUCTION ON STATE HIGHWAY IN ALAMEDA COUNTY IN HAYWARD FROM 0.1 km WEST OF THE SAN MATEO-HAYWARD BRIDGE TOLL PLAZA TO 0.2 km WEST OF ROUTE 880/92 SEPARATION

General work description: Existing 4-lane highway to be widened to 6-lanes by grading and surfacing with rubberized asphalt concrete; 2 bridges widened and retaining walls and toll booths to be constructed.

This project has a goal of 16 percent disadvantaged business enterprise (DBE) participation.

No prebid meeting is scheduled for this project.

THIS PROJECT IS SUBJECT TO THE "BUY AMERICA" PROVISIONS OF THE SURFACE TRANSPORTATION ASSISTANCE ACT OF 1982 AS AMENDED BY THE INTERMODAL SURFACE TRANSPORTATION EFFICIENCY ACT OF 1991.

Bids are required for the entire work described herein.

At the time this contract is awarded, the Contractor shall possess either a Class A license or a combination of Class C licenses which constitutes a majority of the work.

This contract is subject to state contract nondiscrimination and compliance requirements pursuant to Government Code, Section 12990.

Project plans, special provisions, and proposal forms for bidding this project can only be obtained at the Department of Transportation, Plans and Bid Documents, Room 0200, MS #26, Transportation Building, 1120 N Street, Sacramento, California 95814, FAX No. (916) 654-7028, Telephone No. (916) 654-4490. Use FAX orders to expedite orders for project plans, special provisions and proposal forms. FAX orders must include credit card charge number, card expiration date and authorizing signature. Project plans, special provisions, and proposal forms may be seen at the above Department of Transportation office and at the offices of the District Directors of Transportation at Irvine, Oakland, and the district in which the work is situated. Standard Specifications are available through the State of California, Department of Transportation, Publications Unit, 1900 Royal Oaks Drive, Sacramento, CA 95815, Telephone No. (916) 445-3520.

Cross sections for this project are available at the office of the District Director of Transportation of the district in which the work is situated.

The successful bidder shall furnish a payment bond and a performance bond.

The Department of Transportation hereby notifies all bidders that it will affirmatively insure that in any contract entered into pursuant to this advertisement, disadvantaged business enterprises will be afforded full opportunity to submit bids in response to this invitation.

The U.S. Department of Transportation (DOT) provides a toll-free "hotline" service to report bid rigging activities. Bid rigging activities can be reported Mondays through Fridays, between 8:00 a.m. and 5:00 p.m., eastern time, Telephone No. 1-800-424-9071. Anyone with knowledge of possible bid rigging, bidder collusion, or other fraudulent activities should use the "hotline" to report these activities. The "hotline" is part of the DOT's continuing effort to identify and investigate highway construction contract fraud and abuse and is operated under the direction of the DOT Inspector General. All information will be treated confidentially and caller anonymity will be respected.

Pursuant to Section 1773 of the Labor Code, the general prevailing wage rates in the county, or counties, in which the work is to be done have been determined by the Director of the California Department of Industrial Relations. These wages are set forth in the General Prevailing Wage Rates for this project, available at the Labor Compliance Office at the offices of the District Director of Transportation for the district in which the work is situated, and available from the California Department of Industrial Relations' Internet Web Site at: http://www.dir.ca.gov. The Federal minimum wage rates for this project as predetermined by the United States Secretary of Labor are set forth in the books issued for bidding purposes entitled "Proposal and Contract," and in copies of this book that may be examined at the offices described above where project plans, special provisions, and proposal forms may be seen. Addenda to modify the Federal minimum wage rates, if necessary, will be issued to holders of "Proposal and Contract" books. Future effective general prevailing wage rates which have been predetermined and are on file with the California Department of Industrial Relations are referenced but not printed in the general prevailing wage rates.

Attention is directed to the Federal minimum wage rate requirements in the books entitled "Proposal and Contract." If there is a difference between the minimum wage rates predetermined by the Secretary of Labor and the general prevailing wage rates determined by the Director of the California Department of Industrial Relations for similar classifications of labor, the Contractor and subcontractors shall pay not less than the higher wage rate. The Department will not accept lower State wage rates not specifically included in the Federal minimum wage determinations. This includes "helper" (or other classifications based on hours of experience) or any other classification not appearing in the Federal wage determinations. Where Federal wage determinations do not contain the State wage rate determination otherwise available for use by the Contractor and subcontractors, the Contractor and subcontractors shall pay not less than the Federal minimum wage rate which most closely approximates the duties of the employees in question.

Bidder inquiries may be made as follows:

The Department will consider bidder inquiries only when a completed "Bidder Inquiry" form is submitted. A copy of the "Bidder Inquiry" form is available at the Internet address shown below. The bidder inquiry shall include the bidder's name and telephone number. Submit "Bidder Inquiry" forms to :

Construction Program Duty Senior
111 Grand Avenue
Oakland, CA 94612

Fax Number: (510) 286-5171
E-mail: DUTY_SENIOR_DISTRICT04@ dot.ca.gov
Tel. Number: (510) 286-5209

To expedite processing, submittal of "Bidder Inquiry" forms via Fax or E-mail is preferred.

To the extent feasible and at the discretion of the Department, completed "Bidder Inquiry" forms submitted for consideration will be investigated, and responses will be posted on the Internet at:

http://www.dot.ca.gov/dist4/construction/CONTRACTINQUIRIES.html

The responses to bidders' inquiries, unless incorporated into formal addenda to the contract, are not a part of the contract, and are provided for the bidder's convenience only. In some instances, the question and answer may represent a summary of the matters discussed rather than a word-for-word recitation. The availability or use of information provided in the responses to bidders' inquiries is not to be construed in any way as a waiver of the provisions of Section 2-1.03 of the Standard Specifications or any other provision of the contract, the plans, Standard Specifications or Special Provisions, nor to excuse the contractor from full compliance with those contract requirements. Bidders are cautioned that subsequent responses or contract addenda may affect or vary a response previously given.

DEPARTMENT OF TRANSPORTATION

Deputy Director Transportation Engineering

Dated February 14, 2000

EFO

COPY OF ENGINEER'S ESTIMATE

(NOT TO BE USED FOR BIDDING PURPOSES)

04-045034

Item	Item Code	Item	Unit of Measure	Estimated Quantity
1 (S)	017354	ELECTRONIC MOBILE DAILY DIARY COMPUTER SYSTEM DATA DELIVERY	LS	LUMP SUM
2	017355	TIME RELATED OVERHEAD	WDAY	410
3	070010	PROGRESS SCHEDULE (CRITICAL PATH)	LS	LUMP SUM
4	074019	PREPARE STORM WATER POLLUTION PREVENTION PLAN	LS	LUMP SUM
5	074020	WATER POLLUTION CONTROL	LS	LUMP SUM
6 (S)	017356	TEMPORARY COVERED PEDESTRIAN WALKWAY	LS	LUMP SUM
7	017357	TEMPORARY COVER	M2	4000
8	017358	TEMPORARY DRAINAGE INLET PROTECTION	EA	18
9	017359	TEMPORARY DRAINAGE OUTLET PROTECTION (TYPE EC1)	M	260
10	017360	TEMPORARY CONCRETE WASHOUT	EA	6
11	017361	TEMPORARY HIGH VISIBILITY BOUNDARY FENCE	M	3000
12	017362	TEMPORARY CONSTRUCTION ROAD (ENTRANCE/EXIT)	EA	6
13	074029	TEMPORARY SILT FENCE	M	4450
14 (S)	120090	CONSTRUCTION AREA SIGNS	LS	LUMP SUM
15 (S)	120100	TRAFFIC CONTROL SYSTEM	LS	LUMP SUM
16 (S)	120149	TEMPORARY PAVEMENT MARKING (PAINT)	M2	87
17 (S)	120159	TEMPORARY TRAFFIC STRIPE (PAINT)	M	45 300
18	120182	PORTABLE DELINEATOR	EA	13
19 (S)	120300	TEMPORARY PAVEMENT MARKER	EA	12 400
20	129000	TEMPORARY RAILING (TYPE K)	M	31 200

Item	Item Code	Item	Unit of Measure	Estimated Quantity
21 (S)	129100	TEMPORARY CRASH CUSHION MODULE	EA	580
22	150206	ABANDON CULVERT	EA	29
23	150221	ABANDON INLET	EA	22
24	150227	ABANDON PIPELINE	M	1110
25	150608	REMOVE CHAIN LINK FENCE	M	1670
26	150662	REMOVE METAL BEAM GUARD RAILING	M	3820
27	017363	REMOVE DOUBLE METAL BEAM GUARD RAILING	M	92
28	150668	REMOVE FLARED END SECTION	EA	13
29	150710	REMOVE TRAFFIC STRIPE	M	30 300
30	150713	REMOVE PAVEMENT MARKING	M2	10
31	150722	REMOVE PAVEMENT MARKER	EA	6520
32	150742	REMOVE ROADSIDE SIGN	EA	30
33	150747	REMOVE ROADSIDE SIGN (STRAP AND SADDLE BRACKET METHOD)	EA	14
34	150760	REMOVE SIGN STRUCTURE	EA	10
35	150805	REMOVE CULVERT	EA	10
36	017364	REMOVE EXISTING UNDERDRAIN	M	730
37	150820	REMOVE INLET	EA	24
38	150828	REMOVE JUNCTION BOX	EA	1
39	150857	REMOVE ASPHALT CONCRETE SURFACING	M3	3720
40	151274	SALVAGE CONCRETE BARRIER (TYPE K)	M	8090

Item	Item Code	Item	Unit of Measure	Estimated Quantity
41	017365	SALVAGE ROCK SLOPE PROTECTION	M3	400
42 (S)	152355	RELOCATE IRRIGATION CONTROLLER	EA	1
43	152390	RELOCATE ROADSIDE SIGN	EA	10
44 (S)	152394	RELOCATE SIGN STRUCTURE	EA	2
45	152430	ADJUST INLET	EA	12
46	152440	ADJUST MANHOLE TO GRADE	EA	1
47	152604	MODIFY INLET	EA	4
48 (S)	017366	PLANE ASPHALT CONCRETE PAVEMENT (130 MM MAXIMUM)	M2	26 900
49	153214	REMOVE CONCRETE CURB	M	3170
50	153221	REMOVE CONCRETE BARRIER	M	77
51	155003	CAP INLET	EA	13
52	017367	CAP IRRIGATION SUPPLY LINE	EA	10
53	156585	REMOVE CRASH CUSHION	EA	1
54	157561	BRIDGE REMOVAL (PORTION), LOCATION A	LS	LUMP SUM
55	157562	BRIDGE REMOVAL (PORTION), LOCATION B	LS	LUMP SUM
56	157563	BRIDGE REMOVAL (PORTION), LOCATION C	LS	LUMP SUM
57	157564	BRIDGE REMOVAL (PORTION), LOCATION D	LS	LUMP SUM
58	160101	CLEARING AND GRUBBING	LS	LUMP SUM
59	170101	DEVELOP WATER SUPPLY	LS	LUMP SUM
60	190101	ROADWAY EXCAVATION	M3	54 900

Item	Item Code	Item	Unit of Measure	Estimated Quantity
61	017368	EXCAVATION HAZARDOUS MATERIAL	M3	13 800
62 (F)	192001	STRUCTURE EXCAVATION	M3	62
63 (F)	192003	STRUCTURE EXCAVATION (BRIDGE)	M3	470
64 (F)	017369	STRUCTURE EXCAVATION (SIGN STRUCTURE)	M3	56
65 (F)	192037	STRUCTURE EXCAVATION (RETAINING WALL)	M3	900
66 (F)	192055	STRUCTURE EXCAVATION (SOIL NAIL WALL)	M3	45
67 (F)	048165	STRUCTURE EXCAVATION (APPROACH SLAB)	M3	465
68 (F)	193001	STRUCTURE BACKFILL	M3	8
69 (F)	193003	STRUCTURE BACKFILL (BRIDGE)	M3	320
70 (F)	017370	STRUCTURE BACKFILL (SIGN STRUCTURE)	M3	39
71 (F)	048166	LIGHTWEIGHT BACKFILL	M3	3945
72 (F)	193028	STRUCTURE BACKFILL (SOIL NAIL WALL)	M3	25
73	193114	SAND BACKFILL	M3	170
74	194001	DITCH EXCAVATION	M3	84
75 (S)	197060	SOIL NAIL ASSEMBLY	M	535
76	198100	LIGHTWEIGHT FILL	M3	13 800
77	017371	CELLULAR CONCRETE	M3	24 300
78	017372	GEOSYNTHETIC REINFORCED EMBANKMENT (GEOGRID)	M2	8890
79 (S)	017373	EROSION CONTROL (NETTING)	M2	4570
80 (S)	203003	STRAW (EROSION CONTROL)	TONN	22

Item	Item Code	Item	Unit of Measure	Estimated Quantity
81 (S)	017374	FIBER	KG	5960
82 (S)	203021	FIBER ROLLS	M	3050
83 (S)	203045	PURE LIVE SEED (EROSION CONTROL)	KG	550
84 (S)	203056	COMMERCIAL FERTILIZER (EROSION CONTROL)	KG	2690
85 (S)	203061	STABILIZING EMULSION (EROSION CONTROL)	KG	1070
86 (S-F)	208023	75 MM GALVANIZED STEEL PIPE (SUPPLY LINE)	M	53
87 (F)	208038	NPS 3 SUPPLY LINE (BRIDGE)	M	200
88 (S-F)	208798	200 MM WELDED STEEL PIPE CONDUIT (6.35 MM THICK)	M	10
89 (S-F)	208909	EXTEND 200 MM CONDUIT	M	38
90	250401	CLASS 4 AGGREGATE SUBBASE	M3	2370
91	260301	CLASS 3 AGGREGATE BASE	M3	210
92	390145	REPLACE ASPHALT CONCRETE SURFACING	M3	52
93	390152	ASPHALT CONCRETE	TONN	116 000
94	390171	ASPHALT CONCRETE BASE (TYPE A)	TONN	6000
95	390175	ASPHALT CONCRETE (LEVELING)	TONN	16 700
96	390206	RUBBERIZED ASPHALT CONCRETE (TYPE G)	TONN	20 700
97	394002	PLACE ASPHALT CONCRETE (MISCELLANEOUS AREA)	M2	180
98	394040	PLACE ASPHALT CONCRETE DIKE (TYPE A)	M	380
99	394044	PLACE ASPHALT CONCRETE DIKE (TYPE C)	M	280
100	394048	PLACE ASPHALT CONCRETE DIKE (TYPE E)	M	5860

Item	Item Code	Item	Unit of Measure	Estimated Quantity
101	394049	PLACE ASPHALT CONCRETE DIKE (TYPE F)	M	1190
102	397001	ASPHALTIC EMULSION (PAINT BINDER)	TONN	86
103	401000	CONCRETE PAVEMENT	M3	4910
104	048167	FURNISH PRECAST PRESTRESS SHEET PILING	M2	5830
105 (S)	048168	DRIVE PRECAST PRESTRESS SHEET PILE	EA	366
106 (S)	490657	600 MM CAST-IN-DRILLED-HOLE CONCRETE PILING	M	33
107	490757	FURNISH PILING (CLASS 625C)	M	1563
108 (S)	490758	DRIVE PILE (CLASS 625C)	EA	75
109	490759	FURNISH PILING (CLASS 400C)	M	1063
110 (S)	490760	DRIVE PILE (CLASS 400C)	EA	56
111	048169	FURNISH PILING (CLASS 400C) (TOLL BOOTH)	M	526
112 (S)	048170	DRIVE PILE (CLASS 400C) (TOLL BOOTH)	EA	42
113	017375	FURNISH PILING (CLASS 400C) (SIGN FOUNDATION)	M	230
114 (S)	017376	DRIVE PILE (CLASS 400C) (SIGN FOUNDATION)	EA	24
115 (F)	510050	STRUCTURAL CONCRETE	M3	350
116 (F)	510051	STRUCTURAL CONCRETE, BRIDGE FOOTING	M3	97
117 (F)	510053	STRUCTURAL CONCRETE, BRIDGE	M3	1350
118 (F)	510060	STRUCTURAL CONCRETE, RETAINING WALL	M3	60
119 (F)	510085	STRUCTURAL CONCRETE, APPROACH SLAB (TYPE EQ)	M3	53
120 (F)	510087	STRUCTURAL CONCRETE, APPROACH SLAB (TYPE R)	M3	65

Item	Item Code	Item	Unit of Measure	Estimated Quantity
121 (F)	510502	MINOR CONCRETE (MINOR STRUCTURE)	M3	121
122	510507	MINOR CONCRETE (SIGN FOUNDATION)	M3	34
123	510526	MINOR CONCRETE (BACKFILL)	M3	52
124	510800	PAVING NOTCH EXTENSION	M3	9
125	511106	DRILL AND BOND DOWEL	M	340
126	048171	DRILL AND PRESSURE GROUT DOWEL	M	24
127	512230	FURNISH PRECAST PRESTRESSED CONCRETE GIRDER (10 M - 15 M)	EA	24
128	512233	FURNISH PRECAST PRESTRESSED CONCRETE GIRDER (25 M - 30 M)	EA	12
129 (S)	512500	ERECT PRECAST PRESTRESSED CONCRETE GIRDER	EA	36
130 (F)	513501	CONCRETE CLOSURE WALL	M2	85
131	515020	REFINISH BRIDGE DECK	M2	52
132 (S)	515072	CORE CONCRETE (0 - 50 MM)	M	32
133 (S-F)	517961	SOUND WALL (BARRIER) (MASONRY BLOCK)	M2	120
134 (S)	519102	JOINT SEAL (TYPE AL)	M	62
135 (S)	519120	JOINT SEAL (MR 15 MM)	M	210
136 (S)	519121	JOINT SEAL (TYPE B - MR 30 MM)	M	96
137 (S-F)	520101	BAR REINFORCING STEEL	KG	3330
138 (S-F)	520102	BAR REINFORCING STEEL (BRIDGE)	KG	328 800
139 (S-F)	520103	BAR REINFORCING STEEL (RETAINING WALL)	KG	5700
140 (S-F)	520106	BAR REINFORCING STEEL (EPOXY COATED)	KG	43 700

Item	Item Code	Item	Unit of Measure	Estimated Quantity
141 (S-F)	530100	SHOTCRETE	M3	25
142 (F)	560218	FURNISH SIGN STRUCTURE (TRUSS)	KG	63 010
143 (S-F)	560219	INSTALL SIGN STRUCTURE (TRUSS)	KG	63 010
144 (S)	561008	760 MM CAST-IN-DRILLED-HOLE CONCRETE PILE (SIGN FOUNDATION)	M	11
145 (S)	561009	920 MM CAST-IN-DRILLED-HOLE CONCRETE PILE (SIGN FOUNDATION)	M	46
146	562002	METAL (BARRIER MOUNTED SIGN)	KG	830
147	566011	ROADSIDE SIGN - ONE POST	EA	11
148	566012	ROADSIDE SIGN - TWO POST	EA	11
149	568001	INSTALL SIGN (STRAP AND SADDLE BRACKET METHOD)	EA	15
150	568007	INSTALL SIGN OVERLAY	M2	2
151	620905	300 MM ALTERNATIVE PIPE CULVERT (TYPE A)	M	16
152	620906	300 MM ALTERNATIVE PIPE CULVERT (TYPE B)	M	7
153	620910	450 MM ALTERNATIVE PIPE CULVERT (TYPE A)	M	1000
154	620911	450 MM ALTERNATIVE PIPE CULVERT (TYPE B)	M	1540
155	620915	600 MM ALTERNATIVE PIPE CULVERT (TYPE B)	M	240
156	620931	1050 MM ALTERNATIVE PIPE CULVERT (TYPE A)	M	10
157	664010	300 MM CORRUGATED STEEL PIPE (2.01 MM THICK)	M	27
158	665732	450 MM SLOTTED CORRUGATED STEEL PIPE (1.63 MM THICK)	M	160
159	017377	100 MM PLASTIC PIPE (COLLECTOR SYSTEM HORIZONTAL UNDERDRAIN)	M	650
160	682049	CLASS 3 PERMEABLE MATERIAL (BLANKET)	M3	2930

Item	Item Code	Item	Unit of Measure	Estimated Quantity
161	685067	200 MM ALTERNATIVE PIPE UNDERDRAIN	M	690
162	705334	300 MM ALTERNATIVE FLARED END SECTION	EA	1
163	705336	450 MM ALTERNATIVE FLARED END SECTION	EA	5
164	705337	600 MM ALTERNATIVE FLARED END SECTION	EA	3
165	017378	CHECK VALVE	EA	2
166	721010	ROCK SLOPE PROTECTION (BACKING NO. 1, METHOD B)	M3	10
167	017379	GABIONS (PVC)	M3	2540
168	727906	MINOR CONCRETE (GUTTER LINING)	M3	4
169	729010	ROCK SLOPE PROTECTION FABRIC	M2	44
170	731502	MINOR CONCRETE (MISCELLANEOUS CONSTRUCTION)	M3	70
171	017380	MINOR CONCRETE (COLORED PAVING)	M3	210
172 (S-F)	750001	MISCELLANEOUS IRON AND STEEL	KG	16 719
173 (S-F)	750501	MISCELLANEOUS METAL (BRIDGE)	KG	2000
174 (S-F)	800381	CHAIN LINK FENCE (TYPE CL-0.9)	M	82
175 (S)	800385	CHAIN LINK FENCE (TYPE CL-1.2)	M	1610
176 (S)	800391	CHAIN LINK FENCE (TYPE CL-1.8)	M	65
177 (S)	832003	METAL BEAM GUARD RAILING (WOOD POST)	M	1300
178 (F)	833032	CHAIN LINK RAILING (TYPE 7)	M	140
179 (F)	833125	CONCRETE BARRIER (TYPE 25)	M	392
180 (F)	833187	CONCRETE BARRIER (TYPE 27 MODIFIED)	M	49

Item	Item Code	Item	Unit of Measure	Estimated Quantity
181 (S)	839311	DOUBLE THRIE BEAM BARRIER (WOOD POST)	M	68
182 (S)	839532	CABLE ANCHOR ASSEMBLY (BREAKAWAY, TYPE B)	EA	11
183 (S)	839559	TERMINAL SYSTEM (TYPE ET)	EA	1
184 (S)	839565	TERMINAL SYSTEM (TYPE SRT)	EA	13
185 (S)	839566	TERMINAL SYSTEM (TYPE CAT)	EA	2
186 (S)	839591	CRASH CUSHION, SAND FILLED	EA	2
187	839701	CONCRETE BARRIER (TYPE 60)	M	5070
188 (F)	839702	CONCRETE BARRIER (TYPE 60A)	M	126
189	839703	CONCRETE BARRIER (TYPE 60C)	M	480
190	017381	CONCRETE BARRIER (TYPE 60C MODIFIED)	M	1740
191	839705	CONCRETE BARRIER (TYPE 60E)	M	300
192 (S)	840515	THERMOPLASTIC PAVEMENT MARKING	M2	190
193 (S)	840561	100 MM THERMOPLASTIC TRAFFIC STRIPE	M	27 500
194 (S)	840563	200 MM THERMOPLASTIC TRAFFIC STRIPE	M	3140
195 (S)	850101	PAVEMENT MARKER (NON-REFLECTIVE)	EA	8290
196 (S)	850102	PAVEMENT MARKER (REFLECTIVE)	EA	3900
197 (S)	860415	LIGHTING (STAGE CONSTRUCTION)	LS	LUMP SUM
198 (S)	860551	LIGHTING AND SIGN ILLUMINATION (LOCATION 1)	LS	LUMP SUM
199 (S)	860552	LIGHTING AND SIGN ILLUMINATION (LOCATION 2)	LS	LUMP SUM
200 (S)	860553	LIGHTING AND SIGN ILLUMINATION (LOCATION 3)	LS	LUMP SUM

Item	Item Code	Item	Unit of Measure	Estimated Quantity
201 (S)	860554	LIGHTING AND SIGN ILLUMINATION (LOCATION 4)	LS	LUMP SUM
202 (S-F)	048172	50 MM SPRINKLER CONTROL CONDUIT (BRIDGE)	M	200
203 (S)	017382	TRAFFIC OPERATION SYSTEM	LS	LUMP SUM
204 (S)	017383	TELEPHONE AND DATA LINE SURGE PROTECTION DEVICE	EA	1
205 (S)	017384	SWITCH ETHERNET HUB	EA	1
206 (S)	017385	CHANNEL SERVICE UNIT/DATA SERVICE UNIT	EA	1
207 (S)	017386	MODEM ASSEMBLY	EA	1
208 (S)	017387	CAMERA UNIT	EA	1
209 (S)	017388	PAN/TILT UNIT	EA	1
210 (S)	017389	CAMERA CONTROL UNIT	EA	1
211 (S)	017390	VIDEO ENCODER UNIT	EA	1
212 (S)	017391	INTEGRATED SERVICE DIGITAL NETWORK TERMINAL ADAPTER	EA	1
213 (S)	017392	ACCESS NODE	EA	1
214 (S)	017393	ANALOG VIDEO ROUTING SWITCHER	EA	1
215 (S)	017394	UNINTERRUPTIBLE POWER SYSTEM	EA	1
216 (S)	017395	COLOR CCTV MONITOR	EA	1
217 (S)	017396	HUB INTERNALS	LS	LUMP SUM
218 (S)	017397	HUB DATA	LS	LUMP SUM
219 (S)	017398	HUB ANALOG VIDEO	LS	LUMP SUM
220 (S)	017399	WEATHER STATION	LS	LUMP SUM

Item	Item Code	Item	Unit of Measure	Estimated Quantity
221 (S)	994650	BUILDING WORK	LS	LUMP SUM
222	999990	MOBILIZATION	LS	LUMP SUM

STATE OF CALIFORNIA
DEPARTMENT OF TRANSPORTATION

SPECIAL PROVISIONS

Annexed to Contract No. 04-045034

SECTION 1. SPECIFICATIONS AND PLANS

The work embraced herein shall conform to the provisions in the Standard Specifications dated July 1995, and the Standard Plans dated July 1997, of the Department of Transportation insofar as the same may apply, and these special provisions.

Amendments to the Standard Specifications set forth in these special provisions shall be considered as part of the Standard Specifications for the purposes set forth in Section 5-1.04, "Coordination and Interpretation of Plans, Standard Specifications and Special Provisions," of the Standard Specifications. Whenever either the term "Standard Specifications is amended" or the term "Standard Specifications are amended" is used in the special provisions, the indented text following said term shall be considered an amendment to the Standard Specifications. In case of conflict between such amendments and the Standard Specifications, the amendments shall take precedence over and be used in lieu of the conflicting portions.

In case of conflict between the Standard Specifications and these special provisions, the special provisions shall take precedence over and be used in lieu of the conflicting portions.

SECTION 2. PROPOSAL REQUIREMENTS AND CONDITIONS

2-1.01 GENERAL

The bidder's attention is directed to the provisions in Section 2, "Proposal Requirements and Conditions," of the Standard Specifications and these special provisions for the requirements and conditions which the bidder must observe in the preparation of the Proposal form and the submission of the bid.

In addition to the subcontractors required to be listed in conformance with Section 2-1.054, "Required Listing of Proposed Subcontractors," of the Standard Specifications, each proposal shall have listed therein the portion of work that will be performed by each subcontractor listed.

The Bidder's Bond form mentioned in the last paragraph in Section 2-1.07, "Proposal Guaranty," of the Standard Specifications will be found following the signature page of the Proposal.

Submit request for substitution of an "or equal" item, and the data substantiating the request to the Department of Transportation, Division Of Construction - Duty Senior, Mail Station: 3 - B, 111 Grand Avenue / P. O. Box 23660, Oakland, Ca 94623-0660, so that the request is received by the Department by close of business on the fourth day, not including Saturdays, Sundays and legal holidays, following bid opening.

In conformance with Public Contract Code Section 7106, a Noncollusion Affidavit is included in the Proposal. Signing the Proposal shall also constitute signature of the Noncollusion Affidavit.

The contractor, sub recipient or subcontractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this contract. The contractor shall carry out applicable requirements of 49 CFR part 26 in the award and administration of DOT-assisted contracts. Failure by the contractor to carry out these requirements is a material breach of this contract, which may result in the termination of this contract or such other remedy as the recipient deems appropriate. Each subcontract signed by the bidder must include this assurance.

2-1.015 FEDERAL LOBBYING RESTRICTIONS

Section 1352, Title 31, United States Code prohibits Federal funds from being expended by the recipient or any lower tier subrecipient of a Federal-aid contract to pay for any person for influencing or attempting to influence a Federal agency or Congress in connection with the awarding of any Federal-aid contract, the making of any Federal grant or loan, or the entering into of any cooperative agreement.

If any funds other than Federal funds have been paid for the same purposes in connection with this Federal-aid contract, the recipient shall submit an executed certification and, if required, submit a completed disclosure form as part of the bid documents.

A certification for Federal-aid contracts regarding payment of funds to lobby Congress or a Federal agency is included in the Proposal. Standard Form - LLL, "Disclosure of Lobbying Activities," with instructions for completion of the Standard Form is also included in the Proposal. Signing the Proposal shall constitute signature of the Certification.

The above-referenced certification and disclosure of lobbying activities shall be included in each subcontract and any lower-tier contracts exceeding $100,000. All disclosure forms, but not certifications, shall be forwarded from tier to tier until received by the Engineer.

The Contractor, subcontractors and any lower-tier contractors shall file a disclosure form at the end of each calendar quarter in which there occurs any event that requires disclosure or that materially affects the accuracy of the information contained in any disclosure form previously filed by the Contractor, subcontractors and any lower-tier contractors. An event that materially affects the accuracy of the information reported includes:

A. A cumulative increase of $25,000 or more in the amount paid or expected to be paid for influencing or attempting to influence a covered Federal action; or
B. A change in the person(s) or individual(s) influencing or attempting to influence a covered Federal action; or,
C. A change in the officer(s), employee(s), or Member(s) contacted to influence or attempt to influence a covered Federal action.

2-1.02 DISADVANTAGED BUSINESS ENTERPRISE (DBE)

This project is subject to Part 26, Title 49, Code of Federal Regulations entitled "Participation by Disadvantaged Business Enterprises in Department of Transportation Financial Assistance Programs." The Regulations in their entirety are incorporated herein by this reference.

Bidders shall be fully informed respecting the requirements of the Regulations and the Department's Disadvantaged Business Enterprise (DBE) program developed pursuant to the Regulations; particular attention is directed to the following matters:

A. A DBE must be a small business concern as defined pursuant to Section 3 of U.S. Small Business Act and relevant regulations promulgated pursuant thereto;
B. A DBE may participate as a prime contractor, subcontractor, joint venture partner with a prime or subcontractor, vendor of material or supplies, or as a trucking company;
C. A DBE bidder, not bidding as a joint venture with a non-DBE, will be required to document one or a combination of the following:

    1. The bidder will meet the goal by performing work with its own forces.
    2. The bidder will meet the goal through work performed by DBE subcontractors, suppliers or trucking companies.
    3. The bidder, prior to bidding, made adequate good faith efforts to meet the goal.

D. A DBE joint venture partner must be responsible for specific contract items of work, or portions thereof. Responsibility means actually performing, managing and supervising the work with its own forces. The DBE joint venture partner must share in the capital contribution, control, management, risks and profits of the joint venture. The DBE joint venturer must submit the joint venture agreement with the proposal or the DBE Information form required in the Section entitled "Submission of DBE Information" of these special provisions;
E. A DBE must perform a commercially useful function, i.e., must be responsible for the execution of a distinct element of the work and must carry out its responsibility by actually performing, managing and supervising the work;
F. DBEs must be certified by either the California Department of Transportation, or by a participating State of California or local agency which certifies in conformance with Title 49, Code of Federal Regulations, Part 26, as of the date of bid opening. It is the Contractor's responsibility to verify that DBEs are certified. Listings of DBEs certified by the Department are available from the following sources:

    1. The Department's DBE Directory, which is published quarterly. This Directory may be obtained from the Department of Transportation, Materiel Operations Branch, Publication Distribution Unit, 1900 Royal Oaks Drive, Sacramento, California 95815, Telephone: (916) 445-3520;
    2. The Department's Electronic Information Bulletin Board Service, which is accessible by modem and is updated weekly. The Bulletin Board may be accessed by first contacting the Department's Business Enterprise Program at Telephone: (916) 227-8937 and obtaining a user identification and password;
    3. The Department's web site at http://www.dot.ca.gov/hq/bep/index.htm;
    4. The organizations listed in the Section entitled "DBE Goal for this Project" of these special provisions.

G. Credit for materials or supplies purchased from DBEs will be as follows:

    1. If the materials or supplies are obtained from a DBE manufacturer, 100 percent of the cost of the materials or supplies will count toward the DBE goal. A DBE manufacturer is a firm that operates or maintains a factory or establishment that produces, on the premises, the materials, supplies, articles, or equipment required under the contract and of the general character described by the specifications.
    2. If the materials or supplies are purchased from a DBE regular dealer, 60 percent of the cost of the materials or supplies will count toward the DBE goal. A DBE regular dealer is a firm that owns, operates, or maintains a store, warehouse, or other establishment in which the materials, supplies, articles or equipment of the general

character described by the specifications and required under the contract are bought, kept in stock, and regularly sold or leased to the public in the usual course of business. To be a DBE regular dealer, the firm must be an established, regular business that engages, as its principal business and under its own name, in the purchase and sale or lease of the products in question. A person may be a DBE regular dealer in such bulk items as petroleum products, steel, cement, gravel, stone, or asphalt without owning, operating, or maintaining a place of business as provided in this paragraph G.2. if the person both owns and operates distribution equipment for the products. Any supplementing of regular dealers' own distribution equipment shall be by a long-term lease agreement and not on an ad hoc or contract-by-contract basis. Packagers, brokers, manufacturers' representatives, or other persons who arrange or expedite transactions are not DBE regular dealers within the meaning of this paragraph G.2.

3. Credit for materials or supplies purchased from a DBE which is neither a manufacturer nor a regular dealer will be limited to the entire amount of fees or commissions charged for assistance in the procurement of the materials and supplies, or fees or transportation charges for the delivery of materials or supplies required on a job site, provided the fees are reasonable and not excessive as compared with fees charged for similar services.

H. Credit for DBE trucking companies will be as follows:

1. The DBE must be responsible for the management and supervision of the entire trucking operation for which it is responsible on a particular contract, and there cannot be a contrived arrangement for the purpose of meeting the DBE goal;
2. The DBE must itself own and operate at least one fully licensed, insured, and operational truck used on the contract;
3. The DBE receives credit for the total value of the transportation services it provides on the contract using trucks its owns, insures, and operates using drivers it employs;
4. The DBE may lease trucks from another DBE firm, including an owner-operator who is certified as a DBE. The DBE who leases trucks from another DBE receives credit for the total value of the transportation services the lessee DBE provides on the contract;
5. The DBE may also lease trucks from a non-DBE firm, including an owner-operator. The DBE who leases trucks from a non-DBE is entitled to credit only for the fee or commission it receives as a result of the lease arrangement. The DBE does not receive credit for the total value of the transportation services provided by the lessee, since these services are not provided by a DBE;
6. For the purposes of this paragraph H, a lease must indicate that the DBE has exclusive use of and control over the truck. This does not preclude the leased truck from working for others during the term of the lease with the consent of the DBE, so long as the lease gives the DBE absolute priority for use of the leased truck. Leased trucks must display the name and identification number of the DBE.

I. Noncompliance by the Contractor with the requirements of the regulations constitutes a breach of this contract and may result in termination of the contract or other appropriate remedy for a breach of this contract;
J. Bidders are encouraged to use services offered by financial institutions owned and controlled by DBEs.

2-1.02A DBE GOAL FOR THIS PROJECT

The Department has established the following goal for Disadvantaged Business Enterprise (DBE) participation for this project:

Disadvantaged Business Enterprise (DBE): 16 percent

Bidders may use the services of the following firms to contact interested DBEs. These firms are available to assist DBEs in preparing bids for subcontracting or supplying materials.

The following firms may be contacted for projects in the following locations:

Districts 04, 05 (except San Luis Obispo and Santa Barbara Counties), 06 (except Kern County) and 10:	Districts 08, 11 and 12:
Triaxial Management Services, Inc. - Oakland 1545 Willow Street, 1st Floor Oakland, CA 94607 Telephone - (510) 286-1313 FAX No. - (510) 286-6792	Triaxial Management Services, Inc. - San Diego 2725 Congress Street, Suite 1-D San Diego, CA 92110 Telephone - (619) 543-5109 FAX No. - (619) 543-5108

Districts 07 and 08; in San Luis Obispo and Santa Barbara Counties in District 05; and in Kern County in District 06:	Districts 01, 02, 03 and 09:
Triaxial Management Services, Inc. - Los Angeles 2594 Industry Way, Suite 101 Lynwood, CA 90262 Telephone - (310) 537-6677 FAX No. - (310) 637-0128	Triaxial Management Services, Inc. - Sacramento 930 Alhambra Blvd., #205 Sacramento, CA 95816 Telephone - (916) 553-4172 FAX No. - (916) 553-4173

2-1.02B SUBMISSION OF DBE INFORMATION

The required DBE information shall be submitted on the "CALTRANS BIDDER - DBE INFORMATION" form included in the Proposal. If the DBE information is not submitted with the bid, the DBE Information form shall be removed from the documents prior to submitting the bid.

It is the bidder's responsibility to make enough work available to DBEs and to select those portions of the work or material needs consistent with the available DBEs to meet the goal for DBE participation or to provide information to establish that, prior to bidding, the bidder made adequate good faith efforts to do so.

If DBE information is not submitted with the bid, the apparent successful bidder (low bidder), the second low bidder and the third low bidder shall submit DBE information to the Department of Transportation, 1120 N Street, Room 0200, MS #26, Sacramento, California 95814 so the information is received by the Department no later than 4:00 p.m. on the fourth day, not including Saturdays, Sundays and legal holidays, following bid opening. DBE information sent by U.S. Postal Service certified mail with return receipt and certificate of mailing and mailed on or before the third day, not including Saturdays, Sundays and legal holidays, following bid opening will be accepted even if it is received after the fourth day following bid opening. Failure to submit the required DBE information by the time specified will be grounds for finding the bid or proposal nonresponsive. Other bidders need not submit DBE information unless requested to do so by the Department.

The bidder's DBE information shall establish that good faith efforts to meet the DBE goal have been made. To establish good faith efforts, the bidder shall demonstrate that the goal will be met or that, prior to bidding, adequate good faith efforts to meet the goal were made.

Bidders are cautioned that even though their submittal indicates they will meet the stated DBE goal, their submittal should also include their adequate good faith efforts information along with their DBE goal information to protect their eligibility for award of the contract in the event the Department, in its review, finds that the goal has not been met.

The bidder's DBE information shall include the names, addresses and phone numbers of DBE firms that will participate, with a complete description of work or supplies to be provided by each, the dollar value of each DBE transaction, and a written confirmation from the DBE that it is participating in the contract. A copy of the DBE's quote will serve as written confirmation that the DBE is participating in the contract. When 100 percent of a contract item of work is not to be performed or furnished by a DBE, a description of the exact portion of that work to be performed or furnished by that DBE shall be included in the DBE information, including the planned location of that work. The work that a DBE prime contractor has committed to performing with its own forces as well as the work that it has committed to be performed by DBE subcontractors, suppliers and trucking companies will count toward the goal.

The information necessary to establish the bidder's adequate good faith efforts to meet the DBE goal should include:

A. The names and dates of each publication in which a request for DBE participation for this project was placed by the bidder.
B. The names and dates of written notices sent to certified DBEs soliciting bids for this project and the dates and methods used for following up initial solicitations to determine with certainty whether the DBEs were interested.
C. The items of work which the bidder made available to DBE firms, including, where appropriate, any breaking down of the contract work items (including those items normally performed by the bidder with its own forces) into

economically feasible units to facilitate DBE participation. It is the bidder's responsibility to demonstrate that sufficient work to meet the DBE goal was made available to DBE firms.

D. The names, addresses and phone numbers of rejected DBE firms, the firms selected for that work, and the reasons for the bidder's choice.
E. Efforts made to assist interested DBEs in obtaining bonding, lines of credit or insurance, and any technical assistance or information related to the plans, specifications and requirements for the work which was provided to DBEs.
F. Efforts made to assist interested DBEs in obtaining necessary equipment, supplies, materials, or related assistance or services, excluding supplies and equipment the DBE subcontractor purchases or leases from the prime contractor or its affiliate.
G. The names of agencies contacted to provide assistance in contacting, recruiting and using DBE firms.
H. Any additional data to support a demonstration of good faith efforts.

2-1.03 ESCROW OF BID DOCUMENTATION

Bid documentation shall consist of all documentary and calculated information generated by the Contractor in preparation of the bid. The bid documentation shall conform to the requirements in these special provisions, and shall be submitted to the Department and held in escrow for the duration of the contract.

In the resolution of disputes involving the project, the escrowed bid documents will be the only documents accepted from the Contractor regarding preparation of the bid.

In signing the proposal, the bidder certifies that the material submitted for escrow constitutes all the documentary information used in preparation of the bid and that he has personally examined the contents of the container and that they are complete.

The bidder shall include with the proposal, the identification of the bidder's representative authorized to present the bid documentation and the persons responsible for preparing the bidder's estimate.

Nothing in the bid documentation shall be construed to change or modify the terms or conditions of the contract.

Escrowed bid documentation will not be used for pre-award evaluation of the Contractor's anticipated methods of construction, nor to assess the Contractor's qualifications for performing the work.

Bid documentation shall clearly itemize the Contractor's estimated costs of performing the work. The documentation submitted shall be complete and so detailed as to allow for an in-depth analysis of the Contractor's estimate.

The bid documentation shall include, but not be limited to: quantity takeoffs; rate schedules for the direct costs and the time- and nontime-related indirect costs for labor (by craft), plant and equipment ownership and operation, permanent and expendable materials, insurance and subcontracted work; estimated construction schedules, including sequence and duration and development of production rates; quotations from subcontractors and suppliers; estimates of field and home office overhead; contingency and margin for each contract item of work; and other reports, calculations and information used by the bidder to arrive at the estimate submitted with the proposal.

The Contractor shall also submit bid documentation for each subcontractor whose total subcontract exceeds $250,000. Subcontractor bid documentation shall be enclosed with the Contractor's submittal. The examination of subcontractors' bid documentation will be accomplished in the same manner as for the Contractor's bid documentation. If a subcontractor is replaced, bid documentation for the new subcontractor shall be submitted for review and escrow before authorization for the substitution will be granted. Upon request of a subcontractor, the bid documentation from that subcontractor shall be reviewed only by the subcontractor and the Department.

If the bidder is a joint venture, the bid documentation shall include the joint venture agreement, the joint venture estimate comparison and final reconciliation of the joint venture estimate.

Copies of the proposals submitted by the first, second and third low bidders will be provided to the respective bidders for inclusion in the bid documentation to be escrowed.

The first, second, and third apparent low bidders shall present the bid documentation for escrow at the District 04 Office, 111 Grand Avenue, Room 12-816, Oakland, CA, on the first Monday, at 1:00 p.m., following the time indicated in the "Notice to Contractors" for the opening of bids.

Bid documentation shall be submitted in a sealed container, clearly marked with the bidder's name, date of submittal, project contract number and the words, "Bid Documentation for Escrow."

Failure to submit the actual and complete bid documentation as specified herein within the time specified shall be cause for rejection of the proposal.

Upon submittal, the bid documentation of the apparent low bidder will be examined and inventoried by the duly designated representatives of the Contractor and the Department to ensure that the bid documentation is authentic, legible, and in accordance with the terms of this section "Escrow of Bid Documentation." The examination will not include review of, nor will it constitute approval of, proposed construction methods, estimating assumptions or interpretation of the contract. The examination will not alter any conditions or terms of the contract. The acceptance or rejection by the Department that the submitted bid documents are in compliance with this section "Escrow of Bid Documentation" shall be completed within 48 hours of the time the bid documentation is submitted by the Contractor.

At the completion of the examination, the bid documents will be sealed and jointly deposited at an agreed commercial bank.

Bid documentation submitted by the second and third apparent low bidders will be jointly deposited at agreed commercial banks. If the apparent low bid is withdrawn or rejected, the bid documentation of the second low bidder will be examined and inventoried in the manner specified above, then sealed and deposited again in escrow. If the second low bid is withdrawn or rejected, the bid documentation of the third low bidder will be examined and inventoried in the manner specified above, then sealed and deposited again in escrow. Upon execution and final approval of the contract or rejection of all bids, the bid documentation will be returned to any remaining unsuccessful bidders.

The escrowed bid documentation may be examined by the designated representatives of both the Department and the Contractor, at any time deemed necessary by either the Department or the Contractor to assist in the negotiation of price adjustments and change orders, or in the settlement of claims or disputes.

If requested by a Disputes Review Board, the escrowed bid documentation may be utilized to assist the Board in its recommendations.

The bid documentation submitted by the Contractor will be held in escrow until the contract has been completed, the ultimate resolution of all disputes and claims has been achieved and receipt of final payment has been accepted by the Contractor. The escrowed bid documentation will then be released from escrow to the Contractor.

The bid documentation submitted by the bidder is, and shall remain, the property of the bidder, and is subject to only joint review by the Department and the bidder. The Department stipulates and expressly acknowledges that the submitted bid documentation constitutes trade secrets and will not be deemed public records. This acknowledgment is based on the Department's express understanding that the information contained in the bid documentation is not known outside the bidder's business, is known only to a limited extent and only by a limited number of employees of the bidder, is safeguarded while in the bidder's possession, is extremely valuable to the bidder and could be extremely valuable to the bidder's competitors by virtue of it reflecting the bidder's contemplated techniques of construction. The Department acknowledges that the bid documentation includes a compilation of information used in the bidder's business, intended to give the bidder an opportunity to obtain an advantage over competitors who do not know of or use the contents of the documentation. The Department agrees to safeguard the bid documentation, and all information contained therein, against disclosure, including disclosure of subcontractor bid documentation to the Contractor and other subcontractors to the fullest extent permitted by law. However, in the event of arbitration or litigation, the bid documentation shall be subject to discovery, and the Department assumes no responsibility for safeguarding the bid documentation unless the Contractor has obtained an appropriate protective order issued by the arbitrator or the court.

Full compensation for preparing the bid documentation, presenting it for escrow and reviewing it for escrow and upon request of the Engineer shall be considered as included in the contract prices paid for the various items of work, and no additional compensation will be allowed therefor.

The direct cost of depositing the bid documentation in escrow at the agreed commercial bank will be paid by the State.

SECTION 3. AWARD AND EXECUTION OF CONTRACT

The bidder's attention is directed to the provisions in Section 3, "Award and Execution of Contract," of the Standard Specifications and these special provisions for the requirements and conditions concerning award and execution of contract.

The award of the contract, if it be awarded, will be to the lowest responsible bidder whose proposal complies with all the requirements prescribed and who has met the goal for DBE participation or has demonstrated, to the satisfaction of the Department, adequate good faith efforts to do so. Meeting the goal for DBE participation or demonstrating, to the satisfaction of the Department, adequate good faith efforts to do so is a condition for being eligible for award of contract.

A "Vendor Data Record" form will be included in the contract documents to be executed by the successful bidder. The purpose of the form is to facilitate the collection of taxpayer identification data. The form shall be completed and returned to the Department by the successful bidder with the executed contract and contract bonds. For the purposes of the form, vendor shall be deemed to mean the successful bidder. The form is not to be completed for subcontractors or suppliers. Failure to complete and return the "Vendor Data Record" form to the Department as provided herein will result in the retention of 31 percent of payments due the contractor and penalties of up to $20,000. This retention of payments for failure to complete the "Vendor Data Record" form is in addition to any other retention of payments due the Contractor.

SECTION 4. BEGINNING OF WORK, TIME OF COMPLETION AND LIQUIDATED DAMAGES

Attention is directed to the provisions in Section 8-1.03, "Beginning of Work," in Section 8-1.06, "Time of Completion," and in Section 8-1.07, "Liquidated Damages," of the Standard Specifications and these special provisions.

The Contractor shall furnish the Engineer with a statement from the vendor that the order for the electrical materials required for this contract has been received and accepted by the vendor; and the statement shall be furnished within 15 calendar days after the contract has been approved by the Attorney General, or the attorney appointed and authorized to represent the Department of Transportation. The statement shall give the date that the electrical materials will be shipped. If the Contractor has the necessary materials on hand, the Contractor will not be required to furnish the vendor's statement.

The Contractor shall begin work within 5 calendar days after the contract has been approved by the Attorney General or the attorney appointed and authorized to represent the Department of Transportation.

The work shall be diligently prosecuted to completion before the expiration of

410 WORKING DAYS

beginning on the date that work begins, or beginning on the fifth calendar day after approval of the contract, whichever occurs first.

The Contractor shall pay to the State of California the sum of $2,600 per day, for each and every calendar day's delay in finishing the work in excess of the number of working days prescribed above.

The 72 hours advance notice before beginning work as referred to in Section 8-1.03 is changed to 5 days advance notice for this project.

SECTION 5. GENERAL

SECTION 5-1. MISCELLANEOUS

5-1.00 PLANS AND WORKING DRAWINGS

When the specifications require working drawings to be submitted to the Office of Structure Design, the drawings shall be submitted to: Office of Structure Design, Documents Unit, P.O. Box 942874, Mail Station 9, Sacramento, CA 94274-0001 (1801 30th Street, Sacramento, CA 95816), Telephone (916) 227-8252.

5-1.003 TRANSPORTATION LABORATORY

Section 1-1.25, "Laboratory," of the Standard Specifications is amended to read:

1-1.25 Laboratory.— The Office of Materials and Foundations of the Department of Transportation, or established laboratories of the various Districts of the Department, or other laboratories authorized by the Department to test materials and work involved in the contract. When a reference is made in the specifications to the "Transportation Laboratory," the reference shall mean the Office of Materials and Foundations, located at 5900 Folsom Boulevard, Sacramento, CA 95819.

The telephone number of the "Transportation Laboratory" is (916) 227-7000.

5-1.005 CONTRACT BONDS

Attention is directed to Section 3-1.02, "Contract Bonds," of the Standard Specifications and these special provisions.

Each of the two bonds required in Section 3-1.02, "Contract Bonds," of the Standard Specifications shall be in a sum equal to 100 percent of the contract price.

5-1.01 LABOR NONDISCRIMINATION

Attention is directed to the following Notice that is required by Chapter 5 of Division 4 of Title 2, California Code of Regulations.

NOTICE OF REQUIREMENT FOR NONDISCRIMINATION PROGRAM
(GOV. CODE, SECTION 12990)

Your attention is called to the "Nondiscrimination Clause", set forth in Section 7-1.01A(4), "Labor Nondiscrimination," of the Standard Specifications, which is applicable to all nonexempt state contracts and subcontracts, and to the "Standard California Nondiscrimination Construction Contract Specifications" set forth therein. The Specifications are applicable to all nonexempt state construction contracts and subcontracts of $5000 or more.

5-1.02 LABOR CODE REQUIREMENTS

Section 7-1.01A(1), "Hours of Labor," of the Standard Specifications is amended to read:

7-1.01A(1) Hours of Labor.— Eight hours labor constitutes a legal day’s work. The Contractor or any subcontractor under the Contractor shall forfeit, as a penalty to the State of California, $25 for each worker employed in the execution of the contract by the respective Contractor or subcontractor for each calendar day during which that worker is required or permitted to work more than 8 hours in any one calendar day and 40 hours in any one calendar week in violation of the provisions of the Labor Code, and in particular, Section 1810 to Section 1815, thereof, inclusive, except that work performed by employees of Contractors in excess of 8 hours per day, and 40 hours during any one week, shall be permitted upon compensation for all hours worked in excess of 8 hours per day at not less than one and one-half times the basic rate of pay, as provided in Section 1815 thereof.

Section 7-1.01A(2), "Prevailing Wage," of the Standard Specifications is amended to read:

7-1.01A(2) Prevailing Wage.— The Contractor and any subcontractor under the Contractor shall comply with Labor Code Sections 1774 and 1775. Pursuant to Section 1775, the Contractor and any subcontractor under the Contractor shall forfeit to the State or political subdivision on whose behalf the contract is made or awarded a penalty of not more than fifty dollars ($50) for each calendar day, or portion thereof, for each worker paid less than the prevailing rates as determined by the Director of Industrial Relations for the work or craft in which the worker is employed for any public work done under the contract by the Contractor or by any subcontractor under the Contractor in violation of the provisions of the Labor Code and in particular, Labor Code Sections 1770 to 1780, inclusive. The amount of this forfeiture shall be determined by the Labor Commissioner and shall be based on consideration of the mistake, inadvertence, or neglect of the Contractor or subcontractor in failing to pay the correct rate of prevailing wages, or the previous record of the Contractor or subcontractor in meeting their respective prevailing wage obligations, or the willful failure by the Contractor or subcontractor to pay the correct rates of prevailing wages. A mistake, inadvertence, or neglect in failing to pay the correct rate of prevailing wages is not excusable if the Contractor or subcontractor had knowledge of the obligations under the Labor Code. In addition to the penalty and pursuant to Labor Code Section 1775, the difference between the prevailing wage rates and the amount paid to each worker for each calendar day or portion thereof for which each worker was paid less than the prevailing wage rate shall be paid to each worker by the Contractor or subcontractor. If a worker employed by a subcontractor on a public works project is not paid the general prevailing per diem wages by the subcontractor, the prime contractor of the project is not liable for the penalties described above unless the prime contractor had knowledge of that failure of the subcontractor to pay the specified prevailing rate of wages to those workers or unless the prime contractor fails to comply with all of the following requirements:

1. The contract executed between the contractor and the subcontractor for the performance of work on the public works project shall include a copy of the provisions of Sections 1771, 1775, 1776, 1777.5, 1813, and 1815 of the Labor Code.
2. The contractor shall monitor the payment of the specified general prevailing rate of per diem wages by the subcontractor to the employees, by periodic review of the certified payroll records of the subcontractor.
3. Upon becoming aware of the subcontractor's failure to pay the specified prevailing rate of wages to the subcontractor's workers, the contractor shall diligently take corrective action to halt or rectify the failure, including, but not limited to, retaining sufficient funds due the subcontractor for work performed on the public works project.
4. Prior to making final payment to the subcontractor for work performed on the public works project, the contractor shall obtain an affidavit signed under penalty of perjury from the subcontractor that the subcontractor has paid the specified general prevailing rate of per diem wages to the subcontractor's employees on the public works project and any amounts due pursuant to Section 1813 of the Labor Code.

Pursuant to Section 1775 of the Labor Code, the Division of Labor Standards Enforcement shall notify the Contractor on a public works project within 15 days of the receipt by the Division of Labor Standards Enforcement of a complaint of the failure of a subcontractor on that public works project to pay workers the general prevailing rate of per diem wages. If the Division of Labor Standards Enforcement determines that employees of a subcontractor were not paid the general prevailing rate of per diem wages and if the Department did not retain sufficient money under the contract to pay those employees the balance of wages owed under the general prevailing rate of per diem wages, the contractor shall withhold an amount of moneys due the subcontractor sufficient to pay those employees the general prevailing rate of per diem wages if requested by the Division of Labor Standards Enforcement. The Contractor shall pay any money retained from and owed to a subcontractor upon receipt of notification by the Division of Labor Standards Enforcement that the wage complaint has been resolved. If notice of the resolution of the wage complaint has not been received by the Contractor within 180 days of the filing of a valid notice of completion or acceptance of the public works project, whichever occurs later, the Contractor shall pay all moneys retained from the subcontractor to the Department. These moneys shall be retained by the Department pending the final decision of an enforcement action.

Pursuant to the provisions of Section 1773 of the Labor Code, the Department has obtained the general prevailing rate of wages (which rate includes employer payments for health and welfare, pension, vacation, travel time, and subsistence pay as provided for in Section 1773.8 of the Labor Code, apprenticeship or other training programs authorized by Section 3093 of the Labor Code, and similar purposes) applicable to the work to be done, for straight time, overtime, Saturday, Sunday and holiday work. The holiday wage rate listed shall be applicable to all holidays recognized in the collective bargaining agreement of the particular craft, classification or type of workmen concerned. The general prevailing wage rates and any applicable changes to these wage rates are available at the Labor Compliance Office at the offices of the District Director of Transportation for the district in which the work is situated. For work situated in District 9, the wage rates are available at the Labor Compliance Office at the offices of the District Director of Transportation for District 6, located at Fresno. General prevailing wage rates are also available from the California Department of Industrial Relations' Internet Web Site at: http://www.dir.ca.gov.

The wage rates determined by the Director of Industrial Relations for the project refer to expiration dates. Prevailing wage determinations with a single asterisk after the expiration date are in effect on the date of advertisement for bids and are good for the life of the contract. Prevailing wage determinations with double asterisks after the expiration date indicate that the wage rate to be paid for work performed after this date has been determined. If work is

to extend past this date, the new rate shall be paid and incorporated in the contract. The Contractor shall contact the Department of Industrial Relations as indicated in the wage rate determinations to obtain predetermined wage changes.

Pursuant to Section 1773.2 of the Labor Code, general prevailing wage rates shall be posted by the Contractor at a prominent place at the site of the work.

Changes in general prevailing wage determinations which conform to Labor Code Section 1773.6 and Title 8 California Code of Regulations Section 16204 shall apply to the project when issued by the Director of Industrial Relations at least 10 days prior to the date of the Notice to Contractors for the project.

The State will not recognize any claim for additional compensation because of the payment by the Contractor of any wage rate in excess of the prevailing wage rate set forth in the contract. The possibility of wage increases is one of the elements to be considered by the Contractor in determining the bid, and will not under any circumstances be considered as the basis of a claim against the State on the contract.

7-1.01A(2)(a) Travel and Subsistence Payments.— Attention is directed to the requirements of Section 1773.8 of the Labor Code. The Contractor shall make travel and subsistence payments to each workman, needed to execute the work, in accordance with the requirements in Labor Code Section 1773.8.

The first and second paragraphs of Section 7-1.01A(3), "Payroll Records," of the Standard Specifications are amended to read:

7-1.01A(3) Payroll Records.— Attention is directed to the provisions of Labor Code Section 1776, a portion of which is quoted below. Regulations implementing Labor Code Section 1776 are located in Sections 16016 through 16019 and Sections 16207.10 through 16207.19 of Title 8, California Code of Regulations.

"1776. (a) Each contractor and subcontractor shall keep accurate payroll records, showing the name, address, social security number, work classification, straight time and overtime hours worked each day and week, and the actual per diem wages paid to each journeyman, apprentice, worker, or other employee employed by him or her in connection with the public work. Each payroll record shall contain or be verified by a written declaration that it is made under penalty of perjury, stating both of the following:

(1) The information contained in the payroll record is true and correct.

(2) The employer has complied with the requirements of Sections 1771, 1811, and 1815 for any work performed by his or her employees on the public works project.

"(b) The payroll records enumerated under subdivision (a) shall be certified and shall be available for inspection at all reasonable hours at the principal office of the contractor on the following basis:

(1) A certified copy of an employee's payroll record shall be made available for inspection or furnished to the employee or his or her authorized representative on request.

(2) A certified copy of all payroll records enumerated in subdivision (a) shall be made available for inspection or furnished upon request to a representative of the body awarding the contract, the Division of Labor Standards Enforcement, and the Division of Apprenticeship Standards of the Department of Industrial Relations.

(3) A certified copy of all payroll records enumerated in subdivision (a) shall be made available upon request by the public for inspection or for copies thereof. However, a request by the public shall be made through either the body awarding the contract, the Division of Apprenticeship Standards, or the Division of Labor Standards Enforcement. If the requested payroll records have not been provided pursuant to paragraph (2), the requesting party shall, prior to being provided the records, reimburse the costs of preparation by the contractor, subcontractors, and the entity through which the request was made. The public shall not be given access to the records at the principal office of the contractor.

"(c) The certified payroll records shall be on forms provided by the Division of Labor Standards Enforcement or shall contain the same information as the forms provided by the division.

"(d) A contractor or subcontractor shall file a certified copy of the records enumerated in subdivision (a) with the entity that requested the records within 10 days after receipt of a written request.

"(e) Any copy of records made available for inspection as copies and furnished upon request to the public or any public agency by the awarding body, the Division of Apprenticeship Standards, or the Division of Labor Standards Enforcement shall be marked or obliterated in a manner so as to prevent disclosure of an individual's name, address, and social security number. The name and address of the contractor awarded the contract or the subcontractor performing the contract shall not be marked or obliterated.

"(f) The contractor shall inform the body awarding the contract of the location of the records enumerated under subdivision (a), including the street address, city and county, and shall, within five working days, provide a notice of a change of location and address.

"(g) The contractor or subcontractor shall have 10 days in which to comply subsequent to receipt of a written notice requesting the records enumerated in subdivision (a). In the event that the contractor or subcontractor fails to comply within the 10-day period, he or she shall, as a penalty to the state or political subdivision on whose behalf the contract is made or awarded, forfeit twenty-five dollars ($25) for each calendar day, or portion thereof, for each worker, until strict compliance is effectuated. Upon the request of the Division of Apprenticeship Standards or the

Division of Labor Standards Enforcement, these penalties shall be withheld from progress payments then due. A contractor is not subject to a penalty assessment pursuant to this section due to the failure of a subcontractor to comply with this section."

The penalties specified in subdivision (g) of Labor Code Section 1776 for noncompliance with the provisions of Section 1776 may be deducted from any moneys due or which may become due to the Contractor.

5-1.023 INDEMNIFICATION AND INSURANCE

Section 7-1.12, "Responsibility for Damage," of the Standard Specifications is deleted. All references to Section 7-1.12 in the Contract documents shall be deemed to mean Sections 7-1.121, "Indemnification," and 7-1.122, "Insurance," as added below.

The Standard Specifications is amended by adding the following Section 7-1.121, "Indemnification," and Section 7-1.122, "Insurance," before Section 7-1.125, "Legal Action Against the Department":

7-1.121 Indemnification.— With the exception that this section shall in no event be construed to require indemnification by the Contractor to a greater extent than permitted by law, the Contractor shall defend, indemnify and save harmless the State, including its officers, directors, agents (excluding agents who are design professionals), and employees, and each of them (Indemnitees), from any and all claims, demands, causes of action, damages, costs, expenses, actual attorneys' fees, losses or liabilities, in law or in equity, of every kind and nature whatsoever (Claims), arising out of or in connection with the Contractor's performance of this contract for:

A. Bodily injury including, but not limited to, bodily injury, sickness or disease, emotional injury or death to persons, including, but not limited to, the public, any employees or agents of the Contractor, State, Department, or any other contractor and;
B. Damage to property of anyone including loss of use thereof;

caused or alleged to be caused in whole or in part by any negligent or otherwise legally actionable act or omission of the Contractor or anyone directly or indirectly employed by the Contractor or anyone for whose acts the Contractor may be liable.

Except as otherwise provided by law, the indemnification provisions above shall apply regardless of the existence or degree of fault of Indemnitees. The Contractor, however, shall not be obligated to indemnify Indemnitees for Claims arising from conduct delineated in Civil Code section 2782. Further, the Contractor's indemnity obligation shall not extend to Claims to the extent they arise from any defective or substandard condition of the roadway which existed at or prior to the time the Contractor commenced work, unless this condition has been changed by the work or the scope of the work requires the Contractor to maintain existing Roadway facilities and the claim arises from the Contractor's failure to maintain. The Contractor's indemnity obligation shall extend to Claims arising after the work is completed and accepted only if these Claims are directly related to alleged acts or omissions of the Contractor which occurred during the course of the work. No inspection by the Department, its employees or agents shall be deemed a waiver by the Department of full compliance with the requirements of this section.

The Contractor's obligation to defend and indemnify shall not be excused because of the Contractor's inability to evaluate liability or because the Contractor evaluates liability and determines that the Contractor is not liable to the claimant. The Contractor will respond within 30 days to the tender of any claim for defense and indemnity by the State, unless this time has been extended by the State. If the Contractor fails to accept or reject a tender of defense and indemnity within 30 days, in addition to any other remedy authorized by law, so much of the money due the Contractor under and by virtue of the contract as shall reasonably be considered necessary by the Department, may be retained by the State until disposition has been made of the claim or suit for damages, or until the Contractor accepts or rejects the tender of defense, whichever occurs first.

With respect to third party claims against the Contractor, the Contractor waives any and all rights of any type to express or implied indemnity against the State, its directors, officers, employees, or agents (excluding agents who are design professionals).

7-1.122 Insurance .—Insurance shall conform to the following requirements:

7-1.122A Casualty Insurance .—The Contractor shall, at the Contractor's expense, procure and maintain insurance on all of its operations with companies acceptable to the Department as follows. All insurance shall be kept in full force and effect from the beginning of the work through final acceptance by the State. In addition, the Contractor shall maintain completed operations coverage with a carrier acceptable to the Department through the expiration of the patent deficiency in construction statute of repose set forth in Section 337.1 of the Code of Civil Procedure.

7-1.122A(1) Workers' Compensation and Employer's Liability Insurance .—Workers' Compensation insurance shall be provided as specified in Section 7-1.01A(6), "Workers' Compensation." Employer's Liability Insurance shall be provided in amounts not less than:

(a) $1 000 000 for each accident for bodily injury by accident.
(b) $1 000 000 policy limit for bodily injury by disease.
(c) $1 000 000 for each employee for bodily injury by disease.

If there is an exposure of injury to the Contractors' employees under the U.S. Longshoremen's and Harbor Workers' Compensation Act, the Jones Act or under laws, regulations or statutes applicable to maritime employees, coverage shall be included for such injuries or claims.

7-1.122A(2) Liability Insurance .—The Contractor shall carry General Liability and Umbrella or Excess Liability Insurance covering all operations by or on behalf of the Contractor providing insurance for bodily injury liability, and property damage liability for the limits of liability indicated below and including coverage for:

(a) premises, operations and mobile equipment
(b) products and completed operations
(c) broad form property damage (including completed operations)
(d) explosion, collapse and underground hazards
(e) personal injury
(f) contractual liability

7-1.122A(3) Liability Limits/Additional Insureds .—The limits of liability shall be at least:

(a) $1 000 000 for each occurrence (combined single limit for bodily injury and property damage).
(b) $2 000 000 aggregate for products-completed operations.
(c) $2 000 000 general aggregate. This general aggregate limit shall apply separately to the Contractor's work under this Agreement.
(d) $5 000 000 umbrella or excess liability. For projects over $25 000 000 only, an additional $10 000 000 umbrella or excess liability (for a total of $15 000 000). Umbrella or excess policy shall include products liability completed operations coverage and may be subject to $5 000 000 or $15 000 000 aggregate limits. Further, the umbrella or excess policy shall contain a clause stating that it takes effect (drops down) in the event the primary limits are impaired or exhausted.

The State and the Department, including their officers, directors, agents (excluding agents who are design professionals), and State employees, shall be named as additional insureds under the General Liability and Umbrella Liability Policies with respect to liability arising out of or connected with work or operations performed by or on behalf of the Contractor under this contract. Coverage for such additional insureds shall not extend to liability:

(1) arising from any defective or substandard condition of the Roadway which existed at or prior to the time the Contractor commenced work, unless such condition has been changed by the work or the scope of the work requires the Contractor to maintain existing Roadway facilities and the claim arises from the Contractor's failure to maintain; or
(2) for claims occurring after the work is completed and accepted unless these claims are directly related to alleged acts or omissions of the Contractor which occurred during the course of the work; or
(3) to the extent prohibited by Section 11580.04 of the Insurance Code.

The policy shall stipulate that the insurance afforded the additional insureds shall apply as primary insurance. Any other insurance or self insurance maintained by the Department or State will be excess only and shall not be called upon to contribute with this insurance. Such additional insured coverage shall be provided by a policy provision or by an endorsement providing coverage at least as broad as Additional Insured (Form B) endorsement form CG 2010, as published by the Insurance Services Office (ISO).

7-1.122B Automobile Liability Insurance .—The Contractor shall carry automobile liability insurance, including coverage for all owned, hired and non-owned automobiles. The primary limits of liability shall be not less than $1 000 000 combined single limit each accident for bodily injury and property damage. The umbrella or excess liability coverage required under Section 7-1.122A(3), "Liability Limits/Additional Insureds," shall also apply to automobile liability.

7-1.122C Policy Forms, Endorsements and Certificates. —The Contractor's General Liability Insurance shall be provided under Commercial General Liability policy form no. CG0001 as published by the Insurance Services Office (ISO) or under a policy form at least as broad as policy form no. CG0001.

Evidence of insurance in a form acceptable to the Department, including the required "additional insured" endorsements, shall be furnished by the Contractor to the Department at or prior to the pre-construction conference. The evidence of insurance shall provide that there will be no cancellation, lapse, or reduction of coverage without thirty (30) days' prior written notice to the Department. Certificates of Insurance, as evidence of required insurance, for the General Liability, Auto Liability and Umbrella-Excess Liability policies shall set forth deductible amounts applicable to each policy and all exclusions which are added by endorsement to each policy. The Department may expressly allow deductible clauses, which it does not consider excessive, overly broad, or harmful to the interests of the State. Standard ISO form CG 0001 or similar exclusions will be allowed provided they are not inconsistent with the requirements of this section. Allowance of any additional exclusions is at the discretion of the Department. Regardless of the allowance of exclusions or deductions by the Department, the Contractor shall be responsible for any deductible amount and shall warrant that the coverage provided to the Department is consistent with the requirements of this section.

7-1.122D Enforcement .—The Department may take any steps as are necessary to assure Contractor's compliance with its obligations. Should any insurance policy lapse or be canceled during the contract period the Contractor shall, within thirty (30) days prior to the effective expiration or cancellation date, furnish the Department with evidence of renewal or replacement of the policy. Failure to continuously maintain insurance coverage as herein provided is a material breach of contract. In the event the Contractor fails to maintain any insurance coverage required, the Department may, but is not required to, maintain this coverage and charge the expense to the Contractor or terminate this Agreement. The required insurance shall be subject to the approval of Department, but any acceptance of insurance certificates by the Department shall in no way limit or relieve the Contractor of the Contractor's duties and responsibilities under the Contract to indemnify, defend and hold harmless the State, its officers, agents, and employees. Insurance coverage in the minimum amounts set forth herein shall not be construed to relieve the Contractor for liability in excess of such coverage, nor shall it preclude the State from taking other actions as is available to it under any other provision of the contract or law. Failure of the Department to enforce in a timely manner any of the provisions of this section shall not act as a waiver to enforcement of any of these provisions at a later date.

7-1.122E Self-Insurance .—Self-insurance programs and self-insured retentions in insurance policies are subject to separate annual review and approval by the State of evidence of the Contractor's financial capacity to respond. Additionally, self-insurance programs or retentions must provide the State with at least the same protection from liability and defense of suits as would be afforded by first-dollar insurance.

7-1.122F Miscellaneous .—Nothing contained in the Contract is intended to make the public or any member thereof a third party beneficiary of the Insurance or Indemnity provisions of these Standard Specifications, nor is any term, condition or other provision of the Contract intended to establish a standard of care owed to the public or any member thereof.

5-1.025 ARBITRATION

The last paragraph in Section 9-1.10, "Arbitration," of the Standard Specifications, is amended to read:

Arbitration shall be initiated by a Complaint in Arbitration made in compliance with the requirements of those regulations. A Complaint in Arbitration by the Contractor shall be made not later than 90 days after the date of service in person or by mail on the Contractor of the final written decision by the Department on the claim.

5-1.03 PAYMENT OF WITHHELD FUNDS

Section 9-1.065, "Payment of Withheld Funds," of the Standard Specifications, is amended by adding the following after the third paragraph:

Alternatively, and subject to the approval of the Department, the payment of retentions earned may be deposited directly with a person licensed under Division 6 (commencing with Section 17000) of the Financial Code as the escrow agent. Upon written request of an escrow agent that has not been approved by the Department under subdivision (c) of Section 10263 of the Public Contract Code, the Department will provide written notice to that escrow agent within 10 business days of receipt of the request indicating the reason or reasons for not approving that escrow agent. The payments will be deposited in a trust account with a Federally chartered bank or savings association within 24 hours of receipt by the escrow agent. The Contractor shall not place any retentions with the escrow agent in excess of the coverage provided to that escrow agent pursuant to subdivision (b) of Section 17314 of the Financial Code. In all respects not inconsistent with subdivision (c) of Section 10263 of the Public Contract Code, the remaining provisions of Section 10263 of the Public Contract Code shall apply to escrow agents acting pursuant to subdivision (c) of Section 10263 of the Public Contract Code.

5-1.04 INTEREST ON PAYMENTS

Interest shall be payable on progress payments, payments after acceptance, final payments, extra work payments and claim payments as follows:

1. Unpaid progress payments, payment after acceptance and final payments shall begin to accrue interest 30 days after the Engineer prepares the payment estimate.
2. Unpaid extra work bills shall begin to accrue interest 30 days after preparation of the first pay estimate following the receipt of a properly submitted and undisputed extra work bill. To be properly submitted, the bill must be submitted within 7 days of the performance of the extra work and in accordance with the requirements of Section 9-1.03C, "Records," and Section 9-1.06, "Partial Payments," of the Standard Specifications. An undisputed extra work bill not submitted within 7 days of performance of the extra work will begin to accrue interest 30 days after the preparation of the second pay estimate following submittal of the bill.
3. The rate of interest payable for unpaid progress payments, payments after acceptance, final payments and extra work payments shall be 10 percent per annum.
4. The rate of interest payable on a claim, protest or dispute ultimately allowed under this contract shall be 6 percent per annum. Interest shall begin to accrue 61 days after the Contractor submits to the Engineer information in sufficient detail to enable the Engineer to ascertain the basis and amount of said claim, protest or dispute.

The rate of interest payable on any award in arbitration shall be 6 percent per annum if allowed under the provisions of Civil Code Section 3289.

5-1.05 PUBLIC SAFETY

The Contractor shall provide for the safety of traffic and the public in conformance with the provisions in Section 7-1.09, "Public Safety," of the Standard Specifications and these special provisions.

The Contractor shall install temporary railing (Type K) between a lane open to public traffic and an excavation, obstacle, or storage area when the following conditions exist:

(1) Excavations**.—**The near edge of the excavation is 3.6 m or less from the edge of the lane, except:

(a) Excavations covered with sheet steel or concrete covers of adequate thickness to prevent accidental entry by traffic or the public.
(b) Excavations less than 0.3-m deep.
(c) Trenches less than 0.3-m wide for irrigation pipe or electrical conduit, or excavations less than 0.3-m in diameter.
(d) Excavations parallel to the lane for the purpose of pavement widening or reconstruction.
(e) Excavations in side slopes, where the slope is steeper than 1:4 (vertical:horizontal).
(f) Excavations protected by existing barrier or railing.

(2) Temporarily Unprotected Permanent Obstacles**.—**The work includes the installation of a fixed obstacle together with a protective system, such as a sign structure together with protective railing, and the Contractor elects to install the obstacle prior to installing the protective system; or the Contractor, for the Contractor's convenience and with permission of the Engineer, removes a portion of an existing protective railing at an obstacle and does not replace such railing complete in place during the same day.

(3) Storage Areas**.—**Material or equipment is stored within 3.6 m of the lane and the storage is not otherwise prohibited by the provisions of the Standard Specifications and these special provisions.

The approach end of temporary railing (Type K), installed in conformance with the provisions in this section "Public Safety" and in Section 7-1.09, "Public Safety," of the Standard Specifications, shall be offset a minimum of 4.6 m from the edge of the traffic lane open to public traffic. The temporary railing shall be installed on a skew toward the edge of the traffic lane of not more than 0.3-m transversely to 3 m longitudinally with respect to the edge of the traffic lane. If the 4.6-m minimum offset cannot be achieved, the temporary railing shall be installed on the 10 to 1 skew to obtain the maximum available offset between the approach end of the railing and the edge of the traffic lane, and an array of temporary crash cushion modules shall be installed at the approach end of the temporary railing.

Temporary railing (Type K) shall conform to the provisions in Section 12-3.08, "Temporary Railing (Type K)," of the Standard Specifications. Temporary railing (Type K), conforming to the details shown on 1995 Standard Plan T3 or 1992 Standard Plan T3, may be used. Temporary railing (Type K) fabricated prior to January 1, 1993, and conforming to 1988 Standard Plan B11-30 may be used, provided the fabrication date is printed on the required Certificate of Compliance.

The fourteenth paragraph of Section 12-3.08, "Temporary Railing (Type K)," of the Standard Specifications is amended to read:

Each rail unit placed within 3 m of a traffic lane shall have a reflector installed on top of the rail as directed by the Engineer. A Type P marker panel shall also be installed at each end of railing installed adjacent to a two-lane, two-way

highway and at the end facing traffic of railing installed adjacent to a one-way roadbed. If the railing is placed on a skew, the marker shall be installed at the end of the skew nearest the traveled way. Type P marker panels shall conform to the provisions in Section 82, "Markers and Delineators," except that the Contractor shall furnish the marker panels.

Reflectors on temporary railing (Type K) shall conform to the provisions in "Approved Traffic Products" of these special provisions.

Temporary crash cushion modules shall conform to the provisions in "Temporary Crash Cushion Module" of these special provisions.

Except for installing, maintaining and removing traffic control devices, whenever work is performed or equipment is operated in the following work areas the Contractor shall close the adjacent traffic lane unless otherwise provided in the Standard Specifications and these special provisions:

Approach speed of public traffic (Posted Limit) (Kilometers Per Hour)	Work Areas
Over 72 (45 Miles Per Hour)	Within 1.8 m of a traffic lane but not on a traffic lane
56 to 72 (35 to 45 Miles Per Hour)	Within 0.9-m of a traffic lane but not on a traffic lane

The lane closure provisions of this section shall not apply if the work area is protected by permanent or temporary railing or barrier.

When traffic cones or delineators are used to delineate a temporary edge of traffic lane, the line of cones or delineators shall be considered to be the edge of traffic lane, however, the Contractor shall not reduce the width of an existing lane to less than 3 m without written approval from the Engineer.

When work is not in progress on a trench or other excavation that required closure of an adjacent lane, the traffic cones or portable delineators used for the lane closure shall be placed off of and adjacent to the edge of the traveled way. The spacing of the cones or delineators shall be not more than the spacing used for the lane closure.

Suspended loads or equipment shall not be moved nor positioned over public traffic or pedestrians.

Full compensation for conforming to the provisions in this section "Public Safety," including furnishing and installing temporary railing (Type K) and temporary crash cushion modules, shall be considered as included in the contract prices paid for the various items of work involved and no additional compensation will be allowed therefor.

5-1.06 SURFACE MINING AND RECLAMATION ACT

Attention is directed to the Surface Mining and Reclamation Act of 1975, commencing in Public Resources Code, Mining and Geology, Section 2710, which establishes regulations pertinent to surface mining operations.

Material from mining operations furnished for this project shall only come from permitted sites in compliance with the Surface Mining and Reclamation Act of 1975.

The requirements of this section shall apply to all materials furnished for the project, except for acquisition of materials in conformance with Section 4-1.05, "Use of Materials Found on the Work," of the Standard Specifications.

5-1.07 REMOVAL OF ASBESTOS AND HAZARDOUS SUBSTANCES

When the presence of asbestos or hazardous substances are not shown on the plans or indicated in the specifications and the Contractor encounters materials which the Contractor reasonably believes to be asbestos or a hazardous substance as defined in Section 25914.1 of the Health and Safety Code, and the asbestos or hazardous substance has not been rendered harmless, the Contractor may continue work in unaffected areas reasonably believed to be safe, and shall immediately cease work in the affected area and report the condition to the Engineer in writing.

In accordance with Section 25914.1 of the Health and Safety Code, all such removal of asbestos or hazardous substances including any exploratory work to identify and determine the extent of the asbestos or hazardous substance will be performed by separate contract.

If delay of work in the area delays the current controlling operation, the delay will be considered a right of way delay and the Contractor will be compensated for the delay as provided in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

5-1.08 YEAR 2000 COMPLIANCE

This contract is subject to Year 2000 Compliance for automated devices in the State of California. Year 2000 compliance is defined as follows:

> Year 2000 compliance for automated devices in the State of California is achieved when embedded functions have or create no logical or mathematical inconsistencies when dealing with dates prior to and beyond 1999. The year 2000 is recognized and processed as a leap year. The product must also operate accurately in the manner in which it was intended for date operation without requiring manual intervention.

The Contractor shall provide the Engineer a Certificate of Compliance from the manufacturer in accordance with the provisions of Section 6-1.07, "Certificates of Compliance," of the Standard Specifications for all automated devices furnished for the project.

5-1.085 BUY AMERICA REQUIREMENTS

Attention is directed to the "Buy America" requirements of the Surface Transportation Assistance Act of 1982 (Section 165) and the Intermodal Surface Transportation Efficiency Act of 1991 (ISTEA) Sections 1041(a) and 1048(a), and the regulations adopted pursuant thereto. In accordance with the law and regulations, all manufacturing processes for steel and iron materials furnished for incorporation into the work on this project shall occur in the United States; with the exception that pig iron and processed, pelletized and reduced iron ore manufactured outside of the United States may be used in the domestic manufacturing process for such steel and iron materials. The application of coatings, such as epoxy coating, galvanizing, painting and any other coating that protects or enhances the value of steel or iron materials shall be considered a manufacturing process subject to the "Buy America" requirements.

A Certificate of Compliance, conforming to the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications, shall be furnished for steel and iron materials. The certificates, in addition to certifying that the materials comply with the specifications, shall also specifically certify that all manufacturing processes for the materials occurred in the United States, except for the above exceptions.

The requirements imposed by the law and regulations do not prevent a minimal use of foreign steel and iron materials if the total combined cost of the materials used does not exceed one-tenth of one percent (0.1%) of the total contract cost or $2500, whichever is greater. The Contractor shall furnish the Engineer acceptable documentation of the quantity and value of any foreign steel and iron prior to incorporating the materials into the work.

5-1.09 SUBCONTRACTOR AND DBE RECORDS

The Contractor shall maintain records showing the name and business address of each first-tier subcontractor. The records shall also show the name and business address of every DBE subcontractor, DBE vendor of materials and DBE trucking company, regardless of tier. The records shall show the date of payment and the total dollar figure paid to all of these firms. DBE prime contractors shall also show the date of work performed by their own forces along with the corresponding dollar value of the work.

Upon completion of the contract, a summary of these records shall be prepared on Form CEM-2402 (F) and certified correct by the Contractor or the Contractor's authorized representative, and shall be furnished to the Engineer. The form shall be furnished to the Engineer within 90 days from the date of contract acceptance. $10,000 will be withheld from payment until the Form CEM-2402 (F) is submitted. The amount will be returned to the Contractor when a satisfactory Form CEM-2402 (F) is submitted.

Prior to the fifteenth of each month, the Contractor shall submit documentation to the Engineer showing the amount paid to DBE trucking companies listed in the Contractor's DBE information. This monthly documentation shall indicate the portion of the revenue paid to DBE trucking companies which is claimed toward DBE participation. The Contractor shall also obtain and submit documentation to the Engineer showing the amount paid by DBE trucking companies to all firms, including owner-operators, for the leasing of trucks. The DBE who leases trucks from a non-DBE is entitled to credit only for the fee or commission it receives as a result of the lease arrangement. The records must confirm that the amount of credit claimed toward DBE participation conforms with Section 2-1.02, /

The Contractor shall also obtain and submit documentation to the Engineer showing the truck number, owner's name, California Highway Patrol CA number, and if applicable, the DBE certification number of the owner of the truck for all trucks used during that month for which DBE participation will be claimed. This documentation shall be submitted on Form CEM-2404 (F).

5-1.093 DBE CERTIFICATION STATUS

If a DBE subcontractor is decertified during the life of the project, the decertified subcontractor shall notify the Contractor in writing with the date of decertification. If a subcontractor becomes a certified DBE during the life of the project, the subcontractor shall notify the Contractor in writing with the date of certification. The Contractor shall furnish the written documentation to the Engineer.

Upon completion of the contract, Form CEM-2403 (F) indicating the DBE's existing certification status shall be signed and certified correct by the Contractor. The certified form shall be furnished to the Engineer within 90 days from the date of contract acceptance.

5-1.095 PERFORMANCE OF DBE SUBCONTRACTORS AND SUPPLIERS

The DBEs listed by the Contractor in response to the provisions in Section 2-1.02B, "Submission of DBE Information," and Section 3, "Award and Execution of Contract," of these special provisions, which are determined by the Department to be certified DBEs, shall perform the work and supply the materials for which they are listed, unless the Contractor has received prior written authorization to perform the work with other forces or to obtain the materials from other sources.

Authorization to use other forces or sources of materials may be requested for the following reasons:

A. The listed DBE, after having had a reasonable opportunity to do so, fails or refuses to execute a written contract, when such written contract, based upon the general terms, conditions, plans and specifications for the project, or on the terms of such subcontractor's or supplier's written bid, is presented by the Contractor.
B. The listed DBE becomes bankrupt or insolvent.
C. The listed DBE fails or refuses to perform the subcontract or furnish the listed materials.
D. The Contractor stipulated that a bond was a condition of executing a subcontract and the listed DBE subcontractor fails or refuses to meet the bond requirements of the Contractor.
E. The work performed by the listed subcontractor is substantially unsatisfactory and is not in substantial conformance with the plans and specifications, or the subcontractor is substantially delaying or disrupting the progress of the work.
F. It would be in the best interest of the State.

The Contractor shall not be entitled to any payment for such work or material unless it is performed or supplied by the listed DBE or by other forces (including those of the Contractor) pursuant to prior written authorization of the Engineer.

5-1.097 SUBCONTRACTING

Attention is directed to the provisions in Section 8-1.01, "Subcontracting," of the Standard Specifications, and Section 2, "Proposal Requirements and Conditions," and Section 3, "Award and Execution of Contract," of these special provisions.

Section 8-1.01 of the Standard Specifications is amended by adding the following before the sixth paragraph:

Pursuant to the provisions of Section 6109 of the Public Contract Code, the Contractor shall not perform work on a public works project with a subcontractor who is ineligible to perform work on the public works project pursuant to Section 1777.1 or 1777.7 of the Labor Code.

Pursuant to the provisions of Section 1777.1 of the Labor Code, the Labor Commissioner publishes and distributes a list of contractors ineligible to perform work as a subcontractor on a public works project. This list of debarred contractors is available from the Department of Industrial Relations web site at:

http://www.dir.ca.gov/dir/Labor_law/DLSE/Debar.html.

The provisions in the third paragraph of Section 8-1.01, "Subcontracting," of the Standard Specifications, that the Contractor shall perform with the Contractor's own organization contract work amounting to not less than 50 percent of the original contract price, is not changed by the Federal Aid requirement specified under "Required Contract Provisions Federal-Aid Construction Contracts" in Section 14 of these special provisions that the Contractor perform not less than 30 percent of the original contract work with the Contractor's own organization.

Each subcontract and any lower tier subcontract that may in turn be made shall include the "Required Contract Provisions Federal-Aid Construction Contracts" in Section 14 of these special provisions. This requirement shall be enforced as follows:

A. Noncompliance shall be corrected. Payment for subcontracted work involved will be withheld from progress payments due, or to become due, until correction is made. Failure to comply may result in termination of the contract.

The DBE information furnished under Section 2-1.02B, "Submission of DBE Information," of these special provisions is in addition to the subcontractor information required to be furnished under Section 8-1.01, "Subcontracting," and Section 2-1.054, "Required Listing of Proposed Subcontractors," of the Standard Specifications.

In conformance with the Federal DBE regulations Sections 26.53(f)(1) and 26.53(f)(2) Part 26, Title 49 CFR:

A. The Contractor shall not terminate for convenience a DBE subcontractor listed in response to Section 2-1.02B, "Submission of DBE Information," and then perform that work with its own forces, or those of an affiliate without the written consent of the Department, and
B. If a DBE subcontractor is terminated or fails to complete its work for any reason, the Contractor will be required to make good faith efforts to substitute another DBE subcontractor for the original DBE subcontractor, to the extent needed to meet the contract goal.

The requirement in Section 2-1.02, "Disadvantaged Business Enterprise (DBE)," of these special provisions that DBEs must be certified on the date bids are opened does not apply to DBE substitutions after award of the contract.

5-1.098 PROMPT PROGRESS PAYMENT TO SUBCONTRACTORS

Attention is directed to the provisions in Sections 10262 and 10262.5 of the Public Contract Code and Section 7108.5 of the Business and Professions Code concerning prompt payment to subcontractors.

5-1.099 PROMPT PAYMENT OF WITHHELD FUNDS TO SUBCONTRACTORS

The Contractor shall return all moneys withheld in retention from the subcontractor within 30 days after receiving payment for work satisfactorily completed, even if the other contract work is not completed and has not been accepted in conformance with Section 7-1.17, "Acceptance of Contract," of the Standard Specifications. This requirement shall not be construed to limit or impair any contractual, administrative, or judicial remedies otherwise available to the Contractor or subcontractor in the event of a dispute involving late payment or nonpayment by the Contractor or deficient subcontract performance or noncompliance by a subcontractor.

5-1.10 PARTNERING

The State will promote the formation of a "Partnering" relationship with the Contractor in order to effectively complete the contract to the benefit of both parties. The purpose of this relationship will be to maintain cooperative communication and mutually resolve conflicts at the lowest possible management level.

A one-day “Training in Partnering Concepts” forum will be conducted regardless of whether the Contractor requests the formation of a “Partnering” relationship. The forum will be conducted locally for the Contractor and the Engineer’s project representatives. The Contractor shall be represented by a minimum of two representatives, one being the Contractor’s authorized representative pursuant to Section 5-1.06, “Superintendence,” of the Standard Specifications. If, upon the Contractor’s request, “Partnering” is approved by the Engineer, “Training in Partnering Concepts” shall be conducted prior to the “Partnering” workshop. Scheduling of “Training in Partnering Concepts,” selection of the Engineer’s representatives to participate in “Training of Partnering Concepts,” and selection of the partnering concepts trainer and site shall be as determined by the Engineer.

The Contractor may request the formation of a "Partnering" relationship by submitting a request in writing to the Engineer after approval of the contract. If the Contractor's request for "Partnering" is approved by the Engineer, scheduling of a "Partnering" workshop, selecting the "Partnering" facilitator and workshop site, and other administrative details shall be as agreed to by both parties.

The costs involved in providing a trainer and site for the "Training in Partnering Concepts" forum will be borne by the State. The Contractor shall pay all compensation for the wages and expenses of the facilitator and of the expenses for obtaining the workshop site. The State will reimburse the Contractor for these costs as extra work in conformance with the provisions in Section 4-1.03D of the Standard Specifications. Full compensation for the wages and expenses of the Contractor's representatives, including travel costs, shall be considered as included in the contract prices paid for the various items of work and no additional compensation will be allowed therefor.

The costs involved in providing a “Partnering” facilitator and a workshop site will be borne equally by the State and the Contractor. The Contractor shall pay all compensation for the wages and expenses of the facilitator and of the expenses for obtaining the workshop site. The State's share of such costs will be reimbursed to the Contractor in a change order written by the Engineer.

Markups will not be added to the costs of “Training in Partnering Concepts” or the costs of providing a “Partnering” facilitator and workshop site. All other costs associated with the "Partnering" relationship will be borne separately by the party incurring the costs.

The establishment of a "Partnering" relationship will not change or modify the terms and conditions of the contract and will not relieve either party of the legal requirements of the contract.

5-1.11 DISPUTES REVIEW BOARD

To assist in the resolution of disputes or potential claims arising out of the work of this project, a Disputes Review Board, hereinafter referred to as the "DRB", shall be established by the Engineer and Contractor cooperatively upon approval of the contract. The DRB is intended to assist the contract administrative claims resolution process as set forth in the provisions of Section 9-1.04, "Notice of Potential Claim," and Section 9-1.07B, "Final Payment and Claims," of the Standard Specifications. The DRB shall not be considered to serve as a substitute for any requirements in the specifications in regard to filing of potential claims. The requirements and procedures established in this special provision shall be considered as an essential prerequisite to filing a claim, for arbitration or for litigation prior or subsequent to project completion.

The DRB shall be utilized when dispute or potential claim resolution at the job level is unsuccessful. The DRB shall function until the day of acceptance of the contract, at which time the work of the DRB will cease except for completion of unfinished dispute hearings and reports. After acceptance of the contract any disputes or potential claims that the Contractor wants to pursue that have not been settled, shall be stated or restated, by the Contractor, in response to the Proposed Final Estimate within the time limits provided in Section 9-1.07B, "Final Payment and Claims," of the Standard Specifications. The State will review those claims in accordance with Section 9-1.07B, of the Standard Specifications. Following the completion of the State's administrative claims procedure, the Contractor may resort to arbitration as provided in Section 9-1.10, "Arbitration," of the Standard Specifications.

Disputes, as used in this section, shall include all differences of opinion, properly noticed as provided hereinafter, between the State and Contractor on matters related to the work and other subjects considered by the State or Contractor, or by both, to be of concern to the DRB on this project, except matters relating to Contractor, subcontractor or supplier claims not actionable against the State as specified in these special provisions. Whenever the term "dispute" or "disputes" is used herein, it shall be deemed to include potential claims as well as disputes.

The DRB shall serve as an advisory body to assist in the resolution of disputes between the State and the Contractor, hereinafter referred to as the "parties". The DRB shall consider disputes referred to it, and furnish written reports containing findings and recommendations pertaining to those disputes, to the parties to aid in resolution of the differences between them. DRB findings and recommendations are not binding on the parties.

The DRB shall consist of one member selected by the State, one member selected by the Contractor, and a third member selected by the first two members and approved by both the State and the Contractor. The third member shall act as DRB Chairperson.

The first two DRB members shall select a third DRB member subject to the mutual approval of the parties, or may mutually concur on a list of potentially acceptable third DRB members and submit the list to the parties for final selection and approval of the third member. The goal in selection of the third member is to complement the professional experience of the first two members, and to provide leadership for the DRB's activities.

No DRB member shall have prior direct involvement in this contract, and no member shall have a financial interest in this contract or the parties thereto, within a period of 6 months prior to award of this contract, or during the contract, except as follows:

1. Compensation for services on this DRB.
2. Ownership interest in a party or parties, documented by the prospective DRB member, that has been reviewed and determined in writing by the State to be sufficiently insignificant to render the prospective member acceptable to the State.
3. Service as a member of other Disputes Review Boards on other contracts.
4. Retirement payments or pensions received from a party that are not tied to, dependent on or affected by the net worth of the party.
5. The above provisions apply to any party having a financial interest in this contract; including but not limited to contractors, subcontractors, suppliers, consultants, and legal and business services.

DRB members shall be especially knowledgeable in the type of construction and contract documents potentially anticipated by the contract, and shall discharge their responsibilities impartially and as an independent body considering the facts and circumstances related to the matters under consideration, applicable laws and regulations, and the pertinent provisions of the contract.

The State and the Contractor shall select their respective DRB members, in accordance with the terms and conditions of the Disputes Review Board Agreement and these provisions, within 45 days of the approval of the contract. Each party shall provide written notification to the other of the name of their selected DRB member along with the prospective member's written disclosure statement.

Before their appointments are final, the first two prospective DRB members shall submit complete disclosure statements to both the State and the Contractor. The statement shall include a resume of the prospective member's experience, together with a declaration describing all past, present and anticipated or planned future relationships, including indirect relationships through the prospective member's primary or full-time employer, to this project and with all parties involved in this construction contract; including, but not limited to, any relevant subcontractors or suppliers to the parties, the parties' principals or the parties' counsel. The DRB members shall also include a full disclosure of close professional or personal relationships with all key members of all parties to the contract. Either the Contractor or the State may object to the others nominee and that person will not be selected for the DRB. No reason need be given for the first objection. Objections to subsequent nominees must be based on a specific breech or violation of nominee responsibilities under this specification. A different person shall then be nominated within 14 Days. The third DRB member shall supply a full disclosure statement to the first two DRB members and to the parties prior to appointment. Either party may reject any of the three prospective DRB members who fail to fully comply with all required employment and financial disclosure conditions of DRB membership as described in the Disputes Review Board Agreement and elsewhere herein. A copy of the Disputes Review Board Agreement is included in this special provision.

The first duty of the State and Contractor selected members of the DRB is to select and recommend prospective third member(s) to the parties for final selection and approval. The first two DRB members shall proceed with the selection of the third DRB member immediately upon receiving written notification from the State of their selection, and shall provide their recommendation simultaneously to the parties within 21 days of the notification.

An impasse shall be considered to have been reached if the parties are unable to approve a third member within 14 days of receipt of the recommendation of the first two DRB members, or if the first two members are unable to agree upon a recommendation within the 14 day time limit allowed in the preceding paragraph. In the event of an impasse in selection of the third DRB member, the State and the Contractor shall each propose three candidates for the third position. The parties shall select all candidates proposed under this paragraph from the current list of arbitrators certified by the Public Works Contract Arbitration Committee created by Article 7.2 (commencing with Section 10245) of the State Contract Act. The first two DRB members shall then select one of the 6 proposed candidates in a blind draw.

The Contractor, the State, and all three members of the DRB shall complete and adhere to the Disputes Review Board Agreement in administration of this DRB within 14 days of the parties’ concurrence in the selection of the third member. The State authorizes the Engineer to execute and administer the terms of the Agreement. The person(s) designated by the Contractor as authorized to execute Contract Change Orders shall be authorized to execute and administer the terms of this

agreement, or to delegate the authority in writing. The operation of the DRB shall be in conformance with the terms of the Disputes Review Board Agreement.

The State and the Contractor shall bear the costs and expenses of the DRB equally. Each DRB board member shall be compensated at an agreed rate of $1,000.00 per day if time spent per meeting, including all on-site time plus one hour of travel time, is greater than four hours. Each DRB board member shall be compensated at an agreed rate of $600.00 per day if time spent per meeting, including all on-site time plus one hour of travel time, is less than or equal to four hours. The agreed rates shall be considered full compensation for on-site time, travel expenses, transportation, lodging, time for travel and incidentals for each day, or portion thereof, that the DRB member is at an authorized DRB meeting. No additional compensation will be made for time spent by DRB members in review and research activities outside the official DRB meetings unless that time, (such as time spent evaluating and preparing recommendations on specific issues presented to the DRB), has been specifically agreed to in advance by the State and Contractor. Time away from the project, that has been specifically agreed to in advance by the parties, will be compensated at an agreed rate of $100.00 per hour. The agreed amount of $100.00 per hour shall include all incidentals including any expenses for telephone, fax and computer services. Members serving on more than one DRB, regardless of the number of meetings per day, shall not be paid more than the all inclusive rate per day or rate per hour for an individual project. The State will provide, at no cost to the Contractor, administrative services such as conference facilities and secretarial services to the DRB. These special provisions and the Disputes Review Board Agreement state provisions for compensation and expenses of the DRB. All DRB members shall be compensated at the same daily and hourly rate. The Contractor shall make direct payments to each DRB member for their participation in authorized meetings and approved hourly rate charges from invoices submitted by each DRB member. The State will reimburse the Contractor for its share of the costs. There will be no markups applied to any expenses connected with the DRB, either by the DRB members or by the Contractor when requesting payment of the State's share of DRB expenses.

Service of a DRB member may be terminated at any time with not less than 14 days notice as follows:

1. The State may terminate service of the State appointed member.
2. The Contractor may terminate service of the Contractor appointed member.
3. Upon the written recommendation of the State and Contractor members for the removal of the third member.
4. Upon resignation of a member.

When a member of the DRB is replaced, the replacement member shall be appointed in the same manner as the replaced member was appointed. The appointment of a replacement DRB member will begin promptly upon determination of the need for replacement and shall be completed within 14 days. Changes in either of the DRB members chosen by the two parties will not require re-selection of the third member, unless both parties agree to such re-selection in writing. The Disputes Review Board Agreement shall be amended to reflect the change of a DRB member.

The following procedure shall be used for dispute resolution:

1. If the Contractor objects to any decision, act or order of the Engineer, the Contractor shall give written notice of potential claim as specified in Section 9-1.04, "Notice of Potential Claim," of the Standard Specifications, including provision of applicable cost documentation; or file written protests or notices pursuant to Sections 4-1.03A, "Procedure and Protest", 8-1.06, "Time of Completion", 8-1.07, "Liquidated Damages", or 8-1.10, "Utility and Non-Highway Facilities" of the Standard Specifications.
2. The Engineer will respond, in writing, to the Contractor's written protest or notice within 14 days of receipt of the written protest or notice.
3. Within 14 days after receipt of the Engineer's written response, the Contractor shall, if the Contractor still objects, file a written reply with the Engineer, stating clearly and in detail the basis of the objection.
4. Following the Contractor's objection to the Engineer's decision, the Contractor shall refer the dispute to the DRB if the Contractor wishes to further pursue the objection to the Engineer’s decision. The Contractor shall make the referral in writing to the DRB, simultaneously copied to the State, within 21 days after receipt of the written reply from the Engineer. The written dispute referral shall describe the disputed matter in individual discrete segments so that it will be clear to both parties and the DRB what discrete elements of the dispute have been resolved, and which remain unresolved.
5. The Contractor, by failing to submit the written notice of referral of the matter to the DRB, within 21 days after receipt of the State’s written reply, waives any future claims on the matter in contention.
6. The Contractor and the State shall each be afforded an opportunity to be present and to be heard by the DRB, and to offer evidence. Either party furnishing any written evidence or documentation to the DRB must furnish copies of such information to the other party a minimum of 14 days prior to the date the DRB is scheduled to convene the hearing for the dispute. Either party shall produce such additional evidence as the DRB may deem necessary to reach an understanding and determination of the dispute. The party furnishing additional evidence shall furnish copies of such additional evidence to the other party at the same time the evidence is provided to the DRB. The DRB will not consider any evidence not furnished in accordance with the terms specified herein.
7. The DRB shall furnish a report, containing findings and recommendations as described in the Disputes Review Board Agreement, in writing to both the State and the Contractor. The DRB shall complete its reports, including

minority opinion if any, and submit them to the parties within 30 days of the DRB hearing, except that time extensions may be granted at the request of the DRB with the written concurrence of both parties. The report shall include the facts and circumstances related to the matters under consideration, applicable laws and regulations, the pertinent provisions of the Contract and the actual costs and time incurred as shown on the Contractor's cost accounting records.

8. Within 30 days after receiving the DRB's report, both the State and the Contractor shall respond to the DRB in writing signifying that the dispute is either resolved or remains unresolved. Failure to provide the written response within the time specified, or a written rejection of the DRB's recommendation presented in the report by either party, shall conclusively indicate that the party(s) failing to respond accepts the DRB recommendation. Immediately after responses have been received by both parties, the DRB will provide copies of both responses to the parties simultaneously. Either party may request clarification of elements of the DRB's report from the DRB prior to responding to the report. The DRB will consider any clarification request only if submitted within 10 days of receipt of the DRB's report, and if submitted simultaneously in writing to both the DRB and the other party. Each party may submit only one request for clarification for any individual DRB report. The DRB shall respond, in writing, to requests for clarification within 10 days of receipt of such requests.
9. The DRB's recommendations, stated in the DRB's reports, are not binding on either party. Either party may seek a reconsideration of a recommendation of the DRB. The DRB shall only grant a reconsideration based upon submission of new evidence and if the request is submitted within the 30 day time limit specified for response to the DRB's written report. Each party may submit only one request for reconsideration regarding any individual DRB recommendation.
10. If the State and the Contractor are able to resolve their dispute with the aid of the DRB's report, the State and Contractor shall promptly accept and implement the recommendations of the DRB.
11. The State or the Contractor shall not call members who served on the DRB for this contract as witnesses in arbitration proceedings which may arise from this contract, and all documents created by the DRB shall be inadmissible as evidence in subsequent arbitration proceedings, except the DRB’s final written reports on each issue brought before it..
12. The State and Contractor shall jointly indemnify and hold harmless the DRB members from and against all claims, damages, losses, and expenses, including but not limited to attorney's fees, arising out of and resulting from the findings and recommendations of the DRB.
13. The DRB members shall have no claim against the State or the Contractor, or both, from any claimed harm arising out of the parties' evaluations of the DRB's report.

Disputes Involving Subcontractor Claims.—For purposes of this section, a "subcontractor claim" shall include any claim by a subcontractor (including also any pass through claims by a lower tier subcontractor or supplier) against the Contractor that is actionable by the Contractor against the Department which arises from the work, services, or materials provided or to be provided in connection with the contract. If the Contractor determines to pursue a dispute against the Department that includes a subcontractor claim, the dispute shall be processed and resolved in accordance with these special provisions and in accordance with the following:

1. The Contractor shall identify clearly in all submissions pursuant to this section, that portion of the dispute that involves a subcontractor claim or claims.
2. The Contractor shall include, as part of its submission pursuant to Step 4 above, a certification (False Claims Act Certification) by the subcontractor's or supplier's officer, partner, or authorized representative with authority to bind the subcontractor and with direct knowledge of the facts underlying the subcontractor claim. The Contractor also shall submit a certification that the subcontractor claim is acknowledged and forwarded by the Contractor. The form for these certifications are available from the Engineer.
3. At any DRB meeting on a dispute that includes one or more subcontractor claims, the Contractor shall require that each subcontractor that is involved in the dispute have present an authorized representative with actual knowledge of the facts underlying the subcontractor claim to assist in presenting the subcontractor claim and to answer questions raised by the DRB members or the Department's representatives.
4. Failure by the Contractor to declare a subcontractor claim on behalf of its subcontractor (including lower tier subcontractors' and suppliers' pass through claims) at the time of submission of the Contractor's claims, as provided hereunder, shall constitute a release of the Department by the Contractor on account of such subcontractor claim.
5. The Contractor shall include in all subcontracts under this contract that subcontractors and suppliers of any tier (a) agree to submit subcontractor claims to the Contractor in a proper form and in sufficient time to allow processing by the Contractor in accordance with the Dispute Review Board resolution specifications; (b) agree to be bound by the terms of the Dispute Review Board provisions to the extent applicable to subcontractor claims; (c) agree that, to the extent a subcontractor claim is involved, completion of all steps required under these Dispute Review Board special provisions shall be a condition precedent to pursuit by the subcontractor of any other remedies permitted by law, including without limitation of a lawsuit against the Contractor; and (d) agree that the existence of a dispute resolution process for disputes involving subcontractor claims shall not be deemed to create any claim, right, or cause of action by any subcontractor or supplier against the Department.

Notwithstanding the foregoing, this Dispute Review Board special provision shall not apply to, and the DRB shall not have the authority to consider, any subcontractor claim between the subcontractor(s) or supplier(s) and the Contractor that is not actionable by the Contractor against the Department.

A copy of the "Disputes Review Board Agreement" to be executed by the Contractor, State and the three DRB members after approval of the contract follows:

DISPUTES REVIEW BOARD AGREEMENT

_____
(Contract Identification)

Contract No. _____

THIS DISPUTES REVIEW BOARD AGREEMENT , hereinafter called "AGREEMENT", made and entered into this _____ day of _____, _____, between the State of California, acting through the California Department of Transportation and the Director of Transportation, hereinafter called the "STATE"; _____ hereinafter called the "CONTRACTOR"; and the Disputes Review Board, hereinafter called the "DRB" consisting of the following members:

_____ ,
(Contractor Appointee)

_____ ,
(State Appointee)

and _____
(Third Person)

WITNESSETH, that

WHEREAS, the STATE and the CONTRACTOR, hereinafter called the "parties", are now engaged in the construction on the State Highway project referenced above; and

WHEREAS the special provisions for the above referenced contract provides for the establishment and operation of the DRB to assist in resolving disputes; and

WHEREAS, the DRB is composed of three members, one selected by the STATE, one selected by the CONTRACTOR, and the third member selected by the other two members and approved by the parties;

NOW THEREFORE, in consideration of the terms, conditions, covenants, and performance contained herein, or attached and incorporated and made a part hereof, the STATE, the CONTRACTOR, and the DRB members hereto agree as follows:

I
DESCRIPTION OF WORK

To assist in the resolution of disputes between the parties, the contract provides for the establishment and the operation of the DRB. The intent of the DRB is to fairly and impartially consider disputes placed before it and provide written recommendations for resolution of these disputes to both parties. The members of this DRB shall perform the services necessary to participate in the DRB's actions as designated in Section II, Scope of Work.

II
SCOPE OF WORK

The scope of work of the DRB includes, but is not limited to, the following:

A. Objective

The principal objective of the DRB is to assist in the timely resolution of disputes between the parties arising from performance of this contract. It is not intended for either party to default on their normal responsibility to amicably and fairly settle their differences by indiscriminately assigning them to the DRB. It is intended that the mere existence of the DRB will

encourage the parties to resolve disputes without resorting to this review procedure. But when a dispute which is serious enough to warrant the DRB's review does develop, the process for prompt and efficient action will be in place.

B. Procedures

The DRB shall render written reports on disputes between the parties arising from the construction contract. Prior to consideration of a dispute, the DRB shall establish rules and regulations that will govern the conduct of its business and reporting procedures in accordance with the requirements of the contract and the terms of this AGREEMENT. DRB recommendations, resulting from its consideration of a dispute, shall be furnished in writing to both parties. The recommendations shall be based on the pertinent contract provisions, and the facts and circumstances involved in the dispute. The recommendations shall find one responsible party in a dispute; shared or "jury" determinations shall not be rendered.

The DRB shall refrain from officially giving any advice or consulting services to anyone involved in the contract. The individual members shall act in a completely independent manner and while serving as members of the DRB shall have no consulting business connections with either party or its principals or attorneys or any other affiliates (subcontractors, suppliers, etc.) who have a beneficial interest in the contract.

During scheduled meetings of the DRB as well as during dispute hearings, DRB members shall refrain from expressing opinions on the merits of statements on matters under dispute or potential dispute. Opinions of DRB members expressed in private sessions shall be kept strictly confidential. Individual DRB members shall not meet with, or discuss contract issues with individual parties, except as directed by the DRB Chairperson. Any such discussions or meetings shall be disclosed to both parties. Any other discussions regarding the project between the DRB members and the parties shall be in the presence of all three members and both parties. Individual DRB members shall not undertake independent investigations of any kind pertaining to disputes or potential disputes, except with the knowledge of both parties and as expressly directed by the DRB Chairperson.

C. Construction Site Visits, Progress Meetings and Field Inspections

The DRB members shall visit the project site and meet with representatives of the parties to keep abreast of construction activities and to develop familiarity with the work in progress. All scheduled progress meetings shall be held at or near the job site. The DRB shall meet at least once at the start of the project, and at least once every six months thereafter. The frequency, exact time, and duration of additional site visits and progress meetings shall be as recommended by the DRB and approved by the parties consistent with the construction activities or matters under consideration and dispute. Each meeting shall consist of a round table discussion and a field inspection of the work being performed on the contract, if necessary. Each meeting shall be attended by representatives of both parties. The agenda shall generally be as follows:

1. Meeting opened by the DRB Chairperson.
2. Remarks by the STATE's representative.
3. A description by the CONTRACTOR's representative of work accomplished since the last meeting; the current schedule status of the work; and a forecast for the coming period.
4. An outline by the CONTRACTOR's representative of potential problems and a description of proposed solutions.
5. An outline by the STATE's representative of the status of the work as the STATE views it.
6. A brief description by the CONTRACTOR's or STATE's representative of potential claims or disputes which have surfaced since the last meeting.
7. A summary by the STATE's representative, the CONTRACTOR's representative, or the DRB of the status of past disputes and claims.

The STATE's representative will prepare minutes of all regular meetings and circulate them for revision and approval by all concerned.

The field inspection shall cover all active segments of the work, the DRB being accompanied by both parties' representatives. The field inspection may be waived upon mutual agreement of the parties.

D. DRB Consideration and Handling of Disputes

Upon receipt by the DRB of a written referral of a dispute, the DRB shall convene to review and consider the dispute. The DRB shall determine the time and location of DRB hearings, with due consideration for the needs and preferences of the parties while recognizing the paramount importance of speedy resolution of issues. If the matter is not urgent, it may be scheduled for the time of the next scheduled DRB visit to the project. For an urgent matter, and upon the request of either party, the DRB shall meet at its earliest convenience.

Normally, hearings shall be conducted at or near the project site. However, any location which would be more convenient and still provide all required facilities and access to necessary documentation shall be satisfactory.

Both parties shall be given the opportunity to present their evidence at these hearings. It is expressly understood that the DRB members are to act impartially and independently in the consideration of the contract provisions, and the facts and conditions surrounding any dispute presented by either party, and that the recommendations concerning any such dispute are advisory and nonbinding on the parties.

The DRB may request that written documentation and arguments from both parties be sent to each DRB member, through the DRB Chairperson, for review before the hearing begins. A party furnishing any written documentation to the DRB shall furnish copies of such information to the other party at the same time that such information is supplied to the DRB.

DRB hearings shall be informal. There shall be no testimony under oath or cross-examination. There shall be no reporting of the procedures by a shorthand reporter or by any electronic means. Documents and verbal statements shall be received by the DRB in accordance with acceptance standards established by the DRB. Said standards need not comply with prescribed legal laws of evidence.

The third DRB member shall act as Chairperson for dispute hearings and all other DRB activities. The parties shall have a representative at all hearings. Failure to attend a duly noticed meeting by either of the parties shall be conclusively considered by the DRB as indication that the non-attending party considers any written submittals as their entire and complete argument. The claimant shall discuss the dispute, followed by the other party. Each party shall then be allowed one or more rebuttals until all aspects of the dispute are thoroughly covered. DRB members may ask questions, seek clarification, or request further data from either of the parties. The DRB may request from either party documents or information that would assist the DRB in making its findings and recommendations including, but not limited to, documents used by the CONTRACTOR in preparing the bid for the project. A refusal by a party to provide information requested by the DRB may be considered by the DRB as an indication that the requested material would tend to disprove that party's position. Claims shall not necessarily be computed by merely subtracting bid price from the total cost of the affected work. However, if any claims are based on the "total cost method", then, to be considered by the DRB, they shall be supported by evidence furnished by the CONTRACTOR that (1) the nature of the dispute(s) makes it impossible or impracticable to determine cost impacts with a reasonable degree of accuracy, (2) the CONTRACTOR's bid estimate was realistic, (3) the CONTRACTOR's actual costs were reasonable, and (4) the CONTRACTOR was not responsible for the added expenses. As to any claims based on the CONTRACTOR's field or home office accounting records, those claims shall be supported by an audit report of an independent Certified Public Accountant unless the contract includes special provisions that provide for an alternative method to calculate unabsorbed home office overhead. Any of those claims shall also be subject to audit by the DRB with the concurrence of the parties. In large or complex cases, additional hearings may be necessary in order to consider all the evidence presented by both parties. All involved parties shall maintain the confidentiality of all documents and information, as provided in this AGREEMENT.

During dispute hearings, no DRB member shall express an opinion concerning the merit of any facet of the case. All DRB deliberations shall be conducted in private, with all interim individual views kept strictly confidential.

After hearings are concluded, the DRB shall meet in private and reach a conclusion supported by two or more members. Private sessions of the DRB may be held at a location other than the job site or by electronic conferencing as deemed appropriate, in order to expedite the process.

The DRB's findings and recommendations, along with discussion of reasons therefor, shall then be submitted as a written report to both parties. Recommendations shall be based on the pertinent contract provisions, applicable laws and regulations, and facts and circumstances related to the dispute. The report shall be thorough in discussing the facts considered, the contract language, law or regulation viewed by the DRB as pertinent to the issues, and the DRB's interpretation and philosophy in arriving at its conclusions and recommendations. The DRB's report shall stand on its own, without attachments or appendices. The DRB chairman shall complete and furnish a summary report to the DRB Program Manager, Construction Program, M.S. 44, P.O. Box 942874, Sacramento, CA 94274.

With prior written approval of both parties, the DRB may obtain technical services necessary to adequately review the disputes presented; including audit, geotechnical, schedule analysis and other services. The parties' technical staff may supply those services as appropriate. The cost of any technical services, as agreed to by the parties, shall be borne equally by the two parties as specified in an approved contract change order. The CONTRACTOR will not be entitled to markups for the payments made for these services.

The DRB shall resist submittal of incremental portions of information by either party, in the interest of making a fully-informed decision and recommendation.

The DRB shall make every effort to reach a unanimous decision. If this proves impossible, the dissenting member shall prepare a minority opinion, which shall be included in the DRB's report.

Although both parties should place weight upon the DRB's recommendations, they are not binding. Either party may appeal a recommendation to the DRB for reconsideration. However, reconsideration shall only be allowed when there is new evidence to present, and the DRB shall accept only one appeal from each party pertaining to any individual DRB recommendation. The DRB shall hear appeals in accordance with the terms described in the Section entitled "Disputes Review Board" in the special provisions.

E. DRB Member Replacement

Should the need arise to appoint a replacement DRB member, the replacement DRB member shall be appointed in the same manner as the original DRB members were appointed. The selection of a replacement DRB member shall begin promptly upon notification of the necessity for a replacement and shall be completed within 14 days. This AGREEMENT will be amended to indicate change in DRB membership.

III

CONTRACTOR RESPONSIBILITIES

The CONTRACTOR shall furnish to each DRB member one copy of all pertinent documents which are or may become necessary for the DRB to perform their function. Pertinent documents are any drawings or sketches, calculations, procedures, schedules, estimates, or other documents which are used in the performance of the work or in justifying or substantiating the CONTRACTOR's position. The CONTRACTOR shall also furnish a copy of such pertinent documents to the STATE, in accordance with the terms outlined in the special provisions.

IV

STATE RESPONSIBILITIES

The STATE will furnish the following services and items:

A. Contract Related Documents

The STATE will furnish to each DRB member one copy of Notice to Contractors and Special Provisions, Proposal and Contract, Plans, Standard Specifications, and Standard Plans, change orders, written instructions issued by the STATE to the CONTRACTOR, or other documents pertinent to any dispute that has been referred to the DRB and necessary for the DRB to perform its function.

B. Coordination and Services

The STATE, through the Engineer, will, in cooperation with the CONTRACTOR, coordinate the operations of the DRB. The Engineer will arrange or provide conference facilities at or near the project site and provide secretarial and copying services to the DRB without charge to the CONTRACTOR.

V

TIME FOR BEGINNING AND COMPLETION

Once established, the DRB shall be in operation until the day of acceptance of the contract. The DRB members shall not begin any work under the terms of this AGREEMENT until authorized in writing by the STATE.

VI

PAYMENT

A. All Inclusive Rate Payment

The STATE and the CONTRACTOR shall bear the costs and expenses of the DRB equally. Each DRB board member shall be compensated at an agreed rate of $1,000.00 per day if time spent per meeting, including all on-site time plus one hour of travel time, is greater than four hours. Each DRB board member shall be compensated at an agreed rate of $600.00 per day if time spent per meeting, including all on-site time plus one hour of travel time, is less than or equal to four hours. The agreed rates shall be considered full compensation for on-site time, travel expenses, transportation, lodging, time for travel and incidentals for each day, or portion thereof, that the DRB member is at an authorized DRB meeting. No additional compensation will be made for time spent by DRB members in review and research activities outside the official DRB meetings unless that time has been specifically agreed to in advance by the STATE and CONTRACTOR. Time away from the project, that has been specifically agreed to in advance by the parties, will be compensated at an agreed rate of $100.00 per hour. The agreed amount of $100.00 per hour shall include all incidentals including any expenses for telephone, fax and computer services. Members serving on more than one DRB, regardless of the number of meetings per day, shall not be paid more than the all inclusive rate per day or rate per hour for an individual project. The STATE will provide, at no cost to the CONTRACTOR, administrative services such as conference facilities and secretarial services to the DRB.

B. Payments

All DRB members shall be compensated at the same rate. The CONTRACTOR shall make direct payments to each DRB member for their participation in authorized meetings and approved hourly rate charges from invoices submitted by each DRB member. The STATE will reimburse the CONTRACTOR for its share of the costs of the DRB.

The DRB members may submit invoices to the CONTRACTOR for partial payment for work performed and services rendered for their participation in authorized meetings not more often than once per month during the progress of the work. The invoices shall be in a format approved by the parties and accompanied by a general description of activities performed during that billing period. Payment for any hourly fees, at the agreed rate, shall not be paid to a DRB member until the amount and extent of those fees are approved by the STATE and CONTRACTOR.

Invoices shall be accompanied by original supporting documents, which the CONTRACTOR shall include with the extra work billing when submitting for reimbursement of the STATE's share of cost from the STATE. The CONTRACTOR will be reimbursed for one-half of approved costs of the DRB. No markups will be added to the CONTRACTOR's payment.

C. Inspection of Costs Records

The DRB members and the CONTRACTOR shall keep available for inspection by representatives of the STATE and the United States, for a period of three years after final payment, the cost records and accounts pertaining to this AGREEMENT.

If any litigation, claim, or audit arising out of, in connection with, or related to this contract is initiated before the expiration of the three-year period, the cost records and accounts shall be retained until such litigation, claim, or audit involving the records is completed.

VII
ASSIGNMENT OF TASKS OF WORK

The DRB members shall not assign any of the work of this AGREEMENT.

VIII
TERMINATION OF AGREEMENT, THE DRB, AND DRB MEMBERS

DRB members may resign from the DRB by providing not less than 14 days written notice of the resignation to the STATE and CONTRACTOR. DRB members may be terminated by their original appointing power, in accordance with the terms of the contract.

IX
LEGAL RELATIONS

The parties hereto mutually understand and agree that the DRB member in the performance of duties on the DRB, is acting in the capacity of an independent agent and not as an employee of either party.

No party to this AGREEMENT shall bear a greater responsibility for damages or personal injury than is normally provided by Federal or State of California Law.

Notwithstanding the provisions of this contract that require the CONTRACTOR to indemnify and hold harmless the STATE, the parties shall jointly indemnify and hold harmless the DRB members from and against all claims, damages, losses, and expenses, including but not limited to attorney's fees, arising out of and resulting from the findings and recommendations of the DRB.

X
CONFIDENTIALITY

The parties hereto mutually understand and agree that all documents and records provided by the parties in reference to issues brought before the DRB, which documents and records are marked "Confidential - for use by the DRB only", shall be kept in confidence and used only for the purpose of resolution of subject disputes, and for assisting in development of DRB findings and recommendations; that such documents and records will not be utilized or revealed to others, except to officials of the parties who are authorized to act on the subject disputes, for any purposes, during the life of the DRB. Upon termination of this AGREEMENT, said confidential documents and records, and all copies thereof, shall be returned to the parties who furnished them to the DRB. However, the parties understand that such documents shall be subsequently discoverable and admissible in court or arbitration proceedings unless a protective order has been obtained by the party seeking further confidentiality.

XI
DISPUTES

Any dispute between the parties hereto, including disputes between the DRB members and either party or both parties, arising out of the work or other terms of this AGREEMENT, which cannot be resolved by negotiation and mutual concurrence between the parties, or through the administrative process provided in the contract, shall be resolved by arbitration as provided in Section 9-1.10, "Arbitration," of the Standard Specifications.

XII
VENUE, APPLICABLE LAW, AND PERSONAL JURISDICTION

In the event that any party, including an individual member of the DRB, deems it necessary to institute arbitration proceedings to enforce any right or obligation under this AGREEMENT, the parties hereto agree that any such action shall be initiated in the Office of Administrative Hearings of the State of California. The parties hereto agree that all questions shall be resolved by arbitration by application of California law and that the parties to such arbitration shall have the right of appeal from such decisions to the Superior Court in accordance with the laws of the State of California. Venue for the arbitration shall be Sacramento or any other location as agreed to by the parties.

XIII
FEDERAL REVIEW AND REQUIREMENTS

On Federal-Aid contracts, the Federal Highway Administration shall have the right to review the work of the DRB in progress, except for any private meetings or deliberations of the DRB.

All other Federal requirements in this agreement shall only apply to Federal-Aid contracts.

XIV
CERTIFICATION OF THE CONTRACTOR,
THE DRB MEMBERS, AND THE STATE

IN WITNESS WHEREOF, the parties hereto have executed this AGREEMENT as of the day and year first above written.

DRB MEMBER

By: _____

Title: _____

DRB MEMBER

By: _____

Title :_____

DRB MEMBER

By :_____

Title :_____

CONTRACTOR

By: _____

Title: _____

CALIFORNIA STATE DEPARTMENT
OF TRANSPORTATION

By: _____

Title: _____

5-1.12 COST REDUCTION INCENTIVE PROPOSAL

Attention is directed to Section 5-1.14, "Cost Reduction Incentive," of the Standard Specifications.

Prior to preparing a cost reduction proposal, the Contractor shall request a meeting with the Engineer to discuss the proposal in concept and to determine whether the cost reduction proposal will be considered by the Department. Items of discussion will also include permit issues, impact on other projects, impact on the project schedule, traffic considerations, safety, health issues, design criteria, and review times required by the Department and other agencies. Determination by the Engineer that a cost reduction proposal will not be considered will be deemed rejection of the proposal.

5-1.13 FORCE ACCOUNT PAYMENT

The second, third and fourth paragraphs of Section 9-1.03A, "Work Performed by Contractor," of the Standard Specifications, shall not apply.

To the total of the direct costs computed as provided in Sections 9-1.03A(1), "Labor," 9-1.03A(2), "Materials," and 9-1.03A(3), "Equipment Rental," of the Standard Specifications, there will be added a markup of 25 percent to the cost of labor, 10 percent to the cost of materials, and 10 percent to the equipment rental. These markups shall be applied to all force account work, regardless if the added work affects the contract completion date.

The above markups, together with payments made for time related overhead pursuant to "Overhead" of these special provisions, shall constitute full compensation for all overhead costs for work performed on a force account basis. These overhead costs shall be deemed to include all items of expense not specifically designated as cost or equipment rental in Sections 9-1.03A(1), "Labor," 9-1.03A(2), "Materials," and 9-1.03A(3), "Equipment Rental," of the Standard Specifications. The total payment made as provided above and in the first paragraph of Section 9-1.03A, "Work Performed by Contractor," shall be deemed to be the actual cost of the work performed on a force account basis, and shall constitute full compensation therefor.

When extra work to be paid for on a force account basis is performed by a subcontractor, approved in accordance with the provisions in Section 8-1.01, “Subcontracting,” of the Standard Specifications, an additional markup of 5 percent will be added to the total cost of said extra work including all markups specified in this section "Force Account Payment". Said additional 5 percent markup shall reimburse the Contractor for additional administrative costs, and no other additional payment will be made by reason of performance of the extra work by a subcontractor.

5-1.14 OVERHEAD

The Contractor will be compensated for overhead in accordance with these special provisions.

Attention is directed to "Force Account Payment" and "Progress Schedule (Critical Path)" of these special provisions.

Section 9-1.08, "Adjustment of Overhead Costs," of the Standard Specifications shall not apply.

Time related overhead shall consist of those overhead costs, including field and home office overhead, that are in proportion to the time required to complete the work. Time related overhead costs shall not include costs that are not related

to time, including but not limited to mobilization, licenses, permits, and any other charges incurred only once during duration of the contract.

The quantity of time related overhead to be measured for payment will be the number of working days specified in "Beginning of Work, Time of Completion and Liquidated Damages" of these special provisions, adjusted only as a result of suspensions and adjustments of time which revise the current contract completion date and which are also any of the following:

1) suspensions of work ordered in accordance with Section 8-1.05, "Temporary Suspension of Work," of the Standard Specifications, except:

a) suspensions ordered due to the failure on the part of the Contractor to carry out orders given, or to perform any provision of the contract; and

b) suspensions ordered due to unsuitable weather conditions;

2) extensions of time granted by the State in accordance with the provisions of the fifth paragraph of Section 8-1.07, "Liquidated Damages," of the Standard Specifications; or

3) reductions in contract time set forth in approved contract change orders, in accordance with Section 4-1.03, "Changes," of the Standard Specifications.

The contract price paid for time related overhead shall include full compensation for time related overhead measured for payment as specified above, incurred by the Contractor and by any joint venture partner, subcontractor, supplier or other party associated with the Contractor.

No adjustment in compensation will be made for any increase or decrease in the quantities of time related overhead required, regardless of the reason for the increase or decrease. The provisions in Sections 4-1.03B, "Increased or Decreased Quantities" and 4-1.03C, "Changes in Character of the Work," of the Standard Specifications, shall not apply to time related overhead.

For the purpose of making partial payments pursuant to Section 9-1.06, “Partial Payments,” of the Standard Specifications, the amount set forth for the contract item of time related overhead shall not exceed 15 percent of the total contract bid price.

After acceptance of the contract pursuant to Section 7-1.17, “Acceptance of Contract,” of the Standard Specifications, the amount, if any, not yet paid for the total contract item price for time related overhead and the amount, if any, payable for the contarct item price for time related overhead in excess of 15 percent of the original contract amount, will be included for payment in the first estimate made after acceptance of the contract in accordance with Section 9-1.07, “Payment after Acceptance,” of the Standard Specifications. Full compensation for all overhead costs incurred during the performance of extra work and additional item work that is not a controlling operation and for all overhead other than time related overhead measured and paid for as specified above, shall be considered to be included in the markups specified in Section 5-1.13, “Force Account payment,” of these special provisions and in the various items of work and no additional compensation will be allowed therefor.

5-1.15 TIDAL CONDITIONS AND ELEVATION DATUM

Attention is directed to Section 2-1.03, "Examination of Plans, Specifications, Contract, and Site of Work."

Tidal conditions may present significant problems in constructing the work as depicted in the contract plans. Tidal fluctuations may be severe and different from those shown in published tidal and current data due to differences in datum, winter runoff and other causes. Strong currents exist over portions of the project site. Limited time periods of slack water may restrict diving and other underwater activities.

The Contractor is responsible for being knowledgeable of such tidal difficulties, and no payment will be made by the State for any costs incurred by the Contractor in connection with the variations in actual tidal or current conditions during the course of this contract. Any reference to Mean Higher High and Mean Lower Low tides shall be understood to be an estimate used for permit purposes, actual mean tide data shall be determined by the Contractor.

All vertical control data are based on the National Geodetic Vertical Datum of 1929. All vertical control data are based on the National Geodetic Vertical Datum of 1929.

5-1.16 SOUND CONTROL REQUIREMENTS

Sound control shall conform to the provisions in Section 7-1.01I, "Sound Control Requirements," of the Standard Specifications and these special provisions.

The noise level from the Contractor's operations, between the hours of 9:00 p.m. and 6:00 a.m., shall not exceed 86 dbA at a distance of 15 m. This requirement in no way relieves the Contractor from responsibility for complying with local ordinances regulating noise level.

The noise level requirement shall apply to the equipment on the job or related to the job, including but not limited to trucks, transit mixers or transient equipment that may or may not be owned by the Contractor. The use of loud sound signals shall be avoided in favor of light warnings except those required by safety laws for the protection of personnel.

Full compensation for conforming to the requirements of this section shall be considered as included in the prices paid for the various contract items of work involved and no additional compensation will be allowed therefor.

5-1.17 RELATIONS WITH SAN FRANCISCO BAY CONSERVATION DEVELOPMENT COMMISSION (BCDC)

The location of the San Mateo-Hayward Bridge is within an area controlled by the BCDC. A permit has been issued covering work to be performed under this contract. The Contractor shall be fully informed of all rules, regulations and conditions of the permit that may govern the Contractor's operations in said area and shall conduct the Contractor's work accordingly. Said document shall be considered a part of, and shall become, an integral part of the special provisions and contract for this project.

The Contractor's attention is directed to the following conditions which are among those established by the Bay Area Conservation and Development Commission in the permit for this project:

1. No dredging operations are allowed.
2. No piling or other wood structures that have been pressure treated with creosote shall be used in any area subject to the tidal action in the Bay or any certain waterway, in the salt pond, or in any managed wetland within the Commission's jurisdiction.
3. No Contractor's equipment will be allowed to rest on the bottom of the bay.

Copies of the Permit may be obtained at the Department of Transportation, Plans and Bid Documents, Room 0200, Transportation Building, 1120 N Street, P.O. Box 942874, Sacramento, California 94274-0001, Telephone No. (916) 654-4490, and are available for inspection at the Toll Bridge Program Duty Senior at District 04 Office, 111 Grand Avenue, Oakland, California 94612, Telephone No. (510) 286-5549, duty_senior_tollbridge_district04@dot.ca.gov.

Any modifications to the permit which are proposed by the Contractor shall be submitted in writing to the Engineer for transmittal to the BCDC for their consideration.

When the Contractor is notified by the Engineer that a modification to the permit is under consideration, no work will be allowed on the proposed modification until the Department takes action on the proposed modification. Any modifications to any agreement between the Department of Transportation and BCDC shall be fully binding on the Contractor, and the provisions of this section shall be made a part of every subcontract executed pursuant to this contract.

Full compensation for conforming to the above requirements shall be considered as included in the contract prices paid for the various contract items of work and no additional compensation will be allowed therefor.

5-1.18 RELATIONS WITH CALIFORNIA REGIONAL WATER QUALITY CONTROL BOARD

The location of the San Mateo-Hayward Bridge, Br. No. 35-0054 is within an area controlled by the Regional Water Quality Control Board. The Contractor shall be fully informed of all Regional Water Quality Control Board rules, regulations and conditions that may govern the Contractor's operations in said area and shall conduct the Contractor's work accordingly.

Attention is directed to Sections 7-1.11, "Preservation of Property," and 7-1.12, "Responsibility for Damage," of the Standard Specifications.

Attention is directed to Section 8-1.06, "Time of Completion," of the Standard Specifications. Days during which the Contractor's operations are restricted in the floodway by the requirements of this section, shall be considered to be nonworking days if these restrictions cause a delay in the current controlling operation or operations.

Any modifications to this Section which are proposed by the Contractor shall be submitted in writing to the Engineer for transmittal to the Regional Water Quality Control Board for their consideration.

When the Contractor is notified by the Engineer that a modification to the requirements in this Section is under consideration by the Regional Water Quality Control Board, no work will be allowed on the proposed modification area until the RWQCB takes action on the proposed modification.

Any modifications to any agreement between the Department of Transportation and the Regional Water Quality Control Board shall be fully binding on the Contractor, and the provisions of this section shall be made a part of every subcontract executed pursuant to this contract.

Full compensation for conforming to the requirements of this section shall be considered as included in the contract prices paid for the various contract items of work and no additional compensation will be allowed therefor.

5-1.19 RELATIONS WITH U.S. ARMY CORPS OF ENGINEERS

The location of the San Mateo-Hayward Bridge is within an area controlled by the U.S. Army Corps of Engineers. An U.S. Army Corps of Engineers Permit is not applicable to this project. However, any work that would result in the placement of any fill below high tide or mean high tide line, whichever is higher, would require a separate U.S. Army Corps of Engineers action. To request such actions a notification by the Contractor through the Engineer to the U.S. Army Corps of Engineers is required prior to start of such work. The Contractor shall notify the Engineer at least seven days in advance of such an inquiry to the U.S. Army Corps of Engineers.

When the Contractor is notified by the Engineer that a request is under consideration, no such work will be allowed until the U.S. Army Corps of Engineers takes action on the proposed request. Any modifications to any agreement between the

Department of Transportation and the Army Corps of Engineers shall be fully binding on the Contractor, and the provisions of this section shall be made a part of every subcontract executed pursuant to this contract.

Full compensation for conforming to the requirements of this section shall be considered as included in the contract prices paid for the various contract items of work and no additional compensation will be allowed therefor.

5-1.20 COMPENSATION ADJUSTMENTS FOR PRICE INDEX FLUCTUATIONS

The provisions of this section shall apply only to the following contract item:

ITEM CODE	ITEM
390145	REPLACE ASPHALT CONCRETE SURFACING
390152	ASPHALT CONCRETE
390171	ASPHALT CONCRETE BASE (TYPE A)
390175	ASPHALT CONCRETE (LEVELING)
390206	RUBBERIZED ASPHALT CONCRETE (TYPE G)

The compensation payable for replace asphalt concrete surfacing, asphalt concrete, asphalt concrete base, asphalt concrete (leveling), and rubberized asphalt concrete will be subject to being increased or decreased in accordance with the provisions of this section for paving asphalt price fluctuations exceeding 5 percent (Iu/Ib is greater than 1.05 or less than 0.95) which occur during performance of the work.

The adjustment in compensation will be determined in accordance with the following formulae when the item of asphalt concrete, asphalt concrete base, asphalt concrete (leveling), and rubberized asphalt concrete is included in a monthly estimate:

Total monthly adjustment = AQ

For an increase in paving asphalt price index exceeding 5 percent:

$$A = 0.90\ (1.1023)\ (Iu/Ib - 1.05)\ Ib$$

For a decrease in paving asphalt price index exceeding 5 percent:

$$A = 0.90\ (1.1023)\ (Iu/Ib - 0.95)\ Ib$$

Where:

A = Adjustment in dollars per tonne of paving asphalt used to produce asphalt concrete , asphalt concrete base, asphalt concrete (leveling), and rubberized asphalt concrete rounded to the nearest $0.01.

Iu = The California Statewide Paving Asphalt Price Index which is in effect on the first business day of the month within the pay period in which the quantity subject to adjustment was included in the estimate.

Ib = The California Statewide Paving Asphalt Price Index for the month in which the bid opening for the project occurred.

Q = Quantity in tonnes of paving asphalt that was used in producing the quantity of replace asphalt concrete surfacing, asphalt concrete, asphalt concrete base, asphalt concrete (leveling), and rubberized asphalt concrete shown under "This Estimate" on the monthly estimate using the amount of asphalt determined by the Engineer.

The adjustment in compensation will also be subject to the following:

1. The compensation adjustments provided herein, will be shown separately on payment estimates. The Contractor shall be liable to the State for decreased compensation adjustments and the Department may deduct the amount thereof from any moneys due or that may become due the Contractor.
2. Compensation adjustments made under this section will be taken into account in making adjustments under Section 4-1.03B, "Increased or Decreased Quantities," of the Standard Specifications.
3. The total price adjustment for price index increases of paving asphalt on this project shall not exceed $ 584,300.

4. In the event of an overrun of contract time, adjustment in compensation for paving asphalt included in estimates during the overrun period will be determined using the California Statewide Paving Asphalt Price Index in effect on the first business day of the month within the pay period in which the overrun began.

The California Statewide Paving Asphalt Price Index is determined each month on the first business day of the month by the Department using the median of posted prices in effect as posted by Chevron, Mobil and Unocal for the Buena Vista, Huntington Beach, Kern River, Long Beach, Midway Sunset and Wilmington fields.

In the event that any of the companies discontinue posting their prices for any field, the Department will determine an index from the remaining posted prices. The Department reserves the right to include in the index determination the posted prices of additional fields.

5-1.21 PAYMENTS

Attention is directed to Sections 9-1.06, "Partial Payments," and 9-1.07, "Payment After Acceptance," of the Standard Specifications and these special provisions.

For the purpose of making partial payments pursuant to Section 9-1.06, "Partial Payments," of the Standard Specifications, the amount set forth for the contract items of work hereinafter listed shall be deemed to be the maximum value of the contract item of work which will be recognized for progress payment purposes:

Clearing and Grubbing	$27,000.00
Develop Water Supply	$4,500.00
Electronic mobile Daily diary Computer System (Data Delivery)	$ 9,000.00
Bridge Removal Portion, Location A	$24,000.00
Bridge Removal Portion, Location B	$38,000.00
Bridge Removal Portion, Location C	$16,000.00
Bridge Removal Portion, Location D	$4,500.00

After acceptance of the contract pursuant to Section 7-1.17, "Acceptance of Contract," of the Standard Specifications, the amount, if any, payable for a contract item of work in excess of the maximum value for progress payment purposes hereinabove listed for the item, will be included for payment in the first estimate made after acceptance of the contract.

In determining the partial payments to be made to the Contractor, only the following listed materials will be considered for inclusion in the payment as materials furnished but not incorporated in the work:

Piling
Terminal system
Pavement marker
Precast concrete members
Prestressing steel in sealed containers
Prestressing ducts and anchorage devices
Type B joint seal and joint seal assemblies
Bar reinforcing steel
Miscellaneous metal
Miscellaneous iron and steel
Metal sign structures
Soil nail assembly
Alternative pipe culvert
Underdrain pipe
Edge drain pipe
Filter fabric
Metal beam guard railing
Chain link fence
Chain link railing
Crash cushions
Lighting fixtures
Luminaires
Signal & lighting standards
Controller cabinets
Camera assemblies
Twisted pair cable
Fiber optic cable
Fiber optic conduit

5-1.22 AREAS FOR CONTRACTOR'S USE

No area is available within the contract limits for the exclusive use of the Contractor. However, temporary storage of equipment and materials on State property may be arranged with the Engineer, subject to the prior demands of State maintenance forces and to other contract requirements. Use of the Contractor's work areas and other State-owned property shall be at the Contractor's own risk, and the State shall not be held liable for any damage to or loss of materials or equipment located within these areas.

The Contractor shall remove the equipment, materials, and rubbish from the work areas and other State-owned property which the Contractor occupies and shall leave the areas in a presentable condition, in conformance with the provisions in Section 4-1.02, "Final Cleaning Up," of the Standard Specifications.

The Contractor shall secure at the Contractor's own expense any area required for storage of plant, equipment and materials, or for other purposes if sufficient area is not available to the Contractor within the contract limits.

5-1.23 UTILITIES

The Contractor may use electrical power, water, and compressed air from existing State outlets within the contract limits, where the utilities exist, free of charge for contract operations provided that the Contractor does not misuse these services and provided that utility services are in service and are not required by the State for other purposes, and subject to the provisions of "Cooperation" of these special provisions.

The Contractor shall make arrangements to obtain any additional electrical power, water, or compressed air or other utilities required for the Contractor's operations and shall make and maintain the necessary service connections at the Contractor's own expense.

5-1.24 SANITARY PROVISIONS

State sanitary facilities will not be available for use by the Contractor's employees.

5-1.25 BRIDGE TOLLS

Toll-free passage on the San Mateo - Hayward Bridge will be granted only for cars, trucks and special construction equipment which are clearly marked on the exterior with the Contractor's identification and which are being operated by the Contractor exclusively for the project and for the purpose of transporting materials and workers directly to and from the jobsite.

The Contractor shall make application to the Engineer in advance for toll-free passage. The Contractor will be held accountable for the proper use of all passes issued, and upon completion of the work, shall return all unused passes to the Engineer.

Attention is directed to Section 23302, "Evasion of Toll," of the Vehicle Code.

5-1.26 DRAWINGS

Attention is directed to Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications and these special provisions.

When working drawings are required by these special provisions, the drawings shall be submitted in accordance with the provisions in Section 55-1.02, "Drawings," of the Standard Specifications and the following:

1. Working drawings shall be submitted to the Resident Engineer.
2. Working drawings shall not exceed 559 mm x 864 mm in size.
3. Microfilms are required of all approved shop drawings and shall be only a 24x reduction.

5-1.27 PERMITS AND LICENSES

Attention is directed to Section 7-1.04, "Permits and Licenses," of the Standard Specifications and these special provisions.

The Department has obtained the following permits for this project:

U.S. Army Corps of Engineers Permit, File No. 21395S
San Francisco Bay Conservation & Development Commission Permit No. 20-98

Copies of these permits can be obtained at the Department of Transportation, Plans and Bid Documents Section (MS 26), 1120 N Street, Room 200, Sacramento, CA 95814, Telephone No. (916) 654-4490, and are available for inspection at the Toll Bridge Program Duty Senior at District 04 Office, 111 Grand Avenue, Oakland, California 94612, Telephone No. (510) 286-5549, duty_senior_tollbridge_district04@dot.ca.gov.

Full compensation for conforming to the requirements in these permits shall be considered as included in the contract prices paid for the various items of work and no additional compensation will be allowed therefor.

5-1.28 AERIALLY DEPOSITED LEAD, GENERAL

Aerially Deposited Lead is defined as lead deposited within the Department of Transportation (Caltrans) Right of Way primarily due to vehicle emissions. Aerially deposited lead contamination has been discovered through testing of materials from within the project limits.

Attention is directed to "Material with Aerially Deposited Lead" under "Earthwork" of these special provisions regarding the handling of material with aerially deposited lead.

The reports entitled "Site Investigation Report, State Route 92 East Approach Widening, Alameda County, California, October 1999", "Site Investigation Report, San Mateo-Hayward Bridge Projects: Trestle Widening, Landscape Mitigation, including Pedestrian Overcrossing in San Mateo and Alameda Counties, California, June 1999",and "Site Investigation Report, State Routes 92 and 238, Alameda County, California, March 1997" are available for inspection at the Department of Transportation, 111 Grand Avenue Oakland, California 94601. Please call the Toll Bridge Duty Senior, telephone number (510) 286-5549, to reserve a copy of the documents at least 24 hours in advance. Materials with total levels of lead greater than the Total Threshold Limit Concentration (TTLC) of 1000 milligrams per kilogram (mg/kg) or solubility levels, as established by the California Waste Extraction Test (WET), greater than the Solubility Threshold Limit Concentration (STLC) of 5 milligrams per liter (mg/l) shall be considered hazardous pursuant to California Code of Regulations, Title 22. The materials with aerially deposited lead are not regulated under the Federal Resource Conservation and Recovery Act (RCRA).

Caltrans has received from the California Department of Toxic Substances Control (DTSC) a variance regarding the use of these materials. This project is subject to the conditions of the variance and any supplemental amendments. Under the variance any materials with total levels of aerially deposited lead less than 1575 mg/kg and water solubility levels of less than 500 micrograms per liter (µg/l) may be placed under 0.6 m of cover material or under a pavement (e.g., asphalt roadway) within the highway right of way. Water solubility is determined using a modified WET with deionized water as the extraction solution. Materials with total levels of aerially deposited lead from 1575 mg/kg to 4150 mg/kg or water solubility levels greater than 500 µg/l, but less than 50 mg/l, may be placed under pavement (e.g. asphalt roadway) within the highway right of way. Materials reused in accordance with the variance shall be placed a minimum of 1.5 m above maximum ground water level. Cover materials pursuant to this provision are soils with less than 130 mg/kg total lead and 5 mg/l soluble lead (using WET), based on the composite analyses of at least one discrete soil sample obtained for every 200 m^3 of stockpiled soil proposed as cover material. A minimum of four discrete samples shall be used in the composite analyses, if less than 800 m^3 are needed as cover material. All constituents, other than lead, shall be consistent with the surrounding site soils as described in the site investigation reports. The sampling and analyses methods for determining the concentration of lead in the proposed cover material shall be described in a work plan as described elsewhere in this special provisions.

Provisions of this section shall be made a part of every subcontract executed pursuant to this contract. The Contractor shall comply with the conditions of the variance.

Excavation, transportation, placement and handling of soils containing aerially deposited lead shall result in no visible dust. The Contractor shall have a water truck available at all times while performing earthwork, excavation or grubbing activities in work areas containing aerially deposited lead at hazardous levels.

Once the Contractor has completed the placement of materials containing aerially deposited lead, in accordance with the plans, as specified in the Standard Specifications and these special provisions, the Contractor shall have no responsibility for such materials in place and shall not be obligated for further cleanup, removal or remedial actions for such materials.

Excavation, reuse, and disposal of material with aerially deposited lead shall be in accordance with all rules and regulations of agencies including, but not limited to, the following:

United States Department of Transportation (USDOT)
United States Environmental Protection Agency (USEPA)
California Department of Health Services
California Environmental Protection Agency (Cal-EPA)
Department of Toxic Substances Control (DTSC), Region 2
Integrated Waste Management Board
Regional Water Quality Control Board (RWQCB), Region 2
State Air Resources Control Board
Bay Area Air Quality Management District (BAAQMD)
California Division of Occupational Safety and Health Administration (CAL-OSHA)
Alameda County

The Contractor shall prepare a project specific Health and Safety Plan to prevent or minimize exposure to potentially hazardous levels of lead. The Contractor's attention is directed to Title 8, California Code of Regulations, Section 5192 (b) (4) (B) and the Occupational Safety and Health Guidance Manual published by National Institute of Occupational Safety and Health (NIOSH), Occupational Safety and Health Administration (OSHA), and USEPA for elements of the site safety plan. The Health and Safety Plan shall contain as a minimum but not be limited to: identification of key personnel for the project, job hazard analysis for work assignments, summary of risk assessment, air monitoring plan, personal protective equipment, delineation of work zones on-site, decontamination procedures, general safe work practices, security measures, emergency response plans and worker training.

The Health and Safety Plan shall utilize monitoring and exposure standards based on Construction Standards of Title 8, California Code of Regulations Section 1532.1 and as a minimum shall contain a description of activities, specific means employed to achieve compliance, report of the technology considered, air monitoring, schedule for implementation of the program, a work practice program, administrative control schedule, description of arrangements for information transfer between contractors concerning potential exposure to lead and other relevant information. The Health and Safety Plan shall include an air monitoring plan that shall include, but not be limited to, upwind and downwind perimeter monitoring and a discussion of how the air monitoring will be conducted during the progression of roadway work in areas designated as containing aerially deposited lead. The air monitoring plan shall meet the requirements of the immediate area's Air Quality Management District standards, if applicable. The Health and Safety plan shall be approved by the Contractor's Certified Industrial Hygienist before submission to the Engineer. The plan shall be submitted to the Engineer for review and acceptance at least 15 days prior to beginning any work in areas containing aerially deposited lead.

Prior to performing any work in areas containing lead, personnel who have no prior training or are not current in their training status, including State personnel, shall complete a safety training program provided by the Contractor, which meets the requirements of Title 8, California Code of Regulations, Section 1532.1.

Personal protective equipment, training, and medical surveillance required by the Contractor's Health and Safety Plan shall be supplied to State personnel by the Contractor. The number of State personnel will be 10.

Stockpiles of materials containing lead that are designated for reuse under the DTSC/Caltrans variance, including materials placed in fill locations that have not been compacted in accordance with Section 19 "Earthwork" of the Standard Specifications, shall be covered with sheets of 0.3-mm polyethylene at the end of each day to prevent water and wind erosion. Stockpile locations shall be limited to areas within the highway right of way that contain lead. If lead levels in the stockpile areas are not known, the Contractor shall conduct initial sampling in the area to determine lead levels. The Contractor shall prepare a work plan for the handling and stockpiling of materials containing lead that are designated for reuse under the DTSC/Caltrans variance. The work plan shall include a proposal for reuse of this material and a plan for initial and confirmatory sampling of any stockpile area after removal of the materials to ensure that additional lead has not been deposited or has not migrated to surrounding soils. The sampling plan shall meet USEPA, SW 846, "Test Methods for Evaluating Solid Waste," Volume II: Field Manual Physical/Chemical, Chapter Nine, Section 9.1 requirements for the development of the sampling plan, statistical analysis, and reporting of the test results and shall be submitted to the Engineer for review and approval at least 15 days prior to beginning work in any areas containing lead.

The Contractor shall procure all permits and licenses, pay all charges and fees, and give all notices necessary and incidental to the due and lawful prosecution of the work, including registration for transporting vehicles carrying hazardous materials.

Full compensation for conforming to the requirements of this section shall be considered as included in the prices paid for the various contract items of work involved and no additional compensation will be allowed therefor.

SECTION 6. (BLANK)

SECTION 7. (BLANK)

SECTION 8. MATERIALS

SECTION 8-1. MISCELLANEOUS

8-1.01 SUBSTITUTION OF NON-METRIC MATERIALS AND PRODUCTS

Only materials and products conforming to the requirements of the specifications shall be incorporated in the work. When metric materials and products are not available, and when approved by the Engineer, and at no cost to the State, materials and products in the inch-pound (imperial) system which are of equal quality and of the required properties and characteristics for the purpose intended, may be substituted for the equivalent metric materials and products, subject to the following provisions:

Materials and products shown on the plans or in the special provisions as being equivalent may be substituted for the metric materials and products specified or detailed on the plans.

Before other non-metric materials and products will be considered for use the Contractor shall furnish, at the Contractor's expense, evidence satisfactory to the Engineer that the materials and products proposed for use are equal to or better than the materials and products specified or detailed on the plans. The burden of proof as to the quality and suitability of substitutions shall be upon the Contractor and the Contractor shall furnish necessary information as required by the Engineer. The Engineer will be the sole judge as to the quality and suitability of the substituted materials and products and the Engineer's decision will be final.

When the Contractor elects to substitute non-metric materials and products, including materials and products shown on the plans or in the special provisions as being equivalent, the list of sources of material as specified in Section 6-1.01, "Source of Supply and Quality of Materials," of the Standard Specification shall include a list of substitutions to be made

and contract items involved. In addition, for a change in design or details the Contractor shall submit plans and working drawings in conformance with Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications.

Unless otherwise specified, the following substitutions of materials and products will be allowed:

SUBSTITUTION TABLE FOR SIZES OF HIGH STRENGTH STEEL FASTENERS
ASTM Designation: A 325M

METRIC SIZE SHOWN ON THE PLANS mm x thread pitch	IMPERIAL SIZE TO BE SUBSTITUTED inch
M16 x 2	5/8
M20 x 2.5	3/4
M22 x 2.5	7/8
M24 x 3	1
M27 x 3	1-1/8
M30 x 3.5	1-1/4
M36 x 4	1-1/2

SUBSTITUTION TABLE FOR PLAIN WIRE REINFORCEMENT, ASTM Designation: A 82

METRIC SIZE SHOWN ON THE PLANS mm^2	US CUSTOMARY UNITS SIZE TO BE SUBSTITUTED $inch^2$ x 100
MW9	W1.4
MW10	W1.6
MW13	W2.0
MW15	W2.3
MW19	W2.9
MW20	W3.1
MW22	W3.5
MW25	W3.9, except W3.5 in piles only
MW26	W4.0
MW30	W4.7
MW32	W5.0
MW35	W5.4
MW40	W6.2
MW45	W6.5
MW50	W7.8
MW55	W8.5, except W8.0 in piles only
MW60	W9.3
MW70	W10.9, except W11.0 in piles only
MW80	W12.4
MW90	W14.0
MW100	W15.5

SUBSTITUTION TABLE FOR BAR REINFORCEMENT

METRIC BAR DESIGNATION NUMBER SHOWN ON THE PLANS	EQUIVALENT IMPERIAL BAR DESIGNATION NUMBER TO BE SUBSTITUTED
13	4
16	5
19	6
22	7
25	8
29	9
32	10
36	11
43	14
57	18

No adjustment will be required in spacing or total number of reinforcing bars due to a difference in minimum yield strength between metric and non-metric bars.

The sizes in the following tables of materials and products are exact conversions of metric sizes of materials and products and are listed as acceptable equivalents:

CONVERSION TABLE FOR SIZES OF:

(1) STEEL FASTENERS FOR GENERAL APPLICATIONS, ASTM Designation: A 307 or AASHTO Designation: M 314, Grade 36 or 55, and

(2) HIGH STRENGTH STEEL FASTENERS, ASTM Designation: A 325 or A 449

METRIC SIZE SHOWN ON THE PLANS mm	EQUIVALENT IMPERIAL SIZE inch
6, or 6.35	1/4
8 or 7.94	5/16
10, or 9.52	3/8
11, or 11.11	7/16
13 or 12.70	1/2
14, or 14.29	9/16
16, or 15.88	5/8
19, or 19.05	3/4
22, or 22.22	7/8
24, 25, or 25.40	1
29, or 28.58	1-1/8
32, or 31.75	1-1/4
35, or 34.93	1-3/8
38 or 38.10	1-1/2
44, or 44.45	1-3/4
51, or 50.80	2
57, or 57.15	2-1/4
64, or 63.50	2-1/2
70 or 69.85	2-3/4
76, or 76.20	3
83, or 82.55	3-1/4
89 or 88.90	3-1/2
95, or 95.25	3-3/4
102, or 101.60	4

CONVERSION TABLE FOR NOMINAL THICKNESS OF SHEET METAL

UNCOATED HOT AND COLD ROLLED SHEETS		HOT-DIPPED ZINC COATED SHEETS (GALVANIZED)	
METRIC THICKNESS SHOWN ON THE PLANS mm	EQUIVALENT US STANDARD GAGE inch	METRIC THICKNESS SHOWN ON THE PLANS mm	EQUIVALENT GALVANIZED SHEET GAGE inch
7.94	0.3125	4.270	0.1681
6.07	0.2391	3.891	0.1532
5.69	0.2242	3.510	0.1382
5.31	0.2092	3.132	0.1233
4.94	0.1943	2.753	0.1084
4.55	0.1793	2.372	0.0934
4.18	0.1644	1.994	0.0785
3.80	0.1495	1.803	0.0710
3.42	0.1345	1.613	0.0635
3.04	0.1196	1.461	0.0575
2.66	0.1046	1.311	0.0516
2.28	0.0897	1.158	0.0456
1.90	0.0747	1.006 or 1.016	0.0396
1.71	0.0673	0.930	0.0366
1.52	0.0598	0.853	0.0336
1.37	0.0538	0.777	0.0306
1.21	0.0478	0.701	0.0276
1.06	0.0418	0.627	0.0247
0.91	0.0359	0.551	0.0217
0.84	0.0329	0.513	0.0202
0.76	0.0299	0.475	0.0187
0.68	0.0269	--------	--------
0.61	0.0239	--------	--------
0.53	0.0209	--------	--------
0.45	0.0179	--------	--------
0.42	0.0164	--------	--------
0.38	0.0149	--------	--------

CONVERSION TABLE FOR WIRE

METRIC THICKNESS SHOWN ON THE PLANS mm	EQUIVALENT USA STEEL WIRE THICKNESS inch	GAGE NO.
6.20	0.244	3
5.72	0.225	4
5.26	0.207	5
4.88	0.192	6
4.50	0.177	7
4.11	0.162	8
3.76	0.148	9
3.43	0.135	10
3.05	0.120	11
2.69	0.106	12
2.34	0.092	13
2.03	0.080	14
1.83	0.072	15
1.57	0.062	16
1.37	0.054	17
1.22	0.048	18
1.04	0.041	19
0.89	0.035	20

CONVERSION TABLE FOR PIPE PILES

METRIC SIZE SHOWN ON THE PLANS mm x mm	EQUIVALENT IMPERIAL SIZE inch x inch
PP 360 x 4.55	NPS 14 x 0.179
PP 360 x 6.35	NPS 14 x 0.250
PP 360 x 9.53	NPS 14 x 0.375
PP 360 x 11.12	NPS 14 x 0.438
PP 406 x 12.70	NPS 16 x 0.500
PP 460 x T	NPS 18 x T"
PP 508 x T	NPS 20 x T"
PP 559 x T	NPS 22 x T"
PP 610 x T	NPS 24 x T"
PP 660 x T	NPS 26 x T"
PP 711 x T	NPS 28 x T"
PP 762 x T	NPS 30 x T"
PP 813 x T	NPS 32 x T"
PP 864 x T	NPS 34 x T"
PP 914 x T	NPS 36 x T"
PP 965 x T	NPS 38 x T"
PP 1016 x T	NPS 40 x T"
PP 1067 x T	NPS 42 x T"
PP 1118 x T	NPS 44 x T"
PP 1219 x T	NPS 48 x T"
PP 1524 x T	NPS 60 x T"

The thickness in inches (T") represents an exact conversion of the metric thickness in millimeters (T).

CONVERSION TABLE FOR STRUCTURAL TIMBER AND LUMBER

METRIC MINIMUM DRESSED DRY, SHOWN ON THE PLANS mm x mm	METRIC MINIMUM DRESSED GREEN, SHOWN ON THE PLANS mm x mm	EQUIVALENT NOMINAL US SIZE inch x inch
19x89	20x90	1x4
38x89	40x90	2x4
64x89	65x90	3x4
89x89	90x90	4x4
140x140	143x143	6x6
140x184	143x190	6x8
184x184	190x190	8x8
235x235	241x241	10x10
286x286	292x292	12x12

CONVERSION TABLE FOR NAILS AND SPIKES

METRIC COMMON NAIL, SHOWN ON THE PLANS Length, mm Diameter, mm	METRIC BOX NAIL, SHOWN ON THE PLANS Length, mm Diameter, mm	METRIC SPIKE, SHOWN ON THE PLANS Length, mm Diameter, mm	EQUIVALENT IMPERIAL SIZE Penny-weight
50.80 2.87	50.80 2.51	_____	6d
63.50 3.33	63.50 2.87	_____	8d
76.20 3.76	76.20 3.25	76.20 4.88	10d
82.55 3.76	82.55 3.25	82.55 4.88	12d
88.90 4.11	88.90 3.43	88.90 5.26	16d
101.60 4.88	101.60 3.76	101.60 5.72	20d
114.30 5.26	114.30 3.76	114.30 6.20	30d
127.00 5.72	127.00 4.11	127.00 6.68	40d
_____	_____	139.70 7.19	50d
_____	_____	152.40 7.19	60d

8-1.02 APPROVED TRAFFIC PRODUCTS

The Department maintains a List of Approved Traffic Products. The Engineer shall not be precluded from sampling and testing products on the List of Approved Traffic Products.

The manufacturer of products on the List of Approved Traffic Products shall furnish the Engineer a Certificate of Compliance in conformance with the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications for each type of traffic product supplied.

The following is the List of Approved Traffic Products:

PAVEMENT MARKERS, PERMANENT TYPE

RETROREFLECTIVE

Apex, Model 921 (100 mm x 100 mm)
Ray-O-Lite, Models SS (100 mm x 100 mm), RS (100 mm x 100 mm) and AA (100 mm x 100 mm)
Stimsonite, Models 88 (100 mm x 100 mm), 911 (100 mm x 100 mm), 953 (70 mm x 114 mm)
3M Series 290 (89 mm x 100 mm)

RETROREFLECTIVE WITH ABRASION RESISTANT SURFACE (ARS)

Ray-O-Lite "AA" ARS (100 mm x 100 mm)
Stimsonite, Models 911 (100 mm x 100 mm), 953 (70 mm x 114 mm)
3M Series 290 (89 mm x 100 mm)

RETROREFLECTIVE WITH ABRASION RESISTANT SURFACE (ARS)
(Used for recessed applications)

Stimsonite, Model 948 (58 mm x 119 mm)
Ray-O-Lite, Model 2002 (58 mm x 117 mm)
Stimsonite, Model 944SB (51 mm x 100 mm)*
Ray-O-Lite, Model 2004 ARS (51 mm x 100 mm)*

*For use only in 114 mm wide (older) recessed slots

NON-REFLECTIVE FOR USE WITH EPOXY ADHESIVE, 100 mm Round

Apex Universal (Ceramic)
Highway Ceramics, Inc. (Ceramic)

NON-REFLECTIVE FOR USE WITH BITUMEN ADHESIVE, 100 mm Round

Apex Universal (Ceramic)
Apex Universal, Model 929 (ABS)
Elgin Molded Plastics, "Empco-Lite" Model 900 (ABS)
Highway Ceramics, Inc. (Ceramic)
Hi-Way Safety, Inc., Models P20-2000W and 2001Y (ABS)
Interstate Sales, "Diamond Back" (ABS) and (Polypropylene)
Alpine Products, D-Dot (ABS)
Road Creations, Model RCB4NR (Acrylic)

PAVEMENT MARKERS, TEMPORARY TYPE

TEMPORARY MARKERS FOR LONG TERM DAY/NIGHT USE (6 months or less)

Apex Universal, Model 924 (100 mm x 100 mm)
Davidson Plastics Corp., Model 3.0 (100 mm x 100 mm)
Elgin Molded Plastics, "Empco-Lite" Model 901 (100 mm x 100 mm)
Road Creations, Model R41C (100 mm x 100 mm)
Vega Molded Products "Temporary Road Marker" (75 mm x 100 mm)

TEMPORARY MARKERS FOR SHORT TERM DAY/NIGHT USE (14 days or less)
(For seal coat or chip seal applications, clear protective covers are required)

Apex Universal, Model 932
Davidson Plastics, Models T.O.M., T.R.P.M., and "HH" (High Heat)
Hi-Way Safety, Inc., Model 1280/1281

STRIPING AND PAVEMENT MARKING MATERIALS

PERMANENT TRAFFIC STRIPING AND PAVEMENT MARKING TAPE

Advanced Traffic Marking, Series 300 and 400
Brite-Line, Series 1000
Swarco Industries, "Director 35" (For transverse application only)
Swarco Industries, "Director 60"
3M, "Stamark" Series 380 and 5730
3M, "Stamark" Series A320 Bisymetric (For use on low-volume roadways only)
3M, "Stamark" Series A420, A440, N420, and N440 (For transverse application only)

TEMPORARY (REMOVABLE) STRIPING AND PAVEMENT MARKING TAPE
(6 months or less)

Brite-Line, Series 100
P.B. Laminations, Aztec, Grade 102
Swarco Industries, "Director-2"
3M, "Stamark," Series A620
3M Series A145 Removable Black Line Mask
(Black Tape: For use only on Asphalt Concrete Surfaces)
Advanced Traffic Marking Black "Hide-A-Line"
(Black Tape: For use only on Asphalt Concrete Surfaces)

PREFORMED THERMOPLASTIC (Heated in place)

Flint Trading, "Premark" and "Premark 20/20 Flex"
Pavemark, "Hotape"

REMOVABLE TRAFFIC PAINT

Belpro, Series 250/252 and No. 93 Remover

CLASS 1 DELINEATORS

ONE-PIECE DRIVEABLE FLEXIBLE TYPE, 1700 mm

Carsonite, Curve-Flex CFRM-400
Carsonite, Roadmarker CRM-375
Davidson Plastics, "Flexi-Guide Models 400 and 566"
FlexStake, Model 654TM
GreenLine Models HWD1-66 and CGD1-66
J. Miller Industries, Model JMI-375 (with soil anchor)

SPECIAL USE FLEXIBLE TYPE, 1700 mm

Carsonite, "Survivor" with 450 mm U-Channel base
FlexStake, Model 604
GreenLine Models HWD and CGD (with 450 mm U-Channel base)
Safe-Hit with 200 mm pavement anchor (SH248-GP1)
Safe-Hit with 380 mm soil anchor (SH248-GP2) and with 450 mm soil anchor (SH248-GP3)

SURFACE MOUNT FLEXIBLE TYPE, 1200 mm

Bent Manufacturing Company, "Masterflex" Model MF-180EX-48
Carsonite, "Super Duck II"
FlexStake, Surface Mount, Models 704 and 754TM

CHANNELIZERS

SURFACE MOUNT TYPE, 900 mm

Bent Manufacturing Company, "Masterflex" Models MF-360-36 (Round) and MF-180-36 (Flat)
Carsonite, "Super Duck" (Flat SDF-436, Round SDR-336)
Carsonite, Super Duck II Model SDCF203601MB "The Channelizer"
Davidson Plastics, Flex-Guide Models FG300LD and FG300UR
FlexStake, Surface Mount, Models 703 and 753TM
GreenLine, Model SMD-36
The Line Connection, "Dura-Post" Model DP36-3 (Permanent)
The Line Connection, "Dura-Post" Model DP36-3C (Temporary)
Repo, Models 300 and 400
Safe-Hit, Guide Post, Model SH236SMA

CONICAL DELINEATORS, 1070 mm
(For 700 mm Traffic Cones, see Standard Specifications)

Bent Manufacturing Company "T-Top"
Plastic Safety Systems "Navigator-42"
Roadmaker Company "Stacker"
TrafFix Devices "Grabber"

OBJECT MARKERS

TYPE "K", 450 mm

Carsonite, Model SMD-615
FlexStake, Model 701KM
Repo, Models 300 and 400
Safe-Hit, Model SH718SMA
The Line Connection, Model DP21-4K

TYPE "K-4", 450-600 mm
(Shown as Type "Q" in the Traffic Manual)

Carsonite, Super Duck II
FlexStake, Model 701KM
Repo, Models 300 and 400
Safe-Hit, Models SH8 24SMA_WA and SH8 24GP3_WA
The Line Connection, Model DP21-4Q

TEMPORARY RAILING (TYPE K) REFLECTORS AND CONCRETE BARRIER MARKERS

IMPACTABLE TYPE

ARTUK, "FB"
Davidson Plastics, Model PCBM-12
Duraflex Corp., "Flexx 2020" and "Electriflexx"

NON-IMPACTABLE TYPE

ARTUK, JD Series
Stimsonite, Model 967 (with 83 mm Acrylic cube corner reflector)
Stimsonite, Model 967LS
Vega Molded Products, Models GBM and JD

THRIE BEAM BARRIER MARKERS
(For use to the left of traffic)

Duraflex Corp., "Railrider"
Davidson Plastics, "Mini" (75 mm x 254 mm)

CONCRETE BARRIER DELINEATORS, 400 mm
(For use to the right of traffic. When mounted on top of barrier, places top of reflective element at 1200 mm)

Davidson Plastics, Model PCBM T-16
Safe-Hit, Model SH216RBM

CONCRETE BARRIER-MOUNTED MINI-DRUM
(260 mm x 360 mm x 570 mm)

Stinson Equipment Company "SaddleMarker"

SOUND WALL DELINEATOR
(Applied to a vertical surface. Top of reflective element at 1200 mm)

Davidson Plastics, PCBM S-36

GUARD RAILING DELINEATOR
(Top of reflective element at 1200 mm above plane of roadway)

WOOD POST TYPE, 686 mm

Carsonite, Model 427
Davidson Plastics FG 427 and FG 527
FlexStake, Model 102 GR
GreenLine GRD 27
J.Miller Model JMI-375G
Safe-Hit, Model SH227GRD

STEEL POST TYPE

Carsonite, Model CFGR-327 with CFGRBK300 Mounting Bracket

RETROREFLECTIVE SHEETING FOR:

CHANNELIZERS, BARRIER MARKERS, AND DELINEATORS

3M, High Intensity
Reflexite, PC-1000 Metalized Polycarbonate
Reflexite, AC-1000 Acrylic
Reflexite, AP-1000 Metalized Polyester
Reflexite, AR-1000 Abrasion Resistant Coating
Stimsonite, Series 6200 (For rigid substrate devices only)

TRAFFIC CONES, 330 mm Sleeves

Reflexite SB (Polyester), Vinyl or "TR" (Semi-transparent)

TRAFFIC CONES, 100 mm and 150 mm Sleeves

3M Series 3840
Reflexite Vinyl, "TR" (Semi-transparent) or "Conformalite"

BARRELS AND DRUMS

Reflexite, "Super High Intensity" or "High Impact Drum Sheeting"
3M Series 3810

BARRICADES: Type I, Engineering Grade

American Decal, Adcolite
Avery Dennison, 1500 and 1600
3M, Scotchlite, Series CW

BARRICADES: Type II, Super Engineering Grade

Avery Dennison, "Fasign" 2500 Series
Kiwalite Type II
Nikkalite 1800 Series

SIGNS: Type II, Super Engineering Grade

Avery Dennison, "Fasign" 2500 Series
Kiwalite, Type II
Nikkalite 1800 Series

SIGNS: Type III, High-Intensity Grade

3M Series 3800
Nippon Carbide, Nikkalite Brand Ultralite Grade II

SIGNS: Type IV, High-Intensity Prismatic Grade

Stimsonite Series 6200

SIGNS: Type VII, High-Intensity Prismatic Grade

3M Series 3900

SIGNS: Type VI, Roll-Up Signs

Reflexite, Vinyl (Orange), Reflexite "SuperBright" (Fluorescent orange)
3M Series RS34 (Orange) and RS20 (Fluorescent orange)

SIGN SUBSTRATE FOR CONSTRUCTION AREA SIGNS

ALUMINUM

FIBERGLASS REINFORCED PLASTIC (FRP)

Sequentia, "Polyplate"
Fiber-Brite

8-1.03 STATE-FURNISHED MATERIALS

Attention is directed to Section 6-1.02, "State-Furnished Materials," of the Standard Specifications and these special provisions.

The following materials will be furnished to the Contractor:

Sign panels for roadside signs and overhead sign structures.
Sign overlay.

Hardware for mounting sign panels as follows:

1. Blind rivets for mounting overlapping legend at sign panel joints.
2. Closure inserts.
3. Aluminum bolts, nuts, and washers for mounting overhead formed panels.

Master traffic controller unit (including wired cabinet, controller unit and loop detector sensor units).
Operation workstation.
Camera control panel.
Treadle for new lane and toll booth.
Lane Console for the new and relocated toll booths.

The Contractor shall call the electrical supervisor at (510) 782-4453 thirty (30) working days before beginning work on the new toll booth for the delivery of the Treadle and Lane Console.

Operation workstation, camera control panel and master traffic controller unit will be furnished to the Contractor at the Caltrans traffic operation center, 111 Grand Avenue, Oakland, CA 94623.

8-1.04 MEASUREMENT OF QUANTITIES

Attention is directed to the provisions in Section 9-1.01, "Measurement of Quantities," of the Standard Specifications and these special provisions.

The following is added after the third paragraph in Section 9-1.01, "Measurement of Quantities," of the Standard Specifications:

All elements of the material plant controller which affect the accuracy or delivery of data shall be made available for the application of security seals. These devices will be inspected and adjusting elements sealed prior to the first production of materials for the contract. The security seals will be furnished by the Engineer. Material production shall cease when alteration, disconnection, or otherwise manipulation of the security seals occur and production shall not resume until the device is inspected and resealed by the Engineer.

8-1.05 ENGINEERING FABRICS

Engineering fabrics shall conform to the requirements in Section 88, "Engineering Fabrics," of the Standard Specifications and these special provisions.

Filter fabric for this project shall be ultraviolet ray (UV) protected.

Nonwoven and woven rock slope protection fabric shall conform to the following additional requirement:

Specification	ASTM Designation	Requirement
Permittivity, 1/second, Minimum	D 4491	0.5

SECTION 8-2. CONCRETE

8-2.01 PORTLAND CEMENT CONCRETE

Portland cement concrete shall conform to the provisions in Section 90, "Portland Cement Concrete," of the Standard Specifications and these special provisions.

Unless the use of mineral admixture is prohibited, whenever the word "cement" is found in the Standard Specifications or the special provisions, it shall be understood to mean "cementitious material" when both of the following conditions are met:

A. The cement content of portland cement concrete is specified, and
B. Section 90, "Portland Cement Concrete," of the Standard Specifications is referenced.

Portland cement concrete that is produced using equipment where the cement and mineral admixture are proportioned in the same weigh hopper shall be sampled and tested by the Contractor, in the presence of the Engineer, for mix uniformity in conformance with the requirements of ASTM Designation: C 94 Section 11, "Mixing and Delivery," and "Annex A1." The testing shall be performed on concrete produced using an approved project mix design and may be done at the project concrete placement site.

The batch plant producing the portland cement concrete for the project shall have met the requirements of California Test 109 within one year prior to producing concrete for the project.

Sampling for mix uniformity tests shall be performed the first time portland cement concrete, of sufficient volume to perform these tests, is placed on the project. All test results shall be presented to the Engineer no later than 10 days after completion of sampling.

Test results from mixer uniformity testing will not be used for contract compliance, acceptance, or payment.

Prior to placing any concrete on the project, the Contractor shall supply a list of all portland cement concrete mixers to be used. When truck mixers are to be used, the list shall contain the truck identification number, mixer brand, mixer age and mixer condition.

When truck mixers are used, the mix uniformity testing shall be performed on 5 truck mixers per project. The truck mixers selected for testing shall be representative of the different mixer brands, ages, and conditions of the mixers on the list and approved by the Engineer. Mixer selection shall be completed before mix uniformity testing is started. Sampling for the mix uniformity tests from each of the 5 mixers shall be completed within the same work shift, unless otherwise approved in writing by the Engineer. The Contractor shall notify the Engineer, in writing, a minimum of 24 hours prior to performing the sampling for these tests. The letter of notification shall include 1) the truck mixer information, 2) the specific gravity of the coarse aggregate in the mix to be tested, and 3) a copy of the current ACI "Concrete Field Testing Technician, Grade 1" certification for each tester who will perform testing for the Contractor. The Contractor shall provide an adequate number of testers to successfully perform the testing with a minimum amount of impact to the Contractor's operations.

When concrete is completely mixed in stationary mixers, each mixer used for the project shall be tested one time.

Full compensation for the testing of mix uniformity as specified herein will be considered as included in the contract price paid for the concrete work involved and no additional compensation will be allowed therefor.

Unless otherwise specified, Type C accelerating chemical admixture conforming to the requirements of ASTM Designation: C 494, may be used in portland cement concrete for precast steam cured concrete members.

Section 90-1.01, "Description," of the Standard Specifications is amended to read:

90-1.01 Description.—Portland cement concrete shall be composed of cementitious material, fine aggregate, coarse aggregate, admixtures if used, and water, proportioned and mixed as specified in these specifications.

Unless otherwise specified, cementitious material to be used in portland cement concrete shall conform to the requirements for cement and mineral admixtures in Section 90-2, "Materials" and shall be either: 1) "Type IP (MS) Modified" cement; or 2) a combination of "Type II Modified" portland cement and mineral admixture.

Concrete for each portion of the work shall comply with the requirements for the Class, cementitious material content in kilograms per cubic meter, 28-day compressive strength, minor concrete, or commercial quality concrete, as shown on the plans or specified in these specifications or the special provisions.

Class 1 concrete shall contain not less than 400 kg of cementitious material per cubic meter.

Class 2 concrete shall contain not less than 350 kg of cementitious material per cubic meter.

Class 3 concrete shall contain not less than 300 kg of cementitious material per cubic meter.

Class 4 concrete shall contain not less than 250 kg of cementitious material per cubic meter.

Minor concrete shall contain not less than 325 kg of cementitious material per cubic meter unless otherwise specified in these specifications or the special provisions.

Unless otherwise designated on the plans or specified in these specifications or the special provisions, the amount of cementitious material used per cubic meter of concrete in structures or portions of structures shall conform to the following:

Use	Cementitious Material Content (kg/m^3)
Concrete which is designated by compressive strength:	
Deck slabs and slab spans of bridges	400 min., 475 max.
Roof sections of exposed top box culverts	400 min., 475 max.
Other portions of structures	350 min., 475 max.
Concrete not designated by compressive strength:	
Deck slabs and slab spans of bridges	400 min.
Roof sections of exposed top box culverts	400 min.
Prestressed members	400 min.
Seal courses	400 min.
Other portions of structures	350 min.
Concrete for precast members	350 min., 550 max.

Whenever the 28-day compressive strength shown on the plans is greater than 25 MPa, the concrete shall be considered to be designated by compressive strength. If the plans show a 28-day compressive strength which is 31 MPa or greater, an additional 7 days will be allowed to obtain the specified strength. The 28-day compressive strengths shown on the plans which are 25 MPa or less, are shown for design information only and are not to be considered a requirement for acceptance of the concrete.

Concrete designated by compressive strength shall be proportioned such that the concrete will conform to the strength shown on the plans or specified in the special provisions.

The Contractor shall determine the mix proportions for all concrete except pavement concrete. The Engineer will determine the mix proportions for pavement concrete.

Before using concrete for which the mix proportions have been determined by the Contractor, or in advance of revising those mix proportions, the Contractor shall submit in writing to the Engineer a copy of the mix design.

Compliance with cementitious material content requirements will be verified in conformance with procedures described in California Test 518 for cement content. For testing purposes, mineral admixture shall be considered to be cement. Batch proportions shall be adjusted as necessary to produce concrete having the specified cementitious material content.

If any concrete used in the work has a cementitious material content, consisting of cement,-mineral admixture, or cement plus mineral admixture, which is less than the minimum required for the work, the concrete shall be removed. However, if the Engineer determines that the concrete is structurally adequate, the concrete may remain in place and the Contractor shall pay to the State $0.55 for each kilogram of cement, mineral admixture, or cement plus mineral admixture which is less than the minimum required for the work. The Department may deduct the amount from any monies due, or that may become due, the Contractor under the contract. The deductions will not be made unless the difference between the contents required and those actually provided exceeds the batching tolerances permitted by Section 90-5, "Proportioning." No deductions for cementitious material content will be made based on the results of California Test 518.

The requirements of the preceding paragraph shall not apply to minor concrete nor commercial quality concrete.

All concrete for which the mix proportions are determined either by the Contractor or the Engineer shall conform to the requirements of this Section 90.

The first paragraph in Section 90-2.01, "Portland Cement," of the Standard Specifications is amended to read:

90-2.01 Portland Cement.—Unless otherwise specified, portland cement shall be either "Type IP (MS) Modified" cement or "Type II Modified" portland cement.

"Type IP (MS) Modified" cement shall conform to the specifications for Type IP (MS) cement in ASTM Designation: C 595, and shall be comprised of an intimate mixture of Type II cement and not more than 25 percent of a mineral admixture. The type and minimum amount of mineral admixture used in the manufacture of "Type IP (MS) Modified" cement shall be in conformance with the provisions of Section 90-4.08, "Required Use of Mineral Admixtures."

"Type II Modified" portland cement shall conform to the specifications for Type II portland cement in ASTM Designation: C 150.

In addition, "Type IP (MS) Modified" cement and "Type II Modified" portland cement shall conform to the following requirements:

A. The cement shall not contain more than 0.60 percent by mass of alkalies, calculated as the percentage of Na_2O plus 0.658 times the percentage of K_2O, when determined by either direct intensity flame photometry or by the atomic absorption method. The instrument and procedure used shall be qualified as to precision and accuracy in conformance with the requirements of ASTM Designation: C 114.
B. The autoclave expansion shall not exceed 0.50 percent.

C. Mortar, containing the cement to be used and Ottawa sand, when tested in conformance with California Test 527, shall not expand in water more than 0.010 percent and shall not contract in air more than 0.048 percent except that when cement is to be used for precast prestressed concrete piling, precast prestressed concrete members or steam cured concrete products, the mortar shall not contract in air more than 0.053 percent.

The second paragraph in Section 90-2.01, "Portland Cement," of the Standard Specifications is amended to read:

Type III and Type V portland cements shall conform to the specifications in ASTM Designation: C 150, and the additional requirements listed above for Type II Modified portland cement, except that when tested in conformance with California Test 527, mortar containing Type III portland cement shall not contract in air more than 0.075 percent.

The third paragraph in Section 90-2.01, "Portland Cement," of the Standard Specifications is deleted.
The twelfth paragraph in Section 90-2.02, "Aggregates," of the Standard Specifications is deleted.
The first paragraph in Section 90-2.03, "Water," of the Standard Specifications is amended to read:

90-2.03 Water.—In conventionally reinforced concrete work, the water for curing, for washing aggregates, and for mixing shall be free from oil and shall not contain more than 1,000 parts per million of chlorides as Cl, nor more than 1,300 parts per million of sulfates as SO_4. In prestressed concrete work, the water for curing, for washing aggregates, and for mixing shall be free from oil and shall not contain more than 650 parts per million of chlorides as Cl, nor more than 1,300 parts per million of sulfates as SO_4. In no case shall the water contain an amount of impurities that will cause either: 1) a change in the setting time of cement of more than 25 percent when tested in conformance with ASTM Designation: C 191 or ASTM Designation: C 266; or 2) a reduction in the compressive strength of mortar at 14 days of more than 5 percent, when tested in conformance with ASTM Designation: C 109, when compared to the results obtained with distilled water or deionized water, tested in conformance with ASTM Designation: C 109.

The following section is added to Section 90-2, "Materials," of the Standard Specifications:

90-2.04 Admixture Materials.—Admixture materials shall conform to the requirements of the ASTM Designations shown below:

Chemical Admixtures—ASTM Designation: C 494.
Air-entraining Admixtures—ASTM Designation: C 260.
Calcium Chloride—ASTM Designation: D 98.
Mineral Admixtures—Coal fly ash, raw or calcined natural pozzolan as specified in ASTM Designation: C 618, except that the loss on ignition shall not exceed 4 percent, or, silica fume as specified in ASTM Designation: C 1240, with reduction of mortar expansion of 80 percent, minimum, using the cement from the proposed mix design.

Mineral admixtures shall be used in conformance with the provisions in Section 90-4.08, "Required Use of Mineral Admixtures."

Section 90-4.02, "Materials," of the Standard Specifications is amended to read:

90-4.02 Materials.—Admixture materials shall be as specified in Section 90-2.04, "Admixture Materials."

Section 90-4.05, "Optional Use of Chemical Admixtures," of the Standard Specifications is amended to read:

90-4.05 Optional Use of Chemical Admixtures.—The Contractor will be permitted to use Type A or F, water-reducing; Type B, retarding; or Type D or G, water-reducing and retarding admixtures as described in ASTM Designation: C 494 to conserve cementitious material or to facilitate any concrete construction application subject to the following conditions:

When a water-reducing admixture or a water-reducing and retarding admixture is used, the cementitious material content specified or ordered may be reduced by a maximum of 5 percent by mass except that the resultant cementitious material content shall be not less than 300 kilograms per cubic meter.

When a reduction in cementitious material content is made, the dosage of admixture used shall be the dosage used in determining approval of the admixture.

Section 90-4.07, "Optional Use of Air-entraining Admixtures," of the Standard Specifications is amended to read:

90-4.07 Optional Use of Air-entraining Admixtures.—When air-entrainment has not been specified or ordered by the Engineer, the Contractor will be permitted to use an air-entraining admixture to facilitate the use of any construction

procedure or equipment provided that the average air content, as determined by California Test 504, of 3 successive tests does not exceed 4 percent and no single test value exceeds 5.5 percent. If the Contractor elects to use an air-entraining admixture in concrete for pavement, the Contractor shall so indicate at the time the Contractor designates the source of aggregate as provided in Section 40-1.015, "Cement Content."

Section 90-4.08, "Required Use of Mineral Admixtures," of the Standard Specifications is amended to read:

90-4.08 Required Use of Mineral Admixtures.—Unless otherwise specified, mineral admixture shall be combined with cement to make cementitious material for use in portland cement concrete.

The calcium oxide content of mineral admixtures shall not exceed 10 percent and the available alkali, as sodium oxide equivalent, shall not exceed 1.5 percent when measured in conformance with the requirements of ASTM Designation: C 618.

The amounts of cement and mineral admixture used in cementitious material for portland cement concrete shall be sufficient to satisfy the minimum cementitious material content requirements specified in Section 90-1.01, "Description," or Section 90-4.05, "Optional Use of Chemical Admixtures," and shall conform to the following:

The minimum amount of cement shall not be less than 75 percent by mass of the specified minimum cementitious material content.

The minimum amount of mineral admixture to be combined with cement shall be determined using one of the following criteria:

A. When the calcium oxide content of a mineral admixture, measured in conformance with the requirements of ASTM Designation: C 618 and Section 90-2.04, "Admixture Materials," is equal to or less than 2 percent by mass, the amount of mineral admixture shall not be less than 15 percent by mass of the total amount of cementitious material to be used in the mix.
B. When the calcium oxide content of a mineral admixture, measured in conformance with the requirements of ASTM Designation: C 618 and Section 90-2.04, "Admixture Materials," is greater than 2 percent , the amount of mineral admixture shall not be less than 25 percent by mass of the total amount of cementitious material to be used in the mix.
C. When a mineral admixture is used, which conforms to the requirements for silica fume in Section 90-2.04, "Admixture Materials," is used, the amount of mineral admixture shall not be less than 10 percent by mass of the total amount of cementitious material to be used in the mix.

If more than the required amount of cementitious material is used, the additional cementitious material in the mix may be either cement, any mineral admixture conforming to the requirements of Section 90-2.04, "Admixture Materials," or a combination of both; however, the maximum total amount of mineral admixture shall not exceed 35 percent by mass of the total amount of cementitious material to be used in the mix. Where Section 90-1.01, "Description," specifies a maximum cementitious content in kilograms per cubic meter, the total mass of cement and mineral admixture per cubic meter shall not exceed the specified maximum cementitious material content.

Section 90-4.09, "Optional Use of Mineral Admixture," of the Standard Specifications is deleted.

Section 90-4.11, "Storage, Proportioning, and Dispensing of Mineral Admixtures," of the Standard Specifications is amended to read:

90-4.11 Storage, Proportioning, and Dispensing of Mineral Admixtures.—Mineral admixtures shall be protected from exposure to moisture until used. Sacked material shall be piled to permit access for tally, inspection and identification for each shipment.

Adequate facilities shall be provided to assure that mineral admixtures meeting the specified requirements are kept separate from other mineral admixtures in order to prevent any but the specified mineral admixtures from entering the work. Safe and suitable facilities for sampling mineral admixtures shall be provided at the weigh hopper or in the feed line immediately in advance of the hopper.

Mineral admixtures shall be incorporated into concrete using equipment conforming to the requirements for cement weigh hoppers, and charging and discharging mechanisms in ASTM Designation: C 94, in Section 90-5.03, "Proportioning," and in this Section 90-4.11.

When interlocks are required for cement and mineral admixture charging mechanisms by Section 90-5.03A, "Proportioning for Pavement," and cement and mineral admixtures are weighed cumulatively, their charging mechanisms shall be interlocked to prevent the introduction of mineral admixture until the mass of cement in the cement weigh hopper is within the tolerances specified in Section 90-5.02, "Proportioning Devices."

Mineral admixture used in concrete for exposed surfaces of like elements of a structure shall be from the same source and of the same percentage.

Section 90-5.02, "Proportioning Devices," of the Standard Specifications is amended to read:

90-5.02 Proportioning Devices.—All weighing, measuring or metering devices used for proportioning materials shall conform to the requirements in Section 9-1.01, "Measurement of Quantities," and this Section 90-5.02. In addition, any automatic weighing systems used shall comply with the requirements for automatic proportioning devices in Section 90-5.03A, "Proportioning for Pavement." These automatic devices shall be automatic to the extent that the only manual operation required for proportioning the aggregates, cement, and mineral admixture for one batch of concrete is a single operation of a switch or starter.

Proportioning devices shall be tested at the expense of the Contractor as frequently as the Engineer may deem necessary to insure their accuracy.

Weighing equipment shall be insulated against vibration or movement of other operating equipment in the plant. When the plant is in operation, the mass of each batch of material shall not vary from the mass designated by the Engineer by more than the tolerances specified herein.

Equipment for cumulative weighing of aggregate shall have a zero tolerance of ±0.5 percent of the designated total batch mass of the aggregate. For systems with individual weigh hoppers for the various sizes of aggregate, the zero tolerance shall be ±0.5 percent of the individual batch mass designated for each size of aggregate. Equipment for cumulative weighing of cement and mineral admixtures shall have a zero tolerance of ±0.5 percent of the designated total batch mass of the cement and mineral admixture. Equipment for weighing cement or mineral admixture separately shall have a zero tolerance of ±0.5 percent of their designated individual batch masses. Equipment for measuring water shall have a zero tolerance of ±0.5 percent of its designated mass or volume.

The mass indicated for any batch of material shall not vary from the preselected scale setting by more than the following:

A. Aggregate weighed cumulatively shall be within 1.0 percent of the designated total batch mass of the aggregate. Aggregates weighed individually shall be within 1.5 percent of their respective designated batch masses.
B. Cement shall be within 1.0 percent of its designated batch mass. When weighed individually, mineral admixture shall be within 1.0 percent of its designated batch mass. When mineral admixture and cement are permitted to be weighed cumulatively, cement shall be weighed first to within 1.0 percent of its designated batch mass, and the total for cement and mineral admixture shall be within 1.0 percent of the sum of their designated batch masses.
C. Water shall be within 1.5 percent of its designated mass or volume.

Each scale graduation shall be approximately 0.001 of the total capacity of the scale. The capacity of scales for weighing cement, mineral admixture, or cement plus mineral admixture and aggregates shall not exceed that of commercially available scales having single graduations indicating a mass not exceeding the maximum permissible mass variation above, except that no scale shall be required having a capacity of less than 500 kg, with 0.5 kg graduations.

Section 90-5.03, "Proportioning," excluding Section 90-5.03A, "Proportioning for Pavement," of the Standard Specifications is amended to read:

90-5.03 Proportioning.—Proportioning shall consist of dividing the aggregates into the specified sizes, each stored in a separate bin, and combining them with cement, mineral admixture and water as provided in these specifications. Aggregates shall be proportioned by mass.

At the time of batching, all aggregates shall have been dried or drained sufficiently to result in a stable moisture content such that no visible separation of water from aggregate will take place during transportation from the proportioning plant to the point of mixing. In no event shall the free moisture content of the fine aggregate at the time of batching exceed 8 percent of its saturated, surface-dry mass.

Should separate supplies of aggregate material of the same size group, but of different moisture content or specific gravity or surface characteristics affecting workability, be available at the proportioning plant, withdrawals shall be made from one supply exclusively and the materials therein completely exhausted before starting upon another.

Bulk "Type IP (MS) Modified" cement, that conforms to the requirements in Section 90-2.01, "Portland Cement," shall be weighed in an individual hopper and shall be kept separate from the aggregates until the ingredients are released for discharge into the mixer.

Bulk cement to be blended with mineral admixture for use in portland cement concrete for pavement and structures may be weighed in separate, individual weigh hoppers or may be weighed in the same weigh hopper with mineral admixture and shall be kept separate from the aggregates until the ingredients are released for discharge into the mixer. If the cement and mineral admixture are weighed cumulatively, the cement shall be weighed first.

When cement and mineral admixtures are weighed in separate weigh hoppers, the weigh systems for the proportioning of the aggregate, the cement, and the mineral admixture shall be individual and distinct from all other weigh systems. Each weigh system shall be equipped with a hopper, a lever system, and an indicator to constitute an individual and independent material weighing device. The cement and the mineral admixture shall be discharged into the mixer simultaneously with the aggregate.

The scale and weigh hopper for bulk weighing cement, mineral admixture, and cement plus mineral admixture shall be separate and distinct from the aggregate weighing equipment.

When the source of any aggregate is changed for concrete structures, the Contractor shall adjust the mix proportions and submit in writing to the Engineer a copy of the mix design before using such aggregates. When the source of any aggregate is changed for other concrete, the Engineer shall be allowed sufficient time to adjust the mix and such aggregates shall not be used until necessary adjustments are made.

For all batches with a volume of one cubic meter or more, the batching equipment shall conform to one of the following combinations:

A. Separate boxes and separate scale and indicator for weighing each size of aggregate.
B. Single box and scale indicator for all aggregates.
C. Single box or separate boxes and automatic weighing mechanism for all aggregates.

In order to check the accuracy of batch masses, the gross mass and tare mass of batch trucks, truck mixers, truck agitators, and non-agitating hauling equipment shall be determined when ordered by the Engineer. The equipment shall be weighed at the Contractor's expense on scales designated by the Engineer.

Section 90-5.03A, "Proportioning for Pavement," of the Standard Specifications is amended to read:

90-5.03A Proportioning for Pavement.—Aggregates and bulk cement, mineral admixture, and cement plus mineral admixture for use in pavement shall be proportioned by mass by means of automatic proportioning devices of approved type conforming to the requirements specified in this Section 90-5.03A.

The Contractor shall install and maintain in operating condition an electrically actuated moisture meter that will indicate, on a readily visible scale, changes in the moisture content of the fine aggregate as it is batched within a sensitivity of 0.5 percent by mass of the fine aggregate.

The batching of cement, mineral admixture, or cement plus mineral admixture and aggregate shall be interlocked so that a new batch cannot be started until all weigh hoppers are empty, the proportioning devices are within zero tolerance, and the discharge gates are closed. The interlock shall permit no part of the batch to be discharged until all aggregate hoppers and the cement and mineral admixture hoppers or the cement plus mineral admixture hopper are charged with masses which are within the tolerances specified in Section 90-5.02, "Proportioning Devices."

The discharge gate on the cement and mineral admixture hoppers or the cement plus mineral admixture hopper shall be designed to permit regulating the flow of cement, mineral admixture, or cement plus mineral admixture into the aggregate as directed by the Engineer.

When separate weigh boxes are used for each size of aggregate, the discharge gates shall permit regulating the flow of each size of aggregate as directed by the Engineer.

Material discharged from the several bins shall be controlled by gates or by mechanical conveyors. The means of withdrawal from the several bins, and of discharge from the weigh box, shall be interlocked so that not more than one bin can discharge at a time, and that the weigh box cannot be tripped until the required quantity from each of the several bins has been deposited therein. Should a separate weigh box be used for each size of aggregate, all may be operated and discharged simultaneously.

When the discharge from the several bins is controlled by gates, each gate shall be actuated automatically so that the required mass is discharged into the weigh box, after which the gate shall automatically close and lock.

The automatic weighing system shall be designed so that all proportions required may be set on the weighing controller at the same time.

The third paragraph in Section 90-6.01, "General," of the Standard Specifications is amended to read:

All concrete shall be homogeneous and thoroughly mixed, and there shall be no lumps or evidence of undispersed cement, mineral admixture, or cement plus mineral admixture.

The third and fourth paragraphs in Section 90-6.02, "Machine Mixing," of the Standard Specifications are amended to read:

The batch shall be so charged into the mixer that some water will enter in advance of cementitious materials and aggregates. All water shall be in the drum by the end of the first one-fourth of the specified mixing time.

Cementitious materials shall be batched and charged into the mixer by means that will not result either in loss of cementitious materials due to the effect of wind, or in accumulation of cementitious materials on surfaces of conveyors or hoppers, or in other conditions which reduce or vary the required quantity of cementitious material in the concrete mixture.

The sixth paragraph in Section 90-6.02, "Machine Mixing," of the Standard Specifications is amended to read:

The total elapsed time between the intermingling of damp aggregates and all cementitious materials and the start of mixing shall not exceed 30 minutes.

The seventh through tenth paragraphs in Section 90-6.03, "Transporting Mixed Concrete," of the Standard Specifications are amended to read:

When a truck mixer or agitator is used for transporting concrete to the delivery point, discharge shall be completed within 1.5 hours, or before 250 revolutions of the drum or blades, whichever comes first, after the introduction of the cement to the aggregates. Under conditions contributing to quick stiffening of the concrete, or when the temperature of the concrete is 30° C, or above, a time less than 1.5 hours may be required.

When non-agitating hauling equipment is used for transporting concrete to the delivery point, discharge shall be completed within one hour after the addition of the cement to the aggregates. Under conditions contributing to quick stiffening of the concrete, or when the temperature of the concrete is 30° C, or above, the time between the introduction of cement to the aggregates and discharge shall not exceed 45 minutes.

Each load of concrete delivered at the jobsite shall be accompanied by a weight certificate showing the mix identification number, non-repeating load number, date and time at which the materials were batched, the total amount of water added to the load and for transit-mixed concrete, the reading of the revolution counter at the time the truck mixer is charged with cement. This weight certificate shall also show the actual scale masses (kilograms) for the ingredients batched. Theoretical or target batch masses shall not be used as a substitute for actual scale masses.

Weight certificates shall be provided in printed form, or if approved by the Engineer, the data may be submitted in electronic media. Electronic media shall be presented in a tab-delimited format on 90 mm diskette with a capacity of at least 1.4 megabytes. Captured data, for the ingredients represented by each batch shall be LFCR (one line, separate record) with allowances for sufficient fields to satisfy the amount of data required by these specifications.

The Contractor may furnish a weight certificate that is accompanied by a separate certificate which lists the actual batch masses or measurements for a load of concrete provided that both certificates are 1) imprinted with the same non-repeating load number that is unique to the contract and 2) delivered to the jobsite with the load.

All weight certificates furnished by the Contractor shall conform to the requirements of Section 9-1.01, "Measurement of Quantities."

Section 90-6.05, "Hand-Mixing," of the Standard Specifications is amended to read:

90-6.05 Hand-Mixing.—Hand-mixed concrete shall be made in batches not more than one-fourth cubic meter and shall be mixed on a watertight, level platform. The proper amount of coarse aggregate shall be measured in measuring boxes and spread on the platform and the fine aggregate shall be spread on this layer, the 2 layers being not more than 0.3 meters in total depth. On this mixture shall be spread the dry cement and mineral admixture and the whole mass turned no fewer than 2 times dry; then sufficient clean water shall be added, evenly distributed, and the whole mass again turned no fewer than 3 times, not including placing in the carriers or forms.

The table in the first paragraph in Section 90-6.06, "Amount of Water and Penetration," of the Standard Specifications is amended to read:

Type of Work	Nominal Penetration (mm)	Maximum Penetration (mm)
Concrete pavement	0-25	40
Non-reinforced concrete facilities	0-35	50
Reinforced concrete structures:		
Sections over 300 mm thick	0-35	65
Sections 300 mm thick or less	0-50	75
Concrete placed under water	75-100	115
Cast-in-place concrete piles	65-90	100

The first paragraph following the table of penetration ranges in Section 90-6.06, "Amount of Water and Penetration," of the Standard Specifications is amended to read:

The amount of free water used in concrete shall not exceed 183 kg/m^3, plus 20 kg for each required 100 kg of cementitious material in excess of 325 kg/m^3.

The fourth paragraph in Section 90-6.06, "Amount of Water and Penetration," of the Standard Specifications is amended to read:

Where there are adverse or difficult conditions which affect the placing of concrete, the above specified penetration and free water content limitations may be exceeded providing the Contractor is granted permission by the Engineer in

writing to increase the cementitious material content per cubic meter of concrete. The increase in water and cementitious material shall be at a ratio not to exceed 30 kg of water per added 100 kg of cementitious material per cubic meter. The cost of additional cementitious material and water added under these conditions shall be at the Contractor's expense and no additional compensation will be allowed therefor.

Section 90-9.01, "General," of the Standard Specifications is amended to read:

90-9.01 General.—Concrete compressive strength requirements consist of a minimum strength which must be attained before various loads or stresses are applied to the concrete and, for concrete designated by strength, a minimum strength at the age of 28 days or at the age otherwise allowed in Section 90-1.01, "Description." The various strengths required are specified elsewhere or are shown on the plans.

The compressive strength of concrete will be determined from test cylinders which have been fabricated from concrete sampled in conformance with California Test 539. Test cylinders will be molded and initial field cured in conformance with California Test 540. Test cylinders will be cured and tested after receipt at the testing laboratory in conformance with California Test 521. A strength test shall consist of the average strength of 2 cylinders fabricated from material taken from a single load of concrete, except that, if any cylinder should show evidence of improper sampling, molding, or testing, that cylinder shall be discarded and the strength test shall consist of the strength of the remaining cylinder.

When concrete compressive strength is specified as a prerequisite to applying loads or stresses to a concrete structure or member, test cylinders for other than steam cured concrete will be cured in conformance with Method 1 of California Test 540. The compressive strength of concrete determined for these purposes will be evaluated on the basis of individual tests.

When concrete is designated by 28-day compressive strength rather than by cementitious material content, the concrete strength to be used as a basis for acceptance of other than steam cured concrete will be determined from cylinders cured in conformance with Method 1 of California Test 540. If the result of a single compressive strength test at the maximum age specified or allowed is below the specified strength but is 95 percent or more of the specified strength, the Contractor shall, at the Contractor's expense, make corrective changes, subject to approval of the Engineer, in the mix proportions or in the concrete fabrication procedures, before placing additional concrete, and shall pay to the State $14.00 for each in-place cubic meter of concrete represented by the deficient test. If the result of a single compressive strength test at the maximum age specified or allowed is below 95 percent of the specified strength, but is 85 percent or more of the specified strength, the Contractor shall make the corrective changes specified above, and shall pay to the State $20.00 for each in place cubic meter of concrete represented by the deficient test. In addition, such corrective changes shall be made when the compressive strength of concrete tested at 7 days indicates, in the judgment of the Engineer, that the concrete will not attain the required compressive strength at the maximum age specified or allowed. All concrete represented by a single test which indicates a compressive strength of less than 85 percent of the specified 28-day compressive strength will be rejected in conformance with the provisions in Section 6-1.04, "Defective Materials."

If the test result indicates that the compressive strength at the maximum curing age specified or allowed is below the specified strength, but 85 percent or more of the specified strength, payments to the State as required above shall be made, unless the Contractor, at the Contractor's expense, obtains and submits evidence acceptable to the Engineer that the strength of the concrete placed in the work meets or exceeds the specified 28-day compressive strength. If the test result indicates a compressive strength at the maximum curing age specified or allowed below 85 percent, the concrete represented by that test will be rejected, unless the Contractor, at the Contractor's expense, obtains and submits evidence acceptable to the Engineer that the strength and quality of the concrete placed in the work are acceptable. If the evidence consists of tests made on cores taken from the work, the cores shall be obtained and tested in conformance with the specifications of ASTM Designation: C 42.

No single compressive strength test shall represent more than 250 cubic meters.

When a precast concrete member is steam cured, the compressive strength of the concrete will be determined from test cylinders which have been handled and stored in conformance with Method 3 of California Test 540. The compressive strength of steam cured concrete will be evaluated on the basis of individual tests representing specific portions of production. When the concrete is designated by 28-day compressive strength rather than by cementitious material content, the concrete shall be considered to be acceptable whenever its compressive strength reaches the specified 28-day compressive strength provided that strength is reached in not more than the maximum number of days specified or allowed after the member is cast.

When concrete is specified by compressive strength, prequalification of materials, mix proportions, mixing equipment, and procedures proposed for use, will be required prior to placement of the concrete. Prequalification shall be accomplished by the submission of acceptable certified test data or trial batch reports by the Contractor. Prequalification data shall be based on the use of materials, mix proportions, mixing equipment, procedures, and size of batch proposed for use in the work.

Certified test data, in order to be acceptable, must indicate that not less than 90 percent of at least 20 consecutive tests exceed the specified strength at the maximum number of cure days specified or allowed, and none of those tests are

less than 95 percent of specified strength. Strength tests included in the data shall be the most recent tests made on concrete of the proposed mix design and all shall have been made within one year of the proposed use of the concrete.

Trial batch test reports, in order to be acceptable, must indicate that the average compressive strength of 5 consecutive concrete cylinders, taken from a single batch, at not more than 28 days (or the maximum age allowed) after molding shall be at least 4 MPa greater than the specified 28-day compressive strength, and no individual cylinder shall have a strength less than the specified strength at the maximum age specified or allowed. Data contained in the report shall be from trial batches which were produced within one year of the proposed use of specified strength concrete in the project. Whenever air-entrainment is required, the air content of trial batches shall be equal to or greater than the air content specified for the concrete without reduction due to tolerances.

All tests shall be performed in conformance with either the appropriate California Test methods or the comparable ASTM test methods. All equipment employed in testing shall be in good condition and shall be properly calibrated. If the tests are performed during the life of the contract, the Engineer shall be notified sufficiently in advance of performing the tests in order to witness the test procedures.

The certified test data and trial batch test reports shall include the following information:

A. Date of mixing.
B. Mixing equipment and procedures used.
C. The size of batch in cubic meters and the mass, type and source of all ingredients used.
D. Penetration of the concrete.
E. The air content of the concrete if an air-entraining admixture is used.
F. The age at time of testing and strength of all concrete cylinders tested.

All certified test data and trial batch test reports shall be signed by an official of the firm which performed the tests.

When approved by the Engineer, concrete from trial batches may be used in the work at locations where concrete of a lower quality is required and the concrete will be paid for as the type or class of concrete required at that location.

After materials, mix proportions, mixing equipment, and procedures for concrete have been prequalified for use, additional prequalification by testing of trial batches will be required prior to making any changes which, in the judgment of the Engineer, could result in a lowering of the strength of the concrete below that specified.

The Contractor's attention is directed to the time required to test trial batches and the Contractor shall be responsible for production of trial batches at a sufficiently early date so that the progress of the work is not delayed.

When precast concrete members are manufactured at the plant of an established manufacturer of precast concrete members, the mix proportions of the concrete shall be determined by the Contractor, and a trial batch and prequalification of the materials, mix proportions, mixing equipment, and procedures will not be required.

Section 90-10.02A, "Portland Cement," of the Standard Specifications is renamed "Cementitious Material" and is amended to read:

90-10.02A Cementitious Material.—Cementitious material shall conform to the provisions in Section 90-1.01, "Description." Compressive strength requirements consist of a minimum strength which must be attained before various loads or stresses are applied to the concrete and, for concrete designated by strength, a minimum strength at the age of 28 days or at the age otherwise allowed in Section 90-1.01, "Description." The various strengths required are specified elsewhere or are shown on the plans.

The fifth paragraph in Section 90-10.02B, "Aggregate," of the Standard Specifications is deleted.
Section 90-10.03, "Production," of the Standard Specifications is amended to read:

90-10.03 Production.—Cementitious material, water, aggregate, and admixtures shall be stored, proportioned, mixed, transported, and discharged in conformance with recognized standards of good practice, which will result in concrete that is thoroughly and uniformly mixed, that is suitable for the use intended, and which conforms to requirements specified herein. "Recognized standards of good practice" are outlined in various industry publications such as are issued by American Concrete Institute, AASHTO, or California Department of Transportation.

The cementitious material content of minor concrete shall conform to the provisions in Section 90-1.01, "Description."

The amount of water used shall result in a consistency of concrete conforming to the provisions in Section 90-6.06, "Amount of Water and Penetration." Additional mixing water shall not be incorporated into the concrete during hauling or after arrival at the delivery point, unless authorized by the Engineer.

Discharge of ready-mixed concrete from the transporting vehicle shall be made while the concrete is still plastic and before any stiffening occurs. An elapsed time of 1.5 hours (one hour in non-agitating hauling equipment), or more than 250 revolutions of the drum or blades, after the introduction of the cementitious material to the aggregates, or a temperature of concrete of more than 32° C. will be considered as conditions contributing to the quick stiffening of concrete. The Contractor shall take whatever action is necessary to eliminate quick stiffening, except that the addition of water will not be permitted.

The required mixing time in stationary mixers shall be not less than 50 seconds nor more than 5 minutes.

The minimum required revolutions at mixing speed for transit-mixed concrete shall be not less than that recommended by the mixer manufacturer, and shall be increased, if necessary, to produce thoroughly and uniformly mixed concrete.

Each load of ready-mixed concrete shall be accompanied by a weight certificate which shall be delivered to the Engineer at the discharge location of the concrete, unless otherwise directed by the Engineer. The weight certificate shall be clearly marked with the date and time of day when the load left the batching plant and, if hauled in truck mixers or agitators, the time the mixing cycle started.

A Certificate of Compliance in conformance with the provisions in Section 6-1.07, "Certificates of Compliance," shall be furnished to the Engineer, prior to placing minor concrete from a source not previously used on the contract, stating that minor concrete to be furnished meets all contract requirements, including minimum cementitious material content specified.

The third and fourth paragraphs in Section 90-11.02, "Payment," of the Standard Specifications are amended to read:

Should the Engineer order the Contractor to incorporate any admixtures in the concrete when their use is not required by these specifications or the special provisions, furnishing the admixtures and adding them to the concrete will be paid for as extra work as provided in Section 4-1.03D.

Should the Contractor use admixtures as permitted under Sections 90-4.05, "Optional Use of Chemical Admixtures;" or 90-4.07, "Optional Use of Air-entraining Admixtures;" or should the Contractor request and obtain permission to use other admixtures for the Contractor's benefit, the Contractor shall furnish those admixtures and incorporate them in the concrete at the Contractor's expense and no additional compensation will be allowed therefor.

8-2.02 CEMENT AND WATER CONTENT

The amount of free water used in concrete for deck slabs of bridges and structure approach slabs shall not exceed 195 kg/m^3, plus 20 kg for each required 100 kg of cementitious material in excess of 400 kg/m^3.

SECTION 8-3. WELDING

8-3.01 WELDING ELECTRODES

Flux core welding electrodes conforming to the requirements of AWS A5.20 E6XT-4 or E7XT-4 shall not be used to perform any type of welding for this project.

8-3.02 WELDING QUALITY CONTROL

Welding quality control shall conform to the requirements in the AWS welding codes, the Standard Specifications and these special provisions.

Welding quality control shall apply when any work is welded in conformance with the provisions in Section 49, "Piling," Section 52, "Reinforcement," Section 55, "Steel Structures," Section 56-1, "Overhead Sign Structures," Section 75-1.035, "Bridge Joint Restrainer Units," or Section 86-2.04, "Standards, Steel Pedestals and Posts," of the Standard Specifications.

Wherever reference is made to the following AWS welding codes in the Standard Specifications, on the plans or in these special provisions, the year of adoption for these codes shall be as listed:

AWS Code	Year of Adoption
D1.1	1998
D1.4	1992
D1.5	1995
D1.5 (metric only)	1996

All requirements of the AWS welding codes shall apply unless specified otherwise in the Standard Specifications, on the plans or in these special provisions. Wherever the abbreviation AWS is used, it shall be equivalent to the abbreviations ANSI/AWS or ANSI/AASHTO/AWS.

The welding of all fracture critical members (FCMs) shall conform to the provisions specified in the Fracture Control Plan (FCP) and herein.

The Contractor shall designate in writing a welding Quality Control Manager (QCM). The QCM shall be responsible directly to the Contractor for the quality of welding, including materials and workmanship, performed by the Contractor and all subcontractors.

The QCM shall be the sole individual responsible to the Contractor for submitting, receiving, and approving all correspondence, required submittals, and reports to and from the Engineer.

The QCM shall not be employed or compensated by any subcontractor, or by other persons or entities hired by subcontractors, who will provide other services or materials for the project. The QCM may be an employee of the Contractor.

Welding inspection personnel or nondestructive testing (NDT) firms to be used in the work shall not be employed or compensated by any subcontractor, or by other persons or entities hired by subcontractors, who will provide other services or materials for the project, except for the following conditions:

1. The welding is performed at a permanent fabrication facility which is certified under the AISC Quality Certification Program, Category Cbr, Major Steel Bridges.
2. The welding is performed at a permanent fabrication facility which is certified under the AISC Quality Certification Program, Category Sbd, Conventional Steel Building Structures. This condition shall apply only for work welded in conformance with the provisions in Section 56-1, "Overhead Sign Structures" or Section 86-2.04, "Standards, Steel Pedestals and Posts," of the Standard Specifications.

For welding performed at such certified facilities, the inspection personnel or NDT firms may be employed or compensated by the fabrication facility performing the welding.

Prior to submitting the Welding Quality Control Plan (WQCP) required herein, a pre-welding meeting between the Engineer, Contractor and any welding subcontractors or entities hired by these subcontractors to be used in the work, shall be held to discuss the requirements for the WQCP.

Prior to performing any welding, the Contractor shall submit to the Engineer, in conformance with the provisions in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications, 3 copies of a separate WQCP for each item of work for which welding is to be performed. As a minimum, each WQCP shall include the following:

1. The name of the welding firm and the NDT firm to be used;
2. A manual prepared by the NDT firm that shall include equipment, testing procedures, code of safe practices, the Written Practice of the NDT firm, and the names, qualifications and documentation of certifications for all personnel to be used;
3. The name of the QCM and the names, qualifications and documentation of certifications for all Quality Control (QC) Inspectors and Assistant Quality Control Inspectors to be used;
4. An organizational chart showing all QC personnel and their assigned QC responsibilities;
5. The methods and frequencies for performing all required quality control procedures, including QC inspection forms to be used, as required by the specifications including:

   (a) all visual inspections;
   (b) all NDT including radiographic geometry, penetrameter and shim selection, film quality, film processing, radiograph identification and marking system, and film interpretation and reports; and
   (c) calibration procedures and calibration frequency for all NDT equipment;

6. A system for the identification and tracking of all welds, NDT and any required repairs, and a procedure for the reinspection of any repaired welds. The system shall have provisions for 1) permanently identifying each weld and the person who performed the weld, 2) placing all identification and tracking information on each radiograph and 3) a method of reporting nonconforming welds to the Engineer;
7. Standard procedures for performing noncritical repair welds. Noncritical repair welds are–defined as welds to deposit additional weld beads or layers to compensate for insufficient weld size and to fill limited excavations that were performed to remove unacceptable edge or surface discontinuities, rollover or undercut. The depth of these excavations shall not exceed 65 percent of the specified weld size;
8. The welding procedure specification (WPS), including documentation of all supporting Procedure Qualification Record (PQR) tests performed, and the name of the testing laboratory who performed the tests, to verify the acceptability of the WPS. The submitted WPS shall be within the allowable period of effectiveness;
9. Documentation of all certifications for welders for each weld process and position that will be used. Certifications shall list the electrodes used, test position, base metal and thickness, tests performed, and the witnessing authority. All certifications shall be within the allowable period of effectiveness; and
10. One copy each of all AWS welding codes and the FCP which are applicable to the welding to be performed. These codes and the FCP shall become the permanent property of the Department.
11. Example forms to be used for Certificates of Compliance, daily production logs, and daily reports.

The Engineer shall have 10 working days to review the WQCP submittal after a complete plan has been received. No welding shall be performed until the WQCP is approved in writing by the Engineer. Should the Engineer fail to complete the review within this time allowance and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in reviewing the WQCP, the delay will be considered a right of way delay in conformance with the provisions in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

An amended WQCP or addendum shall be submitted to, and approved in writing by the Engineer, for any proposed revisions to the approved WQCP. An amended WQCP or addendum will be required for any revisions to the WQCP, including but not limited to a revised WPS, additional welders, changes in NDT firms or procedures, QC or NDT personnel, or updated systems for tracking and identifying welds. The Engineer shall have 3 working days to complete the review of the amended WQCP or addendum. Work that is affected by any of the proposed revisions shall not be performed until the amended WQCP or addendum has been approved. Should the Engineer fail to complete the review within this time allowance and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in reviewing the amended WQCP or addendum, the delay will be considered a right of way delay in conformance with the provisions in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

After final approval of the WQCP, amended WQCP or addendum, the Contractor shall submit to the Engineer 7 copies each of these approved documents.

It is expressly understood that the Engineer's approval of the Contractor's WQCP shall not relieve the Contractor of any responsibility under the contract for the successful completion of the work in conformity with the requirements of the plans and specifications. The Engineer's approval shall not constitute a waiver of any of the requirements of the plans and specifications nor relieve the Contractor of any obligation thereunder, and defective work, materials and equipment may be rejected notwithstanding approval of the WQCP.

A daily production log for welding shall be kept by the QCM for each day that welding is performed. The log shall clearly indicate the locations of all welding, and shall include the welders' names, amount of welding performed, any problems or deficiencies discovered, and any testing or repair work performed, at each location. The daily report from each Quality Control Inspector shall also be included in the log.

The following items shall be included in a Welding Report that is to be submitted to the Engineer within 7 days following the performance of any welding:

1. Reports of all visual weld inspections and NDT;
2. Radiographs and radiographic reports, and other required NDT reports;
3. Documentation that the Contractor has evaluated all radiographs and other nondestructive tests, corrected all rejectable deficiencies, and all repaired welds have been reexamined by the required NDT and found acceptable; and
4. Daily production log.

All radiographic envelopes shall have clearly written on the outside of the envelope the following information: name of the QCM, name of the nondestructive testing firm, name of the radiographer, date, contract number, complete part description, and all included weld numbers or a report number, as detailed in the WQCP. In addition, all innerleaves shall have clearly written on them the part description and all included weld numbers, as detailed in the WQCP.

All reports regarding NDT, including radiographs, shall be signed by both the NDT technician and the person that performed the review, and then submitted directly to the QCM for review and signature prior to submittal to the Engineer. Corresponding names shall be clearly printed or typewritten next to all signatures.

The Engineer will review the Welding Report to determine if the Contractor is in conformance with the WQCP. Except for steel piling, the Engineer shall be allowed 7 days to review the report and respond in writing after a complete Welding Report has been received. The review time for steel piling shall be as specified in "Piling" of these special provisions. Prior to receiving notification from the Engineer of the Contractor's conformance with the WQCP, the Contractor may encase in concrete or cover any welds for which a Welding Report has been submitted. However, should the Contractor elect to encase or cover those welds prior to receiving notification from the Engineer, it is expressly understood that the Contractor shall not be relieved of the responsibility for incorporating material in the work that conforms to the requirements of the plans and specifications. Any material not conforming to these requirements will be subject to rejection. Should the Contractor elect to wait to encase or cover any welds pending notification by the Engineer, and should the Engineer fail to complete the review and provide notification within this time allowance, and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in notification, the delay will be considered a right of way delay in conformance with the provisions in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

Sections 6.1.2 through 6.1.4.3 of AWS D 1.1, Sections 7.1.1 and 7.1.2 of AWS D 1.4, and Sections 6.1.1.1 through 6.1.3.3 of AWS D 1.5 are replaced with the following:

Quality Control (QC) shall be the responsibility of the Contractor. As a minimum, the Contractor shall perform inspection and testing prior to welding, during welding and after welding as specified in this section and additionally as necessary to ensure that materials and workmanship conform to the requirements of the contract documents.

The Quality Control (QC) Inspector shall be the duly designated person who performs inspection, testing, and quality matters for all welding.

Quality Assurance (QA) is the prerogative of the Engineer. The QA Inspector is the duly designated person who acts for and on behalf of the Engineer.

All QC Inspectors shall be responsible for quality control acceptance or rejection of materials and workmanship, and shall be currently certified as AWS Certified Welding Inspectors (CWI) in conformance with the requirements in AWS QC1, "Standard and Guide for Qualification of Welding Inspectors."

The QC Inspector may be assisted by an Assistant QC Inspector provided that this individual is currently certified as an AWS Certified Associate Welding Inspector (CAWI) in conformance with the requirements in AWS QC1, "Standard and Guide for Qualification of Welding Inspectors," or has equivalent qualifications. The QC Inspector shall monitor the Assistant QC Inspector's work, and shall be responsible for signing all reports.

When the term "Inspector" is used without further qualification, it shall refer to the QC Inspector.

Section 6.14.6, "Personnel Qualification," of AWS D 1.1, Section 7.7.6, "Personnel Qualification," of AWS D 1.4 and Section 6.1.3.4, "Personnel Qualification," of AWS D 1.5 are amended to read:

Personnel performing NDT shall be qualified in conformance with the requirements in the current edition of the American Society for Nondestructive Testing (ASNT) Recommended Practice No. SNT-TC-1A and the Written Practice of the NDT firm. The Written Practice of the NDT firm shall meet or exceed the requirements of the current edition of the ASNT Recommended Practice No. SNT-TC-1A. Only individuals who are 1) qualified for NDT Level II, or 2) Level III technicians who have been directly certified by the ASNT and are authorized to perform the work of Level II technicians, shall perform NDT, review the results, and prepare the written reports.

Section 6.5.4, "Scope of Examination," of AWS D 1.1 and Section 7.5.4 of AWS D 1.4 are amended to read:

The QC Inspector shall inspect and approve the joint preparation, assembly practice, welding techniques, and performance of each welder, welding operator, and tack welder to make certain that the applicable requirements of this code and the approved WPS are met.

Section 6.5.4 of AWS D 1.5 is amended to read:

The QC Inspector shall inspect and approve the joint preparation, assembly practice, welding techniques, and performance of each welder, welding operator, and tack welder to make certain that the applicable requirements of this code and the approved WPS are met. The QC Inspector shall examine the work to make certain that it meets the requirements of section 3 and 9.21. The size and contour of welds shall be measured using suitable gages. Visual inspection for cracks in welds and base metal, and for other discontinuities should be aided by strong light magnifiers, or such other devices as may be helpful. Acceptance criteria different from those specified in this code may be used when approved by the Engineer.

The Engineer shall have the authority to verify the qualifications or certifications of any welder, Quality Control Inspector, or NDT personnel to specified levels by retests or other means.

A sufficient number of QC Inspectors shall be provided to ensure continuous inspection when any welding is being performed. Continuous inspection, as a minimum, shall include (1) having QC Inspectors continually present on all shifts when any welding is being performed, or (2) having a QC Inspector within such close proximity of all welding operations that inspections by the QC Inspector of each operation, at each welding location, shall not lapse for a period exceeding 30 minutes.

Inspection and approval of the joint preparation, assembly practice, welding techniques, and performance of each welder, welding operator, and tack welder shall be documented by the QC Inspector on a daily basis for each day that welding is performed.

The QC Inspector shall provide reports to the QCM on a daily basis for each day that welding is performed.

Except for noncritical weld repairs, base metal repairs, or any other type of repairs not submitted in the WQCP, the Engineer shall be notified immediately in writing when any welding problems or deficiencies are discovered and also of the proposed repair procedures to correct them. The Engineer shall have 5 working days to review these procedures. No remedial work shall begin until the repair procedures are approved in writing by the Engineer. Should the Engineer fail to complete the review within this time allowance and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in reviewing the proposed repair procedures, the delay will be considered a right of way delay in conformance with the provisions in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

When joint details that are not prequalified by the applicable AWS codes are proposed for use in the work, all welders using these details shall perform a qualification test plate using the approved WPS variables and the joint detail to be used in production. The test plate shall be the maximum thickness to be used in production. The test plate shall be mechanically or radiographically tested as directed by the Engineer. Mechanical and radiographic testing and acceptance criteria shall be as specified in the applicable AWS codes.

The period of effectiveness for a welder's or welding operator's qualification shall be a maximum of 3 years for the same weld process, welding position, and weld type. A valid qualification at the beginning of work on a contract will be acceptable for the entire period of the contract, as long as the welder's work remains satisfactory.

All qualification tests for welders, welding operators, and WPSs used in welding operations will be witnessed by the Engineer.

Section 6.6.5, "Nonspecified Nondestructive Testing Other Than Visual," of AWS D 1.1, Section 6.6.5 of AWS D 1.4 and Section 6.6.5 of AWS D 1.5 shall not apply.

For any welding, the Engineer may direct the Contractor to perform NDT that is in addition to the visual inspection or NDT specified in the AWS welding codes, in the Standard Specifications or in these special provisions. Additional NDT required by the Engineer, will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications. Should any welding deficiencies be discovered by this additional NDT, the cost of the testing will not be paid for as extra work, and shall be at the Contractor's expense.

All required repair work to correct welding deficiencies, whether discovered by the required visual inspection or NDT, or by additional NDT directed by the Engineer, and any associated delays or expenses caused to the Contractor by performing these repairs, shall be at the Contractor's expense.

At the completion of all welding, the QCM shall sign and furnish to the Engineer, a certificate of compliance in conformance with the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications for each item of work for which welding was performed. The certificate shall state that all of the materials and workmanship incorporated in the work, and all required tests and inspections of this work, have been performed in conformance with the details shown on the plans and the provisions of the Standard Specifications and these special provisions.

Full compensation for conforming to all of the requirements of this section, Welding Quality Control, shall be considered as included in the contract prices paid for the various contract items of work involved and no additional compensation will be allowed therefor.

SECTION 8-4. ULTIMATE SPLICE TESTING

8-4.01 BAR REINFORCEMENT SPLICE TESTING (ULTIMATE BUTT SPLICES)

Ultimate butt splices and the testing of these splices shall conform to the provisions in "Reinforcement" of these special provisions and the requirements herein.

Ultimate butt splices shall be used at the bent caps of Mt. Eden OH (Widen), Bridge No. 33-0092R/L for transverse No. 36 bars.

The independent qualified testing laboratory used to perform the testing of all ultimate butt sample splices and control bars shall not be employed or compensated by any subcontractor, or by other persons or entities hired by subcontractors, who will provide other services or materials for the project and shall have the following:

1. Proper facilities, including a tensile testing machine capable of breaking the largest size of bar to be tested.
2. A device for measuring the total slip of the reinforcing bars within the splice to the nearest 25 µm. This device shall be placed parallel to the longitudinal axis of the bar and shall be able to simultaneously measure movement on both sides of the splice.
3. Operators who have received formal training for performing the testing requirements of ASTM Designation: A 370/A 370M and California Test 670.
4. A record of annual calibration of testing equipment. The calibration shall be performed by an independent third party that has 1) standards that are traceable to the National Institute of Standards and Technology and 2) a formal reporting procedure, including published test forms.

The Contractor shall designate in writing an ultimate butt splicing Quality Control Manager (QCM). The QCM shall be responsible directly to the Contractor for the quality of all ultimate butt splicing, including materials and workmanship, performed by the Contractor and all subcontractors.

The QCM shall not be employed or compensated by any subcontractor, or by other persons or entities hired by subcontractors, who will provide other services or materials for the project. The QCM may be an employee of the Contractor.

The QCM shall be the sole individual responsible to the Contractor for submitting, receiving, and approving all correspondence, required submittals, and reports regarding ultimate butt splicing to and from the Engineer.

Whenever any lot of ultimate butt splices is rejected, no additional ultimate butt splices shall be placed until the QCM performs a complete review of the Contractor's quality control process and submits written evidence, acceptable to the Engineer, that all remaining splices in this lot conform to the specifications.

Sample splices shall be 1) a minimum length of 1.5 meters for reinforcing bars No. 25 or smaller and 2 meters for reinforcing bars No. 29 or larger, with the splice located at mid-point, and 2) suitably identified prior to shipment with weatherproof markings that do not interfere with the Engineer's tamper-proof markings or seals.

A minimum of one control bar shall be removed from the same bar as, and adjacent to, each sample splice. Control bars shall be 1) a minimum length of 1 meter for reinforcing bars No. 25 or smaller and 1.5 meters for reinforcing bars No. 29 or larger, and 2) suitably identified prior to shipment with weatherproof markings that do not interfere with the Engineer's tamper-proof markings or seals. The portion of adjacent bar remaining in the work shall also be identified with weatherproof markings that correspond to its adjacent control bar.

Shorter length sample splice and control bars may be furnished if approved in writing by the Engineer.

Each sample splice and its associated control bar shall be identified and marked as a set. Each set shall be identified as representing either a prequalification, production, or job control sample splice.

The portion of hoop reinforcing bar, removed to obtain a sample splice and control bar, shall be repaired using a prequalified ultimate mechanical butt splice or the hoop shall be replaced in kind.

Reinforcing bars, other than hoops, from which sample splices are removed shall be repaired using prequalified ultimate mechanical butt splices or the bars shall be replaced in kind.

Section 52-1.08E, "Job Control Tests," of the Standard Specifications shall not apply.

ULTIMATE BUTT SPLICE TEST CRITERIA.—Ultimate prequalification, production, and job control sample splices shall be tensile tested in accordance with the requirements described in ASTM Designation: A 370/A 370M and California Test 670.

Ultimate prequalification and production sample splices shall rupture in the reinforcing bar either: 1) outside of the affected zone or 2) within the affected zone, provided that the sample has achieved at least 95 percent of the ultimate tensile strength of the control bar associated with the sample. In addition, necking of the bar shall be visibly evident at rupture regardless of whether the bar breaks inside or outside the affected zone.

The affected zone is the portion of the reinforcing bar where any properties of the bar, including the physical, metallurgical, or material characteristics, have been altered by fabrication or installation of the splice.

The ultimate tensile strength of each control bar shall be determined by tensile testing the bar to rupture. If two control bars are tested for one sample splice, the bar with the lower ultimate tensile strength shall be considered the control bar.

Testing to determine the minimum tensile strength, in accordance with the ninth paragraph of Section 52-1.08, "Splicing," of the Standard Specifications, will not be required.

PREQUALIFICATION TEST REQUIREMENTS FOR ULTIMATE BUTT SPLICES.—Prior to use in the work, all welded and mechanical ultimate butt splices shall conform to the following prequalification test requirements:

Four prequalification sample splices for each splice type, including ultimate mechanical butt splices, ultimate complete joint penetration butt welded splices, and ultimate resistance butt welded splices that will be used in the work, shall be fabricated by the Contractor and furnished to the Engineer for testing. In addition, for sleeve-filler metal, sleeve-swaged, sleeve-extruded, sleeve-filler grout, and sleeve-lockshear bolt types of couplers, four sample prequalification splices shall be fabricated for each bar deformation pattern that will be used in the work.

If different diameters of hoop reinforcement are shown on the plans, prequalification sample splices, as described above, will only be required for the smallest hoop diameter. In addition, these splices shall be fabricated using the same radius as shown on the plans for said hoops.

Unless otherwise directed in writing by the Engineer, all prequalification sample splices and control bar sets shall be shipped to the Office of Materials Engineering and Testing Services, 5900 Folsom Boulevard, Sacramento, CA 95819, telephone (916) 227-7251.

The 4 sets from each prequalification test shall be securely bundled together and identified by location and contract number with weatherproof markings prior to shipment. Bundles containing fewer than 4 sets will not be tested.

All 4 sample splices from each prequalification test shall conform to the requirements of "Ultimate Butt Splice Test Criteria" specified herein, and Section 52-1.08C, "Mechanical Butt Splices," of the Standard Specifications except the requirement for slip shall be 0.51 mm for No. 36 reinforcing bars.

Test results for each bundle of 4 sets will be reported in writing to the Contractor within 10 working days after receipt of the bundle by the Office of Materials Engineering and Testing Services. In the event that more than one bundle is received on the same day, 2 additional calendar days shall be allowed for providing test results for each additional bundle received. A test report will be made for each bundle received.

Should the Engineer fail to provide the test results within this time allowance and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in providing the test results, the delay will be considered a right of way delay as specified in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

MEASUREMENT AND PAYMENT.—Full compensation for conforming to all of the requirements of this section, Bar Reinforcement Splice Testing (Ultimate Butt Splices), shall be considered as included in the contract prices paid for the various contract items of work involved and no additional compensation will be allowed therefor.

SECTION 9. DESCRIPTION OF BRIDGE WORK

The bridge work consists, in general, of widening existing bridges, constructing soundwall, soil nail retaining wall, and installing sheet pile wall at the following locations:

INDUSTRIAL BOULEVARD UNDERCROSSING (WIDEN)
(Bridge No. 33-0370)

MT. EDEN OVERHEAD (WIDEN)
(Bridge No.33-0092R/L)

HESPERIAN BLVD ON-RAMP SOUNDWALL

SAN MATEO-HAYWARD BRIDGE
TOLL PLAZA RETAINING WALL

SOIL NAILED RETAINING WALL NO.1

SOIL NAILED RETAINING WALL NO. 2

SECTION 10. CONSTRUCTION DETAILS

SECTION 10-1. GENERAL

10-1.01 CONSTRUCTION PROJECT INFORMATION SIGNS

Before any major physical construction work readily visible to highway users is started on this contract, the Contractor shall furnish and erect 2 Type 2 Construction Project Information signs at the locations designated by the Engineer.

The signs and overlays shall be of a type and material consistent with the estimated time of completion of the project and shall conform to the details shown on the plans.

The sign letters, border and Caltrans construction logos shall conform to the colors (non-reflective) and details shown on the plans, and shall be on a white background (non-reflective). The colors blue and orange shall conform to PR Color Number 3 and Number 6, respectively, as specified in the Federal Highway Administration's Color Tolerance Chart.

The sign message to be used for fund types shall consist of the following, in the order shown:

ALAMEDA COUNTY TRANSPORTATION FUNDS

The sign message to be used for type of work shall consist of the following:

HIGHWAY CONSTRUCTION

The sign message to be used for the Year of Completion of Project Construction will be furnished by the Engineer. The Contractor shall furnish and install the "Year" sign overlay within 10 working days of notification of the year date to be used.

The letter sizes to be used shall be as shown on the plans. The information shown on the signs shall be limited to that shown on the plans.

The signs shall be kept clean and in good repair by the Contractor.

Upon completion of the work, the signs shall be removed and disposed of outside the highway right of way in accordance with the provisions in Section 7-1.13 of the Standard Specifications.

Full compensation for furnishing, erecting, maintaining, and removing and disposing of the construction project information signs shall be considered as included in the contract lump sum price paid for construction area signs and no additional compensation will be allowed therefor.

10-1.02 ORDER OF WORK

Order of work shall conform to the provisions in Section 5-1.05, "Order of Work," of the Standard Specifications and these special provisions.

Installing sheet pile wall and all associated work at Toll Plaza Retaining Wall shall be first order of work.

Furnishing pre-construction shotcrete test panels shall be first order of work.

Construction of the HUB building and all associated electrical conduits and pullboxes entering the HUB building shall be a first order of work.

Attention is directed to "Construction Area Signs", "Remove Sign Structures", "Remove Roadside Signs", "Relocate Sign Structures", and "Relocate Roadside Signs" elsewhere in these special provisions. The construction area signs shown on the staging plans shall be in place and ready for use prior to the removal or relocation of corresponding permanent sign structures or roadside signs.

Prior to commencement of the traffic signal functional test at any location, all items of work related to signal control shall be completed and roadside signs and all pavement delineation and pavement markings shall be in place at that location.

The installation work by Pacific Bell will not be performed until all conduits, pull boxes and manholes related to the telephone system have been completed and accepted for use. Approximately 100 working days will be needed for installation done by Pacific Bell.

No overhead sign panel shall be installed until the overhead sign lighting is completely operational.

No above ground electrical work shall be performed on any system within the project site until all Contractor-furnished electrical materials for that individual system have been tested and delivered to the Contractor.

Arrangements have not been made to access any portion of the work from outside the State right-of-of way. The Contractor shall make all arrangements for access to the work from outside of the State right-of-way. All costs to access the work from outside of the State right-of-way shall be at the Contractor's expense.

Attention is directed to "Maintaining Traffic" and "Temporary Pavement Delineation" of these special provisions and to the stage construction sheets of the plans.

Attention is directed to "Progress Schedule (Critical Path)" of these special provisions regarding the submittal of a general time-scaled logic diagram within 10 days after approval of the contract. The diagram shall be submitted prior to performing any work that may be affected by any proposed deviations to the construction staging of the project.

The work shall be performed in conformance with the stages of construction shown on the plans. Nonconflicting work in subsequent stages may proceed concurrently with work in preceding stages, provided satisfactory progress is maintained in the preceding stages of construction.

In each stage, after completion of the preceding stage, the first order of work shall be the removal of existing pavement delineation as directed by the Engineer. Pavement delineation removal shall be coordinated with new delineation so that lane lines are provided at all times on traveled ways open to public traffic.

Before obliterating any pavement delineation that is to be replaced on the same alignment and location, as determined by the Engineer, the pavement delineation shall be referenced by the Contractor, with a sufficient number of control points to reestablish the alignment and location of the new pavement delineation. The references shall also include the limits or changes in striping pattern, including one- and 2-way barrier lines, limit lines, crosswalks and other pavement markings. Full compensation for referencing pavement delineation shall be considered as included in the contract prices paid for new pavement delineation and no additional compensation will be allowed therefor.

Construction of the new structural section adjacent to the existing traveled way shall be performed in successive and, once all operations are under way, concurrent operations of excavating, preparing subgrade, placing base materials and paving. Excavation within 2.4 meters of the existing traveled way shall not precede the paving operation by more than 3 working days unless:

1. approved in writing by the Engineer and;
2. material is placed and compacted against the vertical cuts within 2.4 meters of the existing traveled way. During excavation operations, native material may be used for this purpose, however, once the placing of the structural section commences, structural material shall be used. The material shall be placed to the level of the elevation of the top of existing pavement and tapered at a slope of 1:4 (vertical:horizontal) or flatter to the bottom of the excavation. Full compensation for placing the material on a 1:4 slope, regardless of the number of times it is required, and subsequent removing or reshaping of the material to the lines and grades shown on the plans shall be considered as included in the contract price paid for the materials involved and no additional compensation will be allowed therefor. No payment will be made for material placed in excess of that required for the structural section.

At locations exposed to public traffic where guard railings or barriers are to be constructed, reconstructed, or removed and replaced, the Contractor shall schedule the operations so that at the end of each working day there shall be no post holes open nor shall there be any railing or barrier posts installed without the blocks and rail elements assembled and mounted thereon.

Existing conduits to be extended shall be located as specified under "Extend Irrigation Crossovers" elsewhere in these special provisions prior to the start of other work in these areas.

10-1.03 ENVIRONMENTALLY SENSITIVE AREA (GENERAL)

The Contractors attention is directed to the designated Environmentally Sensitive Area (ESA), shown on the plans. The exact location of the boundaries of environmentally sensitive area shall be established by the Engineer.

Within the boundaries of an ESA, no project related activities shall take place. This specifically prohibits vehicle access, storage or transport of any materials, including hydrocarbon and lead contaminated material, or any other project related activities.

The ESA boundary shall be clearly marked by the installation of Temporary High Visibility Boundary Fence (THVBF), as specified elsewhere in these special provisions. The boundaries shall be checked periodically and the markings repaired or replaced as necessary during construction. Full compensation for checking, repairing or replacing the ESA marking shall be considered as included in the contract price paid per meter for Temporary High Visibility Boundary Fence and no separate payment will be made.

10-1.04 WATER POLLUTION CONTROL

Water pollution control work shall conform to the requirements in Section 7-1.01G, "Water Pollution," of the Standard Specifications and these special provisions.

This project shall conform to the requirements of Permit No.CAS000002 and Permit No. CAS000003 issued by the State Regional Water Resources Control Board. These permits, hereafter referred to as the "Permit," regulates storm water discharges associated with construction activities.

Water pollution control work shall conform to the requirements in the Construction Contractor's Guide and Specifications of the Caltrans Storm Water Quality Handbooks, dated April 1997, and addenda thereto issued up to and including the date of advertisement of the project, hereafter referred to as the "Handbook". Copies of the Handbook may be obtained from the Department of Transportation, Material Operations Branch, Publication Distribution Unit, 1900 Royal Oaks Drive, Sacramento, California 95815, Telephone: (916) 445-3520. In addition, a Conceptual Storm Water Pollution Prevention Plan, hereafter referred to as the "CSWPPP," has been prepared for this project by the Department. The CSWPPP shall be used as a reference for determining and preparing the minimum work required under the Permit and this Special Provision.

Copies of the Handbook, CSWPPP, and the Permit are also available for review at 111 Grand Avenue Oakland, California 94601. Please call the Toll Bridge Duty Senior, Telephone No. (510) 286-5549, to reserve a copy of the documents at least 24 hours in advance..

The Contractor shall become fully informed of and comply with the applicable provisions of the Handbook, Permit and Federal, State and local regulations that govern the Contractor's operations and storm water discharges from both the project site and areas of disturbance outside the project limits during construction. The Contractor shall maintain a copy of the Permit at the project site and shall make the Permit available during construction activities.

Unless arrangements for disturbance of areas outside the project limits are made by the Department and made part of the contract, it is expressly agreed that the Department assumes no responsibility to the Contractor or property owner whatsoever with respect to any arrangements made between the Contractor and property owner to allow disturbance of areas outside the project limits.

The Contractor shall be responsible for the costs and for any liability imposed by law as a result of the Contractor's failure to comply with the requirements set forth in this section "Water Pollution Control", including but not limited to, compliance with the applicable provisions of the Handbook, Permit and Federal, State and local regulations. For the purposes of this paragraph, costs and liabilities include, but are not limited to, fines, penalties and damages whether assessed against the State or the Contractor, including those levied under the Federal Clean Water Act and the State Porter Cologne Water Quality Act.

In addition to any remedy authorized by law, the money due the Contractor under the contract may be retained by the State of California until full compliance with the contract documents, the Permit and disposition has been made of the costs and liabilities.

The retention of money due the Contractor shall be subject to the following:

1. The Department will give the Contractor 30 days notice of its intention to retain funds from any partial payment which may become due to the Contractor prior to acceptance of the contract. Retention of funds from any payment made after acceptance of the contract may be made without prior notice to the Contractor.
2. No retention of additional amounts out of partial payments will be made if the amount to be retained does not exceed the amount being withheld from partial payments pursuant to Section 9-1.06, "Partial Payments," of the Standard Specifications.
3. If the Department has retained funds and it is subsequently determined that the State is not subject to the costs and liabilities in connection with the matter for which the retention was made, the Department shall be liable for interest on the amount retained at the legal rate of interest for the period of the retention.

Conformance with the requirements of this section "Water Pollution Control" shall not relieve the Contractor from the Contractor's responsibilities, as provided in Sections 7-1.11, "Preservation of Property," 7-1.121, "Indemnification," and 7-1.122, "Insurance," of the Standard Specifications.

The Contractor shall, at reasonable times, allow authorized agents of the California Regional Water Quality Control Board, State Water Resources Control Board, U. S. Environmental Protection Agency and local storm water management agency, upon the presentation of credentials and other documents as may be required by law, to:

1. Enter upon the construction site and the Contractor's facilities pertinent to the work;
2. Have access to and copy any records that must be kept as specified in the Permit;
3. Inspect the construction site and related soil stabilization practices and sediment control measures; and
4. Sample or monitor for the purpose of ensuring compliance with the Permit.

The Contractor shall notify the Engineer immediately upon request from regulatory agencies to enter, inspect, sample, monitor or otherwise access the project site or the Contractor's records.

STORM WATER POLLUTION PREVENTION PLAN PREPARATION, APPROVAL AND UPDATES

As part of the water pollution control work, a Storm Water Pollution Prevention Plan, hereafter referred to as the "SWPPP," is required for this contract. The SWPPP shall conform to the requirements in Section 7-1.01G, "Water Pollution," of the Standard Specifications, the requirements in the Handbook, the requirements of the Permit and these special provisions. Upon the Engineer's approval of the SWPPP, the SWPPP shall be deemed to fulfill the requirements of Section 7-1.01G, "Water Pollution," of the Standard Specifications for development and submittal of a Water Pollution Control Program.

No work having potential to cause water pollution, as determined by the Engineer, shall be performed until the SWPPP has been approved by the Engineer.

Within 20 days after the approval of the contract, the Contractor shall submit 3 copies of the SWPPP to the Engineer. The Contractor shall allow 10 days for the Engineer to review the SWPPP. If revisions are required, as determined by the Engineer, the Contractor shall revise and resubmit the SWPPP within 5 days of receipt of the Engineer's comments and shall allow 5 days for the Engineer to review the revisions. Upon the Engineer's approval of the SWPPP, 3 additional copies of the SWPPP, incorporating the required changes, shall be submitted to the Engineer. In order to allow construction activities to proceed, the Engineer may conditionally approve the SWPPP while minor revisions are being completed.

The objectives of the SWPPP shall be to identify pollution sources that may adversely affect the quality of storm water discharges associated with the project and to identify, construct, implement and maintain water pollution control measures, hereafter referred to as control measures, to reduce to the extent feasible pollutants in storm water discharges from the construction site both during and after construction is completed under this contract.

The SWPPP shall incorporate control measures in the following categories:

1. Soil stabilization practices;
2. Sediment control practices;
3. Sediment tracking control practices;
4. Wind erosion control practices; and
5. Non-storm water management and waste management and disposal control practices.

Specific objectives and minimum requirements for each category of control measures are contained in the Handbook.

The Contractor shall consider the objectives and minimum requirements presented in the Handbook for each of the above categories. The special minimum requirements listed below supersede the minimum requirements listed in the Handbook for the same category. When minimum requirements are listed for any category, the Contractor shall incorporate into the SWPPP, and implement on the project, one or more of the listed minimum controls required in order to meet the pollution control objectives for the category. In addition, the Contractor shall consider other control measures presented in the Handbook and shall incorporate into the SWPPP and implement on the project the control measures necessary to meet the objectives of the SWPPP. The Contractor shall document the selection process in accordance with the procedure specified in the Handbook. The following special minimum requirements are established:

Category:	Minimum Requirements:
Non-Storm Water and Waste Management Controls	CD7(2) Dewatering, CD8(2) Paving Operations, CD10(2) Material Delivery and Storage, CD11(2) Material Use, CD12(2) Spill Prevention and Control, CD13(2) Solid Waste Management, CD14(2) Hazardous Waste Management, CD15(2) Contaminated Soil Management, CD16(2) Concrete Waste Management, CD17(2) Sanitary/Septic Waste Management, CD18(2) Vehicle and Equipment Cleaning, CD19(2) Vehicle and Equipment Fueling, CD20(2) Vehicle and Equipment Maintenance, CD22(2) Scheduling, CD44(2) Illicit Discharge/Illegal Dumping Reporting
Erosion & Sediment Source Controls	CD23(2) Preservation of Existing Vegetation, CD22(2) Scheduling, CD25(2) Mulching,
Wind Erosion Controls	CD22(2) Scheduling, CD26B(2) Geotextiles, Mats/Plastic Covers & Erosion Control Blankets,
Sediment Treatment Controls	CD22(2) Scheduling, CD29A(2) Stabilized Construction Entrance, CD34(2) Check Dams, CD37(2) Straw Bale Barriers, CD39(2) Brush or Rock Filter, and CD41(2) Sediment Traps

The following contract items of work, as shown on the project plans, shall be incorporated into the SWPPP as critical temporary control measures: Temporary High Visibility Boundary Fence, Temporary Drainage Inlet Protection, Temporary Drainage Outlet Protection (Type EC1), Temporary Construction Road, Temporary Concrete Washout, Temporary Cover, and Temporary Silt Fence. The Contractor shall consider other control measures to supplement the critical temporary control measures when necessary to meet the pollution control objectives of the SWPPP.

The following contract items of work, as shown on the project plans, shall be incorporated into the SWPPP as permanent post-construction control measures: Erosion Control (Type D), Erosion Control (Netting), and Fiber Rolls. These control measures shall be utilized as construction period control measures. Attention is directed to "Order of Work" of these special provisions. The Contractor shall consider other control measures to supplement these permanent, post-construction control measures when necessary to meet the pollution control objectives of the SWPPP. The Contractor shall maintain and protect the permanent control measures throughout the duration of the project and shall restore these controls to the lines and grades shown on the plans prior to acceptance of the project.

The SWPPP shall include, but not be limited to, the following items as described in the Handbook and Permit:

1. Source Identification;
2. Erosion and Sediment Controls;
3. Non-Storm Water Management;
4. Waste Management and Disposal;
5. Maintenance, Inspection and Repair;
6. Training;
7. List of Contractors and Subcontractors;
8. Post-Construction Storm Water Management;
9. Preparer;
10. Copy of the Permits;
11. BMP Consideration Checklist;
12. SWPPP Checklist;
13. Schedule of Values; and
14. Water Pollution Control Drawings.

The Contractor shall amend the SWPPP, graphically and in narrative form, whenever there is a change in construction activities or operations which may affect the discharge of significant quantities of pollutants to surface waters, ground waters, municipal storm drain systems, or when deemed necessary by the Engineer. The SWPPP shall also be amended if it is in violation of any condition of the Permit, or has not effectively achieved the objective of reducing pollutants in storm water discharges. Amendments shall show additional control measures or revised operations, including those in areas not shown in the initially approved SWPPP, which are required on the project to control water pollution effectively. Amendments to the SWPPP shall be submitted for review and approval by the Engineer in the same manner specified for the initially approved SWPPP. Approved amendments shall be dated and logged in the SWPPP. Upon approval of the amendment, the Contractor shall implement the additional control measures or revised operations.

The Contractor shall keep a copy of the SWPPP and approved amendments at the project site. The SWPPP shall be made available upon request of a representative of the Regional Water Quality Control Board, State Water Resources Control Board, U.S. Environmental Protection Agency or local storm water management agency. Requests by the public shall be directed to the Engineer.

By June 15 of each year, the Contractor shall submit an annual certification to the Engineer stating compliance with the requirements governing the Permit. Throughout the duration of the contract, if the project is determined to be in non-compliance at any time by the Engineer, the Contractor shall submit a written report to the Engineer within 48 hours of identification of non-compliance. The report shall specify the time and nature of the non-compliance and include a course of action to correct the deficiency. Non-compliance shall include, but not be limited to, any of the following conditions:

1. Any written notices, violations, or orders presented by a regulatory agency.
2. Control measures in the field are not implemented or maintained as provided for in the approved SWPPP or amendments.
3. The Contractor fails to respond to written requests for corrective actions.
4. The Contractor refuses to submit an annual certification.
5. Contractor discharges unauthorized materials, residues, or liquids to drainage conveyances or water bodies.

If the project is in non-compliance, the Engineer may impose one or more of the following actions:

1. Directing the Contractor to revise the operations and SWPPP.
2. No further work on any contract item out of compliance until the water pollution control measures are adequate and the SWPPP is amended and acceptable to the Engineer.

3. No further work or the beginning of any new work on any contract items until an inspection is conducted of the work area to determine the adequacy of measures deployed to address all other work.
4. Notification to the Regional Water Quality Control Board (RWQCB) of the non-compliance status.

SCHEDULE OF VALUES

The Contractor shall submit with the SWPPP, for approval by the Engineer, a schedule of values detailing the cost breakdown of the contract lump sum item for water pollution control. The cost breakdown shall include both the special minimum requirements required by the Department and those selected by the Contractor for this project. The combined requirements shall be considered as items of work as part of the lump sum bid. The schedule of values shall reflect the total items of work including both those required by the Department and those selected by the Contractor. The Contractor shall include quantities, values, and amounts for all control measures shown in the schedule of values, except for critical temporary controls and permanent control measures which are shown on the project plans and for which there is a contract item of work. Adjustments in the items of work and quantities listed in the schedule of values shall be made when required to address approved amendments to the SWPPP.

The sum of the amounts for the units of work listed in the schedule of values shall be equal to the contract lump sum price for water pollution control.

If approved in writing by the Engineer, the schedule of values will be used to determine progress payments for water pollution control during the progress of the work, and as the basis for calculating any adjustment in compensation for the contract item for water pollution control due to changes in the work ordered by the Engineer.

SWPPP IMPLEMENTATION

Upon approval of the SWPPP, the Contractor shall be responsible throughout the duration of the project for installing, constructing, inspecting and maintaining the control measures included in the SWPPP and any amendments thereto and for removing and disposing of temporary control measures. Unless otherwise directed by the Engineer or specified in these special provisions, the Contractor's responsibility for SWPPP implementation shall continue throughout any temporary suspension of work ordered in accordance with Section 8-1.05, "Temporary Suspension of Work," of the Standard Specifications. Requirements for installation, construction, inspection, maintenance, removal and disposal of control measures are specified in the Handbook and these special provisions.

Soil stabilization practices and sediment control measures, including minimum requirements, shall be provided throughout the winter season, defined as between October 1 and May 1.

Implementation of soil stabilization practices and sediment control measures for soil-disturbed areas of the project site shall be completed, except as provided for below, no later than 20 days prior to the beginning of the winter season or upon start of applicable construction activities for projects which begin either during or within 20 days of the winter season.

Throughout the winter season, the active, soil-disturbed area of the project site shall be no more than 2 hectares. The Engineer may approve, on a case-by-case basis, expansions of the active, soil-disturbed area limit. The Contractor shall demonstrate the ability and preparedness to fully deploy soil stabilization practices and sediment control measures to protect soil-disturbed areas of the project site before the onset of precipitation. The Contractor shall maintain a quantity of soil stabilization and sediment control materials on site equal to 125 percent of that sufficient to protect unprotected, soil-disturbed areas on the project site and shall maintain a detailed plan for the mobilization of sufficient labor and equipment to fully deploy control measures required to protect unprotected, soil-disturbed areas on the project site prior to the onset of precipitation. The Contractor shall include a current inventory of control measure materials and the detailed mobilization plan as part of the SWPPP.

Throughout the winter season, soil-disturbed areas of the project site shall be considered to be nonactive whenever soil disturbing activities are expected to be discontinued for a period of 10 or more days and the areas are fully protected. Areas that will become nonactive either during the winter season or within 20 days thereof shall be fully protected with soil stabilization practices and sediment control measures within 10 days of the discontinuance of soil disturbing activities or prior to the onset of precipitation, whichever is first to occur.

Throughout the winter season, active soil-disturbed areas of the project site shall be fully protected at the end of each day with soil stabilization practices and sediment control measures unless fair weather is predicted through the following work day. The weather forecast shall be monitored by the Contractor on a daily basis. The National Weather Service forecast shall be used, or an alternative weather forecast proposed by the Contractor may be used if approved by the Engineer. If precipitation is predicted prior to the end of the following work day, construction scheduling shall be modified, as required, and the Contractor shall deploy functioning control measures prior to the onset of the precipitation.

The Contractor shall implement, year-round and throughout the duration of the project, control measures included in the SWPPP for sediment tracking, wind erosion, non-storm water management and waste management and disposal.

The Engineer may order the suspension of construction operations which create water pollution if the Contractor fails to conform to the requirements of this section "Water Pollution Control" as determined by the Engineer.

MAINTENANCE

To ensure the proper implementation and functioning of control measures, the Contractor shall regularly inspect and maintain the construction site for the control measures identified in the SWPPP. The Contractor shall identify corrective actions and time frames to address any damaged measures or reinitiate any measures that have been discontinued.

The construction site inspection checklist provided in the CSWPPP shall be used to ensure that the necessary measures are being properly implemented, and to ensure that the control measures are functioning adequately. The Contractor shall submit one copy of each site inspection record to the Engineer.

Inspections of the construction site shall be conducted by the Contractor to identify deficient measures, as follows:

1. Prior to a forecast storm;
2. After each storm event;
3. At 24 hour intervals during extended precipitation events; and
4. Routinely, on a weekly basis..

If the Contractor or the Engineer identifies a deficiency in the deployment or functioning of an identified control measure, the deficiency shall be corrected by the Contractor immediately, or by a later date and time if requested by the Contractor and approved by the Engineer in writing, but not later than the onset of subsequent precipitation events. The correction of deficiencies shall be at no additional cost to the State. When any deficiency is not corrected within the timeframe prescribed by the Engineer, then the project shall be in non-compliance.

PAYMENT

The contract lump sum price paid for prepare storm water pollution prevention plan shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals for doing all the work involved in developing, preparing, obtaining approval of, revising and amending the SWPPP as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Attention is directed to Sections 9-1.06, "Partial Payments," and 9-1.07, "Payment After Acceptance," of the Standard Specifications. Payments for prepare storm water pollution prevention plan will be made as follows:

1. After the SWPPP has been approved by the Engineer, 75 percent of the contract item price for prepare storm water pollution prevention plan will be included in the monthly partial payment estimate; and
2. After acceptance of the contract pursuant to Section 7-1.17, "Acceptance of Contract," the remaining 25 percent of the contract item price for prepare storm water pollution prevention plan will be made in accordance with Section 9-1.07.

The contract lump sum price paid for water pollution control shall include full compensation for furnishing all labor, materials, tools, equipment and incidentals, and for doing all the work involved in installing, constructing, maintaining, removing and disposing of control measures, except those shown on the project plans and for which there is a contract item of work, and excluding developing, preparing, obtaining approval of, revising and amending the SWPPP, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Full compensation for changes in control measures required by an approved amendment to the SWPPP shall be considered as included in the lump sum bid for the various items of work and no additional compensation will be allowed thereafter except changes to control measures required as a result of extra work will be compensated in accordance with Section 4-1.03D of the standard specifications and the following:

If the control measure is listed in the approved SWPPP schedule of values, an adjustment in compensation for the contract item for water pollution control will be made by applying the increase or decrease in quantities to the approved schedule of values. No adjustment of compensation will be made to the unit price listed for any item in the schedule of values due to any increase or decrease in the quantities, regardless of the reason for the increase or decrease. The provisions in Section 4-1.03B, "Increased or Decreased Quantities," shall not apply to items listed in the schedule of values.

If the control measure is not listed in the approved SWPPP schedule of values, payment will be made by force account.

Those control measures which are shown on the project plans and for which there is a contract item of work will be measured and paid for as that item of work.

The Engineer will retain an amount equal to 25 percent of the estimated value of the contract work performed during estimate periods in which the Contractor fails to conform to the requirements of this section "Water Pollution Control" or if the project is in non-compliance as determined by the Engineer.

Retentions for failure to conform to the requirements of this section "Water Pollution Control" shall be in addition to the other retentions provided for in the contract. The amounts retained for failure of the Contractor to conform to the requirements of this section will be released for payment on the next monthly estimate for partial payment following the date that an approved SWPPP has been implemented and maintained, and water pollution is adequately controlled, as determined by the Engineer.

10-1.05 NON-STORM WATER DISCHARGES

Non-storm water discharges shall conform to the requirements in Section 7-1.01G, “Water Pollution” of the Standard Specifications and these special provisions

Conformance with the requirements of this section shall in no way relieve the Contractor from the Contractor's responsibilities, as provided in Section 7-1.11, "Preservation of Property," and Section 7-1.12, "Responsibility for Damage," of the Standard Specifications.

Dewater.--Suspended solids shall be removed during the dewatering operation of any excavation, as specified in these special provisions.

Suspended solids shall be removed to the extent that visible, floating products are not apparent within the discharge. Also, the discharge shall be of a purity such that turbidity and color beyond present natural background levels are not apparent within the receiving water body. The turbidity, measured in Nephelometric Turbidity Units (NTU), of the discharge shall be less than 20 percent of the background turbidity of the receiving water body. The point of effluent discharge shall not cause bottom sediments, aquatic vegetation, or surface soils to become dislodged or disturbed.

The Contractor shall describe the dewatering apparatus within the appropriate sections of the SWPPP. The description shall include, but not be limited to, an estimate of the discharge volume, flow rate, and frequency; location of discharge; and the inspection and monitoring procedures related to the discharge.

The Contractor shall conduct a daily inspection of the dewatering equipment, when in use, to ensure that all components are functional and routinely maintained to prevent leakage prior to removal of suspended solids. Any component of the apparatus that is found to be damaged or to affect the performance of the apparatus shall be either immediately repaired or replaced.

The Contractor shall monitor both the discharge and the affected area of the receiving water body. The observations made during monitoring shall include the color, size of affected area, presence of suspended material, presence of water fowl or aquatic wildlife, wind direction and velocity, tidal condition, atmospheric condition, time, and date. In addition, the Contractor shall supplement the observations with photographs. During monitoring events, the Contractor shall obtain NTU measurements for the discharge turbidity and the receiving water (background) turbidity. The contractor shall conduct monitoring, at a minimum, one hour prior to discharge, during the first ten minutes of initiating discharge, every four hours during discharge, and upon cessation of discharge. Monitoring data shall be recorded daily in a tabular format known as the monitoring report to be provided by the Engineer. Completed monitoring reports, including photographs, shall be provided weekly to the Engineer, or as directed by the Engineer.

Observations or measurements which indicate that the discharge is of a visible purity such that turbidity and apparent color are beyond the present natural background levels or the requirements of these special provisions shall be immediately reported to the Engineer. The discharge activity shall immediately cease, so that corrective actions are undertaken to repair, modify, or replace the equipment. The commencement of discharge activities shall be allowed upon approval by the Engineer.

Spill Contingency.--The Contractor shall prepare and submit to the Engineer a contingency plan for the management of spills or leaks of any materials or wastes that may impact the water quality of the San Francisco Bay.

The spill contingency plan shall be incorporated within the Storm Water Pollution Prevention Plan (SWPPP), as specified in “Water Pollution Control” of these special provisions.

The contingency plan shall include instructions and procedures for reporting spills, and a list of spill containment and collection materials and equipment to be maintained onsite. The contingency plan shall be reviewed and updated at least every 90 calendar days.

Liquids, Residues, and Debris.--The control and disposal of liquids, residues, and debris associated with all activities shall be described within the SWPPP, as specified in “Water Pollution Control” of these special provisions. The SWPPP shall, at a minimum, depict and describe the procedural and structural methods of detaining, collecting, and disposing of all slurries, liquids, residues, and debris associated with the operations. Sufficient redundancy shall be incorporated into the procedural and structural methods such that the liquids, residues, and debris are not conveyed into or become present in drainage systems, San Francisco Bay, or other water bodies.

Measurement and Payment.--Full compensation for conforming to the requirements of this section shall be considered as included in the prices paid for the various contract items of work affected by this section and no additional compensation will be allowed therefor.

10-1.06 TEMPORARY SILT FENCE

Temporary silt fence shall conform to the details shown on the plans and these special provisions.

Temporary silt fence shall be furnished, installed, maintained and removed at the locations shown on the plans.

Preparation shall conform to the requirements in Section 20-3.02, "Preparation," of the Standard Specifications.

Attention is directed to "Water Pollution Control" elsewhere in these special provisions.

The Contractor shall use temporary silt fence as one of the various measures to prevent water pollution. The Storm Water Pollution Prevention Plan shall graphically show the use of temporary silt fence in relation to other water pollution control work specified elsewhere in these special provisions.

MATERIALS.—Materials shall conform to Section 20-2, "Materials," of the Standard Specifications and the following:

Temporary silt fence shall be a prefabricated silt fence with a minimum woven polypropylene fabric width of 900 mm and a minimum tensile strength of 0.44-kN, conforming to ASTM Designation: D 4632 and having an integral reinforcement layer. The reinforcement layer shall be a polypropylene or equivalent net provided by the manufacturer.

INSTALLATION.—Temporary silt fence shall be installed as shown on the plans and as follows:

When joints are necessary, the temporary silt fence shall overlap a minimum of 150 mm with both posts tied together.

Temporary silt fences shall be maintained to provide for adequate sediment holding capacity. Sediment deposits shall be removed when the sediment deposit reaches approximately one-third of the fence height. Removed sediment shall be deposited within the project in such a way that it is not subject to erosion by wind or water, or as directed by the Engineer.

When no longer required for the intended purpose, as determined by the Engineer, temporary silt fence shall be removed from the site of the work.

Holes, depressions or any other ground disturbance caused by the removal of the temporary silt fence shall be backfilled and repaired in accordance with the provisions in the second paragraph of Section 15-1.02, "Preservation of Property," of the Standard Specifications.

MEASUREMENT AND PAYMENT.—The quantity of temporary silt fence to be paid for will be determined by the meter from actual measurements, the measurements to be made parallel with the ground slope along the line of the completed temporary silt fence, deducting the widths of openings.

The contract price paid per meter for temporary silt fence shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in installing temporary silt fence, complete in place, including trench excavation and backfill, and maintenance and removal of temporary silt fence, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Temporary silt fence placed at location other than as shown on the project plans or directed by the Engineer, in accordance with the Contractor's Storm Water Pollution Prevention Plan, will not be measured and will be paid for as specified in "Water Pollution Control" elsewhere in these special provisions.

No adjustment of compensation will be made for any increase or decrease in the quantities of temporary silt fence required, regardless of the reason for the increase or decrease. The provisions in Section 4-1.03B, "Increased or Decreased Quantities," shall not apply to temporary silt fence.

10-1.07 TEMPORARY COVER

Temporary cover material shall be either plastic sheeting or fabric. If fabric is used, the material shall conform to the provisions in the Standard Specifications and these special provisions.

The Conceptual Storm Water Pollution Prevention Plan (CSWPPP), developed for this project, graphically shows the location of temporary cover for reference only. The quantity of temporary cover for this project is derived from the location of temporary cover shown in the CSWPPP.

The Contractor shall use temporary cover as one of the various measures to prevent water pollution. The Storm Water Pollution Prevention Plan shall graphically show the use of temporary cover in relation to other water pollution control work specified elsewhere in these special provisions.

If fabric is used, the fabric shall be a minimum 115-170 g slit film woven fabric made of monofilaments of polypropylene. The fabric shall be non biodegradable, resistant to sunlight deterioration, inert to most soil chemicals and furnished with sealed edges on all sides to prevent unraveling. The fabric shall also conform to the following:

Properties

Grab tensile strength (minimum)	90-137 kg
Elongation at break (minimum)	40%

If plastic sheeting is used, the sheeting shall be new and a minimum of 12 mil in thickness.

Fabric or plastic sheeting shall be placed over the critical slope with a 0.6 m overlap. A weight such as rock bags or sandbags shall be placed on the overlap area at a maximum spacing of 2.4 m. Edges shall be embedded a minimum of 150 mm in native soil.

Temporary cover damaged as a result of the Contractors operations shall be replaced by the Contractor at his expense.

MEASUREMENT AND PAYMENT.--The quantity of temporary cover to be paid for shall be determined by the square meter of sheeting cover installed. If the Contractor removes the sheeting cover in order to facilitate any other work, the cover shall be replaced and secured by the contractor at no additional cost to the State.

Temporary cover placed at locations other than as shown on the project plans or directed by the Engineer, in accordance with the Contractor's Storm Water Pollution Prevention Plan, will not be measured and will be paid for as specified in "Water Pollution Control" elsewhere in these special provisions.

No adjustment of compensation will be made for any increase or decrease in the quantities of temporary cover required, regardless of the reason for the increase or decrease. The provisions in Section 4-1.03B, "Increased or Decreased Quantities," shall not apply to temporary cover.

10-1.08 TEMPORARY DRAINAGE INLET PROTECTION

Temporary drainage inlet protection shall be installed, maintained and later removed as shown on the plans, as specified in these special provisions, and as directed by the Engineer.

The Conceptual Storm Water Pollution Prevention Plan (CSWPPP), developed for this project, graphically shows the location of temporary drainage inlet protection for reference only. The quantity of temporary drainage inlet protection for this project is derived from the location of temporary drainage inlet protection shown in the CSWPPP.

The Contractor shall select the appropriate drainage inlet protection commensurate to the field condition around the drainage inlet.

Throughout the duration of the Contract, the Contractor shall be required to provide protection commensurate with the existing condition of the drainage inlet. The percentage of completion of the drainage inlet varies subject to the staging and phasing of construction. It is recognized that the drainage inlet changes during the course of construction and the actual protection provided may require selecting the appropriate type or types of drainage inlet protection as it changes during the course of construction. The Contractor shall use temporary drainage inlet protection as one of the various measures to prevent water pollution. The Storm Water Pollution Prevention Plan shall graphically show the use of temporary drainage inlet protection in relation to other water pollution control work specified elsewhere in these special provisions.

MATERIALS.--Materials shall conform to the provisions in Section 20-2, "Materials," of the Standard Specifications and these special provisions.

SILT FENCE.—Sedimentation control fabric for temporary silt fence shall be a prefabricated silt fence with a minimum woven polypropylene fabric width of 900 mm and a minimum tensile strength of 0.44-kN, conforming to ASTM Designation: D 4632.

SEDIMENT BAG.—Sedimentation control fabric for sediment bags shall be a prefabricated sedimentation control fabric envelop with a woven polypropylene fabric and sewn with a double stitched seam using nylon thread. The fabric shall have a grab tensile strength of at least 120 kg and grab elongation of 20 percent, per ASTM Designation: D4632. The fabric shall have a mullen burst strength of at least 2895 kPa, per ASTM Designation: D3786 and an ultraviolet (UV) stability exceeding 90 percent. The sedimentation control fabric shall be capable of a flow rate of 70.3 L/minute/m^2, per ASTM Designation: D4491.

The sediment bag shall be sized to fit the catch basin or drop inlet and be complete with lifting loops and dump straps attached at the bottom to facilitate emptying of the sediment bag. The sediment bags shall have a restraint cord approximately halfway up the bag to keep the sides away from the catch basin walls.

TEMPORARY FLEXIBLE DIKE.—Temporary flexible dike fabric cover and skirt shall be a woven polypropylene fabric with a minimum tensile strength of 0.44-kN, conforming to ASTM Designation: D 4632. The prefabricated fabric shall be high visibility orange in color that is integral to the fabric; painting shall not be allowed. The fabric shall have an ultraviolet (UV) stability exceeding 70 percent.

Temporary flexible dike inner material shall be urethane foam and shall be shaped and dimensioned as shown on the plans.

Adhesive for temporary flexible dike shall be a solvent free rubber modified asphalt emulsion. The color of the emulsion is brown when wet and with a drying period up to 3 hours.

Anchoring nails or spikes for temporary flexible dike shall be a minimum of 25 mm in length and capable of penetrating concrete and asphalt surfaces.

EROSION CONTROL BLANKET.—Erosion control blanket shall consist of straw and coconut or wood excelsior mats secured in place with wire staples and shall conform to one of the following:

EXCELSIOR BLANKET.—Excelsior blanket material shall consist of machine produced mats of curled wood excelsior with 80 percent of the fiber 150 mm or longer. The erosion control blanket shall be of consistent thickness and the wood fiber shall be evenly distributed over the entire area of the blanket. The top surface of the blanket shall be covered with a photo-degradable extruded plastic mesh. The blanket shall be smolder resistant without the use of chemical additives and shall be non-toxic and non-injurious to plant and animal life. Erosion control blanket

shall be furnished in rolled strips, 1220 mm -2440 mm in width, and shall have an average mass of 0.5-kg/m^2, ± 10 percent, at the time of manufacture.

STRAW AND COCONUT BLANKET.—Straw and coconut blanket shall be machine produced mats of straw and coconut with a light weight photo-degradable netting on top. The straw and coconut shall adhered to the netting with biodegradable thread or glue strip. The straw and coconut erosion control blanket shall be of consistent thickness with the straw and coconut evenly distributed over the entire area of the blanket. Straw and coconut erosion control blanket shall be furnished in rolled strips with a minimum width of 1.8 meters, minimum length of 20 meters (± 1 meter) and a minimum weight of 0.27-kg/m^2.

STAPLES.—Staples for erosion control blankets shall be made of 11-gage minimum steel wire and shall be U-shaped with 150-mm legs and 25-mm crown or 200-mm legs and a 50-mm crown.

INSTALLATION.—Temporary flexible dikes consists of individual sections of dike installed in conjunction with one another adjacent to existing drainage inlets as shown on the plans. The spacing and angle of placement shall be in accordance with the table shown on the plans. Temporary flexible dikes shall be installed flush against the sides of concrete or asphalt curbs, dikes and pavement with the inner material and fabric cover cut smoothly and evenly to provide a tight flush joint.

Temporary flexible dikes installed as part of temporary drainage inlet protection shall be maintained to provide for adequate sediment holding capacity. Sediment deposits shall be removed when the deposit reaches one-half of the temporary flexible dike height. Removed sediment shall be deposited within the project in such a way that it is not subject to erosion by wind or water, or as directed by the Engineer.

Sediment bags shall be installed by removing the drainage inlet grate, placing the sediment bag in the opening, and replacing the grate to secure the sediment bag in place. Removal of the bag shall be facilitated by the use of 25 mm steel reinforcing bars placed through the lifting loops.

Sediment bags installed as part of temporary drainage inlet protection shall be emptied when the restraint cords are no longer visible. Emptying of the bag shall be facilitated by the use of 25 mm steel reinforcing bars placed through the dump loops. The sediment bag shall be emptied of material with a shovel and rinsed before replacement in the catch basin or drop inlet.

When no longer required for the purpose, as determined by the Engineer, temporary drainage inlet protection facilities shall be removed. Removed facilities shall become the property of the Contractor and shall be removed from the site of the work.

Temporary drainage inlet protection damaged due to storms or as a result of the Contractors operations shall be replaced by the Contractor at his expense.

MEASUREMENT AND PAYMENT.—The quantity of temporary drainage inlet protection to be paid for will be determined from actual count of each drainage inlet protected conforming to the details shown on the plans. The protection is measured one time only and no additional measurement is recognized, and no additional compensation made, if it changes during the course of construction.

The contract unit price paid per temporary drainage inlet protection shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in installing temporary drainage inlet protection, complete in place, including excavation and backfill, all modifications occuring during the course of construction, and maintenance and removal of temporary drainage inlet protection, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Temporary drainage inlet protection for protection at drainage inlets other than as shown on the project plans or directed by the Engineer, in accordance with the Contractor's Storm Water Pollution Prevention Plan, will not be measured and will be paid for as specified in "Water Pollution Control" elsewhere in these special provisions.

No adjustment of compensation will be made for any increase or decrease in the quantities of temporary drainage inlet protection required, regardless of the reason for the increase or decrease. The provisions in Section 4-1.03B, "Increased or Decreased Quantities," shall not apply to temporary drainage inlet protection.

10-1.09 TEMPORARY DRAINAGE OUTLET PROTECTION (TYPE EC1)

Temporary drainage outlet protection shall be installed and maintained as shown on the plans, as specified in these special provisions, and as directed by the Engineer.

MATERIALS.--Materials shall conform to the provisions in Section 20-2, "Materials," of the Standard Specifications and these special provisions.

FIBER ROLL .--- Fiber roll shall consist of a prefabricated wheat or rice straw in rolls with a minimum diameter of 0.4m. The rolls shall be bound with an ultraviolet (UV) degradable plastic netting and weigh no less than 1.9 kg per linear meter.

Stakes used to anchor fiber roll shall be fir or pine, and shall have a cross-sectional area of at least 360 mm^2 and a minimum length of 600 mm.

Stakes shall be installed 1.2 meters apart along the total length of the rolls and 125 mm from the end of each individual roll. Stakes shall be driven flush or a maximum of 50 mm above the roll.

MEASUREMENT AND PAYMENT.—The quantity of temporary drainage outlet protection (Type EC1) to be paid for will be measured by the meter from end to end along the centerline of the installed fiber roll of each temporary drainage outlet protected conforming to the details shown on the plans

The contract price paid per temporary drainage outlet protection (Type EC1) shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in installing temporary drainage outlet protection, complete in place, including furnishing and installing fiber roll ,stakes, and maintenance of temporary drainage outlet protection (Type EC1), as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.10 TEMPORARY CONCRETE WASHOUT

The Contractor shall use temporary concrete washouts as one of the various measures to prevent water pollution. The Storm Water Pollution Prevention Plan shall graphically show the use of temporary concrete washouts in relation to other water pollution control work specified elsewhere in these special provisions.

The Contractor shall have a concrete washout within 50 m of any concrete operation, unless otherwise directed by the Engineer. The location of any concrete washout shall not be allowed within 15 m of drainage inlets and 26 m of defined water bodies.

Concrete washouts shall be installed prior to beginning any placement operation of concrete. The washout shall be maintained so that the volume of waste within the device is less than 75% of the capacity. The washout shall be removed and disposed of at the completion of concrete operations, or as directed by the Engineer. The Contractor shall be responsible for all costs associated with replacing any displaced soil or vegetation.

At the Contractor's option, concrete washouts may be installed below grade according to the plans. When concrete washout are installed below grade, removal of the washout shall consist of removing all concrete wastes and disposing of the waste in accordance with Section 7-1.13 'Disposal of Material Outside of the Highway Right of Way' of the Standard Specifications. At the discretion of the Engineer, disposal of concrete wastes within the Right of Way may be allowed. Approval to dispose of concrete wastes within the Right of Way will only be allowed by written authority from the Engineer. Upon disposal of concrete wastes, all below grade pits shall be backfilled, graded and compacted to conform to the surrounding area. Native material excavated to construct the washout shall be used as backfill material. Additional fill material may be used if native material is insufficient to allow the level of the excavated area to conform with the surrounding grade after compaction, with written approval by the Engineer.

MEASUREMENT AND PAYMENT.--The quantity of concrete washout to be paid for shall be determined by each unit installed. If the Contractor removes the concrete washout in order to facilitate any other work the concrete washout shall be replaced by the contractor at no additional cost to the State.

The contract unit price paid for temporary concrete washout shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in temporary concrete washout, complete in place, including maintaining, removing and disposing of temporary concrete washout, including installing, maintaining, removing and disposing of concrete washout sign, as shown on the plans, as specified in Section 15-3.02, "Removal Methods," of the Standard Specifications and these special provisions, and as directed by the Engineer.

No adjustment of compensation will be made for any increase or decrease in the quantities of temporary concrete washouts required, regardless of the reason for the increase or decrease. The provisions in Section 4-1.03B, "Increased or Decreased Quantities," shall not apply to temporary concrete washout .

Concrete washouts installed below grade at the Contractor's option, will be measured and paid for as concrete washouts installed above grade at no additional cost to the State.

10-1.11 TEMPORARY HIGH VISIBILITY BOUNDARY FENCE

Temporary high visibility boundary fence shall be furnished and constructed, maintained, and later removed as shown on the plans, as specified in these special provisions, and as directed by the Engineer.

Attention is directed to "Remove Chain Link Fence" elsewhere in these special provisions.

Temporary high visibility boundary fence shall be placed immediately after removal of existing chain link fence to provide for continuous access control along the existing fence line. The Contractor is responsible for maintaining access-control for those areas in which the existing fencing has been removed, and the planned permanent fencing is not yet in place.

Temporary access-control fencing, when in use for the purpose of access control along the highway right of way line (existing fence line), shall be maintained by the Contractor on a daily basis. Damage to temporary high visibility boundary fence shall be repaired by the Contractor, at his expense, within twenty-four (24) hours from initial observation or report.

Access-control shall be maintained at all times. Failure to maintain access-control by the Contractor will result in the Engineer directing State forces or another Contractor to perform remedial access-control work. If performed, the cost for

State forces or another Contractor to perform remedial access-control work will be tabulated and deducted from other payments due the Contractor via an administrative deduction taken on the next available progress payment. Such a deduction shall be permanent.

Used materials may be installed providing the used materials are good, sound, and are suitable for the purpose intended, as determined by the Engineer.

Materials may be commercial quality providing the dimensions and sizes of the materials are equal to, or greater than, the dimensions and sizes shown on the plans or specified herein. The fabric shall also conform to the following:

Properties

Material:	Polypropylene
Color:	Orange
Opening Size:	50.8 mm x 50.8 mm
Ultraviolet Resistance:	Fully Stabilized
Minimum Fabric Width:	1.22 m

Posts shall be either metal or wood at the Contractor's option and shall be suitable for the purpose intended. Post spacing shall be adequate to completely support the fence in an upright position.

Galvanizing and painting of steel items will not be required.

Treating wood with wood preservatives will not be required.

Concrete footings for metal or wood posts will not be required.

Temporary high visibility boundary fences that are damaged from any cause during the progress of the work shall be repaired or replaced by the Contractor at the Contractor's expense.

When no longer required for the work as determined by the Engineer, temporary high visibility boundary fence shall be removed. Removed facilities shall become the property of the Contractor and shall be removed from the site of the work, except as otherwise provided in this section.

Removed temporary high visibility boundary fence materials that are not damaged may be reused in the permanent work providing such materials conform to all of the requirements specified for the permanent work and such materials are new when used for the temporary high visibility boundary fence.

Holes caused by the removal of temporary high visibility boundary fences shall be backfilled in accordance with the provisions in the second paragraph of Section 15-1.02, "Preservation of Property," of the Standard Specifications.

The contract price paid per meter for temporary high visibility boundary fence shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in temporary high visibility fence, complete in place, including installation, maintenance, removal and disposal of materials, and for conducting daily checks on the integrity of temporary high visibility boundary fence when used along the highway right of way line for temporary access-control purposes, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

No adjustment of compensation will be made for any increase or decrease in the quantities of temporary high visibility boundary fence required, regardless of the reason for the increase or decrease. The provisions in Section 4-1.03B, “Increased or Decreased Quantities,” shall not apply to temporary high visibility boundary fence.

10-1.12 TEMPORARY CONSTRUCTION ROAD ENTRANCE/EXITS

Temporary construction road entrance/exits and clean out sumps shall conform to the details shown on the plans and these special provisions. The minimum quantity of temporary construction road entrance/exits required for this project shall be 6.

Temporary construction road entrance/exits shall be furnished, installed, maintained and removed at the locations shown on the Contractor's Storm Water Pollution Prevention Plan (SWPPP), as approved by the Engineer.

The Contractor shall provide as many temporary construction road entrance/exits and clean out sumps as required for the duration of the contract. Attention is directed to "Water Pollution Control" elsewhere in these special provisions.

The Contractor shall use temporary construction road entrance/exit as one of the various measures to prevent water pollution. The Storm Water Pollution Prevention Plan shall graphically show the use of temporary construction road entrance/exit in relation to other water pollution control work specified elsewhere in these special provisions.

MATERIALS.—Materials shall conform the following:

Subgrade Enhancement Fabric.--Subgrade enhancement fabric shall be placed where shown on the plans and at locations designated by the Engineer in accordance with this special provision.

Subgrade enhancement fabric shall be manufactured from one or more of the following materials: polyester, nylon or polypropylene. Subgrade enhancement fabric shall be, at the option of the contractor, either a woven filament or nonwoven type fabric conforming to the following:

	Woven	Non-Woven
Weight, grams per Square Meter, Min. ASTM Designation: D3776	205	205
Grab Tensile Strength, Newtons (N), Min. ASTM Designation: D4632	890	800
Tensile Strength at 10% Elongation, N, Min. ASTM Designation: D4632	490	--
Elongation at Break, Percent, Max. ASTM Designation: D4632	35 Max.	50 Min.

Subgrade enhancement fabric shall be furnished in an appropriate protective cover which shall protect it from ultraviolet radiation and from abrasion due to shipping and handling, and shall remain in said cover until installation. Subgrade enhancement fabric shall be accompanied by a Certificate of Compliance conforming to the provision in Section 6-1.07, "Certificate of Compliance" of the Standard Specifications.

Aggregate.--Aggregate shall be uniformly graded angular rock or cobble ranging in size from 76.2mm-180mm. Rock shall be clean and free of organic matter and shall conform to the provisions in Section 26, "Aggregate Base, " of the Standard Specifications and these special provisions.

INSTALLATION.—Temporary construction road entrance/exit and clean out sump shall be installed as shown on the plans and as follows:

Prior to placing the Subgrade enhancement, the areas shall be cleared of all trash and debris. Weeds shall be removed to the ground level. Cleared trash, debris, and removed weeds shall be disposed of outside the highway right of way in accordance with the provisions in Section 7-1.13 of the Standard Specifications.

Subgrade enhancement fabric shall be handled and placed in accordance with the manufacturer's recommendation and shall be positioned longitudinally along the alignment, pulled taut to form a tight wrinkle-free mat. The Subgrade to receive the fabric, immediately prior to placing, shall conform to the compaction and elevation tolerance specified in Section 25-1.03, "Subgrade", of the Standard Specifications and these special provisions and shall be free of loose or extraneous material and sharp objects that may damage the fabric during the installation

Adjacent borders of the fabric shall be overlapped a minimum of 450 mm.

The amount of Subgrade enhancement fabric placed shall be limited to that which can be covered with aggregate material within 72 hours.

Aggregate material to be placed directly over the Subgrade enhancement fabric shall be spread in the direction of fabric overlaps. Stockpiling of materials directly on the Subgrade enhancement fabric is not allowed. Once a sufficient working platform has been constructed, all remaining materials shall be uniformly placed and spread with 1:4 (V:H) tapers at the perimeter edges of the temporary construction road entrance/exit where it conforms to existing roadway and in accordance with the applicable sections of the special provisions and the Standard Specifications.

During spreading of the aggregate material, vehicles or equipment shall not be driven directly on the fabric. A sufficient thickness of material shall be maintained between the fabric and the equipment to prevent damage to the fabric. Damage to the fabric resulting from the Contractor's vehicles, equipment, or operations shall be repaired at the Contractor's expense.

Should the fabric be damaged during placing, the damaged section shall be repaired by placing a new piece of fabric over the damaged area. Said piece of fabric shall be large enough to cover the damaged area and provide a minimum 900 mm overlay on all edges.

A clean out sump shall be installed as shown on the plans and located within 6m of the temporary construction road entrance/exit facility. The sump shall be sized sufficiently to hold soil removed from the surfacing of the temporary construction road entrance /exit facility in order to maintain efficiency.

MAINTENANCE.-- The Contractor shall maintain the temporary construction road entrance/exits and clean out sumps throughout the contract period. The Contractor shall prevent displacement or migration of the aggregate surfacing or metal track panels. Any significant depressions, as determined by the Engineer, which form due to settling or heavy traffic shall be repaired by the Contractor.

Sediment and soil built up in the temporary construction road entrance/exits surface which compromises the efficiency for sediment and soil removing capability shall be removed as necessary or as determined by the Engineer. Removal and disposal of sediment and soils from the temporary construction road entrance/exits and the clean out sumps shall be the responsibility of the Contractor.

Once the temporary construction road entrance/exits and clean out sumps are no longer needed, the aggregate, Subgrade enhancement fabric and any soil and sediments shall be removed and disposed of as provided for in Section 7-1.13 'Disposal

of Material Outside of the Highway Right of Way' of the Standard Specifications. Following removal of the temporary construction road entrance/exits and clean out sumps, the areas shall be graded smooth and compacted to conform with adjacent areas.

The contract unit price paid for temporary construction road entrance/exits shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in placing, maintaining and removing temporary construction road entrance/exit and clean out sump, complete in place, including transporting and disposing of soil and sediments removed from temporary construction road entrance/exit and any incidental grading required to grade and compact areas within the limits of temporary construction road entrance/exit and clean out sump as shown on the plans, as specified in these special provisions, and as directed by the Engineer.

10-1.13 PRESERVATION OF PROPERTY

Attention is directed to the provisions in Section 7-1.11, "Preservation of Property," of the Standard Specifications and these special provisions.

Existing trees, shrubs and other plants, that are not to be removed as specified elsewhere in these special provisions, and are injured or damaged by reason of the Contractor's operations, shall be replaced by the Contractor. The minimum size of tree replacement shall be No. 15 container. Replacement planting shall conform to the requirements in Section 20-4.07, "Replacement," of the Standard Specifications.

Damaged or injured plants shall be removed and disposed of outside the highway right of way in accordance with the provisions in Section 7-l.13 of the Standard Specifications. At the option of the Contractor, removed trees and shrubs may be reduced to chips. The chipped material shall be spread within the highway right of way at locations designated by the Engineer.

Replacement planting of injured or damaged trees, shrubs and other plants shall be completed not less than 20 working days prior to acceptance of the contract. Replacement plants shall be watered as necessary to maintain the plants in a healthy condition.

10-1.14 COOPERATION

Attention is directed to Sections 7-1.14, "Cooperation," and 8-1.10, "Utility and Non-Highway Facilities," of the Standard Specifications and these special provisions.

In the event of a loss caused to the Contractor due to unnecessary delays or failure to finish the work within the time specified for completion caused by another contractor under contract with the Department performing work for the State, the State will reimburse the delayed contractor in conformance with the provisions in Section 8-1.09 "Right of Way Delays," of the Standard Specifications. Deductions will be made from any moneys due or that may become due the contractor causing the loss or delay.

It is anticipated that work will be in progress by State forces and other Contractors within or adjacent to the project limits of this contract.

Contracts which may be in progress during the working period of this contract include but are not necessarily limited to the following:

Contract No. 04-0436V4: Work consists of the seismic retrofit of the San Mateo-Hayward Bridge on the west approach (high rise section of the bridge) which includes the superstructure and expansion joints, substructures, foundations and Pier 38A; the installation of seismic instruments; and the construction of a Changeable Message Sign. Construction is currently in progress.

Contract number 04-045014: Construct new San Mateo-Hayward Bridge Trestle (north of existing structure) on Route 92 just west of the existing toll booth structures. Work on Contract number 04-045014 involves roadway construction within 0.3 KM of the work on this contract.

NOTE: Contract 04-045014 will allow this contract exclusive use of the freeway shoulder area from WOS Line Stations 9+60 - 11+80 for the purpose of storage of materials and pile driving for the installation of concrete sheet piles for Retaining Walls #1 and #2 under this contract. Exclusive use of this area shall be limited to that timeframe necessary for pile driving for Retaining Walls #1 and #2 and shall not exceed two hundred fifty (250) working days from the first chargeable working day under this contract. Coordinate use of this area through the Resident Engineer.

Contract No. 04-045024: Work consists of constructing three toll booths and a walkway at San Mateo-Hayward Bridge Toll Plaza from WOS Line Stations 12+05 - 16+00 It is anticipated that Contract 04-045024 will begin construction after this contract has completed work in the area.

Contract No. 04-150404: Work consists of installing a traffic operating system on Route 92 from Route 101 in San Mateo County to Route 880 in Alameda County.

Progress schedules for other work in progress, if available, may be inspected by the Contractor upon approval by the Engineer. Such progress schedules are tentative and cannot be guaranteed to be accurate.

The Contractor shall attend joint weekly meetings, to be organized by the Engineer with other Contractors on adjacent projects, in order to minimize potential conflicts. The Contractor shall coordinate with and accommodate other bridge Contractors when preparing operations and work schedules.

10-1.15 PROGRESS SCHEDULE (CRITICAL PATH)

Progress schedules will be required for this contract. Progress schedules shall utilize the Critical Path Method (CPM).
Definitions - The following definitions apply to this section "Progress Schedule (Critical Path)":

- Activity: Any task, or portion of a project, which takes time to complete.
- Baseline Schedule: The initial CPM schedule representing the Contractor's original work plan, as accepted by the Engineer.
- Controlling Operation: The activity considered at the time by the Engineer, within that series of activities defined as the critical path, which if delayed or prolonged, will delay the time of completion of the contract.
- Critical Path: The series of activities, which determines the earliest completion of the contract (Forecast Completion Date).
- Critical Path Method: A mathematical calculation to determine the earliest completion of the contract represented by a graphic representation of the sequence of activities that shows the interrelationships and interdependencies of the elements composing a project.
- Current Contract Completion Date: The extended date for completion of the contract shown on the weekly statement of working days furnished by the Engineer in accordance with Section 8-1.06, "Time of Completion," of the Standard Specifications.
- Early Completion Time: The difference in time between the current contract completion date and the Contractor's scheduled early forecast completion date as shown on the accepted baseline schedule, or schedule updates and revisions.
- Float: The amount of time between the early start date and the late start date, or the early finish date and the late finish date, of any activity or group of activities in the network.
- Forecast Completion Date: The completion date of the last scheduled work activity identified on the critical path.
- Fragnet: A section or fragment of the network diagram comprised of a group of activities.
- Free Float: The amount of time an activity can be delayed before affecting a subsequent activity.
- Hammock Activity: An activity added to the network to span an existing group of activities for summarizing purposes.
- Milestone: A marker in a network, which is typically used to mark a point in time or denote the beginning or end of a sequence of activities. A milestone has zero duration, but will otherwise function in the network as if it were an activity.
- Revision: A change in the future portion of the schedule that modifies logic, adds or deletes activities, or alters activities, sequences, or durations.
- Tabular Listing: A report showing schedule activities, their relationships, durations, scheduled and actual dates, and float.
- Total Float: The amount of time that an activity may be delayed without affecting the total project duration of the critical path.
- Update: The modification of the CPM progress schedule through a regular review to incorporate actual progress to date by activity, approved time adjustments, and projected completion dates.
- Time Scaled Logic Diagram: A schematic display of the logical relationships of project activities, drawn from left to right to reflect project chronology with the positioning and length of the activity representing its duration.
- Bar Chart (Gantt Chart): A graphic display of scheduled-related information, activities or other project elements are listed down the left side of the chart, date are shown across the top, and activity durations are shown as date-placed horizontal bars.

Pre-construction Scheduling Conference - The Engineer shall schedule and conduct a Preconstruction Scheduling Conference with the Contractor's Project Manager and Construction Scheduler within seven days after the bidder has received the contract for execution. At this meeting, the requirements of this section of the special provisions will be reviewed with the Contractor. The Contractor shall be prepared to discuss its schedule methodology, proposed sequence of operations, the activity identification system for labeling all work activities, the schedule file numbering system, and any deviations it proposes to make from the Stage Construction Plans. The Engineer shall submit a diskette of a scheduling shell project, displaying an activity code dictionary consisting of fields populated with the Caltrans Scope Breakdown Structure Code. The Contractor shall utilize these codes, and may add other codes as necessary, to group and organize the work activities. Periodically the Engineer may request the Contractor to utilize additional filters, layouts or activity codes to be able to further group or summarize work activities.

Also, the Engineer and the Contractor shall review the requirements for all submittals applicable to the contract and discuss their respective preparation and review durations. All submittals and reviews are to be reflected on the Interim Baseline Schedule and the Baseline Schedule.

Interim Baseline Schedule - Within 15 days after approval of the contract, the Contractor shall submit to the Engineer an Interim Baseline Project Schedule which will serve as the progress schedule for the first 120 days of the project, or until the Baseline Schedule is accepted, whichever is sooner. The Interim Baseline Schedule shall utilize the critical path method. The Interim Baseline Schedule shall depict how the Contractor plans to perform the work for the first 120 days of the contract. Additionally, the Interim Baseline Schedule shall show all submittals required early in the project, and shall provide for all permits, and other non-work activities necessary to begin the work. The Interim Baseline Schedule submittal shall include a 3 l/2 inch floppy diskette which contains the data files used to generate the schedule.

The Engineer shall be allowed 10 days to review the schedule and to provide comments, including the Contractor's application of the supplied scope breakdown structure. The Interim Baseline Schedule does not require Caltrans approval but all comments are to be implemented into the Baseline Schedule. Re-submittal of the Interim Baseline Schedule is not required. Late review of the Interim Baseline Schedule shall not restrain the submittal of the Baseline Schedule.

Baseline Schedule - Within 30 days, after approval of the contract, the Contractor shall submit to the Engineer a Baseline Project Schedule including the incorporation of all comments provided to the Interim Baseline Schedule. The Baseline Schedule shall have a data date of the day prior to the first working day of the contract. The schedule shall not include any actual start dates, actual finish dates, or constraint dates. The Baseline Progress Schedule shall meet interim milestone dates, contract milestone dates, stage construction requirements, internal time constraints, show logical sequence of activities, and must not extend beyond the number of days originally provided for in the contract.

All task activities shall be assigned to a project calendar. Each calendar shall identify a workweek, and holidays. Use different calendars for work activities that occur on different work schedules.

The Contractor shall not add job inefficiencies or weather days to a project calendar without prior approval by the Engineer.

The Baseline CPM Schedule submitted by the Contractor shall have a sufficient number of activities to assure adequate planning of the project and to permit monitoring and evaluation of progress and the analysis of time impacts. The Baseline Schedule shall depict how the Contractor plans to complete the whole work involved, and shall show all activities that defines the critical path. Each construction activity shall have durations of not more than 20 working days, and not less than one working day unless permitted otherwise by the Engineer. All activities in the schedule, with the exception of the first and last activities, shall have a minimum of one predecessor and a minimum of one successor.

The Baseline Schedule shall not attribute negative float to any activity. Float shall not be considered as time for the exclusive use of or benefit of either the State or the Contractor but shall be considered as a jointly owned, expiring resource available to the project and shall not be used to the financial detriment of either party. Any accepted schedule, revision or update having an early completion date shall show the time between the early completion date and the current Contract Completion Date as "total float".

The Contractor shall be responsible for assuring that all work sequences are logical and the network shows a coordinated plan for complete performance of the work. Failure of the Contractor to include any element of work required for the performance of the contract in the network shall not relieve the Contractor from completing all work within the time limit specified for completion of the contract. If the Contractor fails to define any element of work, activity or logic, the Contractor in the next monthly update or revision of the schedule shall correct it.

The Baseline Progress Schedule shall be supplemented with resource allocations for every task activity to a level of detail that facilitates report generation based on labor craft and equipment class for the Contractor and subcontractors. The Contractor shall use average composite crews to display the labor loading of on-site construction activities. The Contractor shall optimize and level labor to reflect a reasonable plan for accomplishing the work of the contract and to assure that resources are not duplicated in concurrent activities. Along with the baseline progress schedule, the Contractor shall also submit to the Engineer time-scaled resource histograms of the labor crafts and equipment classes to be utilized on the contract.

The Contractor shall not create hammock activities for the purpose of resources loading.

The Contractor shall require each subcontractor to submit in writing a statement certifying that the subcontractor has concurred with the Contractor's CPM, including major updates, and that the subcontractor's related schedule has been incorporated accurately, including the duration of activities, labor and equipment loading. Should the Baseline Schedule or schedule update, submitted for acceptance, show variances from the requirements of the contract, the Contractor shall make specific mention of the variations in the letter of transmittal, in order that, if accepted, proper adjustments to the project schedule can be made. The Contractor will not be relieved of the responsibility for executing the work in strict accordance with the requirements of the contract documents. In the event of a conflict between the requirements of the contract documents and the information provided or shown on an accepted schedule, the requirements of the contract documents shall take precedence.

Each schedule submitted to the Engineer shall comply with all limits imposed by the contract, with all specified intermediate milestone and contract completion dates, and with all constraints, restraints or sequences included in the contract. The degree of detail shall include factors including, but not limited to:

1) Physical breakdown of the project;

2) Contract milestones and completion dates, substantial completion dates, constraints, restraints, sequences of work shown in the contract, the planned substantial completion date, and the final completion date;
3) Type of work to be performed, the sequences, and the major subcontractors involved;
4) All purchases, submittals, submittal reviews, manufacture, fabrication, tests, delivery, and installation activities for all major materials and equipment.
5) Preparation, submittal and approval of shop and working drawings and material samples, showing time, as specified elsewhere, for the Engineer's review. The same time frame shall be allowed for at least one resubmittal on all major submittals so identified in the contract documents;
6) Identification of interfaces and dependencies with preceding, concurrent and follow-on contractors, railroads, and utilities as shown on the plans or specified in the specifications;
7) Identification of each and every utility relocation and interface as a separate activity, including activity description and responsibility coding that identifies the type of utility and the name of the utility company involved.
8) Actual tests, submission of test reports, and approval of test results;
9) All start-up, testing, training, and assistance required under the Contract;
10) Punchlist and final clean-up;

- Identification of any manpower, material, or equipment restrictions, as well as any activity requiring unusual shift work, such as double shifts, 6-day weeks, specified overtime, or work at times other than regular days or hours; and
- Identification of each and every ramp closing and opening event as a separate one-day activity, including designation by activity coding and description that it is a north-bound, south-bound, east-bound, west-bound, and entry or exit ramp activity.

The Baseline Schedule submittal shall include a 3 l/2 inch floppy diskette which contains the data files used to generate the schedule, a schedule narrative describing the critical path, and all schedule reports.

The Engineer shall be allowed 15 days to review and accept or reject the baseline project schedule submitted. Rejected schedules shall be resubmitted to the Engineer within 5 days, at which time a new 15 day review period by the Engineer will begin.

Project Schedule Reports - Schedules submitted to the Engineer including Interim Baseline, Baseline, and update schedules shall include time scaled network diagrams in a layout format requested by the Engineer. The network diagrams submitted to the Engineer shall also be accompanied by four computer-generated mathematical analysis tabular reports for each activity included in the project schedule. The reports (8 1/2” x 11” size) shall include a network diagram report showing the activity columns only, a predecessor and successor report, a resource report (Interim Baseline and Baseline Schedules), and a scheduling and leveling calculation report. The network diagram reports shall include, at a minimum, the following for each activity:

1) Activity number and description;
2) Activity codes;
3) Original, actual and remaining durations;
6) Early start date (by calendar date);
7) Early finish date (by calendar date);
8) Actual start date (by calendar date);
9) Actual finish date (by calendar date);
10) Late start date (by calendar date);
11) Late finish date (by calendar date);
12) Identify activity calendar ID;
14) Total Float and Free Float, in work days and;
15) Percentage complete.

Network diagrams shall be sorted and grouped in a format requested by the Engineer reflecting the project breakdown per the Caltrans scope breakdown structure codes. They shall show a continuous flow of information from left to right per the project sorting and grouping codes, e.g., project milestones, submittals sub-grouped by description, and the construction activities sub-grouped by the scope breakdown structure. The primary paths of criticality shall be clearly and graphically identified on the networks. The network diagram shall be prepared on E-size sheets (36" x 48"), shall have a title block in the lower right-hand corner, and a timeline on each page. Exceptions to the size of the network sheets and the use of computer graphics to generate the networks shall be subject to the approval of the Engineer.

Schedule network diagrams the tabular reports shall be submitted to the Engineer for acceptance in the following quantities:

a) 2 sets of the Network Diagrams;
b) 2 copies of the tabular reports (8 1/2" x 11" size); and
c) 3 computer diskettes.

Weekly Schedule Meetings - The Engineer and the Contractor shall hold weekly scheduling meetings to discuss the near term schedule activities, to address any long-term schedule issues, and to discuss any relevant technical issues. The Contractor shall develop a rolling 4-week schedule identifying the previous week worked and a 3-week look ahead. It shall provide sufficient detail to address all activities to be performed and to identify issues requiring engineering action or input.

Monthly Update Schedules - The Contractor shall submit a Monthly Update Schedule to the Engineer once in each month within 5 days of the data date. The proposed update schedule prepared by the Contractor shall include all information available as of the 20th calendar day of the month, or other data date as established by the Engineer. A detailed list of all proposed schedule changes such as logic, duration, lead/lag, forecast completion date, additions and deletions shall be submitted with the update.

The monthly update of the schedule shall focus on the period from the last update to the current cut-off data date. Changes to activities or logic beyond the data date are classified as revisions and need to be addressed per the schedule revision section of this specification. Activities that have either started or finished shall be reported as they actually occurred and designated as complete, if actually completed. For activities in progress that are forecasted to complete longer than planned, the remaining durations shall be revised, not the original durations. All out of sequence activities are to be reviewed and their relationships either verified or changed.

The Monthly Update Schedule submitted to the Engineer shall be accompanied by a Schedule Narrative Report. The report shall describe the physical progress during the report period, plans for continuing the work during the forthcoming report period, actions planned to correct any negative float, and an explanation of potential delays or problems and their estimated impact on performance, milestone completion dates, forecast completion date, and the overall project completion date. In addition, alternatives for possible schedule recovery to mitigate any potential delay or cost increases shall be included for consideration by the Engineer. The report shall follow the outline set forth below:

Contractor's Schedule Narrative Report Outline:

1) Contractor's Transmittal Letter
2) Work completed during the period
3) Description of the current critical path
4) Description of problem areas
5) Current and anticipated delays
    a) Cause of the delay
    b) Corrective action and schedule adjustments to correct the delay
    c) Impact of the delay on other activities, milestones, and completion dates
6) Changes in construction sequences
7) Pending items and status thereof
    a) Permits
    b) Change Orders
    c) Time Extensions
    d) Non-Compliance Notices
8) Contract completion date(s) status
    a) Ahead of schedule and number of days
    b) Behind schedule and number of days
9) Include updated Network Diagram and Reports

The Contractor shall provide to the Engineer a 3 1/2” electronic disk of the schedule, together with printed copies of the network diagrams and tabular reports described under "Project Schedule Reports", and the Schedule Narrative Report.

Portions of the network diagram on which all activities are complete need not be reprinted and submitted in subsequent updates. However, the electronic disk file of the submitted schedule and the related reports shall constitute a clear record of progress of the work from award of contract to final completion.

On a date determined by the Engineer, the Contractor shall meet with the Engineer to review the monthly schedule update. At the monthly progress meeting, the Contractor and the Engineer shall review the updated schedule and shall discuss the content of the Narrative Report. The Engineer shall be allowed 10 days after the meeting to review and accept or reject the update schedule submitted. Rejected schedules shall be resubmitted to the Engineer within 5 days, at which time a new 5 day review period by the Engineer will begin. All efforts shall be made between the Engineer and the Contractor to complete the review and the approval process prior to the next update schedule cutoff date. To expedite the process a second meeting between the Engineer and the Contractor shall be held.

Schedule Revisions - If the Contractor desires to make a change to the accepted schedule, the Contractor shall request permission from the Engineer in writing, stating the reasons for the change, and proposed revisions to activities, logic and duration. The Contractor shall submit for acceptance an analysis showing the effect of the revisions on the entire project. The analysis shall include:

1. An updated schedule not including the revisions. The schedule shall have a data date just prior to implementing the proposed revisions and include a project completion date;
2. A revised schedule that includes the proposed revisions. The schedule will have the same data date as the updated schedule and include a project completion date;
3. A narrative explanation of the revisions and their impact to the schedule; and
4. Computer files of the updated schedule and the revised schedule sequentially numbered or renamed for archive (record) purposes.

The Engineer will provide a response within 10 days. No revision to the accepted baseline schedule or the schedule updates shall be made without the prior written approval of the Engineer.

The Engineer will request the Contractor to submit a proposed revised schedule within 15 days when:

a) there is a significant change in the Contractor's operations that will affect the critical path;
b) the current updated schedule indicates that the contract progress is 30 days or more behind the planned schedule, as determined by the Engineer; or
c) the Engineer determines that an approved or anticipated change will impact the critical path, milestone or completion dates, contract progress, or work by other contractors.

The Engineer shall be allowed 10 days to review and accept or reject a schedule revision. Rejected schedule revisions shall be revised and resubmitted to the Engineer within 10 days, at which time a new 10 day review period by the Engineer will begin. Only upon approval of a change by the Engineer shall it be reflected in the next schedule update submitted by the Contractor.

Schedule Time Extension Requests - When the Contractor requests a time extension due to contract change orders or delays, the Contractor shall submit to the Engineer a written Time Impact Analysis illustrating the influence of each change or delay on the current contract completion date or milestone completion date, utilizing the current accepted schedule. Each Time Impact Analysis shall include a schedule revision demonstrating how the Contractor proposes to incorporate the Change Order or delay into the current schedule. The schedule shall include the sequence of activities and any revisions to the existing activities to demonstrate the influence of the delay, the proposed method for incorporating the delay, and its impact into the schedule.

Each time impact analysis shall demonstrate the estimated time impact based on the events of delay, the anticipated or actual date of the contract change order work performance, the status of construction at that point in time, and the event time computation of all activities affected by the change or delay. The event times used in the analysis shall be those included in the latest update of the current schedule in effect at the time the change or delay was encountered.

Time extensions will be granted only to the extent that equitable time adjustments for the activity or activities affected exceed the total or remaining float along the critical path of activities at the time of actual delay, or at the time the contract change order work is performed. Float time is not for the exclusive use or benefit of the Engineer or the Contractor, but is an expiring resource available to all parties as needed to meet contract milestones and the contract completion date. Time extensions will not be granted nor will delay damages be paid unless:

a) the delay is beyond the control and without the fault or negligence of the Contractor and its subcontractors or suppliers, at any tier; and,
b) the delay extends the actual performance of the work beyond the applicable current contract completion date and the most recent date predicted for completion of the project on the accepted schedule update current as of the time of the delay or as of the time of issuance of the contract change order.

Time Impact Analyses shall be submitted in triplicate within 15 days after the delay occurs or after issuance of the contract change order. A schedule file diskette is also to be submitted.

Acceptance or rejection of each Time Impact Analysis by the Engineer will be made within 15 days after receipt of the Time Impact Analysis, unless subsequent meetings and negotiations delay the review. A copy of the Time Impact Analysis accepted by the Engineer shall be returned to the Contractor and the accepted schedule revisions illustrating the influence of the contract change orders or delays shall be incorporated into the project schedule during the first update after acceptance.

Final Schedule Update - Within 15 days after the acceptance of the contract by the Director, the Contractor shall submit a final update of the schedule with actual start and actual finish dates for all activities. This schedule submission shall be accompanied by a certification, signed by an officer of the company and the Contractor's Project Manager stating "To the best of my knowledge, the enclosed final update of the project schedule reflects the actual start and completion dates of the activities contained herein."

Equipment and Software - The Contractor shall provide for the State's exclusive possession and use a complete computer system specifically capable of creating, storing, updating and producing CPM schedules. Before delivery and setup of the computer system, the Contractor shall submit to the Engineer for approval a detailed list of all computer hardware and software the Contractor proposes to furnish. The minimum computer system to be furnished shall include the following:

1) Complete computer system, including keyboard, mouse, 20 inch color SVGA monitor (1,024x768 pixels), Intel Pentium 350 MHz micro processor chip, or equivalent, or better;

2) Computer operating system software, compatible with the selected processing unit, for Windows 95 or later, or equivalent;

3) Minimum sixty-four (64) megabytes of random access memory (RAM);

4) A 3.2 gigabyte minimum hard disk drive, a 1.44 megabyte 3 l/2 inch floppy disk drive, 32x speed minimum CD-ROM drive, Ethernet card and 56k modem;

5) A color-ink-jet plotter with a minimum 36Megabytes RAM, capable of 300 dots per inch color, 600 dots per inch monochrome, or equivalent. Capable of printing fully legible, timescaled charts, and network diagrams, in four colors, with a minimum size of 36 inches by 48 inches (E size) and is compatible with the selected system. Capable of plotting 3 E sized sheets within one hour. Plotter paper and ink cartridges throughout the contract.

6) CPM software shall be Primavera Project Planner, the latest version for Windows 95, or later;

7) Scheduler Analyzer Pro or equivalent – a suite of programs to assist in schedule analysis, the latest version for Windows 95, Windows NT or later and,

8) Microsoft Office software, the latest version for Windows 95, Windows NT or later, and McAfee Virus software or equivalent.

The computer hardware and software furnished shall be compatible with that used by the Contractor for the production of the CPM progress schedule required by the Contract, and shall include original instruction manuals and other documentation normally provided with the software.

The Contractor shall furnish, install, set up, maintain and repair the computer hardware and software ready for use at a location determined by the Engineer. The hardware and software shall be installed and ready for use by the first submission of the baseline schedule. The Contractor shall provide 24 hours of formal training for the Engineer, and three other agents of the department designated by the Engineer, in the use of the hardware and software to include schedule analysis, reporting, and resource and cost allocations. An authorized vendor of Project Primavera shall perform the training.

All computer hardware and software furnished shall remain the property of the Contractor and shall be removed by the Contractor upon acceptance of the contract when no claims involving contract progress are pending. When claims involving contract progress are pending, computer hardware or software shall not be removed until the final estimate has been submitted to the Contractor.

Payment - Progress schedule (critical path) will be paid for at a lump sum price. The contract lump sum price paid for progress schedule (critical path) shall include full compensation for furnishing all labor, materials (including computer hardware and software), tools, equipment, and incidentals; and for doing all the work involved in preparing, furnishing, updating and revising CPM progress schedules. Also for maintaining and repairing the computer hardware and training the Engineer in the use of the computer hardware and software as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Payments for progress schedule (critical path) will be made as follows:

Interim baseline schedule accepted, then 10 percent payment for progress schedule (critical path) will be made.

Baseline schedule accepted, then 10 percent payment for progress schedule (critical path) will be made.

Monthly update schedules accepted, then 75 percent payment for progress schedule (critical path) will be made equally for each update.

Final schedule update accepted, then 5 percent payment for progress schedule (critical path) will be made.

The Department will retain an amount equal to 25 percent of the estimated value of the work performed during the first estimate period in which the Contractor fails to submit an interim baseline, baseline, revised or updated CPM schedule conforming to the requirements of this section, as determined by the Engineer. Thereafter, on subsequent successive estimate periods the percentage the Department will retain will be increased at the rate of 25 percent per estimate period in which acceptable CPM progress schedules have not been submitted to the Engineer. Retention's for failure to submit acceptable CPM progress schedules shall be additional to all other retention's provided for in the contract. The retention for failure to submit acceptable CPM progress schedules will be released for payment on the next monthly estimate for partial payment following the date that acceptable CPM progress schedules are submitted to the Engineer.

The adjustment provisions in Section 4-1.03, "Changes," of the Standard Specifications, shall not apply to the item of progress schedule (critical path). Adjustments in compensation for the project schedule will not be made for any increased or decreased work ordered by the Engineer in furnishing project schedules.

10-1.16 ELECTRONIC MOBILE DAILY DIARY COMPUTER SYSTEM DATA DELIVERY

Attention is directed to Sections 5-1.10, "Equipment and Plants," and 7-1.01A(3), "Payroll Records," of the Standard Specifications, and these special provisions.

The Contractor shall submit to the Engineer a list of each piece of equipment and its identifying number, type, make, model and rate code in accordance with the Department of Transportation publication entitled "Labor Surcharge and Equipment Rental Rate" which is in effect on the date upon the work is performed, and the names, labor rates and work classifications for all field personnel employed by the Contractor and all subcontractors in connection with the public work, together with such additional information as is identified below. This information shall be updated and submitted to the Engineer weekly through the life of the project.

This personnel information will only be used for this mobile daily diary computer system and it will not relieve the Contractor and subcontractors from all the payroll records requirements as required by Section 7-1.01A(3), "Payroll Records," of the Standard Specifications.

The Contractor shall provide the personnel and equipment information not later than 11 days after the contract award for its own personnel and equipment, and not later than 5 days before start of work by any subcontractor for the labor and equipment data of that subcontractor.

The minimum data to be furnished shall comply with the following specifications:

DATA CONTENT REQUIREMENTS

1. The Contractor shall provide the following basic information for itself and for each subcontractor that will be used on the contract:

Company name.	Alphanumeric; up to 30 characters.
Federal tax ID	Alphanumeric; up to 10 characters.
State contractor license	Alphanumeric; up to 20 characters.
Company type (prime or sub)	Alphanumeric; up to 10 characters.
Address (line 1).	Alphanumeric; up to 30 characters.
Address (line 2).	Alphanumeric; up to 30 characters.
Address (city).	Alphanumeric; up to 30 characters.
Address (2-letter state code).	Alphanumeric; up to 2 characters.
Address (zip code)	Alphanumeric; up to 14 characters.
Contact name.	Alphanumeric; up to 30 characters
Telephone number (with area code).	Alphanumeric; up to 20 characters.
Company code: short company name.	Alphanumeric; up to 10 characters.
Type of work (Department-supplied codes)	Alphanumeric; up to 30 characters
DBE status (Department-supplied codes)	Alphanumeric; up to 20 characters.
Ethnicity for DBE status (Department-supplied codes).	Alphanumeric; up to 20 characters.
List of laborers to be used on this contract (detail specified below).	
List of equipment to be used on this contract (detail specified below).	

For example, one such set of information for a company might be:

XYZ CONSTRUCTION, INC.
94-2991040
AL1649T
SUB
1240 9TH STREET

SUITE 600
OAKLAND
CA
94612
JOHN SMITH
(510) 834-9999
XYZ
PAVING
MBE
BLACK

2. The Contractor shall provide the following information for each laborer who will be used on the contract:

Company code (as defined above).	Alphanumeric; up to 10 characters.
Employee ID	Alphanumeric; up to 10 characters.
Last name.	Alphanumeric; up to 20 characters.
First name.	Alphanumeric; up to 15 characters.
Middle name.	Alphanumeric; up to 15 characters.
Suffix	Alphanumeric; up to 15 characters
Labor trade (Department-provided codes).	Alphanumeric; up to 10 characters.
Labor classification (Department-provided codes).	Alphanumeric; up to 10 characters.
Regular hourly rate.	Alphanumeric; up to (6,2)
Overtime hourly rate.	Alphanumeric; up to (6,2)
Doubletime hourly rate	Alphanumeric; up to (6,2)
Standby hourly rate.	Alphanumeric; up to (6,2)
Ethnicity (Department-provided codes).	Alphanumeric; up to 20 characters.
Gender.	Alphanumeric; up to 1 character.

For example, one such set of information might be:
XYZ
1249
GONZALEZ
HECTOR
VINCENT
JR.
OPR
JNY
22.75
30.25
37.75
0.00
HISPANIC
M

3. The Contractor shall provide the following information for each piece of equipment that will be used on the contract:

Company code (as defined above).	Alphanumeric; up to 10 characters.
Company's equipment ID number.	Alphanumeric; up to 10 characters.
Company's equipment description.	Alphanumeric; up to 60 characters.
Equipment type (from Department ratebook).	Alphanumeric; up to 60 characters.
Equipment make (from Department ratebook).	Alphanumeric; up to 60 characters.
Equipment model (from Department ratebook).	Alphanumeric; up to 60 characters.
Equipment rate code (from Department ratebook).	Alphanumeric; up to 10 characters
Regular hourly rate.	Alphanumeric; up to (6,2)
Overtime hourly rate.	Alphanumeric; up to (6,2)
Standby hourly rate	Alphanumeric; up to (6,2)
Idle hourly rate.	Alphanumeric; up to (6,2)
Rental flag.	Alphanumeric; up to 1 character.

For example, one such set of information might be:

XYZ
B043
CAT TRACTOR D-6C
TRACC
CAT
D-6C
3645
28.08
25.27
14.04
0.00
N

DATA DELIVERY REQUIREMENTS

1. All data described in "Data Requirements" of this section shall be delivered to the Department electronically, on 3 1/4" floppy disks compatible with the Microsoft Windows operating system. The Contractor shall provide a weekly disk and hard copy of the required correct updated personnel and equipment information for the Contractor and all the subcontractors and verified correct by the Engineer.

2. Data of each type described in the previous section (contractor, labor, and equipment information) will be delivered separately, each type in one or more files on floppy disk. Any given file may contain information from one contractor or from multiple contractors, but only one type of data (contractor, labor, or equipment information).

3. The file format for all files delivered to Caltrans shall be standard comma-delimited, plain text files. This type of file (often called “CSV”) is the most standard type for interchange of formatted data; it can be created and read by all desktop spreadsheet and desktop database applications. Characteristics of this type of file are:

- All data is in the form of plain ASCII characters.
- Each row of data (company, person, equipment) is delimited by a carriage return character.
- Within rows, each column (field) of data is delimited by a comma character.

4. The files shall have the following columns (i.e., each row shall have the following fields):

- Contractor info: 15 columns (fields) as specified in "Data Requirements #1", above.
- Labor info: 14 columns (fields) as specified in "Data Requirements #2", above.
- Equipment info: 12 columns (fields) as specified in "Data Requirements #3", above.

For each type of file, columns (fields) must be in the order specified under "Data Requirements", above. All columns (fields) described under "Data Requirements" must be present for all rows, even if some column (field) values are empty. The first row of each file may contain column headers (in plain text) rather than data, if desired.

5. Column (field) contents must conform to the data type and length requirements described in the "Data Requirement" section, above. In addition, column (field) data must conform to the following restrictions:

- All data shall be uppercase.
- Company type shall be either “PRIME” or “SUB”.
- Labor trade and classification codes must conform to a list of standard codes that will be supplied by Department.
- Contractor type of work codes and DBE status codes must conform to a list of standard codes that will be supplied by Department.
- Ethnicity codes must conform to standard codes that will be supplied by Department.
- Data in the "gender" column must be either "M" or "F".
- Data in the "rental equipment" column must be either "Y" or "N".
- Equipment owner's description may not be omitted. (The description, together with the equipment number, is how the equipment will be identified in the field.) Include manufacturer, rated capacity & trade description
- Equipment type, make, model, and ratebook code shall conform to the Department of Transportation Publication entitled “Labor Surcharge and Equipment Rental Rate”, which is in effect on the date upon the work is performed. If the equipment in question does not have an entry in the book then alternate, descriptive entries may be made in these fields as directed by the Engineer.

6. The name of each file must indicate its contents, e.g., "XYZlab.csv" for laborers from XYZ Company, Inc. Each floppy disk supplied to Caltrans must be accompanied by a printed list of the files it contains with a brief description of the contents of each file.

PAYMENT

Payment for providing electronic mobile daily diary computer system data delivery will be made on a lump sum basis .The lump sum bid price for electronic mobile daily diary computer system data delivery will be made according to the following schedule:

The Contractor will receive not more than 3.33 per cent per month of the total bid price for electronic mobile daily diary computer system data delivery .

After the completion of the work, 100 per cent payment will be made for electronic mobile daily diary computer system data delivery less the permanent deduction, if any, for failure to deliver complete weekly electronic mobile daily diary computer system data in each month.

The contract lump sum price paid for electronic mobile daily diary computer system data delivery shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in electronic mobile daily diary computer system data delivery as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

The Department will retain an amount equal to 25 percent of the estimated value of the work performed during the first estimate period in which the Contractor fails to submit electronic mobile daily diary computer system data delivery conforming to the requirements of this section, as determined by the Engineer. Thereafter, on subsequent successive estimate periods the percentage the Department will retain will be increased at the rate of 25 percent per estimate period in which acceptable electronic mobile daily diary computer system data have not been submitted to the Engineer. Retentions for failure to submit acceptable electronic mobile daily diary computer system data shall be additional to all other retentions provided for in the contract. The retention for failure to submit acceptable electronic mobile daily diary computer system data will be released for payment on the next monthly estimate for partial payment following the date that acceptable electronic mobile daily diary computer system data is submitted to the Engineer.

The adjustment provisions in Section 4-1.03, "Changes," of the Standard Specifications, shall not apply to the item of electronic mobile daily diary computer system data delivery. Adjustments in compensation for electronic mobile daily diary computer system data delivery will not be made for any increased or decreased work ordered by the Engineer in furnishing electronic mobile daily diary computer system data.

10-1.17 OBSTRUCTIONS

Attention is directed to Sections 8-1.10, "Utility and Non-Highway Facilities," and 15, "Existing Highway Facilities," of the Standard Specifications and these special provisions.

The Contractor's attention is directed to the existence of certain underground facilities that may require special precautions be taken by the Contractor to protect the health, safety and welfare of workers and of the public. Facilities requiring special precautions include, but are not limited to: conductors of petroleum products, oxygen, chlorine, and toxic or flammable gases; natural gas in pipelines greater than 150 mm in diameter or pipelines operating at pressures greater than

415 kPa (gage); underground electric supply system conductors or cables, with potential to ground of more than 300 V, either directly buried or in duct or conduit which do not have concentric grounded or other effectively grounded metal shields on sheaths.

If these facilities are not located on the plans in both alignment and elevation, no work shall be performed in the vicinity of the facilities, except as provided herein for conduit to be placed under pavement, until the owner, or the owner's representative, has located the facility by potholing, probing or other means that will locate and identify the facility. Conduit to be installed under pavement in the vicinity of these facilities shall be placed by the trenching method in conformance with the provisions in "Conduit" of these special provisions. If, in the opinion of the Engineer, the Contractor's operations are delayed or interfered with by reason of the utility facilities not being located by the owner or the owner's representative, the State will compensate the Contractor for the delays to the extent provided in Section 8-1.09, "Right of Way Delays," of the Standard Specifications, and not otherwise, except as provided in Section 8-1.10, "Utility and Non-Highway Facilities," of the Standard Specifications.

The Contractor shall notify the Engineer and the appropriate regional notification center for operators of subsurface installations at least 2 working days, but not more than 14 calendar days, prior to performing any excavation or other work close to any underground pipeline, conduit, duct, wire or other structure. Regional notification centers include but are not limited to the following:

Notification Center	Telephone Number
Underground Service Alert-Northern California (USA)	1-800-642-2444 1-800-227-2600
Underground Service Alert-Southern California (USA)	1-800-422-4133 1-800-227-2600

The Contractor shall notify the Engineer and the Southern Pacific Telecom 24-hour Notification Center at telephone number 1-800-283-4237 at least 72 hours prior to performing any work on Southern Pacific Transportation Company property. No work shall be performed on Southern Pacific Transportation Company property until fiber optic conduit structures have been located and flagged by Southern Pacific Telecom.

The Contractor shall notify the Engineer and the Union Pacific 24-hour Telecommunications "Call Before You Dig" number 1-800-336-9193 at least 48 hours prior to performing any work on Union Pacific Railroad Company property. No work shall be performed on Union Pacific Railroad Company property until optical conduit structures have been located and flagged by Union Pacific Telecommunications personnel.

Full compensation for conforming to the requirements of this section, not otherwise provided for, shall be considered as included in the prices paid for the various contract items of work involved and no additional compensation will be allowed therefor.

10-1.18 DUST CONTROL

Dust control shall conform to the provisions in Section 10, "Dust Control," of the Standard Specifications and these special provisions.

10-1.19 MOBILIZATION

Mobilization shall conform to the provisions in Section 11, "Mobilization," of the Standard Specifications.

10-1.20 CONSTRUCTION AREA SIGNS

Construction area signs shall be furnished, installed, maintained, and removed when no longer required in conformance with the provisions in Section 12, "Construction Area Traffic Control Devices," of the Standard Specifications and these special provisions.

Attention is directed to "Relocate Roadside Signs" elsewhere in these special provisions. The construction area signs shown on the staging plans shall be in place and ready for use prior to the relocation of corresponding permanent roadside signs.

All construction area signs used to direct public traffic (detour signs and those signs shown on the staging plans) shall be covered when not needed to direct public traffic.

The Contractor shall notify the appropriate regional notification center for operators of subsurface installations at least 2 working days, but not more than 14 calendar days, prior to commencing excavation for construction area sign posts. The regional notification centers include but are not limited to the following:

Notification Center	Telephone Number
Underground Service Alert-Northern California (USA)	1-800-642-2444 1-800-227-2600
Underground Service Alert-Southern California (USA)	1-800-422-4133 1-800-227-2600

Excavations required to install construction area signs shall be performed by hand methods without the use of power equipment, except that power equipment may be used if it is determined there are no utility facilities in the area of the proposed post holes.

The second sentence of the third paragraph in Section 12-3.02, "Barricades," of the Standard Specifications is amended to read:

> The entire area of orange and white stripes shall be Type I, engineering grade, or Type II, super engineering grade, retroreflective sheeting conforming to the requirements of ASTM Designation: D 4956-95.

The third paragraph in Section 12-3.06A, "Stationary Mounted Signs," of the Standard Specifications is amended to read:

> Sign panels for stationary mounted signs shall consist of Type III or Type IV reflective sheeting applied to an aluminum substrate conforming to the requirements in the Department's "Specifications for Reflective Sheeting Aluminum Signs." The type of reflective sheeting, Type III or Type IV, shall be at the Contractor's option and sign substrates fabricated from materials other than aluminum may be used when specified in the special provisions.
>
> Legend and border may be applied by a screening process or by use of pressure sensitive cut-out sheeting. Size and spacing of letters and symbols shall be as depicted on the sign specification sheets published by the Department.
>
> Rectangular signs over 1375 mm measured along the horizontal axis, and diamond-shaped signs 1500 mm and larger shall be framed unless otherwise specified. Frames shall be constructed in conformance with the requirements of the Department's "Framing Details for Sheet Aluminum Signs," Sheets 1 through 4 and Table 1 on Sheet 5.
>
> Copies of the Department's "Specifications for Reflective Sheeting Aluminum Signs," "Framing Details for Sheet Aluminum Signs," and sign specification sheets may be obtained from the Department's Office of Business Management, Materiel Operations Branch, 1900 Royal Oaks Drive, Sacramento, CA 95815.

The second paragraph in Section 12-3.06B, "Portable Signs," of the Standard Specifications is amended to read:

> Sign panels for portable signs shall conform to the provisions of sign panels for stationary mounted signs in Section 12-3.06A, "Stationary Mounted Signs," or shall be Type VI reflective sheeting as specified in the special provisions, or shall be cotton drill fabric, flexible industrial nylon fabric, or other approved fabric. Fabric signs shall not be used during the hours of darkness. Size, color, and legend requirements for portable signs shall be as described for stationary mounted sign panels in Section 12-3.06A. The height to the bottom of the sign panel above the edge of traveled way shall be at least 0.3-m.

The third paragraph in Section 12-3.06B, "Portable Signs," of the Standard Specifications is deleted.

Sign substrates for stationary mounted construction area signs may be fabricated from fiberglass reinforced plastic as specified under "Approved Traffic Products" of these special provisions.

Type VI reflective sheeting for sign panels for portable construction area signs shall conform to the provisions in "Approved Traffic Products" of these special provisions.

10-1.21 MAINTAINING TRAFFIC

Attention is directed to Sections 7-1.08, "Public Convenience," 7-1.09, "Public Safety," and 12, "Construction Area Traffic Control Devices," of the Standard Specifications and to the Section entitled "Public Safety" elsewhere in these special provisions, and these special provisions. Nothing in these special provisions shall be construed as relieving the Contractor from the responsibilities specified in Section 7-1.09.

Lane closures shall conform to the provisions in the section of these special provisions entitled "Traffic Control System for Lane Closure."

Personal vehicles of the Contractor's employees shall not be parked on the traveled way or shoulders, including any section closed to public traffic.

The Contractor shall notify local authorities of the Contractor's intent to begin work at least 5 days before work is begun. The Contractor shall cooperate with local authorities relative to handling traffic through the area and shall make all arrangements relative to keeping the working area clear of parked vehicles.

Whenever vehicles or equipment are parked on the shoulder within 1.8 m of a traffic lane, the shoulder area shall be closed as shown on the plans.

Lanes shall be closed only during the hours shown on the charts included in this section "Maintaining Traffic." Except work required under Sections 7-1.08 and 7-1.09, work that interferes with public traffic shall be performed only during the hours shown for lane closures.

Designated legal holidays are: January 1st, the third Monday in February, the last Monday in May, July 4th, the first Monday in September, November 11th, Thanksgiving Day, and December 25th. When a designated legal holiday falls on a Sunday, the following Monday shall be a designated legal holiday. When November 11th falls on a Saturday, the preceding Friday shall be a designated legal holiday.

Minor deviations from the requirements of this section concerning hours of work which do not significantly change the cost of the work may be permitted upon the written request of the Contractor if in the opinion of the Engineer, public traffic will be better served and the work expedited. These deviations shall not be adopted by the Contractor until the Engineer has approved them in writing. All other modifications will be made by contract change order.

Precast concrete members shall not be cast, assembled or stored within the right of way of Route 92 or within the median or within 6 meters of the edge of pavement of Industrial Boulevard Undercrossing, (Bridge No. 33-0370). During work on the members, no men, equipment or materials shall occupy any area within 1.2 m of the edge of any existing pavement except as permitted during lane or shoulder closures.

Erection of girders over Route 92 or Industrial Boulevard shall be undertaken on one span at a time. During girder erection, public traffic on Industrial Boulevard in the lanes over and directly adjacent to which girders are being placed shall be routed around the work area by means of a local detour as shown on the plans.Erection and removal of falsework at locations where falsework openings are required shall be undertaken at one location at a time. During falsework erection and removal, public traffic in the lanes over which falsework is being erected or removed shall be routed around the work area on adjacent streets or where 2 falsework openings are called for at one location, the public traffic may be routed by means of a local detour as shown on the plans. Removal shall include lowering falsework, blowing sand from sand jacks, turning screws on screw jacks and removing wedges.

Regardless of the construction procedure, methods and equipment selected, the Contractor shall have all necessary materials and equipment on the site to erect or remove the girders or falsework in any one span prior to detouring public traffic, and shall erect or remove the girders or falsework in an expeditious manner in order that inconvenience to public traffic will be a minimum.

Chart No. 1
Multilane Lane Requirements

Location: Eastbound Rte 92 - From Toll Plaza to Rte 880/Rte 92 Connector

	a.m.												p.m.												
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■	■														■	■	■	■	■	
Fridays	■	■	■	■	■	■														■	■	■	■	■	
Saturdays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Day before designated legal holiday	■	■	■	■	■	■														■	■	■	■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	

Legend:

■ One lane open in direction of travel

■ Two adjacent lanes open in direction of travel

□ No lane closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS:

Chart No. 2
Multilane Lane Requirements

Location: Westbound Rte 92 - At Toll Plaza

FROM HOUR TO HOUR	a.m. 12	1	2	3	4	5	6	7	8	9	10	11	p.m. 12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	2	2	2	2	2	4				6	5	5	5	5	5					3	3	3	3	2	
Fridays	2	2	2	2	2	4				5	5	5	5	5	5					4	3	3	3	3	
Saturdays	2	2	2	1	1	2	2	2	4	4	4	6	6	6	6	6	6	5	5	4	4	3	3	3	
Sundays	2	2	2	2	2	2	2	2	2	3	4	5	5	5	5	4	4	4	4	4	4	3	3	2	
Day before designated legal holiday	2	2	2	2	2	4				5	5	5	5	5	5					4	3	3	3	3	
Designated legal holidays	2	2	2	2	2	2	2	2	2	3	4	5	5	5	5	4	4	4	4	4	4	3	3	2	

Legend:

[#] Number of toll-booths to be provided

[ ] No toll-booth closure permitted

REMARKS:
HOV lane shall remain open during weekdays from 5:00 AM to 10:00 AM, and from 3:00 PM to 6:00 PM

Chart No. 3
Multilane Lane Requirements

Location: Westbound Rte 92 - From Toll Plaza east approach to Hesperian Blvd off-ramp

FROM HOUR TO HOUR	a.m. 12	1	2	3	4	5	6	7	8	9	10	11	p.m. 12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays																									
Fridays																									
Saturdays																									
Sundays																									
Day before designated legal holiday																									
Designated legal holidays																									

Legend:

[ ] One lane open in direction of travel

[ ] Two adjacent lanes open in direction of travel

[ ] No lane closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS:
HOV lane shall remain open during weekdays from 5:00 AM to 10:00 AM, and from 3:00 PM to 6:00 PM

Chart No. 4
Multilane Lane Requirements

Location: Westbound Rte 92 - From Hesperian Blvd off-ramp to SB Rte 880/WB Rte 92 connector

	a.m.												p.m.											
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays																									
Fridays																									
Saturdays																									
Sundays																									
Day before designated legal holiday																									
Designated legal holidays																									

Legend:

One lane open in direction of travel

Two adjacent lanes open in direction of travel

No lane closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS:

Chart No. 5
Ramp Lane Requirements

Location: Eastbound Rte 92 - On Clawiter Rd off-ramp

	a.m.												p.m.											
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays																									
Fridays																									
Saturdays																									
Sundays																									
Day before designated legal holiday																									
Designated legal holidays																									

Legend:

Ramp may be closed

No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS:

Chart No. 6
Ramp Lane Requirements

Location: Eastbound Rte 92 - On Clawiter Rd on-ramp

	a.m.												p.m.												
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■	■															■	■	■	■	■
Fridays	■	■	■	■	■	■															■	■	■	■	■
Saturdays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■
Day before designated legal holiday	■	■	■	■	■	■															■	■	■	■	■
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■

Legend:

■ Ramp may be closed

□ No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS: See detour plan #1

Chart No. 7
Ramp Lane Requirements

Location: Eastbound Rte 92 - On Industrial Blvd off-ramp

	a.m.												p.m.												
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■	■				■	■	■	■	■	■						■	■	■	■	■
Fridays	■	■	■	■	■	■				■	■	■									■	■	■	■	■
Saturdays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■
Day before designated legal holiday	■	■	■	■	■	■				■	■	■									■	■	■	■	■
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■

Legend:

■ Ramp may be closed

□ No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS:

Chart No. 8
Ramp Lane Requirements

Location: Eastbound Rte 92 - On Industrial Blvd on-ramp

	a.m.												p.m.											
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■															■	■	■	■	■	
Fridays	■	■	■	■	■															■	■	■	■	■	
Saturdays	■	■	■	■	■	■	■	■	■	■								■	■	■	■	■	■	■	
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Day before designated legal holiday	■	■	■	■	■															■	■	■	■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	

Legend:

■ Ramp may be closed

□ No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS: See detour plan #2

Chart No. 9
Ramp Lane Requirements

Location: Westbound Rte 92 - On Hesperian Blvd off-ramp

	a.m.												p.m.											
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■	■														■	■	■	■	■	
Fridays	■	■	■	■	■	■														■	■	■	■	■	
Saturdays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Day before designated legal holiday	■	■	■	■	■	■														■	■	■	■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	

Legend:

■ Ramp may be closed

□ No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS:

Chart No. 10
Ramp Lane Requirements

Location: Eastbound Rte 92 - On Hesperian Blvd on-ramp

	a.m.												p.m.												
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■																	■	■	■	
Fridays	■	■	■	■	■																	■	■	■	
Saturdays	■	■	■	■	■	■															■	■	■	■	
Sundays	■	■	■	■	■	■	■															■	■	■	
Day before designated legal holiday	■	■	■	■	■																	■	■	■	
Designated legal holidays	■	■	■	■	■	■	■														■	■	■	■	

Legend:

■ Ramp may be closed

□ No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS: See detour plan #3

Chart No. 11
Ramp Lane Requirements

Location: Westbound Rte 92 - On Clawiter Rd on- ramp

	a.m.												p.m.												
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■	■													■	■	■	■	■	■	
Fridays	■	■	■	■	■	■													■	■	■	■	■	■	
Saturdays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Day before designated legal holiday	■	■	■	■	■	■													■	■	■	■	■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	

Legend:

■ Ramp may be closed

□ No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS: See detour plan #4

Chart No. 12
Ramp Lane Requirements

Location: Westbound Rte 92 - On Clawiter Rd off-ramp

	a.m.												p.m.											
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■														■	■	■	■	■	■	
Fridays	■	■	■	■	■														■	■	■	■	■	■	
Saturdays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Day before designated legal holiday	■	■	■	■	■														■	■	■	■	■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	

Legend:

■ Ramp may be closed

□ No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS:

Chart No. 13
Ramp Lane Requirements

Location: Westbound on Rte 92 - On Industrial Blvd on-ramp

	a.m.												p.m.											
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■															■	■	■	■	■	
Fridays	■	■	■	■	■															■	■	■	■	■	
Saturdays	■	■	■	■	■	■	■	■	■									■	■	■	■	■	■	■	
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Day before designated legal holiday	■	■	■	■	■															■	■	■	■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	

Legend:

■ Ramp may be closed

□ No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS: See detour plan #5

Chart No. 14
Ramp Lane Requirements

Location: Westbound Rte 92 - On Industrial Blvd off-ramp

	a.m.												p.m.											
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays																									
Fridays																									
Saturdays																									
Sundays																									
Day before designated legal holiday																									
Designated legal holidays																									

Legend:

Ramp may be closed

No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS:

Chart No. 15
Ramp Lane Requirements

Location: Westbound Rte 92 - On Hesperian Blvd on-ramp

	a.m.												p.m.											
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays																									
Fridays																									
Saturdays																									
Sundays																									
Day before designated legal holiday																									
Designated legal holidays																									

Legend:

Ramp may be closed

No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS: See detour plan #6

Chart No. 16
Ramp Lane Requirements

Location: Westbound Rte 92 - On Hesperian Blvd off-ramp

	a.m.												p.m.												
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■	■																■	■	■	
Fridays	■	■	■	■	■	■																■	■	■	
Saturdays	■	■	■	■	■	■																■	■	■	
Sundays	■	■	■	■	■	■	■	■	■													■	■	■	
Day before designated legal holiday	■	■	■	■	■	■																■	■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■													■	■	■	

Legend:

■ Ramp may be closed

□ No ramp closure, shoulder closure, or work that interferes with public traffic allowed

REMARKS:

Chart No. 17
Industrial Blvd Lane Requirements

Location: On Industrial Boulevard, Northbound and Southbound

	a.m.												p.m.												
FROM HOUR TO HOUR	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■	■																	■	■	
Fridays																							■	■	
Saturdays	■	■	■	■	■	■	■	■	■	■											■	■	■	■	
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Day before designated legal holiday																							■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	

Legend:

■ Roadway may be closed completely in both directions. Traffic to be detoured as shown in the detour plans.

□ No lane closure or work that interferes with public traffic will be allowed

REMARKS: This lane closure chart for bridge demolition, erection of precast concrete girders, and concrete bridge pours for Industrial Blvd UC (Br. No.33-0370) only.

Chart No. 18
Southbound Industrial Boulevard Lane Requirements

Location:

FROM HOUR TO HOUR	a.m. 12	1	2	3	4	5	6	7	8	9	10	11	p.m. 12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■	■	■	■	■	■	■	■	□	□	□	□	□	□	□	□	■	■	■	■	
Fridays	■	■	■	■	■	■	■	■	■	■	■	■	□	□	□	□	□	□	□	□	■	■	■	■	
Saturdays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Day before designated legal holiday	■	■	■	■	■	■	■	■	■	■	■	■	□	□	□	□	□	□	□	□	■	■	■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	

Legend:

■ Roadway may be completely closed in both directions. Traffic to be detoured as shown in the Detour Plans.
At Contractor's option, Traffic may also be regulated using one-way traffic control per standard plan T-13. Minimum width of traffic lane for one-way control is 3.6 m.

■ A minimum of one paved traffic lane, not less than 3.6 m wide, shall be open for use by public traffic in each direction of travel per standard plan T-12.

□ No lane closure or work that interferes with public traffic will be allowed

REMARKS: This lane closure chart for bridge demolition, erection of precast concrete girders, and concrete pours for Industrial Boulevard UC (Br. No. 33-0370) only.

Chart No. 19
Northbound Industrial Boulevard Lane Requirements

Location:

FROM HOUR TO HOUR	a.m. 12	1	2	3	4	5	6	7	8	9	10	11	p.m. 12	1	2	3	4	5	6	7	8	9	10	11	12
Mondays through Thursdays	■	■	■	■	■	■	□	□	□	■	■	■	■	■	■	□	□	□	□	□	■	■	■	■	
Fridays	■	■	■	■	■	■	□	□	□	■	■	■	■	■	■	□	□	□	□	□	■	■	■	■	
Saturdays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Sundays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	
Day before designated legal holiday	■	■	■	■	■	■	□	□	□	■	■	■	■	■	■	□	□	□	□	□	■	■	■	■	
Designated legal holidays	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	■	

Legend:

■ Roadway may be completely closed in both directions. Traffic to be detoured as shown in the Detour Plans.
At Contractor's option, Traffic may also be regulated using one-way traffic control per standard plan T-13. Minimum width of traffic lane for one-way control is 3.6 m.

■ A minimum of one paved traffic lane, not less than 3.6 m wide, shall be open for use by public traffic in each direction of travel per standard plan T-12.

□ No lane closure or work that interferes with public traffic will be allowed

REMARKS: This lane closure chart for bridge demolition, erection of precast concrete girders, and concrete pours for Industrial Boulevard UC (Br. No. 33-0370) only.

10-1.22 CLOSURE REQUIREMENTS AND CONDITIONS

Lane closures shall conform to the provisions in "Maintaining Traffic" of these special provisions and these special provisions.

The term closure, as used herein, is defined as the closure of a traffic lane or lanes, including ramp or connector lanes, within a single traffic control system.

CLOSURE SCHEDULE

By Noon Monday, the Contractor shall submit a written schedule of planned closures for the next week period, defined as Friday Noon through the following Friday Noon.

The Closure Schedule shall show the locations and times when the proposed closures are to be in effect. The Contractor shall use closure schedule request forms furnished by the Engineer for this purpose. Closure schedules submitted with incomplete, unintelligible or inaccurate information will be returned for correction. The Contractor will be notified of disapproved closures or closures that will require coordination with other parties as a condition of approval.

Amendments to the Closure Schedule, including additional closures, shall be submitted to the Engineer, in writing, at least 3 working days in advance of any planned closure. Approval of amendments to the Closure Schedule will be at the discretion of the Engineer.

The Contractor shall confirm, in writing, all scheduled closures by no later than 8:00 a.m. 3 working days prior to the date on which the closure is to be made. Approval or denial of scheduled closures will be made by no later than 4:00 p.m. 2 working days prior to the date on which the closure is to be made. Closures not confirmed or approved will not be allowed.

Confirmed closures that are cancelled due to unsuitable weather may be rescheduled at the discretion of the Engineer for the next working day.

CONTINGENCY PLAN

The Contractor shall prepare a contingency plan for reopening closures to public traffic. The Contractor shall submit the contingency plan for a given operation to the Engineer within one working day of the Engineer's request.

LATE REOPENING OF CLOSURES

If a closure is not reopened to public traffic by the specified time, work shall be suspended in conformance with the provisions in Section 8-1.05, "Temporary Suspension of Work," of the Standard Specifications. The Contractor shall not make any further closures until the Engineer has accepted a work plan, submitted by the Contractor, that will insure that future closures will be reopened to public traffic at the specified time. The Engineer will have 2 working days to accept or reject the Contractor's proposed work plan. The Contractor will not be entitled to any compensation for the suspension of work resulting from the late reopening of closures.

For each 10-minute interval, or fraction thereof past the time specified to reopen the closure, the Department will deduct $2,700 per interval from moneys due or that may become due the Contractor under the contract.

COMPENSATION

The Contractor shall notify the Engineer of any delay in the Contractor's operations due to the following conditions, and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of those conditions, and the Contractor's loss due to that delay could not have been avoided by rescheduling the affected closure or by judicious handling of forces, equipment and plant, the delay will be considered a right of way delay within the meaning of Section 8-1.09, "Right of Way Delays," and compensation for the delay will be determined in conformance with the provisions in Section 8-1.09:

A. The Contractor's proposed Closure Schedule is denied and his planned closures are within the time frame allowed for closures in "Maintaining Traffic" of these special provisions, except that the Contractor will not be entitled to any compensation for amendments to the Closure Schedule that are not approved.
B. The Contractor is denied a confirmed closure.

Should the Engineer direct the Contractor to remove a closure prior to the time designated in the approved Closure Schedule, any delay to the Contractor's schedule due to removal of the closure will be considered a right of way delay within the meaning of Section 8-1.09, "Right of Way Delays," and compensation for the delay will be determined in conformance with the provisions in Section 8-1.09.

10-1.23 TRAFFIC CONTROL SYSTEM FOR LANE CLOSURE

A traffic control system shall consist of closing traffic lanes and ramps in accordance with the details shown on the plans, the provisions of Section 12, "Construction Area Traffic Control Devices," of the Standard Specifications, the provisions under "Maintaining Traffic" and "Construction Area Signs" elsewhere in these special provisions and these special provisions.

The provisions in this section will not relieve the Contractor from the responsibility to provide additional devices or take the measures as may be necessary to comply with the provisions in Section 7-1.09, "Public Safety," of the Standard Specifications.

During traffic stripe operations and pavement marker placement operations using bituminous adhesive, traffic shall be controlled, at the option of the Contractor, with either stationary or moving type lane closures. During all other operations traffic shall be controlled with stationary type lane closures. The Contractor's attention is directed to the provisions in Section 84-1.04, "Protection From Damage," and Section 85-1.06, "Placement," of the Standard Specifications.

If any component in the traffic control system is displaced, or ceases to operate or function as specified, from any cause, during the progress of the work, the Contractor shall immediately repair the component to its original condition or replace the component and shall restore the component to its original location.

STATIONARY TYPE LANE CLOSURE.—When lane and ramp closures are made for work periods only, at the end of each work period, all components of the traffic control system, except portable delineators placed along open trenches or excavation adjacent to the traveled way, shall be removed from the traveled way and shoulder. If the Contractor so elects, the components may be stored at selected central locations, approved by the Engineer, within the limits of the highway right of way.

Each vehicle used to place, maintain and remove components of a traffic control system on multilane highways shall be equipped with a Type II flashing arrow sign which shall be in operation when the vehicle is being used for placing, maintaining, or removing the components. Vehicles equipped with Type II flashing arrow sign not involved in placing, maintaining, or removing the components when operated within a stationary type lane closure shall only display the caution display mode. The sign shall be controllable by the operator of the vehicle while the vehicle is in motion. The flashing arrow sign shown on the plans shall not be used on the vehicles which are doing the placing, maintaining and removing of components of a traffic control system, and shall be in place before a lane closure requiring its use is completed.

MOVING TYPE LANE CLOSURE.—Flashing arrow signs used in moving lane closures shall be truck-mounted. Changeable message signs used in moving lane closure operations shall conform to Section 12-3.12, "Portable Changeable Message Signs," of the Standard Specifications, except the signs shall be truck-mounted and the full operation height of the bottom of the sign may be less than 2.1 m above the ground, but should be as high as practicable.

Truck-mounted crash cushions (TMCC) for use in moving lane closures shall be any of the following approved models, or equal:

Hexfoam TMA Series 3000, Alpha 1000 TMA Series 1000, Alpha 2001 TMA Series 2001, manufactured by Energy Absorption Systems, Inc., One East Wacker Drive, Chicago, IL 60601-2076, Telephone (312) 467-6750.

Distributor(Northern): Traffic Control Service, Inc., 8585 Thys Court, Sacramento, CA 95828, Telephone 1-800-884-8274, FAX (916) 387-9734.

Distributor(Southern): Traffic Control Service, Inc., 1881 Betmor Lane, Anaheim, CA 92805, Telephone 1-800-222-8274.

Cal T-001 Model 2 or Model 3, manufacturer and distributor; Hexcel Corporation, 11711 Dublin Boulevard, P.O. Box 2312, Dublin, CA 94568, Telephone (510) 828-4200.

Renco Rengard Model Nos. CAM 8-815 and RAM 8-815, manufacturer and distributor, Renco Inc., 1582 Pflugerville Loop Road, P.O. Box 730, Pflugerville, TX 78660-0730, Telephone 1-800-654-8182.

Each TMCC shall be individually identified with the manufacturer's name, address, TMCC model number, and a specific serial number. The names and numbers shall each be a minimum 13 mm high, and located on the left (street) side at the lower front corner. The TMCC shall have a message next to the name and model number in 13 mm high letters which states, "The bottom of this TMCC shall be _____ mm ±_____ mm above the ground at all points for proper impact performance." Any TMCC which is damaged or appears to be in poor condition shall not be used unless recertified by the manufacturer. The Engineer shall be the sole judge as to whether used TMCCs supplied under this contract need recertification. Each unit shall be certified by the manufacturer to meet the requirements for TMCCs in accordance with the standards established by the Transportation Laboratory Structures Research Section.

Approvals for new TMCC designs proposed as equal to the above approved models shall be in accordance with the procedures (including crash testing) established by the Transportation Laboratory Structures Research Section. For information regarding submittal of new designs for evaluation contact: Transportation Laboratory, Structures Research Section, P.O. Box 19128, 5900 Folsom Boulevard, Sacramento, CA 95819.

New TMCCs proposed as equal to approved TMCCs or approved TMCCs determined by the Engineer to need recertification shall not be used until approved or recertified by the Transportation Laboratory Structures Research Section.

PAYMENT.—The contract lump sum price paid for traffic control system shall include full compensation for furnishing all labor, materials (including signs), tools, equipment and incidentals, and for doing all the work involved in placing, removing, storing, maintaining, moving to new locations, replacing and disposing of the components of the traffic control system as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the

Engineer.The adjustment provisions in Section 4-1.03, "Changes," of the Standard Specifications, shall not apply to the item of traffic control system. Adjustments in compensation for traffic control system will be made only for increased or decreased traffic control system required by changes ordered by the Engineer and will be made on the basis of the cost of the increased or decreased traffic control necessary. The adjustment will be made on a force account basis as provided in Section 9-1.03, "Force Account Payment," of the Standard Specifications for increased work, and estimated on the same basis in the case of decreased work.

Traffic control system required by work which is classed as extra work, as provided in Section 4-1.03D of the Standard Specifications, will be paid for as a part of the extra work.

10-1.24 TEMPORARY PAVEMENT DELINEATION

Temporary pavement delineation shall be furnished, placed, maintained, and removed in conformance with the provisions in Section 12-3.01, "General," of the Standard Specifications and these special provisions. Nothing in these special provisions shall be construed as reducing the minimum standards specified in the Manual of Traffic Controls published by the Department or as relieving the Contractor from the responsibilities specified in Section 7-1.09, "Public Safety," of the Standard Specifications.

GENERAL

Whenever the work causes obliteration of pavement delineation, temporary or permanent pavement delineation shall be in place prior to opening the traveled way to public traffic. Laneline or centerline pavement delineation shall be provided at all times for traveled ways open to public traffic. On multilane roadways (freeways and expressways) edgeline delineation shall be provided at all times for traveled ways open to public traffic.

The Contractor shall perform the work necessary to establish the alignment of temporary pavement delineation, including required lines or marks. Surfaces to receive temporary pavement delineation shall be dry and free of dirt and loose material. Temporary pavement delineation shall not be applied over existing pavement delineation or other temporary pavement delineation. Temporary pavement delineation shall be maintained until superseded or replaced with a new pattern of temporary pavement delineation or permanent pavement delineation.

Temporary pavement markers, including underlying adhesive, and removable traffic tape which is applied to the final layer of surfacing or existing pavement to remain in place or which conflicts with a subsequent or new traffic pattern for the area shall be removed when no longer required for the direction of public traffic, as determined by the Engineer.

TEMPORARY LANELINE AND CENTERLINE DELINEATION

Whenever lanelines or centerlines are obliterated and temporary pavement delineation to replace the lines is not shown on the plans, the minimum laneline and centerline delineation to be provided for that area shall be temporary reflective pavement markers placed at longitudinal intervals of not more than 7.3 m. The temporary reflective pavement markers shall be the same color as the laneline or centerline the pavement markers replace. Temporary reflective pavement markers shall be, at the option of the Contractor, one of the temporary pavement markers listed for short term day/night use (14 days or less) or long term day/night use (6 months or less) in "Approved Traffic Products" of these special provisions.

Temporary reflective pavement markers shall be placed in conformance with the manufacturer's instructions and shall be cemented to the surfacing with the adhesive recommended by the manufacturer, except epoxy adhesive shall not be used to place pavement markers in areas where removal of the pavement markers will be required.

Temporary laneline or centerline delineation consisting entirely of temporary reflective pavement markers placed on longitudinal intervals of not more than 7.3 m shall be used on lanes opened to public traffic for a maximum of 14 days. Prior to the end of the 14 days the permanent pavement delineation shall be placed. If the permanent pavement delineation is not placed within the 14 days, the Contractor shall provide additional temporary pavement delineation and shall bear the cost thereof. The additional temporary pavement delineation to be provided shall be equivalent to the pattern specified for the permanent pavement delineation for the area, as determined by the Engineer.

Full compensation for furnishing, placing, maintaining and removing the temporary reflective pavement markers for those areas where temporary laneline and centerline delineation is not shown on the plans and for providing equivalent patterns of permanent traffic lines for those areas when required, shall be considered as included in the contract prices paid for the items of work that obliterated the laneline and centerline pavement delineation and no separate payment will be made therefor.

TEMPORARY EDGELINE DELINEATION

On multilane roadways (freeways and expressways) whenever edgelines are obliterated and temporary pavement delineation to replace those edgelines is not shown on the plans, the edgeline delineation to be provided for those areas adjacent to lanes open to public traffic shall be as follows:

Temporary pavement delineation for right edgelines shall, at the option of the Contractor, consist of either a solid 100-mm wide traffic stripe of the same color as the stripe the temporary edgeline delineation replaces, or traffic cones, portable delineators or channelizers placed at longitudinal intervals not to exceed 30 m.

Temporary pavement delineation for left edgelines shall, at the option of the Contractor, consist of either solid 100 mm wide traffic stripe of the same color as the stripe the temporary edgeline delineation replaces, or traffic cones, portable delineators or channelizers placed at longitudinal intervals not to exceed 30 m; or temporary reflective pavement markers placed at longitudinal intervals of not more than 1.8 m. Temporary pavement markers used for temporary left edgeline delineation shall be one of the types of temporary pavement markers listed for short term day/night use (14 days or less) or long term day/night use (6 months or less) in "Approved Traffic Products" of these special provisions.

100-mm wide traffic stripe placed for temporary edgeline delineation which will require removal shall conform to the provisions of "Temporary Traffic Stripe (Tape)" of these special provisions. Where removal of the 100-mm wide traffic stripe will not be required, painted traffic stripe conforming to the provisions of "Temporary Traffic Stripe (Paint)" of these special provisions may be used. The quantity of temporary traffic stripe (tape) or temporary traffic stripe (paint) used for this temporary edgeline delineation will not be included in the quantities of tape or paint to be paid for.

The lateral offset for traffic cones, portable delineators or channelizers used for temporary edgeline delineation shall be as determined by the Engineer. If traffic cones or portable delineators are used as temporary pavement delineation for edgelines, the Contractor shall provide personnel to remain at the job site to maintain the cones or delineators during the hours of the day that the portable delineators are in use.

Channelizers used for temporary edgeline delineation shall be the surface mounted type and shall be orange in color. Channelizer bases shall be cemented to the pavement in the same manner provided for cementing pavement markers to pavement in the section of these special provisions entitled "Pavement Markers," except epoxy adhesive shall not be used to place channelizers on the top layer of pavement. Channelizers shall be, at the Contractor's option, one of the surface mount types (900 mm) listed in "Approved Traffic Products" of these special provisions.

Temporary edgeline delineation shall be removed when no longer required for the direction of public traffic as determined by the Engineer.

Full compensation for furnishing, placing, maintaining, and removing temporary edgeline delineation, including underlying adhesive, for those areas where temporary edgeline delineation is not shown on the plans shall be considered as included in the contract prices paid for the items of work that obliterated the edgeline pavement delineation and no separate payment will be made therefor.

TEMPORARY TRAFFIC STRIPE (PAINT)

Temporary traffic stripe consisting of painted traffic stripe shall be applied and maintained at the locations shown on the plans. The painted temporary traffic stripe shall be complete in place at the location shown, prior to opening the traveled way to public traffic. Removal of painted temporary traffic stripe will not be required.

Temporary painted traffic stripe shall conform to "Paint Traffic Stripes and Pavement Markings" of these special provisions, except for payment and the number of coats shall be, at the option of the Contractor, either one or 2 coats regardless of whether on new or existing pavement.

At the Contractor's option, temporary removable striping tape listed in "Approved Traffic Products" of these special provisions may be used instead of painted temporary traffic stripes. When traffic stripe tape is used in place of painted temporary traffic stripes, the tape will be measured and paid for as temporary traffic stripe (paint).

When painted traffic stripe is specified for temporary left edgeline delineation, temporary reflective pavement markers placed at longitudinal intervals of not more than 1.8 m may be used in place of the temporary painted traffic stripe. Temporary reflective pavement markers shall be one of the types of temporary pavement markers listed for long term day/night use (6 months or less) in "Approved Traffic Products" of these special provisions. When temporary reflective pavement markers are used in place of temporary painted traffic stripe, payment for those temporary pavement markers will be made on the basis of the theoretical quantity of temporary traffic stripe (paint), required for the left edgeline the temporary pavement markers replace.

TEMPORARY PAVEMENT MARKING (PAINT)

Temporary pavement marking consisting of painted pavement marking shall be applied and maintained at the locations shown on the plans. The painted temporary pavement marking shall be complete in place at the location shown, prior to opening the traveled way to public traffic. Removal of painted temporary pavement marking will not be required.

Temporary painted pavement marking shall conform to "Paint Traffic Stripes and Pavement Markings" of these special provisions, except for payment and the number of coats shall be, at the option of the Contractor, either one or two coats regardless whether on new or existing pavement.

At the Contractor's option, temporary removable pavement marking tape or permanent pavement marking tape listed in "Approved Traffic Products" of these special provisions may be used instead of painted temporary pavement markings. When pavement marking tape is used, regardless of which type of tape is placed, the tape will be measured and paid for as temporary pavement marking (paint).

TEMPORARY PAVEMENT MARKERS

Temporary pavement markers shall be applied at the locations shown on the plans. The pavement markers shall be applied complete in place at the location shown, prior to opening the traveled way to public traffic.

Temporary pavement markers shown on the plans shall be, at the option of the Contractor, one of the temporary reflective pavement markers for long term day/night use (6 months or less) listed in "Approved Traffic Products" of these special provisions.

Temporary pavement markers shall be placed in conformance with the manufacturer's instructions and shall be cemented to the surfacing with the adhesive recommended by the manufacturer, except epoxy adhesive shall not be used in areas where removal of the pavement markers will be required.

Where the temporary pavement delineation shown on the plans for lanelines or centerlines consists entirely of a pattern of broken traffic stripe and pavement markers, the Contractor may use groups of the temporary reflective pavement markers for long term day/night use (6 months or less) listed in "Approved Traffic Products" of these special provisions, in place of the temporary traffic stripe tape or painted temporary traffic stripe. The groups of pavement markers shall be spaced as shown on the plans for a similar pattern of permanent traffic line, except pavement markers shown to be placed in the gap between the broken traffic stripe shall be placed as part of the group to delineate the pattern of broken temporary traffic stripe. The kind of laneline and centerline delineation selected by the Contractor shall be continuous within a given location. Payment for temporary pavement markers used in place of temporary traffic stripe will be made on the basis of the theoretical quantities of temporary traffic stripe (tape), temporary traffic stripe (paint) and temporary pavement markers required for the pattern the pavement markers replace.

Reflective pavement markers conforming to the provisions of "Pavement Markers" of these special provisions may be used in place of temporary reflective pavement markers for long term day/night use (6 months or less) except to simulate patterns of broken traffic stripe. Placement of the reflective pavement markers used for temporary pavement markers shall conform to "Pavement Markers" of these special provisions except the waiting period provisions before placing the pavement markers on new asphalt concrete surfacing as specified in Section 85-1.06, "Placement," of the Standard Specifications shall not apply and epoxy adhesive shall not be used to place pavement markers in areas where removal of the pavement markers will be required.

MEASUREMENT AND PAYMENT

Temporary traffic stripe (paint) and temporary pavement marking (paint) will be measured and paid for in the same manner as specified for paint traffic stripe (1-coat) and paint pavement marking (1-coat) specified in Section 84-3.06, "Measurement," and Section 84-3.07, "Payment," of the Standard Specifications. 200 mm temporary traffic stripe (paint) will be paid for at double the quantity of the same length of 100 mm temporary traffic stripe (paint).

Temporary pavement markers, shown on the plans, will be measured and paid for as units in the same manner specified for reflective pavement markers as provided in Section 85-1.08, "Measurement," and Section 85-1.09, "Payment," of the Standard Specifications. Temporary pavement markers, used for temporary laneline and centerline delineation for areas which are not shown on the plans will not be included in the quantities of temporary pavement markers to be paid for. Full compensation for removing temporary pavement markers, when no longer required, shall be considered as included in the contract unit price paid for temporary pavement marker and no separate payment will be made therefor.

10-1.25 PORTABLE CHANGEABLE MESSAGE SIGN

Portable changeable message signs shall be furnished, placed, operated, and maintained at locations directed by the Engineer and shall conform to the provisions of Section 12, "Construction Area Traffic Control Devices," of the Standard Specifications and these special provisions.

Portable Changeable Message Signs furnished, placed, operated, and maintained at locations directed by the Engineer shall be paid for as extra work in accordance with Section 4-1.03D of the Standard Specifications.

10-1.26 TEMPORARY RAILING

Temporary railing (Type K) shall be placed as shown on the plans, specified in the Standard Specifications or in these special provisions or ordered by the Engineer, and shall conform to the provisions in Section 12, "Construction Area Traffic Control Devices," of the Standard Specifications and these special provisions.

Attention is directed to "Salvage Concrete Barrier (Type K)" elsewhere in these special provisions. The existing concrete barrier (Type K) to be salvaged can be used at the Contractor's option as temporary railing (Type K). If the existing concrete barrier (Type K) to be salvaged is used as temporary railing (Type K), it must first be repaired and brought to an acceptable condition (repair all spalls over 25mm in width, smooth finish to original shape within +/- 3 mm tolerance) and be freshly coated prior to their first use as specified in Section 12-3.08 of the Standard Specifications. Existing concrete barrier (Type K) that is not used as temporary railing (Type K), shall be salvaged per "Salvage Concrete Barrier (Type K)" elsewhere in these special provisions. If the existing concrete barrier (Type K) to be salvaged is used as temporary railing (Type K), there shall be no deductions from either salvage concrete barrier (Type K) or temporary railing (Type K) pay quantities. Quantities of salvage concrete barrier (Type K) used as temporary railing (Type K) will be paid for as both salvage concrete barrier (Type K) and temporary railing (Type K).

The fourth paragraph of Section 12-4.01, "Measurement and Payment," of the Standard Specifications is amended to read:

When the Engineer's Estimate includes a contract item for temporary railing (Type K), the temporary railing (Type K) will be measured by the meter along the top of the railing, at each location shown on the plans, specified, or ordered

by the Engineer. If the Engineer orders a lateral move of the temporary railing (Type K), and the repositioning is not shown on the plans, moving the temporary railing will be paid for as extra work as provided in Section 4-1.03D and the temporary railing will not be measured in the new position. Temporary railing (Type K) placed in excess of the length shown, specified, or ordered will not be paid for. The contract price paid per meter for temporary railing (Type K) shall include full compensation for furnishing all labor, materials (including reinforcement and Type P marker panels), tools, equipment and incidentals, and for doing all the work involved in furnishing, placing, maintaining, repairing, replacing, and removing the temporary railing, including excavation and backfill, including all costs for using existing concrete barrier (Type K) to be salvaged as temporary railing (Type K), including drilling holes and bonding threaded rods or dowels when required, removing threaded rods or dowels and filling the drilled holes with mortar, furnishing and installing reflectors, and moving and replacing removable panels as required, complete in place, as shown on the plans, as specified in these Specifications and the special provisions, and as directed by the Engineer.

Reflectors on temporary railing (Type K) shall conform to the provisions in "Approved Traffic Products" of these special provisions.

Temporary railing (Type K), conforming to the details shown on 1995 Standard Plan T3 or 1992 Standard Plan T3, may be used. Temporary railing (Type K) fabricated prior to January 1, 1993, and conforming to 1988 Standard Plan B11-30 may be used, provided the fabrication date is printed on the required Certificate of Compliance.

The Contractor's attention is directed to the provisions in "Public Safety" and "Order of Work" of these special provisions.

Temporary railing (Type K) placed in conformance with the provisions in "Public Safety" of these special provisions will be neither measured nor paid for.

10-1.27 PORTABLE DELINEATORS

Portable Delineators shall be surface mounted type and shall be furnished, placed, and maintained at the locations shown on the plans and shall conform to the provisions in Section 12, "Construction Area Traffic Control Devices," of the Standard Specifications and these special provisions.

Portable Delineators shall conform to the provisions in "Approved Traffic Products" of these special provisions.

When no longer required for the work as determined by the Engineer, portable delineators and underlying adhesive used to cement the portable delineator bases to the pavement shall be removed. Removed portable delineators and adhesive shall become the property of the Contractor and shall be removed from the site of work.

10-1.28 TEMPORARY CRASH CUSHION MODULE

This work shall consist of furnishing, installing and maintaining sand filled temporary crash cushion modules in groupings or arrays at each location shown on the plans, specified in the special provisions or directed by the Engineer. The grouping or array of sand filled modules shall form a complete sand filled temporary crash cushion in accordance with the details shown on the plans and these special provisions.

Attention is directed to "Public Safety" and "Temporary Railing" of these special provisions.

GENERAL

Whenever the work or the Contractor's operations establishes a fixed obstacle, the exposed fixed obstacle shall be protected with a sand filled temporary crash cushion. The sand filled temporary crash cushion shall be in place prior to opening the lanes adjacent to the fixed obstacle to public traffic.

Sand filled temporary crash cushions shall be maintained in place at each location, including times when work is not actively in progress. Sand filled temporary crash cushions may be removed during a work period for access to the work provided that the exposed fixed obstacle is 4.6 m or more from a lane carrying public traffic and the temporary crash cushion is reset to protect the obstacle prior to the end of the work period in which the fixed obstacle was exposed. When no longer required, as determined by the Engineer, sand filled temporary crash cushions shall be removed from the site of the work.

MATERIALS

At the Contractor's option, the modules for use in sand filled temporary crash cushions shall be either Energite III Inertial Modules, Fitch Inertial Modules manufactured after March 31, 1997, or equal:

Energite III Inertial Modules manufactured by Energy Absorption Systems, Inc., One East Wacker Drive, Chicago, IL 60601-2076, Telephone 1-312-467-6750, FAX 1-800-770-6755.

Distributor (Northern): Traffic Control Service, Inc., 8585 Thys Court, Sacramento, CA 95828, Telephone 1-800-884-8274, FAX 1-916-387-9734

Distributor (Southern): Traffic Control Service, Inc., 1881 Betmor Lane, Anaheim, CA 92805, Telephone 1-800-222-8274, FAX 1-714-937-1070.

Fitch Inertial Modules, national distributor; Roadway Safety Service, Inc., 1050 North Rand Road, Wauconda, IL 60084, Telephone 1-800-426-0839, FAX 1-847-487-9820.

Distributor: Singletree Sales Company, 1533 Berger Drive, San Jose, CA 95112, Telephone 1-800-822-7735, FAX 1-408-287-1929.

Modules contained in each temporary crash cushion shall be of the same type at each location. The color of the modules shall be the standard yellow color as furnished by the vendor, with black lids. The modules shall exhibit good workmanship free from structural flaws and objectionable surface defects. The modules need not be new. Good used undamaged modules conforming to color and quality of the types specified above may be utilized. If used Fitch modules requiring a seal are furnished, the top edge of the seal shall be securely fastened to the wall of the module by a continuous strip of heavy duty tape.

Modules shall be filled with sand in accordance with the manufacturer's directions, and to the sand capacity in kilograms for each module as shown on the plans. Sand for filling the modules shall be clean washed concrete sand of commercial quality. At the time of placing in the modules, the sand shall contain not more than 7 percent water, as determined by California Test 226.

Modules damaged due to the Contractor's operations shall be repaired immediately by the Contractor at the Contractor's expense. Modules damaged beyond repair, as determined by the Engineer, due to the Contractor's operations shall be removed and replaced by the Contractor at the Contractor's expense.

INSTALLATION

Temporary crash cushion modules shall be placed on movable pallets or frames conforming to the dimensions shown on the plans. The pallets or frames shall provide a full bearing base beneath the modules. The modules and supporting pallets or frames shall not be moved by sliding or skidding along the pavement or bridge deck.

A Type R or P marker panel shall be attached to the front of the crash cushion as shown on the plans, when the closest point of crash cushion array is within 3.6 m of the traveled way. The marker panel, when required, shall be firmly fastened to the crash cushion with commercial quality hardware or by other methods approved by the Engineer.

At the completion of the project, temporary crash cushion modules, sand filling, pallets or frames, and marker panels shall become the property of the Contractor and shall be removed from the site of the work. Temporary crash cushion modules shall not be installed in permanent work.

MEASUREMENT AND PAYMENT

Temporary crash cushion modules will be measured by the unit determined from the actual count of modules used in the work or ordered by the Engineer at each location. Temporary crash cushion modules placed in accordance with the provisions in "Public Safety" elsewhere in these special provisions and modules placed in excess of the number specified or shown will not be measured nor paid for.

Repairing modules damaged by public traffic will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications. Modules damaged beyond repair by public traffic, when ordered by the Engineer, shall be removed and replaced immediately by the Contractor. Modules replaced due to damage by public traffic will be measured and paid for as temporary crash cushion module.

If the Engineer orders a lateral move of sand filled temporary crash cushions and the repositioning is not shown on the plans, moving the sand filled temporary crash cushion will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications and these temporary crash cushion modules will not be counted for payment in the new position.

The contract unit price paid for temporary crash cushion module shall include full compensation for furnishing all labor, materials (including sand, pallets or frames and marker panels), tools, equipment and incidentals, and for doing all work involved in furnishing, installing, maintaining, moving and resetting during a work period for access to the work, and removing from the site of the work when no longer required (including those damaged by public traffic, and the complete cleanup of all damaged module components for same) the sand filled temporary crash cushion modules, complete in place, as shown on the plans, as specified in these special provisions and as directed by the Engineer.

10-1.29 TEMPORARY COVERED PEDESTRIAN WALKWAY

Attention is directed to "Reconstruct Chain Link Handrailing" shown on the plans, Section 7-1.06 "Safety and Health Provisions" of the Standard Specifications, and Section 15-2.05 "Reconstruction" of the Standard Specifications.

A temporary covered pedestrian walkway shall be provided at the east side of Industrial Boulevard adjacent to the existing sidewalk under Industrial Boulevard Undercrossing, as shown on the plans. The temporary covered pedestrian walkway shall be completely constructed within 5 calendar days after the removal of the existing chain link fence handrailing.

The existing chain link fence handrailing on the east side of Industrial Blvd adjacent to the existing sidewalk under Industrial Boulevard Undercrossing shall be removed and reconstructed between IB-Line station 5+70 to 6+35. The existing chain link fence handrailing shall be removed and the undamaged portions of the existing chain link fence handrailing shall then be stored for the duration the temporary covered pedestrian walkway is in use. The chain link fence handrailing shall then be reconstructed within 5 calendar days after the removal of the temporary covered pedestrian walkway.

New posts shall be furnished and used to reconstruct existing chain link fence handrailing. Posts from the removed existing chain link fence handrailing shall be disposed of. New posts shall conform to the provisions in Section 80, "Fences", of the Standard Specifications.

The Contractor shall submit to the Engineer, for approval, working drawings of the temporary covered walkway in accordance with the Provisions in Section 5-1.02, “Plans and Working Drawings,” of the Standard Specifications. The working drawings shall show all necessary details for the construction of the temporary covered walkway and shall include a detailed erection and removal plans of the temporary covered walkway. The working drawings for the temporary covered walkway shall be submitted at least two weeks in advance of the time the Contractor intends to begin the construction of the temporary covered walkway.

Hand railing shall be provided on each side of the temporary covered walkway as necessary to protect pedestrian traffic from hazards due to the Contractor's operations and vehicular traffic. The hand railing shall be constructed of wood and painted white.

Construction of the temporary covered walkway shall be completed to the satisfaction of the Engineer before the existing side walk or any portions thereof are closed.

The temporary covered pedestrian walkway shall be a minimum of 1.2 meters wide and 3 meters high.

The temporary covered pedestrian walkway shall be paved or provided with full width continuous wood walks and shall be kept clear of obstructions and maintained in good condition by the Contractor. The temporary covered walkway shall provide protection from falling objects and drips from the overhead structure. Overhead protection for pedestrians shall extend not less than 3.0 meters beyond the edge of the bridge deck of the widened structure.

The walkway shall be illuminated in accordance with the provisions in Section 86-6.11, “Falsework Lighting." The temporary covered walkway shall be designed for 24 –hour illumination.

Measurement and Payment

The temporary covered pedestrian walkway will be paid for on a lump sum basis.

The contract lump sum price paid for temporary covered pedestrian walkway shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, including reconstruction of the existing chain link handrailing, and for doing all the work involved in constructing, maintaining, and removing when no longer required, the temporary covered pedestrian walkway, complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Fence removed in excess of that required for reconstructing chain link handrailing shall be disposed of. Full compensation for removing and disposing of excess fence shall be considered as included in the lump sum price paid for temporary covered pedestrian walkway and no separate payment will be made therefor.

10-1.30 EXISTING HIGHWAY FACILITIES

The work performed in connection with various existing highway facilities shall conform to the provisions in Section 15, "Existing Highway Facilities," of the Standard Specifications and these special provisions.

Except as otherwise provided for damaged materials in Section 15-2.04, "Salvage," of the Standard Specifications, the materials to be salvaged shall remain the property of the State, and shall be cleaned, packaged, bundled, tagged, and hauled to the District recycle center at the San Francisco-Oakland Bay Bridge warehouse supply area adjacent to the San Francisco-Oakland Bay Bridge Toll Plaza and stockpiled.

The Contractor shall notify the Engineer and the District Recycle Coordinator, telephone (510) 286-6111 a minimum of 48 hours prior to hauling salvaged material to the Recycle Center.

The recycle center is open from 8:00 AM to Noon, and from 1:00 PM to 2:30 PM Mondays through Fridays except legal holidays.

Plans of the existing bridges may be requested by fax from the Office of Structure Maintenance and Investigations, 1801 30th Street, Sacramento, CA, Fax (916) 227-8357.

Plans of existing bridges available to the Contractor are reproductions of the original contract plans with significant changes noted and working drawings and do not necessarily show normal construction tolerances and variances. Where dimensions of new construction required by this contract are dependent on the dimensions of existing bridges, the Contractor shall verify the controlling field dimensions and shall be responsible for adjusting dimensions of the work to fit existing conditions.

10-1.30A ABANDON CULVERTS AND PIPE LINES

Existing culverts and utility pipe lines, where shown on the plans to be abandoned, shall be abandoned in place or, at the option of the Contractor, the culverts and pipe lines shall be removed and disposed of. All resulting openings into existing structures, that are to remain in place, shall be plugged with commercial quality concrete containing not less than 300 kg of cement per cubic meter.

Abandoning culverts and pipe lines in place shall conform to the following:

Culverts and pipe lines, that intersect the side slopes, shall be removed to a depth of not less than one meter measured normal to the plane of the finished side slope, before being abandoned.

Culverts and pipe lines, 600 mm in diameter and larger, shall be backfilled with sand by any method, acceptable to the Engineer, which completely fills the pipe. Sand backfill material shall be clean, free draining, and free from roots and other deleterious substances.

The ends of culverts and pipe lines shall be securely closed by a 150 mm thick tight fitting plug or wall of commercial quality concrete.

Culverts and pipe lines shall not be abandoned until their use is no longer required. The Contractor shall notify the Engineer in advance of any intended culvert or pipe abandonment.

Full compensation for plugs, pipe removal, structure excavation, and backfill shall be considered as included in the contract unit price paid for abandon culvert and pipe line, and no additional compensation will be allowed therefor.

10-1.30B ABANDON INLETS

Existing pipe and drainage inlets, where shown on the plans to be abandoned, shall be abandoned.

The top portion of the inlets shall be removed to a depth of 1 meter below finished grade. Concrete aprons, when present, shall be removed and disposed of.

Frames and grates shall be salvaged. Full compensation for salvaging existing frames and grates, and for removing and disposing of concrete aprons when present, shall be considered as included in the contract unit price paid for abandon inlet and no separate payment will be made therefor.

10-1.30C SALVAGE ROCK SLOPE PROTECTION

Existing rock slope protection , where shown on the plans to be salvaged, shall be removed and salvaged.

Salvaged rock slope protection shall be re-incorporated into the work as Rock Slope Protection (Backing No. 1, Method B) during the course of this contract.

Full compensation for salvaging rock slope protection, including initial removal, temporary storage, handling (outside of placement when re-incorporated into the work as Rock Slope Protection (Backing No. 1, Method B)), and all transportation (outside of placement when re-incorporated into the work as Rock Slope Protection (Backing No. 1, Method B)) shall be considered as included in the contract price paid per cubic meter for salvage rock slope protection and no separate payment will be made therefor.

10-1.30D SALVAGE CONCRETE BARRIER (TYPE K)

Existing concrete barrier (Type K), where shown on the plans to be salvaged, shall be removed and salvaged.

Attention is directed to "temporary railing (Type K)" elsewhere in these special provisions. The existing concrete barrier (Type K) to be salvaged can be used at the Contractor's option as temporary railing (Type K). Existing concrete barrier (Type K) to be salvaged not used as temporary railing (Type K) shall be salvaged. If the existing concrete barrier (Type K) to be salvaged is used as temporary railing (Type K), there shall be no deductions from either salvage concrete barrier (Type K) or temporary railing (Type K) pay quantities. Quantities of salvage concrete barrier (Type K) used as temporary railing (Type K) will be paid for as both salvage concrete barrier (Type K) and temporary railing (Type K).

Salvaged concrete barrier (Type K) shall be hauled to Alameda County, Route 880, KP 33.3, under the Route 880/238 Separation behind the "Target" Store at 15555 Hesperian Blvd., San Leandro, CA and neatly stockpiled.

10-1.30E REMOVE GUARD RAILING

Existing single and double metal beam guard railing, where shown on the plans to be removed, shall be removed and disposed of.

Existing concrete anchors shall be removed to a depth of not less than 0.3-m below subgrade or 0.3-m below finished grade, whichever is greater in depth. Full compensation for removing concrete anchors shall be considered as included in the contract price paid per meter for remove metal beam guard railing and no separate payment will be made therefor.

Full compensation for removing cable anchor assemblies shall be considered as included in the contract price paid per meter for remove metal beam guard railing and no separate payment will be made therefor.

10-1.30F REMOVE SIGN STRUCTURES

Existing sign structures, where shown on the plans to be removed, shall be removed and disposed of.

Attention is directed to "Construction Area Signs" elsewhere in these Special Provisions. Existing sign structures for which temporary construction area signs exist, as shown in the staging plans, shall not be removed until the corresponding temporary construction area sign is in place and ready for use.

Sign structure removal shall consist of removing posts, frames, portions of foundations, sign panels, and sign lighting electrical equipment.

A sign structure shall not be removed until the structure is no longer required for the direction of public traffic.

Concrete foundations may be abandoned in place, except that the top portion, including anchor bolts, reinforcing steel, and conduits shall be removed to a depth of not less than 1 meter below the adjacent finished grade. The resulting holes shall be backfilled and compacted with material equivalent to the surrounding material.

Electrical wiring shall be removed to the nearest pull box.

Electrical equipment, as shown on the plans, shall be salvaged.

10-1.30G REMOVE PAVEMENT MARKERS

Existing pavement markers, including underlying adhesive, when no longer required for traffic lane delineation as directed by the Engineer, shall be removed and disposed of.

10-1.30H REMOVE TRAFFIC STRIPES AND PAVEMENT MARKINGS

Traffic stripes and pavement markings to be removed shall be removed at the locations shown on the plans and at the locations designated by the Engineer.

The first paragraph of Section 15-2.02B, "Traffic Stripes and Pavement Markings," of the Standard Specifications is amended to read:

> 15-2.02B Traffic Stripes and Pavement Markings.— Traffic stripes and pavement markings shall be removed by any method that does not materially damage the existing pavement. Pavement marking images shall be removed in such a manner that the old message cannot be identified. Where grinding is used, the pavement marking image shall be removed by grinding a rectangular area. The minimum dimensions of the rectangle shall be the height and width of the pavement marking. Residue resulting from removal operations shall be removed from pavement surfaces by sweeping or vacuuming before the residue is blown by the action of traffic or wind, migrates across lanes or shoulders, or enters into drainage facilities.

Section 15-2.07, "Payment," of the Standard Specifications is amended by adding the following paragraph:

> Full compensation for any additional grinding outside the limits of the existing pavement marking image to obtain a rectangular area shall be considered as included in the contract price paid for the item of work involved and no additional compensation will be allowed therefor.

Nothing in these special provisions shall relieve the Contractor from the Contractor's responsibilities as provided in Section 7-1.09, "Public Safety," of the Standard Specifications.

10-1.30I REMOVE DRAINAGE FACILITIES

Existing junction boxes, flared end section, culverts, inlets and underdrains, where any portion of these structures is within one meter of the grading plane in excavation areas, or within 0.3-m of original ground in embankment areas, or where shown on the plans to be removed, shall be completely removed and disposed of.

Frames and grates shall be salvaged. Full compensation for salvaging existing frames and grates shall be considered as included in the contract unit price for remove inlet and no separate payment will be made therefor.

10-1.30J REMOVE ASPHALT CONCRETE SURFACING

Existing asphalt concrete surfacing, where shown on the plans to be removed, shall be removed.

For the purposes of this contract, the contract item "remove asphalt concrete surfacing" shall include the removal of existing cement treated base (CTB) as necessary to meet new subgrade elevations as shown in the plans.

Prior to removing the surfacing, the outside edge of the asphalt concrete to remain in place shall be cut to a neat line. The cut shall be a minimum depth of 50 mm.

The surfacing shall be removed in such a manner so that the surfacing which is to remain in place is not damaged.

The surfacing may be buried in embankments in the same manner as provided for burying concrete in embankments in Section 15-3, "Removing Concrete," of the Standard Specifications.

The contract price paid per cubic meter for remove asphalt concrete surfacing shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in removing asphalt concrete surfacing, including removal and disposal of existing cement treated base as described above, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.30K REMOVE ROADSIDE SIGNS

Existing roadside signs, at locations shown on the plans to be removed, shall be removed and disposed of.

Attention is directed to "Construction Area Signs" elsewhere in these Special Provisions. Existing roadside signs for which temporary construction area signs exist, as shown in the staging plans, shall not be removed until the corresponding temporary construction area sign is in place and ready for use.

Sign panels, as shown on the plans, shall be salvaged.

Existing roadside signs shall not be removed until replacement signs have been installed or until the existing signs are no longer required for the direction of public traffic, unless otherwise directed by the Engineer.

Full compensation for salvaging sign panels shall be considered as included in the contract unit price paid for remove roadside sign and no separate payment will be made therefor.

10-1.30L REMOVE ROADSIDE SIGNS (STRAP AND SADDLE BRACKET METHOD)

Existing roadside signs (strap and saddle bracket method), at locations shown on the plans to be removed, shall be removed and disposed of.

Attention is directed to "Construction Area Signs" elsewhere in these Special Provisions. Existing roadside signs for which temporary construction area signs exist, as shown in the staging plans, shall not be removed until the corresponding temporary construction area sign is in place and ready for use.

Sign panels, as shown on the plans, shall be salvaged.

Existing roadside signs (strap and saddle bracket method) shall not be removed until replacement signs have been installed or until the existing signs are no longer required for the direction of public traffic, unless otherwise directed by the Engineer.

Full compensation for salvaging sign panels shall be considered as included in the contract unit price paid for remove roadside sign (strap and saddle bracket method) and no separate payment will be made therefor.

10-1.30M REMOVE CHAIN LINK FENCE

Existing chain link fence shall be removed as shown on the plans.

Attention is directed to "Temporary High Visibility Boundary Fence" elsewhere in these special provisions.

Removal of existing chain link fencing shall be timed with new construction to minimize both the length and duration where permanent fencing is not in place. The contractor is responsible for maintaining access-control for those areas in which the existing fencing has been removed, and the planned permanent fencing is not yet in place. The contractor shall use temporary high visibility boundary fence conforming to the requirements of "Temporary High Visibility Boundary Fence" elsewhere in these special provisions for this purpose.

Fence removed shall be disposed of. Full compensation for removing and disposing of fence shall be considered as included in the contract price paid per meter for remove chain link fence and no separate payment will be made therefor.

10-1.30N RELOCATE SIGN STRUCTURES

Relocating sign structures shall consist of removing and relocating existing sign structures as shown on the plans.

Attention is directed to "Construction Area Signs" elsewhere in these Special Provisions. Existing sign structures for which temporary construction area signs exist, as shown in the staging plans, shall not be relocated until the corresponding temporary construction area sign is in place and ready for use.

Each existing concrete foundation, including anchor bolts, reinforcing steel, and conduit shall be removed to a depth of not less than 1 meter below the adjacent finished grade. Electrical wiring, if any, shall be removed to the nearest pull box. Removed portions of concrete foundations shall be disposed of outside the highway right of way in conformance with the provisions in Section 7-1.13, "Disposal of Material Outside the Highway Right of Way," of the Standard Specifications.

New foundation work for relocated sign structures shall conform to the provisions in Section 56-1, "Overhead Sign Structures," of the Standard Specifications, except that full compensation for furnishing and installing new anchor bolt assembly on each new foundation shall be considered as included in the contract price paid for the new foundation and no separate payment will be made therefor.

Sign lighting electrical work is provided for elsewhere in these special provisions.

10-1.30O RELOCATE ROADSIDE SIGNS

Attention is directed to "Construction Area Signs" elsewhere in these Special Provisions.

Existing roadside signs shall be removed and relocated at new locations shown on the plans. Existing roadside signs for which temporary construction area signs exist, as shown in the staging plans, shall not be relocated until the corresponding temporary construction area sign is in place and ready for use.

Each roadside sign shall be removed, stored in a sheltered location, and installed as shown on the plans once contract roadwork in the area of each sign has been completed as shown in the staging plans. Roadside signs shall be re-installed as shown in the plans within thirty (30) calendar days of the associated contract staging work being completed. At the time the permanent roadside sign is re-installed, any corresponding temporary construction area sign shall either by covered or removed so as not to conflict with the permanent roadside sign.

Attention is directed to Section 15-2.05 "Reconstruction" of the Standard Specifications. At least five (5) working days before removal, the Contractor shall supply the Engineer with a written list of existing roadside signs and sign materials which may be damaged beyond repair. The Engineer will then confirm the Contractor's list of damaged roadside signs and materials in writing within ten (10) working days. Any roadside signs later found to be damaged beyond repair after removal, storage, and transportation, shall be replaced by the contractor at his expense in accordance with Section 15-2.05 "Reconstruction" of the Standard Specifications. Roadside signs and materials listed and confirmed by the Engineer as having been damaged beyond repair at the beginning of this contract will be paid for as Extra Work in accordance with Section 4-1.03D of the Standard Specifications.

Two holes shall be drilled in each existing post as required to provide a breakaway feature as shown on the plans.

Full compensation for relocating roadside signs, including initial removal, temporary storage, handling, transportation, and installation to permanent locations within thirty (30) calendar days of associated contract staging work completion shall be considered as included in the contract price paid per unit for relocate roadside sign and no separate payment will be made therefor.

10-1.30P ADJUST INLETS

Existing pipe and drainage inlets shall be adjusted as shown on the plans. Concrete aprons, where present, shall be removed and disposed of.

Portland cement concrete shall conform to the provisions in Section 90-10, "Minor Concrete," of the Standard Specifications, or may be produced from commercial quality concrete containing not less than 350 kilograms of cement per cubic meter.

Where inlets are located in areas to be paved or surfaced, no individual structure shall be constructed to final grade until the paving or surfacing has been completed immediately adjacent to the structure.

The contract unit price paid for adjust inlet shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in adjusting inlets, including removing portions of inlets, removing and disposing of concrete aprons, bar reinforcing steel, concrete and structure excavation and structure backfill, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.30Q ADJUST MANHOLE TO GRADE

Existing manholes, frames and covers and frames and grates, or other facilities shall be adjusted to grade in accordance with the provisions in Section 15-2.05, "Reconstruction," of the Standard Specifications.

10-1.30R MODIFY INLETS

Existing drainage inlets shall be modified as shown on the plans. Concrete aprons, where present, shall be removed and disposed of. Existing frames and grates, that are not to be re-used in the work, shall be salvaged.

Portland cement concrete shall conform to the provisions in Section 90-10, "Minor Concrete," of the Standard Specifications, or may be produced from commercial quality concrete containing not less than 350 kilograms of cement per cubic meter.

Where inlets are located in areas to be paved or surfaced, no individual structure shall be constructed to final grade until the paving or surfacing has been completed immediately adjacent to the structure.

The contract unit price paid for modify inlet shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in modifying inlets, including removing portions of inlets, removing and disposing of concrete aprons, salvaging existing frames and grates not to be re-used in the work, bar reinforcing steel, concrete and structure excavation and structure backfill, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.30S PLANE ASPHALT CONCRETE PAVEMENT

Existing asphalt concrete pavement shall be planed at the locations and to the dimensions shown on the plans.

Except as provided herein, planing asphalt concrete pavement shall be performed, at the option of the Contractor, either by the cold planing or heater planing method. The use of the heater planing method shall be subject to approval of the local Air Pollution Control Officer.

Cold planing machines shall be equipped with a cutter head not less than 750 mm in width and shall be operated so as not to produce fumes or smoke. The cold planing machine shall be capable of planing the pavement without requiring the use of a heating device to soften the pavement during or prior to the planing operation.

Heater planing machines shall have, in combination or separately, a means for heating and cutting the asphalt concrete surface and blading the displaced material into windrows in one continuous forward motion. Heat shall be applied uniformly to the area to be planed and shall be accurately controlled according to conditions and road surfacing being planed. The cutting width of the blade shall be not less than 900 mm.

Heater planing operations shall not be performed at any time where there is danger of igniting entrapped gases from sewers or gas mains, if an open flame is used in the heater. The heater planing method shall not be used in areas where the heat generated by the heater planing equipment may damage adjacent shrubs or the foliage on overhanging tree limbs.

The depth, width and shape of the cut shall be as indicated on the typical cross sections or as directed by the Engineer. The final cut shall result in a uniform surface conforming to the typical cross sections. The outside lines of the planed area shall be neat and uniform. Planing asphalt concrete pavement operations shall be performed without damage to the surfacing to remain in place.

Planed widths of pavement shall be continuous except for intersections at cross streets where the planing shall be carried around the corners and through the conform lines. Following planing operations, a drop-off of more than 45 mm will not be allowed at any time between adjacent lanes open to public traffic.

Where transverse joints are planed in the pavement at conform lines no drop-off shall remain between the existing pavement and the planed area when the pavement is opened to public traffic. If asphalt concrete has not been placed to the level of existing pavement before the pavement is to be opened to public traffic a temporary asphalt concrete taper shall be constructed. Asphalt concrete for temporary tapers shall be placed to the level of the existing pavement and tapered on a slope of 1:30 (vertical: horizontal) or flatter to the level of the planed area.

Asphalt concrete for temporary tapers shall be commercial quality and may be spread and compacted by any method that will produce a smooth riding surface. Temporary asphalt concrete tapers shall be completely removed, including the removal of all loose material from the underlying surface, before placing the permanent surfacing. The removed material shall be disposed of outside the highway right of way in accordance with the provisions in Section 7-1.13 of the Standard Specifications.

Operations shall be scheduled so that not more than 7 days shall elapse between the time when transverse joints are planed in the pavement at the conform lines and the permanent surfacing is placed at the conform lines.

The material planed from the roadway surface, including material deposited in existing gutters or on the adjacent traveled way, shall be removed and disposed of outside the highway right of way in accordance with the provisions in Section 7-1.13 of the Standard Specifications. Removal operations of planed material shall be concurrent with planing operations and follow within 15 m of the planer, unless otherwise directed by the Engineer.

Plane asphalt concrete pavement will be measured by the square meter for the depth (maximum) designated in the Engineer's Estimate. The quantity to be paid for will be the actual area of surface planed for the depth (maximum) designated in the Engineer's Estimate, irrespective of the number of passes required to obtain the depth shown on the plans.

The contract price paid per square meter for plane asphalt concrete pavement for the depth (maximum) designated in the Engineer's Estimate shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in planing asphalt concrete surfacing and disposing of planed material, including furnishing the asphalt concrete for and constructing, maintaining, removing, and disposing of temporary asphalt concrete tapers, as specified in these special provisions and as directed by the Engineer.

10-1.30T CAP INLETS

Existing drainage inlets where shown on the plans to be capped, shall be capped and the bottoms of the inlets shall be rounded with portland cement concrete as shown on the plans. Concrete aprons, where present, shall be removed and disposed of. Existing frames and grates, that are not to be re-used in the work, shall be salvaged.

Portland cement concrete shall conform to the provisions in Section 90-10, "Minor Concrete," of the Standard Specifications, or may be produced from commercial quality aggregates and cement containing not less than 350 kg of cement per cubic meter.

Inlets shall be removed to a depth of at least 0.3-m below the grading plane.

Concrete removal shall be performed without damage to any portion that is to remain in place. All damage to the existing concrete, which is to remain in place, shall be repaired by the Contractor to a condition equal to that existing prior to the beginning of removal operations. The repair of existing concrete damaged by the Contractor's operations shall be at the Contractor's expense.

Existing reinforcement that is to be incorporated in new work shall be protected from damage and shall be thoroughly cleaned of all adhering material before being embedded in new concrete.

The quantity of capping inlets will be determined as units from actual count.

The contract unit price paid for cap inlet shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in capping inlets, including removing portions of inlets, removing and disposing of concrete aprons, salvaging existing frames and grates not to be re-used in the work, rounding bottoms of inlets, bar reinforcing steel, and structure excavation and structure backfill, as shown on the plans, as specified in the Standard Specifications and these special provisions and as directed by the Engineer.

10-1.30U CAP IRRIGATION SUPPLY LINES

Existing irrigation supply lines where shown on the plans to be capped, shall be capped in accordance with Section 20 "Erosion Control and Highway Planting" of the Standard Specifications.

The contract unit price paid for cap irrigation supply line shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in capping irrigation supply line, as shown on the plans, as specified in the Standard Specifications and these special provisions and as directed by the Engineer.

10-1.30V REMOVE CRASH CUSHION

Existing sand filled crash cushions, where shown on the plans to be removed, shall be removed and disposed of.

Removing sand filled crash cushions, regardless of the number of modules in each crash cushion array, will be measured and paid for by the unit at each location as remove crash cushion. The quantity to be paid for will be determined from actual count of the crash cushion arrays in place.

10-1.30W BRIDGE REMOVAL

Removing portions of bridge shall conform to the requirements in Section 15-4, "Bridge Removal," of the Standard Specifications and these special provisions.

Industrial Boulevard Undercrossing (Bridge No. 33-0370), location A.
MT. Eden Overhead (Bridge No. 33-0092R/L), location B.
Hesperian Blvd On-Ramp Soundwall, location C.
San Mateo-Hayward Bridge Toll Plaza Retaining Wall, location D.

All removed materials that are not to be salvaged or used in the reconstruction shall become the property of the Contractor and shall be disposed outside the highway right of way in accordance with the provisions in Section 7-1.13, "Disposal of Material Outside the Highway Right of Way," of the Standard Specifications.

The following additional requirements apply to the removal of portions of bridges whenever the removal work is to be performed over traffic or railroad property:

A protective cover supported by falsework or members of the existing structure shall be constructed before beginning bridge removal work.

The construction and removal of the protective cover and the installation and removal of temporary railings shall conform to the requirements under "Order of Work" "Maintaining Traffic" and "Temporary Railings" of these special provisions.

The protective cover shall prevent any materials, equipment or debris from falling onto the traffic or railroad property. The protective cover shall have a minimum strength equivalent to that provided by good, sound Douglas fir planking having a nominal thickness of 50 mm. Additional layers of material shall be furnished as necessary to prevent fine materials or debris from sifting down upon the traveled way and shoulders.

The protective cover shall conform to the provisions for falsework in Section 51-1.06, "Falsework," of the Standard Specifications.

The Contractor shall be responsible for designing and constructing a safe and adequate protective cover, and shoring and falsework needed to support the protective cover, all with sufficient strength and rigidity to support the entire load to be imposed.

Bridge removal methods shall be described in the working drawings and calculations in sufficient detail to substantiate live loads used in the protective cover design. Dead and live load values assumed for designing the protective cover shall be shown on the working drawings.

At locations where the bridge railing is to be removed, the protective cover shall extend from, face of exterior girder or at least 0.6-m inside of the bridge railing to be removed whichever is less, to at least 1.2 m beyond the outside face of the bridge railing.

The protective cover shall extend at least 3 m beyond the outside face of the bridge railing, except that, at locations where the bridge railing is to be removed and new girders are not constructed, the protective cover shall extend from, face of exterior girder or at least 0.6-m inside of the bridge railing to be removed whichever is less, to at least 1.2 m beyond the outside face of the bridge railing.

During the removal of bridge segments, and when portions of the bridge, such as deck slabs or box girder slabs, comply with the requirements for the protective cover, a separate protective cover need not be constructed.

Before removal, the protective cover shall be cleaned of all debris and fine material.

The protective cover shall provide the openings specified under "Maintaining Traffic" of these special provisions, except that when no openings are specified for bridge removal a vertical opening of 4.6 m and a horizontal opening of 9.8 m shall be provided for the passage of public traffic.

The protective cover shall provide minimum clearances as required under "Relations with Railroad Company" of these special provisions for the passage of railroad traffic.

Falsework or supports for protective cover shall not extend below the vertical clearance level nor to the ground line at any location within the roadbed.

The construction of the protective cover as herein specified shall in no way be construed to relieve the Contractor of responsibilities specified in Sections 7-1.121, "Indemnification," and 7-1.122, "Insurance," of the Standard Specifications.

10-1.30X REMOVE CONCRETE CURB

Concrete curb, designated on the plans to be removed, shall be removed.

The pay quantities of concrete curb to be removed will be measured by the meter, measured before and during removal operations.

10-1.30Y REMOVE CONCRETE BARRIER

Concrete barrier, designated on the plans to be removed, shall be removed.

The pay quantities of concrete barrier to be removed will be measured by the meter, measured before and during removal operations.

10-1.31 CLEARING AND GRUBBING

Clearing and grubbing shall conform to the provisions in Section 16, "Clearing and Grubbing," of the Standard Specifications and these special provisions.

Attention is directed to "Aerially Deposited Lead, General" elsewhere in these specifications.

Clearing and grubbing operations shall result in no visible dust. No material containing lead shall be deposited on public roads. The Contractor shall indemnify the State from any costs due to spillage of material containing lead during transport. The Contractor shall separate soil from vegetation, and the soils will remain on the site.

Vegetation shall be cleared and grubbed only within the excavation and embankment slope lines.

At locations where there is no grading adjacent to a bridge or other structure, clearing and grubbing of vegetation shall be limited to 1.5 meters outside the physical limits of the bridge or structure.

Existing vegetation outside the areas to be cleared and grubbed, shall be protected from injury or damage resulting from the Contractor's operations.

Activities controlled by the Contractor, except cleanup or other required work, shall be confined within the graded areas of the roadway.

Nothing herein shall be construed as relieving the Contractor of the Contractor's responsibility for final cleanup of the highway as provided in Section 4-1.02, "Final Cleaning Up," of the Standard Specifications.

10-1.32 WATERING

Watering shall conform to the provisions in Section 17, "Watering," of the Standard Specifications.

10-1.33 EARTHWORK

Earthwork shall conform to the provisions in Section 19, "Earthwork," of the Standard Specifications and these special provisions.

Where a portion of existing surfacing is to be removed, the outline of the area to be removed shall be cut on a neat line with a power-driven saw to a minimum depth of 50 mm before removing the surfacing. Full compensation for cutting existing surfacing shall be considered as included in the contract price paid per cubic meter for roadway excavation and no additional compensation will be allowed therefor.

The "0.2-m" dimensions in the fifth paragraph of Section 19-3.08, "Payment," of the Standard Specifications are increased to "0.3-m" on this project.

10-1.33A CONTROLLED LOW STRENGTH MATERIAL

Controlled low strength material shall consist of a workable mixture of aggregate, cementitious materials and water, and shall conform to the provisions in Section 19-3, "Structure Excavation and Backfill," of the Standard Specifications and these special provisions.

At the option of the Contractor, controlled low strength material may be used as structure backfill for pipe culverts, except that controlled low strength material shall not be used as structure backfill for culverts having a span greater than 6.1 m.

When controlled low strength material is used for structure backfill, the width of the excavation shown on the plans may be reduced so that the clear distance between the outside of the pipe and the side of the excavation, on each side of the pipe, is a minimum of 300 mm. This minimum may be reduced to 150 mm when, either the height of cover is less than or equal to 6.1 m or the pipe diameter or span is less than 1050 mm.

Controlled low strength material in new construction shall not be permanently placed higher than the basement soil. For trenches in existing pavements, permanent placement shall be no higher than the bottom of any existing pavement permeable drainage layer. If a drainage layer does not exist, permanent placement in existing pavements shall be no higher than 25 mm below the bottom of the existing asphalt concrete, or no higher than the top of base below the existing Portland cement concrete pavements. The minimum height that controlled low strength material shall be placed, relative to the pipe invert, is 0.5D (D=Diameter) for rigid pipe and 0.7D for flexible pipe.

When controlled low strength material is proposed for use, the Contractor shall submit a mix design and test data to the Engineer for approval prior to excavating the trench for which controlled low strength material is proposed for use. The test data shall demonstrate that the mix design provides:

a) For pipe culverts having a height of cover of 6.1 m or less, a 28-day compressive strength between 345 and 690 kPa is required; for height of cover greater than 6.1 m, a minimum 28-day compressive strength of 690 kPa is required. Compressive strength shall be determined by ASTM Designation: D 4832, "Preparation and Testing of Soil-Cement Slurry Test Cylinders."
b) When controlled low strength material is used as structure backfill for pipe culverts, the sections of pipe culvert in contact with the controlled low strength material shall meet the requirements of Chapter 850 of the Highway Design Manual using the minimum resistivity, pH, chloride content, and sulfate content of the hardened controlled low

strength material. Minimum resistivity and pH shall be determined by California Test 643, the chloride content shall be determined by California Test 422 and the sulfate content shall be determined by California Test 417.

c) Cement shall be any type of Portland cement conforming to the provisions of ASTM Designation: C 150; or any type of blended hydraulic cement conforming to either ASTM Designation: C 595M or the physical requirements of ASTM Designation: C 1157M. Testing of cement will not be required.

d) Admixtures may be used in conformance with Section 90-4, "Admixtures," of the Standard Specifications. Chemical admixtures containing chlorides as Cl in excess of one percent by mass of admixture, as determined by California Test 415, shall not be used.

Materials for controlled low strength material shall be thoroughly machine-mixed in a pugmill, rotary drum, or other approved mixer. Mixing shall continue until the cementitious material and water are thoroughly dispersed throughout the material. Controlled low strength material shall be placed in the work within 3 hours after introduction of the cement to the aggregates.

Controlled low strength material shall be placed in a uniform manner that will prevent voids in, or segregation of, the backfill, and will not float or shift the culvert. Foreign material which falls into the trench prior to or during placing of the controlled low strength material shall be immediately removed.

When controlled low strength material is to be placed within the traveled way or otherwise to be covered by paving or embankment materials, the material shall achieve a maximum indentation diameter of 76 mm prior to covering and opening to traffic. Penetration resistance shall be as measured by ASTM Designation: C 6024, "Standard Test Method for Ball Drop on Controlled Low Strength Material to Determine Suitability for Load Application."

Controlled low strength material used as structure backfill for pipe culverts will be considered structure backfill for compensation purposes.

If the Contractor elects to use the "Weep Hole and Geocomposite Drain" alternative where permitted on the plans, the geocomposite drain shall conform to the details shown on the plans and the following:

Attention is directed to "Engineering Fabrics" under "Materials" of these special provisions.

Geocomposite drain shall consist of a manufactured core not less than 6.35 mm thick nor more than 50 mm thick with one or both sides covered with a layer of filter fabric that will provide a drainage void. The drain shall produce a flow rate, through the drainage void, of at least 25 liters per minute per meter of width at a hydraulic gradient of 1.0 and a minimum externally applied pressure of 168 kPa.

A Certificate of Compliance conforming to the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications shall be furnished for the geocomposite drain certifying that the drain produces the required flow rate and complies with these special provisions. The Certificate of Compliance shall be accompanied by a flow capability graph for the geocomposite drain showing flow rates for externally applied pressures and hydraulic gradients. The flow capability graph shall be stamped with the verification of an independent testing laboratory.

Filter fabric for the geocomposite drain shall conform to the provisions for fabric for underdrains in Section 88, "Engineering Fabrics," of the Standard Specifications.

The manufactured core shall be either a preformed grid of embossed plastic, a mat of random shapes of plastic fibers, a drainage net consisting of a uniform pattern of polymeric strands forming 2 sets of continuous flow channels, or a system of plastic pillars and interconnections forming a semirigid mat.

The core material and filter fabric shall be capable of maintaining the drainage void for the entire height of geocomposite drain. Filter fabric shall be integrally bonded to the side of the core material with the drainage void. Core material manufactured from impermeable plastic sheeting having nonconnecting corrugations shall be placed with the corrugations approximately perpendicular to the drainage collection system.

The geocomposite drain shall be installed with the drainage void and the filter fabric facing the embankment. The fabric facing the embankment side shall overlap a minimum of 75 mm at all joints and wrap around the exterior edges a minimum of 75 mm beyond the exterior edge. If additional fabric is needed to provide overlap at joints and wrap-around at edges, the added fabric shall overlap the fabric on the geocomposite drain at least 150 mm and be attached thereto.

Should the fabric on the geocomposite drain be torn or punctured, the damaged section shall be replaced completely or repaired by placing a piece of fabric that is large enough to cover the damaged area and provide a minimum 150-mm overlap.

Plastic pipe shall conform to the provisions for pipe for edge drains and edge drain outlets in Section 68-3, "Edge Drains," of the Standard Specifications.

Treated permeable base to be placed around slotted plastic pipe at the bottom of the geocomposite drain shall be cement treated permeable base conforming to the provisions for cement treated permeable base in Section 29, "Treated Permeable Bases," of the Standard Specifications and these special provisions.

The treated permeable base shall be enclosed with a high density polyethylene sheet or PVC geomembrane, not less than 250 µm thick, which is bonded with a suitable adhesive to the concrete and geocomposite drain. Surfaces to receive the polyethylene sheet shall be cleaned before applying the adhesive. The treated permeable base shall be compacted with a vibrating shoe type compactor.

MEASUREMENT AND PAYMENT (EARTHWORK)

Measurement and payment for earthwork shall conform to all provisions for "Measurement" and "Payment" in Section 19, "Earthwork," of the Standard Specifications and these special provisions.

Pervious backfill material in connection with the bridge work will be measured and paid for as structure backfill (bridge).

If structure excavation or structure backfill involved in bridges is not otherwise designated by type, and payment for the structure excavation or structure backfill has not otherwise been provided for in the Standard Specifications or these special provisions, the structure excavation or structure backfill will be paid for at the contract price per cubic meter for structure excavation (bridge) or structure backfill (bridge).

Full compensation for filter fabric shall be considered as included in the contract price paid per cubic meter for structure backfill (soil nail wall) and no additional compensation will be allowed therefor.

10-1.33B MATERIAL WITH AERIALLY DEPOSITED LEAD

Attention is directed to "Aerially Deposited Lead, General" elsewhere in these special provisions.

Existing materials within the highway right of way, as shown in the plans, contain less than 1575 mg/kg total lead and less than 500 µg/l water soluble lead and are designated as Type Y materials. Type Y materials excavated in accordance with the contract documents shall be placed in embankments or backfill as shown on the plans and as specified in these special provisions.

All other existing material excavated from areas within the project limits may be used as cover material provided that it meets the conditions described in "Aerially Deposited Lead, General" elsewhere in these special provisions.

Type Y materials, when excavated from outside the limits of payment for contract items, shall be the responsibility of the Contractor. This material shall not be placed in embankments or backfill unless the Contractor can show that such placement will not cause Type Y materials from within the limits of payment for contract items to become surplus material.

All materials excavated from areas within the project limits are designated as containing aerially deposited lead. Temporary surplus material will be generated on this project due to stage construction constraints. Such surplus material shall not leave the highway right of way. In order to conform with this provision, it is anticipated that the Contractor will have to stockpile materials for subsequent stages and/or construct some embankments out of stage and/or handle the surplus material more than once.

No USEPA manifesting of these materials is required. A bill of lading will be furnished by the State for each truck load of these materials being transported from the project to a mandatory disposal site shown on the plans and traveling on public roads. The bill of lading shall include: US DOT description including shipping name, hazard class and ID number, handling codes, and quantity of materials in cubic yards. Trucks shall have all extraneous dirt and materials extending outside of the vehicle removed and the vehicle covered prior to traveling on public roads. The Contractor shall be responsible for costs due to spillage of lead containing material during transport.

The Contractor shall monitor the air quality during excavation operations in accordance with the Health and Safety Plan. Results of air quality tests shall be made available to the Engineer upon request.

Surplus material from outside the limits of Type Y material, which cannot be used in accordance with Section 19-2.06, "Surplus Material," of the Standard Specifications, shall become the property of the Contractor and shall be disposed of in accordance with Section 7-1.13, "Disposal of Material Outside the Highway Right of Way," of the Standard Specifications. The written authorization from the property owner shall include acknowledgment that the material contains lead, and shall state the levels of lead reported from testing.

The Contractor shall conduct any further investigation deemed necessary by the owner of the disposal site for acceptance of the material. This investigation shall be at the Contractor's expense. The Contractor shall submit to the Engineer, his sampling and analysis procedure and name of laboratory fifteen days prior to beginning any sampling or analysis. The Contractor shall use a laboratory certified by the California Department of Health Services. Characterization of the material shall be based on guidelines in USEPA, SW 846, "Test Methods for Evaluating Solid Waste, Volume II: Field Manual Physical/Chemical , Chapter Nine, Section 9.1.

Surplus hazardous material shall remain the property of the State and shall be disposed of at the direction of the Engineer. Disposal and transportation of materials with hazardous levels of lead shall be performed in accordance with all applicable Federal, State and Local hazardous waste laws and regulations. Laws and regulations which govern this work include; but, are not limited to:

1. Health and Safety Code, Division 20, Chapter 6.5 (California Hazardous Waste Control Act).
2. Title 22, California Code of Regulations, Chapter 30 (Minimum Standard for Management of Hazardous and Extremely Hazardous Materials)
3. Title 8, California Code of Regulations

The Engineer will obtain the Environmental Protection Agency Generator Identification Number and Board of Equalization Identification Number for hazardous material disposal. The Engineer will sign all hazardous waste manifest, as the State is the generator.

Transportation and disposal of surplus hazardous material will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications. Sampling, analyzing, transporting, and disposing of additional materials containing lead, excavated outside of the pay limits for contract items, shall be at the Contractor's expense.

Full compensation for conforming to the requirements of this section involving materials containing aerially deposited lead, except as otherwise specifically provided in these special provisions, shall be considered as included in the contract prices paid for the various contract items of work involved and no additional compensation will be allowed therefor.

10-1.33C EXCAVATION HAZARDOUS MATERIAL

Attention is directed to "Aerially Deposited Lead, General", "Water Pollution Control", and "Material with Aerially Deposited Lead" elsewhere in these special provisions. Excavation of Hazardous Material shall consist of all excavation involving Type Y materials excavated from within planned excavation limits as shown on the plans, and the grading and construction of the roadway and embankments using this Type Y material, as shown on the plans.

Excavation Hazardous Material shall conform to the provisions in Section 19, "Earthwork," of the Standard Specifications and these special provisions.

MEASUREMENT AND PAYMENT

Excavation Hazardous Material shall be measured and paid for by the cubic meter. The contract price paid per cubic meter for Excavation Hazardous Material shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in excavating, handling, stockpiling, protecting, loading and re-loading (as many times as required for planned work), grading and placement, complete in place, including compliance with all regulatory and contractual requirements for the use of the Type Y material, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.34 LIGHTWEIGHT FILL MATERIAL

Lightweight fill material shall consist of a rotary kiln expanded shale aggregate of the extruded type or a processed, naturally-occurring volcanic aggregate. The dominant shape of the lightweight fill material shall be subangular or angular. Lightweight fill material shall conform to the provisions in Section 19-3, "Structure Excavation and Backfill," of the Standard Specifications and these special provisions.

Attention is directed to "Asphalt Concrete" elsewhere in these special provisions. A sixty (60) calendar day settlement period shall elapse between placement of lightweight fill and any subsequent asphalt concrete paving over the same.

The lightweight fill material, when delivered to the project site, shall consist of fine-graded lightweight fill material conforming to the following:

Sieve Size	Percent Passing	Test Method
25 mm [1 inch]	95 - 100	202 (1)
19 mm [3/4 inch]	90 - 100	
9.5 mm [3/8 inch]	45 - 95	
4.75 mm [#4]	25 - 60	
600 µm [#30]	8 - 30	
75 µm [#200]	0 - 15	

(1) Shaking in the sieves shall be limited to 5 minutes.

The lightweight fill material shall also conform to the following:

Durability Index	35	229
Resistance (R-Value)	50 min	301

The lightweight fill material shall have a maximum dry unit density of 1 090 kg/m^3 (68.0 pcf) when determined in accordance with California Test Method 212, Compaction Method b (by jigging). The lightweight fill material shall have a maximum saturated surface dry state unit density of 1 185 kg/m^3 (74.0 pcf) after twenty four hour absorption, when determined in accordance with California Test Method 206.

Lightweight fill material shall be placed and compacted to the designated dimensions as specified in Sections 19-1.03, "Grade Tolerance," and 19-6, "Embankment Construction," of the Standard Specifications.

The fine-graded lightweight fill material may be placed by dumping from trucks or by any other method approved by the Engineer. Lightweight fill material shall be spread or placed in a uniform manner that will prevent bulking of the material and minimize particle breakdown. Lightweight fill material shall be compacted in lifts of a maximum 200 mm (8 in) thickness of the uncompacted material. Compaction shall be obtained by a minimum of three complete coverage passes using smooth drum steel roller compaction equipment imposing contact pressure of 5 360 kg per-meter-width of the roller face (3,600 pounds per-foot-width of the roller faces), or as approved by the Engineer. The fine-graded lightweight fill

placed within 1.2 m of the gabions shall be compacted by 5 passes of a vibratory plate, or as approved by the Engineer. Compaction using pneumatic-tired equipment or compaction within trenches or other limited access areas shall be of a method approved by the Engineer.

With the concurrence of the Contractor and the Engineer, a test fill may be constructed and compaction equipment and procedures developed to meet project and site conditions.

The total quantity of lightweight fill material placed will be computed in the same manner as specified for roadway excavation in Section 19-2.08, "Measurement," of the Standard Specifications. No adjustment in the quantity of lightweight fill material to be paid for will be made in the event that subsidence or consolidation occurs after the placing of the lightweight fill material has begun.

The contract price paid per cubic meter for lightweight fill material shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in constructing the lightweight fill material, complete in place, as shown on the plans, as specified in these special provisions, and as directed by the Engineer.

10-1.35 CELLULAR CONCRETE

DESCRIPTION

The work shall consist of constructing a cellular concrete fill to the lines, grades, and dimensions shown on the plans and in accordance with these special provisions or as directed by the Engineer.

MATERIALS

FOAMING AGENT tested in accordance with ASTM C796.

PORTLAND CEMENT shall comply with ASTM C150, Types I, II, or III.

Pozzolans and other cementitious materials may be used when specifically approved by the manufacturer of the foaming agent.

MIXING WATER shall be potable and free of deleterious amounts of acids, alkali, salts, oils, and organic materials which would adversely affect the setting or strength of the cellular concrete.

ADMIXTURES for accelerating the set time may be used in accordance with the manufacturer's recommendations.

MIX DESIGN

The Contractor shall furnish a mix design which will produce a cast density (at point of placement) of 3.7 to 5.6 kN/m^3 with a minimum compressive strength of 551 kPa at 28 days.

TESTING

At the point of placement, the density shall be in accordance with the specified cast density. No single cast density test shall represent more than 230 cubic meters or one day's production, whichever is less.

The compressive strength shall be tested in accordance with ASTM C495, except as follows:

1.Unless otherwise approved by the Engineer, the specimens shall be 76.2 mm (3-inch) by 152.4 mm (6-inch) cylinders. Specimens shall be covered and protected immediately after casting to prevent damage and loss of moisture.

2. Specimens shall be moist cured for a period of seven (7) days prior to the 28-day compressive strength test. Specimens shall not be over dried.

MIXING AND PLACING

Cellular concrete shall be job site batched, mixed with the foaming agent, and placed with specialized equipment certified by the manufacturer. Slurry coats and multilayer casting are acceptable methods of installation.

Subgrade to receive cellular concrete shall be free of all loose and extraneous material. Subgrade shall be uniformly moist, and any excess water standing on the surface shall be removed prior to placing cellular concrete.

The exposed surface of cellular concrete shall be covered with a seal coat to fill surface voids. The seal coat SS-1 shall be applied uniformly at a rate of between 0.68 and 1.13 liter per square-meter.

MEASUREMENT

Pay quantities for cellular concrete will be measured by the cubic meter as shown on the plans or as directed by the Engineer.

PAYMENT

The contract price per cubic meter for cellular concrete shall include full compensation for furnishing all labor, tools, equipment, materials and incidentals and for doing all the work to install the cellular concrete as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.36 GEOSYNTHETIC REINFORCED EMBANKMENT (GEOGRID)

Geosynthetic reinforced embankment shall consist of placing geosynthetic reinforcement material between layers of lightweight fill and gabions in accordance with the details shown on the plans, as specified in Section 19 "Earthwork," of the Standard Specifications, these special provisions, and as directed by the Engineer. Only one type of geosynthetic reinforcement material shall be used for an entire embankment.

If shown on the plans, a drainage system shall be constructed with the geosynthetic reinforced embankment. Specifications for the drainage system will be found elsewhere in these special provisions.

MATERIAL CONFIGURATION SPECIFICATIONS.--The geosynthetic reinforcement material shall be configured as a geosynthetic and shall meet the requirements described under “Material Specifications” found elsewhere in this section. The Engineer shall be furnished a Certificate of Compliance according to the provisions found in Section 6-1.07, “Certificate of Compliance,” of the Standard Specifications for the geosynthetic reinforcement material a minimum of one week prior to beginning placement of geosynthetic reinforcement material. The Certificate of Compliance shall be prepared and signed by a representative of the manufacturer who is a California-registered Civil Engineer.

Geosynthetic reinforcement material shall consist of material designed for use in subsurface geotechnical slope reinforcement applications. Geosynthetic reinforcement material shall be configured as either a geogrid or geotextile material. Geogrid shall have in addition to the requirements for geosynthetic reinforcement, a regular and defined open area. Geogrid shall obtain pullout resistance from the soil by a combination of soils shearing friction on the plane surfaces parallel to the direction of shearing and soils bearing on transverse grid surfaces normal to the direction of grid movement. The percentage of the open area for geogrids shall not vary outside of the range of 50 to 90 percent of the total projection of a section of the material. Geotextiles shall have in addition to the requirements for geosynthetic reinforcement material, an irregular or regular open area with the spacing of open areas being less than 6.3 millimeters in any direction.

Geosynthetic reinforcement material shall meet the following requirements in addition to the requirements described under “Materials Specifications” elsewhere in this section:

1. Long Term Design Strength (LTDS) for geosynthetic reinforcement material shall be equal to or greater than values shown on the plans or elsewhere in these specifications as determined by Geosynthetic Research Institute (GRI) Test Methods. LTDS for geogrid reinforcement and geotextile reinforcement shall be determined by Standard Practice GRI G4 (a) and (b) and GRI GT7, respectively. These values are minimum average roll values.

Long Term Design Strength is the strength of the geogrid or the geotextile calculated by applying all partial factors of safety in accordance with GRI Standard Practice GG4 (a) and (b) or GT7. The factor of safety for creep deformation shall be determined for a 75-year design life as determined by GRI G4 (a) and (b) for geogrids or GRI GT7 for geotextiles. The 75-year design life strength is determined from the creep curve which becomes asymptotic to a constant strain line of 10 percent or less.

In the absence of specific test data, the partial factor of safety default values (installation damage, creep deformation, chemical degradation, biological degradation, and joint) as indicated in the Standard Practice GRI G4 (a) and (b) and GRI GT7 shall be applied to the calculations of the LTDS.

2. Geosynthetic reinforcement material shall be resistant to naturally occurring alkaline and acidic soil conditions, and to attack by bacteria.

All test results which contributed to the calculations of the LTDS shall be submitted to the Engineer no less than one week prior to beginning placement of the geosynthetic reinforced embankment. All test results which contribute to the calculations of the LTDS shall be prepared and signed by a California-registered Civil Engineer.

MATERIAL.--Geosynthetic reinforcement material shall consist of high density polyethylene, polypropylene, high density polypropylene sheets, high tenacity polyester yarn, or polyaramide and shall meet the applicable material requirements found below.

High Density Polyethylene.--Geosynthetic reinforcement material consisting of high density polyethylene shall meet or exceed the following material requirements:

1) Be manufactured from high density polyethylene (HDPE) which conforms to ASTM Method D 1248.
2) Shall have a LTDS in the primary strength direction greater than or equal to ≠20.4 kilonewtons.

Polypropylene.--Geosynthetic reinforcement material consisting of polypropylene or high-density polypropylene sheets shall meet or exceed the following material requirements:

1) Shall meet the requirements of ASTM Designation: D 4101, Group 1/Class1/Grade 2.

2) Shall have a LTDS in the primary strength direction greater than or equal to 20.4 kilonewtons.

High Tenacity Polyester Encapsulated.--Geosynthetic reinforcement material consisting of high tenacity polyester yarn shall meet or exceed the following material requirements:

1) Be manufactured from high tenacity polyester yarn as determined by ASTM Designation: D 629. In addition to meeting the requirements for geosynthetic, geogrid shall be encapsulated in an acrylic latex coating or similar.
2) Shall have a LTDS in the primary strength direction greater than or equal to-/20.4 kilonewtons.

Polyaramides.--Geosynthetic reinforcement material consisting of polyaramide shall meet or exceed the following material requirements:

1) Be manufactured from high tenacity polyester yarn as determined by ASTM Designation: D 629.
2) Shall have a LTDS in the primary strength direction greater than or equal to 20.4 kilonewtons .

EMBANKMENT BACKFILL.--The backfill material used in the geosynthetic reinforced embankment shall consist of light weight fill.

HANDLING AND STORAGE.--Geosynthetic reinforcement material shall be handled and stored in accordance with the manufacturer's recommendations and these special provisions. Geosynthetic reinforcement material shall be furnished in an appropriate protective cover which shall protect it from ultraviolet radiation and from abrasion during shipping and handling. Only as much geosynthetic reinforcement material shall be placed as can be placed and covered with backfill in the same work shift.

CONSTRUCTION.--The Contractor shall prepare the grade that is to receive the layers of geosynthetic reinforcement material to the compaction and elevation tolerances described in the Standard Specifications under Section 19-3, "Structure Excavation and Backfill," and these special provisions. The grade shall be free of loose or extraneous material and objects that may damage the geosynthetic reinforcement material during installation.

Geosynthetic reinforcement material shall be handled and placed in accordance with the manufacturer's recommendations and these special provisions. The geosynthetic reinforcement material shall be laid horizontally at the elevation specified on the plans, on compacted light weight fill at the face of the gabion to the required embedment length. The geosynthetic reinforcement material shall be placed in a wrinkle free manner, pulled taut, aligned, and anchored before backfill placement. Slack in geosynthetic reinforcement material shall be removed in a manner, and to such a degree, as approved by the Engineer. Geosynthetic reinforcement material shall be installed in a horizontal plane at the intervals, elevations, and for the minimum embedment length shown on the plans. Each layer of geosynthetic reinforcement material shall not vary more than 0.15 meter from the theoretical horizontal plane established for that layer for the entire width and length of the reinforced reinforcement.

Geosynthetic reinforcement material shall be placed as shown on the plans and shall extend to the lengths shown on the plans Where the full embedment length of geosynthetic reinforcement material as shown on the plans cannot be achieved along the sides or for other limited areas of the reinforcement zone, the geosynthetic reinforcement material shall be trimmed as necessary to avoid the obstruction and to achieve the maximum embedment possible.

Geosynthetic reinforcement material shall be secured in place with staples, pins, sand bags, or backfill as required by construction conditions, weather conditions, or as directed by the Engineer to prevent the displacement of the geosynthetic reinforcement material during compaction and placement of the reinforcement material.

Geosynthetic reinforcement material shall not extend into the pavement structural section.

Each layer of geosynthetic reinforcement material shall be placed (unrolled) into the grade to form a continuous mat. Overlapping and splicing geosynthetic embankment material shall conform to the following:

Uniaxial geogrid and geotextile geotechnical fabric does not need to be overlapped along edges parallel to the direction of working tensile strength. Uniaxial geogrid and woven geotechnical fabric shall not be overlapped or spliced along edges perpendicular to the direction of working tensile strength, or as directed by the Engineer.

If a drainage feature or other feature is shown on the plans within or adjacent to the geosynthetic reinforced embankment, the construction of that feature shall be done in a time sequence relative to the geosynthetic reinforced embankment as best meets the project requirements.

The geosynthetic reinforcement material shall be placed in such a manner that the direction of maximum strength is oriented perpendicular to the project centerline. The Contractor shall verify correct orientation of the geosynthetic reinforcement material. Each layer of geosynthetic reinforcement material shall be placed onto the embankment material to form a continuos mat. Adjacent strips of geosynthetic reinforcement material placed in this manner need not be overlapped.

During spreading and compacting of the backfill, at least 150 millimeters, measured vertically, of backfill shall be maintained between the geosynthetic reinforcement material and the Contractor's equipment. Equipment or vehicles shall not be operated or driven directly on the geosynthetic reinforcement material .

Splicing of geosynthetic reinforcement material shall not be allowed.

If the geosynthetic reinforcement material is damaged during construction operations, the damaged sections shall be replaced, at the Contractor's expense.

MEASUREMENT AND PAYMENT.--Geosynthetic (Geogrid) will be measured and paid for by the square meter for the total area shown on the plans and for any additional area as directed by the Engineer.

The contract price paid per square meter of geosynthetic reinforced embankment (Geogrid) shall include full compensation for furnishing all labor and materials, including tools and equipment, and incidentals, for developing, placing and compacting light weight fill and for doing all the work involved in placing the geosynthetic reinforcement complete and in place, including splicing, overlapping and anchoring as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Full compensation for revisions to drainage systems or other facilities made necessary by the use of an alternative geosynthetic reinforcement material embankment material shall be considered as included in the contract price paid per square foot for geosynthetic reinforced embankment (geogrid) and no adjustment in compensation will be made therefor.

10-1.37 SOIL NAIL WALL EARTHWORK

Soil nail wall earthwork consisting of excavation, drilling holes for installation of soil nail assemblies, and backfilling around these completed walls shall conform to the requirements in Section 19-3, "Structure Excavation and Backfill," of the Standard Specifications and these special provisions.

Working Drawings--The Contractor shall submit complete working drawings for earthwork for each soil nail to the Office of Structure Design, Documents Unit, P.O. Box 942874, Sacramento, CA 94274-0001 (1801 30th Street, Sacramento, CA, 95816; 916-227-8230), in conformance with the provisions in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications. All working drawings for wall earthwork shall be 559 mm by 864 mm in size. For initial review, 5 sets of drawings shall be submitted. After review, between 6 and 12 sets, as requested by the Engineer, shall be submitted to the said Office for final approval and use during construction.

Working drawings for wall earthwork shall show the State assigned designations for the contract number, structure number, full name of the structure as shown on the contract plans, and District-County-Route-Kilometer Post on each drawing and calculation sheet. The Contractor name, address, and phone number shall be shown on the working drawings. Each sheet shall be numbered in the lower right hand corner.

Working drawings for wall earthwork shall contain all information required for the construction and quality control of the earthwork, including the following:

1. The proposed schedule and detailed construction sequence. Construction sequence shall include measures to ensure wall and slope stability during various stages of wall construction including provisions for discontinuous rows of nails or anchors.
2. Methods of excavation to the staged lifts indicated and excavation equipment types.
3. Temporary shoring plans.
4. Drilling methods and equipment including proposed drill hole size and any variation of these along the alignments.
5. Information on space requirements for installation equipment.
6. A detailed construction dewatering plan addressing all elements necessary to divert, control and dispose of surface water.

A supplement to the working drawings shall include the following:

1. Independently checked calculations for wall and slope stability during various stages of wall construction including geotechnical assessment of information provided by Caltrans for this contract. At the Contractor's option, the Contractor may conduct additional geotechnical investigation for the purpose of developing wall earthwork working drawings.
2. Information on provisions for working in the proximity of underground facilities.

The working drawings and supplement shall be stamped and signed by an engineer who is registered as a Civil Engineer in the State of California.

The Contractor shall allow the Engineer 4 weeks to review the working drawing submittal after a complete set has been received.

Should the Engineer fail to review the complete working drawing submittal within the time specified and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in reviewing the wall earthwork working drawing submittal, an extension of time commensurate with the delay in completion of the work thus caused will be granted in accordance with Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

Excavation--Care shall be taken during excavation for soil nail wall to prevent disturbing the natural foundation materials behind the face of excavation. During initial mass grading, the Contractor shall not excavate the full wall height to the wall alignment as shown on the plans, but the Contractor shall maintain a working berm of native material in front of the wall to serve as a work bench for the drill equipment. The working berm shall extend out from the wall a minimum distance

of 6 meters and shall be cut down from that point at a slope as shown on the approved wall earthwork working drawings. The original ground beyond the wall alignment for the back or ends of the wall as shown on the plans shall not be over excavated. Any such over excavation shall be restored by the Contractor, at the Contractor's expense, using methods and materials approved in writing by the Engineer. Soil stabilization methods or temporary backing or lagging placed behind the excavation face may be required to prevent disturbing the natural foundation materials.

Excavation for walls shall be limited to that area which can be nailed and covered with shotcrete during the same work shift in which the excavation is done. Subsequent excavation shall not be made within 3 meters of previously nailed and covered portions of the wall until those nailed or anchored and covered portions are structurally complete. A portion of the wall will be considered structurally complete when the soil nail assemblies have been installed, the shotcrete cover has set, the soil nail bearing assemblies installed, and specified anchorage testing has been completed for that portion of the wall, and the test results have been furnished to the Engineer.

Excavation to the final wall alignment for the full wall height shall incorporate a working berm which shall be constructed from the top down in a staged lift sequence as shown on the approved wall earthwork working drawings. The ground level in front of the wall face shall not be excavated more than 3 feet below the level of the row of soil nails or tieback anchors to be installed in that same lift.

In addition to the working berm described above, the Contractor shall maintain a stabilizing berm to support the excavation face during nail installation. The stabilizing berm shall extend horizontally from the bottom of the shotcrete a minimum distance of 0.3 meters and shall be cut down from that point at a slope as shown on the approved wall earthwork working drawings.

After soil nails are complete in place for a given lift, the stabilizing berm shall be removed during excavation to the final wall alignment. The complete excavated face shall be cleaned of all loose materials, mud, rebound, and other materials that could prevent or reduce shotcrete bond to the excavated face and soil nails.

Temporary backing or lagging for excavation at walls may be left in place. There shall be no voids behind wall temporary backing or lagging that is left in place. Fillers used to eliminate voids between the excavation face and temporary backing or lagging shall be dimensionally stable, non-deteriorating material capable of supporting the earth pressures in both a water saturated and dry condition.

The Contractor shall remove all cobbles, boulders, rubble, or debris which are encountered at the final wall alignment during wall face excavation and which protrude from the excavated face more than 5mm into the design shotcrete thickness as shown on the plans. Such over excavation shall be backfilled with shotcrete.

The Contractor shall immediately notify the Engineer of the occurrence of raveling or local instability of the final wall face excavation due to the presence of groundwater, soil conditions, equipment vibration or other causes.

Unstable area shall be temporarily stabilized by means of buttressing the exposed excavation face with an earth berm or other methods approved in writing by the Engineer. Construction of the wall in unstable areas shall be suspended until remedial measures, submitted by the Contractor, and approved by the Engineer, have been taken.

The Contractor shall protect installed nails during excavation and subsequent operations. Any nails damaged during construction shall be replaced by the Contractor, at the Contractor's expense.

Measurement and Payment--Excavation for soil nail wall construction will be measured and paid for as structure excavation (soil nail wall).

Full compensation for working drawings and supplements, and for furnishing, constructing and removing shoring, working berms, and stabilizing berms, for soil nail wall construction shall be considered as included in the contract price paid per cubic meter for structure excavation (soil nail wall) and no additional compensation will be allowed therefor.

Full compensation for shotcrete used to fill voids created by the removal of cobbles and boulders or other obstructions shall be considered as included in the contract price paid per cubic meter for shotcrete and no additional compensation will be allowed therefor.

10-1.38 SOIL NAIL ASSEMBLY

Soil nail assemblies and test soil nail assemblies, consisting of installing and grouting steel bars in holes drilled in natural foundation materials, anchorage system, and testing of test soil nail assemblies, shall conform to the details shown on the plans and the provisions of the Standard Specifications and these special provisions.

Foundation recommendations are included in the "Materials Information" available to the Contractor as provided for in Section 2-1.03, "Examination of Plans, Specifications, Contract, and Site of Work," of the Standard Specifications.

Attention is directed to "Order of Work" and "Soil Nail Wall Earthwork," elsewhere in these special provisions.

WORKING DRAWINGS.—The Contractor shall submit complete a working drawing submittal for soil nail assemblies to the Office of Structure Design (OSD) in conformance with the provisions in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications. All working drawings for soil nail assemblies shall be 559 mm by 864 mm in size. For initial review, 5 sets of drawings shall be submitted. After review, between 6 and 12 sets, as requested by the Engineer, shall be submitted to OSD for final approval and use during construction.

A working drawing submittal consists of working drawings and supplement. Working drawing submittals shall be submitted sufficiently in advance of the start of the work to allow time for review by the Engineer and correction by the Contractor without delaying the work.

Working drawing submittals for soil nail assemblies shall show the State assigned designations for the contract number,

structure number, full name of the structure as shown on the contract plans, and District-County-Route-Post mile on each drawing and calculation sheet. The Contractor's name, address, and phone number shall be shown on the working drawings. Each working drawing sheet shall be numbered in the lower right hand corner.

Working drawings for soil nail assemblies shall contain all information required for the construction and quality control of the soil nail wall construction, including the following:

1. The proposed schedule and detailed construction sequence of the installation and grouting of soil nails, application of shotcrete and construction of cast-in-place reinforced concrete.
2. Complete details and specifications of the soil nail and test soil nail including encapsulation materials and method of grouting the encapsulation, anchorage system, type of packers or other appropriate devices to be used to ensure partial length grouting of test soil nails.
3. Grout mix designs and procedures involved in testing grout.
4. Grout placement procedures and equipment including minimum required cure time.
5. Details of the equipment proposed for testing soil nails including jacking frame and appurtenant bracing, and the method and equipment for determining any displacement of the test soil nail relative to the grout during applications of test loads.
6. Information on space requirements for installation equipment.

The supplement to the working drawings shall include the following:

1. Calculations of soil nail capacities for each length of soil nail specified.
2. Calculations for design capacity of the load frame.
3. Construction details, structural details, and load test results from at least three previous successful installations by the Contractor for the same wall type as shown on the plans. The installations shall be from three separate construction sites. The installations shall be similar to those proposed for this contract.

The working drawing submittal shall be stamped and signed by an engineer who is registered as a Civil Engineer in the State of California.

The Contractor shall allow the Engineer 4 weeks to review the soil nail working drawing submittal, including supplement, after a complete set has been received. No soil nails shall be fabricated and installed until the Engineer has approved, in writing, the working drawing submittal for soil nail assemblies.

Should the Engineer fail to review the complete working drawing submittal within the time specified and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in reviewing the soil nail working drawing submittal, an extension of time commensurate with the delay in completion of the work thus caused will be granted in accordance with Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

MATERIALS.—The materials specified below shall be used for construction of soil nail assemblies and test soil nail assemblies.

Soil Nail.—Soil nail shall conform to the provisions for bar reinforcement in Section 52, “Reinforcement,” of the Standard Specifications. The soil nail shall be either a reinforcing bar encapsulated full length in a grouted corrugated plastic sheathing or an epoxy coated reinforcing bar. The bar shall be centered in the sheathing and the space between the sheathing and the bar shall be filled with grout. The epoxy coating shall have a minimum thickness of 305 µm.

Soil nail assemblies shall be lengthened or additional soil nail assemblies shall be installed when ordered by the Engineer. The lengthening or addition of soil nail assemblies when ordered by the Engineer will be paid for in accordance with the requirements in Section 4-1.03D of the Standard Specifications.

Soil nails shall be threaded a minimum length of 150 mm on the anchorage end. Threading may be continuous spiral deformed ribbing provided by the bar deformations or may be cut into a reinforcing bar. If threads are cut into a reinforcing bar, the bar size shall be the next larger bar designation number from that shown on the plans and coarse threads shall be used. The epoxy coating at the anchorage end of epoxy coated bars may be omitted for a maximum length of 150 mm. Metal surfaces of assembled splices of epoxy coated bars shall be epoxy coated.

Corrugated plastic sheathing shall be either polyvinyl chloride (PVC) or high density polyethylene (HDPE). The minimum sheathing wall thickness shall be 1015 µm.

HDPE shall have a density between 0.940- and 0.960-gram/cm^3 when measured in accordance with ASTM Designation: D 792, A-2.

The sheathing shall have sufficient strength to prevent damage during construction operations, shall be watertight, chemically stable without embrittlement or softening, and nonreactive with concrete.

Splicing of soil nails shall be made only at the locations shown on the plans or at ends of soil nails ordered by the Engineer to be lengthened.

Test Soil Nail.—Test soil nail shall conform to the provisions for bar reinforcement in Section 52, “Reinforcement,” of the Standard Specifications.

Test soil nail assemblies shall be lengthened or additional test soil nail assemblies shall be installed when ordered by the Engineer. The lengthening or addition of test soil nail assemblies when ordered by the Engineer will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications.

Test soil nail need not be epoxy coated or encapsulated in grouted plastic sheathing.

Splicing of test soil nails shall be made only at locations outside of the bonded length.

Anchorage System.—Anchorage for soil nail shall conform to the details shown on the plans and the provisions in Section 75-1.02, "Miscellaneous Iron and Steel," of the Standard Specifications except that nuts, washers, wedges and bearing plates to be fully encased in concrete, grout or shotcrete need not be galvanized. Concrete anchors on the bearing plates shall conform to the requirements for stud connectors in Section 55-2, "Materials," of the Standard Specifications.

The ultimate strength of the soil nail anchorage shall be at least the value shown below for the size of the soil nail bar shown on the plans.

BAR SIZE	ANCHORAGE ULTIMATE STRENGTH (Kilonewtons)
No. 16	120
No. 19	173
No. 25	302
No. 29	463

Grout.—Grout shall conform to the provisions in Section 50-1.09, "Bonding and Grouting," of the Standard Specifications. California Test 541 will not be required nor will the grout be required to pass through a screen with a 1.70 mm maximum clear opening prior to being introduced into the grout pump. Fine aggregate may be added to the grout mixture of portland cement and water in drilled holes 150 mm or greater in diameter, but only to the extent that the cement content of the grout is not less than 500 kilograms per cubic meter of grout. Fine aggregate, if used, shall conform to the provisions in Section 90-2, "Materials," and Section 90-3, "Aggregate Gradings," of the Standard Specifications.

CONSTRUCTION.—Soil nails shall be installed in drilled holes in an expeditious manner so that caving or deterioration of the drilled hole does not occur. No portion of the drilled hole shall be left open for more than 60 minutes prior to soil nail placement and grouting unless otherwise approved by the Engineer.

Drilling.—Drilling equipment shall be designed to drill straight and clean holes. The size and capability of the drilling equipment shall be as approved in the working drawings. Drill rigs shall have the capability of anchorage installation and grout placement through the use of drill casing or hollow-stem augers. Where caving conditions are specified elsewhere in these special provisions, sufficient casing and auger lengths shall be available on site to maintain uninterrupted installation of anchors.

Where hard drilling conditions such as rock, cobbles, boulders, or obstructions are specified elsewhere in these special provisions, a down hole pneumatic hammer drill rig and drill bit shall be available on site to drill holes for soil nail assemblies.

The Engineer will suspend wall construction if subsidence or other detrimental impact from drilling occurs.

Drilled holes for walls shall not extend beyond the right-of-way or easements limits as shown on the plans or as specified in these special provisions.

Holes shall be drilled in the natural foundation materials. Holes for soil nail assemblies and for test soil nail assemblies and represented soil nails shall be the same diameter.

Casing may be used to stabilize the holes, but shall be removed prior to or during the grouting operation.

Holes shall be cleaned to remove material resulting from the drilling operations and to remove any other material that would impair the strength of the soil nail assemblies or test soil nail assemblies. Foreign material dislodged or drawn into the holes during construction of the assemblies shall be removed. Water for cleaning holes shall not be used, unless full hole length hollow-stem augers or casing maintained in the same hole during cleaning and anchorage installation . Anchors and test anchors shall not be installed in the drilled holes until the holes have been inspected by the Engineer.

Installing Soil Nails and Test Soil Nails.—Soil nails and test soil nails shall be installed in the drilled holes using centralizers. Centralizers shall adequately support the bar in the center of the drilled hole and shall be spaced at a maximum of 1.5 meters on center.

Where the soil nail cannot be completely inserted, the Contractor shall remove the bar and clean or redrill the hole to permit unobstructed installation. Partially installed bars shall not be driven or forced into the drilled hole and will be rejected. When open-hole drilling methods are being used, the Contractor shall have hole cleaning tools on-site suitable for cleaning drilled holes along their full length just prior to bar insertion and grouting.

Grouting.—The length of drilled hole shall be verified by the Contractor before grouting.

Grout shall be injected at the low end of the drilled hole. The grout shall fill the drilled hole with a dense grout free of voids or inclusion of foreign material.

Soil nails shall be grouted full length.

Only the bonded length of test soil nails shall be initially grouted. Initial grouting shall be confined to the bonded length

by packers or other approved devices. For test soil nails, grouting of the remainder of the drilled hole shall not be done until after completing pullout tests.

Cold joints shall not be used in grout placement.

After placing the grout for soil nails and test soil nails, they shall remain undisturbed for the cure time stated in the approved soil nail working drawings.

Securing Soil Nails.—Any remaining void at the exterior end of the drilled hole for a soil nail assembly shall be filled with shotcrete and the soil nail secured at the face of the shotcrete. The steel bearing plate shall be seated with full bearing on the shotcrete surface and the nut for the soil nail shall be hand tightened before the initial set of the shotcrete. The nut shall be made wrench tight after the shotcrete has set for 24 hours, unless a shorter time is approved by the Engineer.

Securing Test Soil Nails.—Testing shall be performed against a temporary bearing yoke which bears directly on the shotcrete facing. Test loads transmitted through the temporary bearing yoke shall not fracture the shotcrete or cause displacement or sloughing of the soil surrounding the drilled hole. No part of the yoke shall bear within 150 mm of the edge of blockout.

Test soil nails shall be removed to behind the front face of the shotcrete after testing has been completed. The remaining length of void in the drilled hole shall be grouted and the blockout in the shotcrete facing filled with either grout or shotcrete.

TESTING.—Test soil nail assemblies shall be pullout tested by the Contractor in the presence of the Engineer. A pullout test consists of incrementally loading the test soil nail assembly to the maximum test load or failure point, whichever occurs first. Failure point shall be the point where the movement of the test soil nail continues without an increase in the load or when the soil nail has displaced 13 mm. The failure load corresponding to the failure point shall be recorded as part of the test data.

The Contractor shall monitor and record displacement of the test soil nail relative to the grout during application of the test load.

Applied test loads shall be determined with either a calibrated pressure gage or a load cell. Movements of the end of the soil nail, relative to an independent fixed reference point, shall be measured and recorded to the nearest 25 µm at each increment of load, including the ending alignment load, during the load tests.

The pressure gage shall have an accurately reading dial at least 150 mm in diameter and each jack and its gage shall be calibrated as a unit with the cylinder extension in the approximate position that it will be at final jacking force, and shall be accompanied by a certified calibration chart. The load cell shall be calibrated and shall be provided with an indicator by means of which the test load in the soil nail may be determined. The range of the load cell shall be such that the lower 10 percent of the manufacturer's rated capacity will not be used in determining the jacking force.

The test load may be verified by State forces with either State-furnished load cells or pressure cells, or with State-furnished Vibra-Tension equipment operated in accordance with California Test 677. The Contractor shall provide sufficient labor, equipment, and material to install and support such testing equipment at the soil nails and to remove the testing equipment after the testing is complete, as ordered by the Engineer.

The pullout test procedures shall conform to the following:

The pullout test shall be conducted by measuring the test load applied to the test soil nail and the test soil nail end movement at each load listed in the following loading schedule.

PULLOUT TEST LOADING SCHEDULE

AL
0.25M
0.375M
0.50M
0.625M
0.75M
0.875M
1.00M (PULLOUT TEST LOAD)
AL

(M = PULLOUT TEST LOAD (kN) = $(0.01414\sigma_b)(D)$)
(σ_b = Ultimate bond stress between grout and the drilled hole, as shown on the plans, in kPA)
(D = Actual drilled hole diameter in millimeters)
(Round value of M to the nearest 10kN)
(AL = ALIGNMENT LOAD = 0.125M)

Each increment of load shall be applied in less than one minute and held for at least one minute but not more than 2 minutes, except that load equal to 1.0M shall be held for 10 minutes. During the 10-minute load hold, the movement of the end of the soil nail shall be measured at 1, 2, 3, 4, 5, 6, and 10 minutes. The observation period for the 10-minute load hold shall start when the pump begins to apply the increment of load from 0.875M to 1.0M. If the movement measured between one minute and 10 minutes is less than 2 mm, the remaining increments of load shall be applied, including the ending alignment load.

If the load of 1.0M cannot be maintained for 10 minutes with 2 mm or less movement, the 1.0M load shall be maintained for an additional 50 minutes. Total movement shall be measured at 15, 20, 25, 30, 45, and 60 minutes. After the 60 minute movement measurement the remaining increments of load shall be applied, including the ending alignment load.

The soil nail shall be unloaded only after completion of the test.

The Contractor shall furnish to the Engineer complete test results for each soil nail assembly tested. Data for each test shall list key personnel, test loading equipment, test soil nail location, hole diameter and depth, bonded length, type of soil, and amount of ground water encountered within the bond length. Test data shall also include the dates and times of drilling, test soil nail installation, grouting, and testing. The test load and amount of displacement shall be included in the test data when any displacement of the test soil nail relative to the grout occurs during application of the test load.

MEASUREMENT.—Soil nail assembly and test soil nail assembly will be measured and paid for by the meter. The length to be paid for will be the length of soil nail assembly or test soil nail assembly measured along the bar centerline from the back face of shotcrete to the tip end shown on the plans or ordered in writing by the Engineer.

PAYMENT.—The contract price paid per meter for soil nail assembly shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in constructing the soil nail assemblies, complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Test soil nail assemblies will be paid for as soil nail assembly.

Full compensation for testing of the test soil nail assemblies shown on the plans shall be considered as included in the contract price paid per meter for soil nail assembly and no separate payment will be made therefor.

Full compensation for architectural treatment at soil nail retaining walls shall be considered as included in the contract price paid per meter for soil nail assembly and no additional compensation will be allowed therefor.

10-1.39 EROSION CONTROL (NETTING)

Erosion control (netting) shall conform to the details shown on the plans, the provisions in Section 20-3, "Erosion Control," of the Standard Specifications and these special provisions.

Erosion control (netting) work shall consist of installing erosion control netting to embankment slopes and excavation slopes as shown on the plans and other areas designated by the Engineer.

Following the installation of erosion control netting, erosion control materials shall be applied onto the netting face as specified in Erosion Control (Type D) of these specifications.

MATERIALS.—Materials shall conform to the provisions in Section 20-2, "Materials," of the Standard Specifications and the following:

SEED.—Seed for erosion control (netting) shall conform to the provisions specified for seed under "Erosion Control (Type D)" elsewhere in these special provisions.

EROSION CONTROL NETTING.—Erosion control netting shall consist of 100 percent spun coil fiber and shall conform to the following:

Properties

ASTM 3776C Weight	400 grams/sq. mtr.
Roll Width (minimum	2 m
Area/roll (minimum)	200 sq. mtr.
Open Area (maximum)	63-70%
D4595-86 Minimum tensile Strength	0.23/0.14 KN (dry) 0.17/0.11 KN (wet)

Staples for erosion control netting shall be made of 11-gage minimum steel wire and shall be U-shaped with 203 mm legs and 25-mm crown.

INSTALLATION.—Erosion control (netting) materials shall be placed as shown on the plans.

Erosion control netting strips shall be placed loosely on the slope with the longitudinal joints perpendicular to the slope contour lines. Longitudinal and transverse joints of netting shall be overlapped a minimum of mm and stapled as shown on the plans. Staples shall be driven perpendicular to the netting such that the top of the staple is flush with the ground surface. Stapling pattern shall be located and spaced in accordance with the plans.

MEASUREMENT AND PAYMENT.—The quantity of erosion control (netting) to be paid for will be determined by the square meter from actual measurement of the area covered by the erosion control netting.

The contract price paid per square meter for erosion control (netting) shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in erosion control (netting), complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.40 EROSION CONTROL (TYPE D)

Erosion control (Type D) shall conform to the provisions in Section 20-3, "Erosion Control," of the Standard Specifications and these special provisions.

Erosion control (Type D) work shall consist of applying erosion control materials to the surface of all reconstructed embankment slopes and excavation slopes, and all other areas disturbed by grading, removal of pavement, or clearing of vegetation and other areas designated by the Engineer. Erosion control (Type D) shall be applied at the completion of all work during the period starting September 1 and ending October 31; or, if the slope on which the erosion control is to be placed is finished during the winter season as specified in "Water Pollution Control" elsewhere in these special provisions the erosion control shall be applied immediately; or, if the slope on which the erosion control is to be placed is finished outside both specified periods and the contract work will be completed before September 1, the erosion control shall be applied as a last item of work.

In locations shown on plans with Erosion Control Netting, Erosion Control (Type D) shall be applied upon completion of Erosion Control Netting and fiber rolls installation as described elsewhere in these special provisions.

Prior to installing erosion control materials, soil surface preparation shall conform to the provisions in Section 19-2.05, "Slopes," of the Standard Specifications, except that rills and gullies exceeding 50 mm in depth or width shall be leveled. Vegetative growth, temporary erosion control materials and other debris shall be removed from areas to receive erosion control.

MATERIALS.—Materials shall conform to Section 20-2, "Materials," of the Standard Specifications and the following:

SEED.—Seed shall conform to the provisions in Section 20-2.10, "Seed," of the Standard Specifications. Individual seed species shall be measured and mixed in the presence of the Engineer.

Seed not required to be labeled under the California Food and Agricultural Code shall be tested for purity and germination by a seed laboratory certified by the Association of Official Seed Analysts, or a seed technologist certified by the Society of Commercial Seed Technologists.

Seed shall have been tested for purity and germination not more than one year prior to application of seed.

Results from testing seed for purity and germination shall be furnished to the Engineer prior to applying seed.

NON-LEGUME SEED.—Non-legume seed shall consist of the following:

NON-LEGUME SEED

Botanical Name (Common Name)	Percent Germination (Minimum)	Kilograms pure live seed per hectare (Slope measurement)
Bromus carinatus* (California Brome)	40	20
Hordeum brachyantherum salt* (California Meadow Barley)	40	20
Deschampsia caespitosa* (Tufted Hairgrass)	40	5
Leymus triticoides 'Rio'* (Creeping Wildrye)	40	15
Atriplex patula (Beach Saltbush)	40	1
Hordeum vulgare 'UC 603" Cereal Barley	95	50
Vulpia microstachys* (Three Weeks Fescue)	45	5

Seed shall be delivered to the job site in unopened separate containers with the seed tag attached. Containers without a seed tag will not be accepted.

A sample of approximately 30 g of seed will be taken from each seed container by the Engineer.

COMMERCIAL FERTILIZER.—Commercial fertilizer shall conform to the provisions in Section 20-2.02, "Commercial Fertilizer," of the Standard Specifications and shall have a guaranteed chemical analysis of 6 percent nitrogen, 20 percent phosphoric acid and 20 percent water soluble potash.

STRAW.—Straw shall be derived from wheat and barley. Wheat and barley straw shall not be derived from dry farmed cereal crops.

STABILIZING EMULSION.—Stabilizing emulsion shall conform to the provisions in Section 20-2.11, "Stabilizing Emulsion," of the Standard Specifications and these special provisions.

The requirement of an effective life of at least one year for stabilizing emulsion shall not apply.

Stabilizing emulsion shall be in a dry powder form, may be reemulsifiable, and shall be a processed organic adhesive derivative of Plantago insularis used as a soil binder.

APPLICATION.—Erosion control materials shall be applied to the areas shown on the plans as Erosion Control (Type D) in 3 separate applications in the sequence specified below: At areas shown on plans as Erosion Control Netting with Erosion Control (Type D), only the first of the following applications specified below shall be applied after installation of Erosion Control Netting and Fiber Rolls.

The following mixture in the proportions indicated shall be applied with hydro-seeding equipment within 30 minutes after the seed has been added to the mixture: Hydroseeding shall be applied by hose from the ground to all areas with erosion control netting. Erosion control materials shall be applied at close range onto the netting face such that the materials are well integrated into the netting and in close contact with the ground surface. Application shall be perpendicular to the slope face such that the application does not displace the netting. Any netting displacement shall be immediately repaired by the Contractors.

Material	Kilograms per hectare (Slope measurement)
Fiber	560
Non-Legume Seed	121

Straw shall be applied at the rate of 4 tonnes per hectare based on slope measurements. Incorporation of straw will not be required.

The following mixture in the proportions indicated shall be applied with hydro-seeding equipment:

Material	Kilograms per hectare (Slope measurement)
Fiber	560
Stabilizing emulsion (solids)	135

The ratio of total water to total stabilizing emulsion in the mixture shall be as recommended by the manufacturer.

Once straw work is started in an area, the remaining applications shall be completed in that area on the same working day.

The proportions of erosion control materials may be changed by the Engineer to meet field conditions.

MEASUREMENT AND PAYMENT.—The quantity of pure live seed (erosion control) to be paid for by the kilogram will be determined by multiplying the percentage of purity by the percentage of germination by the marked mass on the sack.

Pure live seed (erosion control) will be paid for by the kilogram in the same manner specified for seed in Section 20-3.07 of the Standard Specifications.

10-1.41 FIBER ROLLS

Fiber rolls shall conform to the details shown on the plans and to the provisions in these special provisions.

MATERIALS

Fiber rolls shall consist of one of the following:

Fiber rolls shall be constructed on the site with manufactured blankets consisting of one or a combination of wood excelsiorrice, wheat or coconut fibers. The blanket shall measure approximately 3.5 meters wide by 26 to 29 meters in length. Wood excelsior material shall have individual fibers, 80 percent of which shall be 150 mm or longer in fiber length. The blanket shall have a photodegradable plastic netting. The blanket shall be rolled on the blanket's width and secured with jute twine spaced 2 meters apart along the roll for the full length and 150 mm from each end of the individual rolls. The blanket shall be rolled so that the netting is on the outside of the finished roll. The finished roll diameter shall be a minimum of 175 mm and a maximum of 225 mm and shall weigh not less than 1.3 kg per meter.

Fiber rolls shall be pre-manufactured rice or wheat straw, wood excelsior or coconut fiber rolls encapsulated within a photodegradable plastic netting. Each roll shall be a minimum of 175 mm and a maximum of 225 mm in diameter and 7 to 9 meters in length and shall weigh not less than 1.3 kg per meter. The netting shall be ultraviolet (UV) degradable plastic. The netting shall have a minimum durability of one year after installation. The netting shall be secured tightly at each end of the individual rolls.

Stakes shall be fir or pine and shall be a minimum of 25 mm x 25 mm x 600 mm in length. Metal stakes may be used as an alternative. The Contractor shall submit a sample of the metal stake to the Engineer prior to installation. The tops of the metal stakes shall be bent over at a 90-degree angle. No additional compensation will be allowed for the use of a metal stake.

INSTALLATION

Fiber rolls shall be joined tightly together to form a single linear roll that is installed approximately parallel to the slope contour. Fiber rolls shall be installed prior to the application of other erosion control materials.

Furrows shall be constructed at a slight angle to the slope contour shown on the plans, to a depth of 50 to 100 mm, and at a sufficient width to hold the fiber rolls.

Individual rolls shall be placed with adjacent ends butted firmly to each other to create a continuous linear roll.

Stakes shall be installed 1.2 meters apart along the total length of the rolls and 125 mm from the end of each individual roll. Stakes shall be driven flush or a maximum of 50 mm above the roll.

MEASUREMENT AND PAYMENT

Fiber rolls will be measured by the meter from end to end along the centerline of the installed rolls.

The contract price paid per meter for fiber rolls shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in installing fiber rolls, complete in place, including stakes, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.42 IRRIGATION CROSSOVERS

Irrigation crossovers shall conform to the provisions in Section 20-5, "Irrigation Systems," of the Standard Specifications and these special provisions.

Irrigation crossovers shall include conduits, water line crossovers, sprinkler control crossovers and appurtenances. Sizes of the conduits, water line crossovers and sprinkler control crossovers shall be as shown in the table for "Irrigation Crossovers" in the plans.

Conduits shall be installed in accordance with the provisions in Section 20-5.03B, "Conduit for Water Line Crossovers and Sprinkler Control Crossovers," of the Standard Specifications.

Conduits shall be 200 mm welded steel pipe conduit (6.35 mm thick).

Water line crossovers shall conform to the provisions in Section 20-5.03C, "Water Line Crossovers," of the Standard Specifications, and shall be polyvinyl chloride (PVC) plastic pipe, 1120 or 1220. PVC plastic pipe water line crossovers shall have a minimum pressure rating (PR) of 315 unless otherwise shown on the plans.

Sprinkler control crossovers shall conform to the provisions in Section 20-5.027D, "Sprinkler Control Crossovers," of the Standard Specifications.

Installation of pull boxes shall conform to the provisions in Section 20-5.027I, "Conductors, Electrical Conduit and Pull Boxes," of the Standard Specifications. When no conductors are installed in electrical conduits, pull boxes for irrigation crossovers shall be installed on a foundation of compacted soil.

Full compensation for sprinkler control crossovers, water line crossovers, pavement markers, and appurtenances, and for pressure testing water line crossover in the conduit shall be considered as included in the contract price paid per meter for 200 mm welded steel pipe conduit (6.35 mm thick) and no additional compensation will be allowed therefor.

10-1.43 EXTEND IRRIGATION CROSSOVERS

Extend existing irrigation crossovers shall conform to the provisions in Section 20-5, "Irrigation Systems," of the Standard Specifications and these special provisions.

Extend irrigation crossovers shall include conduit, water line crossover extensions, and sprinkler control crossover and appurtenances, locating existing irrigation crossovers and pressure testing existing and new water line crossovers. The sizes of conduit, water line crossover extensions and sprinkler control crossover shall be as shown on the plans.

Before any work is started in an area where an existing irrigation crossover conduit is to be extended, the existing conduit shall be located by the Contractor. When exploratory holes are used to locate the existing conduit, the exploratory holes shall be excavated in accordance with the provisions in Section 20-5.03B, "Conduit for Water Line Crossovers and Sprinkler Control Crossovers," of the Standard Specifications.

If debris is encountered in the ends of conduits to be extended, the debris shall be removed prior to extending conduits. Removal of debris within the first meter in the conduits shall be at the Contractor's expense. If debris is encountered in the conduit more than one meter from the ends of the conduits to be extended, the additional debris shall be removed as directed by the Engineer. When directed by the Engineer, removal of debris more than one meter from the ends in the conduits will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications.

Prior to installation of water line crossover extensions, the existing water lines shall be pressure tested for leakage in accordance with the provisions in Section 20-5.03H, "Pressure Testing," of the Standard Specifications. Repairs to the existing water line crossover, when ordered by the Engineer, will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications.

Conduit extensions shall be high density polyethylene (HDPE) pipe and nestable corrugated steel pipe as shown on the plans.

Water line crossover extensions shall be plastic pipe (PR 315) supply line.

Sprinkler control crossovers shall be Type 3 electrical conduit.

Conductors shall be removed from existing sprinkler control crossovers.

After installation of the sprinkler control crossover, new conductors shall be installed without splices. New conductors shall match the removed conductors in color and size and shall be spliced to the existing conductors in adjacent pull boxes. After the new conductors are installed, the conductors shall be tested in the same manner specified for traffic signal, sign illumination, and lighting circuits in accordance with the provisions in Section 86-2.14B, "Field Testing," of the Standard Specifications.

After water line crossover extensions have been installed, existing and extended water line crossovers shall be retested for leakage in accordance with the provisions in Section 20-5.03H, "Pressure Testing," of the Standard Specifications. Leaks that develop shall be repaired at the Contractor's expense, and the water line crossovers shall be retested until a satisfactory pressure test is achieved.

Full compensation for locating, excavation, backfill, water line crossovers, sprinkler control crossovers, conductors, appurtenances, testing and retesting shall be considered as included in the contract price paid per meter for extend 200 mm conduit and no additional compensation will be allowed therefor.

10-1.44 NPS 3 SUPPLY LINE (BRIDGE)

Water supply lines identified on the plans as supply line (bridge) shall be of the size shown and shall conform to the details shown on the plans, the provisions in Section 20-5, "Irrigation Systems," of the Standard Specifications and these special provisions.

Unless otherwise shown on the project plans, casings shall be installed at each abutment, and casings shall be extended to the greater of: (1) 1.5 m beyond the approach slab, (2) 1.5 m beyond the end of the adjacent wingwall or (3) 6 m beyond the abutment.

Working Drawings.—The Contractor shall submit complete working drawings for temporary support of the casing at the abutments to the Office of Structure Design (OSD) in accordance with the provisions in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications.

For initial review, 5 sets of drawings shall be submitted. After review, between 6 and 12 sets, as requested by the Engineer, shall be submitted to OSD for final approval and use during construction.

MATERIALS:

Pipe and Fittings for Supply Lines Less Than NPS 4.—Pipe and fittings for supply lines less than NPS 4 shall conform to the provisions of Section 20-2.15A, "Steel Pipe," of the Standard Specifications.

Air Release Valve Assembly for Supply Lines Less Than NPS 4.—Air release valve assembly for supply lines less than NPS 4 shall consist of a line size threaded tee or pipe saddle, NPS 1 ball valve, automatic air release valve, and tank vent. Air release valve shall have cast iron body with stainless steel trim, stainless steel float, NPS 1 inlet pipe connection and 4.8 mm orifice. Tank vent shall be the size of air release valve outlet and have a double opening facing down with screen cover.

Casing Insulators for Supply Lines Less Than NPS 4.—Casing insulators for supply lines less than NPS 4 shall be designed for the size of casing and supply line shown on the plans. Casing insulators for supply line shall be high density, injection molded polyethylene, 2-piece construction with cadmium plated nuts and bolts and shall have a non-conductive inner liner. Casing insulators shall be factory constructed to ensure the supply line is centered in the casing to avoid any pipe to pipe contact and shall have at least two runners seated on the bottom of the casing.

Pipe End Seals for Supply Lines Less Than NPS 4.—Pipe end seals for supply lines less than NPS 4 shall cover the space between the supply line and the end of the casing. Pipe end seals shall be made with 50 mm thick construction grade redwood and cut to fit the supply line.

Expansion Assembly for Supply Lines Less Than NPS 4.—Expansion assembly for supply lines less than NPS 4 shall be hose type.

Hose shall be medium or heavy weight, oil resistant, flexible, rubber or synthetic rubber cover and tube, reinforced with not less than 2-plies of synthetic yarn or steel wire and shall be equipped with steel flanges. The hose and flange assembly shall be rated for a working pressure of not less than 1.4 MPa.

Hoses carrying potable water shall meet Food and Drug Administration standards.

All parts of the expansion assembly shall have the same nominal inside diameter as the supply line.

Insulated Flange Connection.—Insulated flange connection shall consist of dielectric flange gasket, insulating washers and sleeves held in place with steel bolts and nuts. Gasket shall have dielectric rating of 500 V/0.025-mm, minimum.

Casing.—Casing shall be welded steel pipe conforming to Section 70-1.02B, "Welded Steel Pipe," of the Standard Specifications, except that the pipe shall be treated in accordance with the following requirements, prior to shipping. Exterior surfaces of welded steel pipe shall be cleaned and coated in accordance with specifications of ANSI/AWWA C213 or at the option of the Contractor cleaned, primed and coated in accordance with specifications of ANSI/AWWA C214.

Pipe Wrapping Tape.—Wrapping tape for pipe in contact with the earth shall be a pressure sensitive polyvinyl chloride or polyethylene tape with a minimum thickness of 1.27 mm.

Pipe Hanger Assembly.—Pipe hanger assembly shall consist of concrete clevis plate or embedded steel welded linked eye rods, adjustable steel yoke, cast iron pipe roller, steel roller rod and hex nuts. Parts shall be galvanized. The pipe hanger assembly shall be suitable for the type and size of pipe installed and shall be as shown on the plans.

Steel hangers, anchor bolts, pipe clamps, nuts and bolts, and other fittings shall be suitable for the type and size of the supply lines or casing and shall conform to the provisions in Section 75-1.03, "Miscellaneous Bridge Metal," of the Standard Specifications.

Concrete Pipe Supports.—Concrete pipe supports shall consist of either a precast or cast-in-place concrete pipe cradle, galvanized steel pipe clamp, 2 anchor bolts and where shown on the plans, a stainless steel pipe protection shield.

Concrete pipe supports and pipe stops shall conform to the dimensions shown on the plans and shall be constructed of commercial quality concrete with a cement content not less than 350 kg of portland cement per cubic meter, and commercial quality wire mesh. The concrete for pipe supports and pipe stops shall be moist cured for not less than 3 days.

Steel anchor bolts, nuts, pipe clamps and other fittings shall be suitable for the type and size of the supply lines or casing and shall conform to the provisions in Section 75-1.03, "Miscellaneous Bridge Metal," of the Standard Specifications.

Epoxy Adhesive.—Epoxy adhesive shall conform to the provisions in Section 95-1, "General," of the Standard Specifications and at the option of the Contractor, shall conform to the provisions in Section 95-2.03, "Epoxy Resin Adhesive for Bonding New Concrete to Old Concrete," or in Section 95-2.04, "Rapid Set Epoxy Adhesive for Pavement Markers," or in Section 95-2.05, "Standard Set Epoxy Adhesive for Pavement Markers," of the Standard Specifications.

INSTALLATION:

Water supply lines in bridge structures shall be supported as shown on the plans and in conformance with these special provisions.

If a blockout is provided in the bridge abutment wall for casing, the space between the casing and bridge abutment wall shall be filled with portland cement mortar conforming to the provisions in Section 51-1.135, "Mortar," of the Standard Specifications.

When the bridge superstructure is to be prestressed, the space around supply lines through abutments shall not be filled until the prestressing has been completed.

Openings for supply lines through bridge superstructure concrete shall either be formed or shall consist of pipe sleeves.

Cleaning and Closing of Pipe.—The interior of the pipe shall be cleaned before installation. Openings shall be capped or plugged as soon as the pipe is installed to prevent the entrance of any materials. The caps or plugs shall remain in place until their removal is necessary for completion of the installation.

Wrapping and Coating Pipe.—Damaged coating on supply line pipe in contact with the earth shall be wrapped with tape as follows:

1. Pipe to be wrapped shall be thoroughly cleaned and primed as recommended by the tape manufacturer.
2. Tapes shall be tightly applied with one-half uniform lap, free from wrinkles and voids to provide not less than 2.5 mm thickness.
3. Field joints and fittings for wrapped pipe shall be covered by double wrapping 1.27 mm thick tape. Wrapping at joints shall extend a minimum of 150 mm over adjacent pipe coverings. Width of tape for wrapping fittings shall not exceed 50 mm. Adequate tension shall be applied so tape will conform closely to contours of joint.

Where a casing passes through the abutment wall, the casing shall be wrapped with an additional 2 layers of asphalt-felt building paper conforming to the requirements of ASTM Designation: D 226, Type I, securely taped or wired in place.

TESTING

Water supply lines less than NPS 4 shall be tested in accordance with the provisions in Section 20-5.03H(1), "Method A," of the Standard Specifications, except that the testing period shall be 4 hours minimum with no leakage or pressure drop.

The Contractor shall furnish pipe anchorages to resist thrust forces occurring during testing. Leaks shall be repaired and defective materials shall be replaced by the Contractor at the Contractor's expense.

Pressure testing and necessary repairing of water lines shall be completed prior to backfilling, placing deck slabs over supply lines in box girder cells, or otherwise covering the supply lines.

Each end of water line shall be capped prior to and after the testing.

The supply line shall be tested as one unit. The limits of the unit shall be 1.5 m beyond the casing at each end of the bridge.

MEASUREMENT AND PAYMENT

Measurement and payment for NPS 3 supply line (bridge) for each size listed in the Engineer's Estimate shall be made in the same manner as galvanized steel pipe and plastic pipe supply lines in Sections 20-5.04, "Measurement," and 20-5.05, "Payment," of the Standard Specifications.

Full compensation for furnishing and installing air release valves, steel hangers, steel brackets, and other fittings, casing and casing insulators, pipe end seals, concrete supports, testing and checking, concrete pipe stops, pipe wrapping tape, epoxy adhesives and expansion assemblies shall be considered as included in the contract prices paid per meter for the sizes of NPS 3 supply line (bridge) involved and no additional compensation will be allowed therefor.

10-1.45 AGGREGATE SUBBASE

Aggregate subbase shall be Class 4 and shall conform to the provisions in Section 25, "Aggregate Subbases," of the Standard Specifications and these special provisions.

Section 25-1.02B, "Class 4 Aggregate Subbase," of the Standard Specifications is amended by adding the following sentences:

Aggregate may include or consist of material processed from reclaimed asphalt concrete, portland cement concrete, lean concrete base, cement treated base, glass or a combination of any of these materials. Aggregate subbase incorporating reclaimed glass shall not be placed at locations where surfacing will not be placed over the aggregate subbase.

The percentage composition by mass of Class 4 aggregate subbase shall conform to the following grading requirements:

Grading Requirements (Percentage Passing)

Sieve Sizes	Operating Range	Contract Compliance
63-mm	100	100
4.75-mm	35-65	25-70
75-µm	0-15	0-18

Class 4 aggregate subbase shall also conform to the quality requirements shown in the following table:

Quality Requirements

Test	Operating Range	Contract Compliance
Sand Equivalent	21 Min.	18 Min.
Resistance (R-value)	----	50 Min.

The requirements of the last four paragraphs in Section 25-1.02A, "Class 1, Class 2, and Class 3 Aggregate Subbases," of the Standard Specifications shall apply to Class 4 aggregate subbase.

At the option of the Contractor, Class 1 aggregate subbase conforming to the grading and quality requirements in Section 25-1.02A, may be used in place of Class 4 aggregate subbase. Once a class of aggregate subbase is selected, the class shall not be changed without written approval of the Engineer. The first paragraph of Section 25-1.02A is amended by adding the following sentences:

Aggregate may include or consist of material processed from reclaimed asphalt concrete, portland cement concrete, lean concrete base, cement treated base, glass or a combination of any of these materials. Aggregate subbase incorporating reclaimed glass shall not be placed at locations where surfacing will not be placed over the aggregate subbase.

Regardless of the class of aggregate subbase supplied under the provisions in this section, payment for all such aggregate subbase will be made as Class 4 aggregate subbase.

10-1.46 AGGREGATE BASE

Aggregate base shall be Class 3 and shall conform to the provisions in Section 26, "Aggregate Bases," of the Standard Specifications and these special provisions.

The first paragraph of Section 26-1.02B, "Class 3 Aggregate Base," of the Standard Specifications is amended by adding the following sentences:

Aggregate may include or consist of material processed from reclaimed asphalt concrete, portland cement concrete, lean concrete base, cement treated base, glass or a combination of any of these materials. Aggregate base incorporating reclaimed glass shall not be placed at locations where surfacing will not be placed over the aggregate base.

Aggregate for Class 3 aggregate base shall conform to the following requirements:

Grading Requirements (Percentage Passing)
19-mm Maximum

Sieve Sizes	Operating Range	Contract Compliance
50-mm	----	----
37.5-mm	----	----
25-mm	100	100
19-mm	90-100	87-100
4.75-mm	35-60	30-65
600-µm	10-30	5-35
75-µm	2-11	0-14

Except for the grading requirements, all requirements pertaining to Class 2 aggregate base as given in Section 26-1.02A, "Class 2 Aggregate Base," of the Standard Specifications shall apply to Class 3 aggregate base.

The aggregate shall not be treated with lime, cement or other chemical material before the Durability Index test is performed. Untreated reclaimed asphalt concrete and portland cement concrete will not be considered to be treated with lime, cement or other chemical material for purposes of performing the Durability Index test.

10-1.47 SOIL STERILIZATION

Soil shall be sterilized in minor concrete (colored paving) areas in accordance with these special provisions.

Pesticides used for soil sterilization shall conform to the provisions in Section 20-4.026, "Pesticides," of the Standard Specification, except recommendations from a Pest Control Adviser will not be required.

Soil under minor concrete (colored paving) areas shown on the plans to be sterilized, shall be graded to a neat and smooth surface. Immediately prior to sterilizing, the soil shall be scarified to a minimum depth of 75 mm and rocks, large lumps of earth, weeds, and other debris shall be removed and disposed of. Removed material shall be disposed of outside the highway right of way in accordance with the provisions in Section 7-1.13, "Disposal of Material Outside the Highway Right of Way," of the Standard Specifications. The loosened material shall then be brought to as finely divided condition as the material will permit.

After scarification, the soil shall be sterilized with one of the following pesticides applied at the maximum label rate unless otherwise directed by the Engineer:

1. Trifluralin emulsifiable concentrate.

Pesticides shall be mixed according to the manufacturer's recommendations and shall be applied by a device approved by the Engineer. Pesticides shall not be applied more than 8 hours prior to placing minor concrete (colored paving) surfacing. Pesticides shall not be applied more than 300 mm beyond the areas to be surfaced.

Treated areas shall be thoroughly compacted by use of power rollers. When power rollers cannot be operated in certain areas due to the shape or size of the areas, compaction shall be obtained by hand rollers, impactors, or other methods approved by the Engineer.

Full compensation for sterilizing soil shall be considered as included in the contract prices paid for minor concrete (colored paving) involved and no additional compensation will be allowed therefor.

10-1.48 ASPHALT CONCRETE

Asphalt concrete, asphalt concrete (leveling), and asphalt concrete base shall be Type A and shall conform to the provisions in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions and these special provisions.

Attention is directed to "Lightweight Fill Material" elsewhere in these special provisions. A sixty (60) calendar day settlement period shall elapse between placement of lightweight fill and any subsequent asphalt concrete paving over the same.

Surfacing of miscellaneous areas with asphalt concrete shall conform to the provisions in "Asphalt Concrete (Miscellaneous Areas)" elsewhere in these special provisions.

The aggregate for asphalt concrete base (type A) shall conform to the 19mm Maximum, Coarse grading specified in Section 39-2.03, "Aggregate," in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions. The aggregate for asphalt concrete (type A) shall conform to the 19 mm Maximum, Medium grading specified in Section 39-2.03, "Aggregate," in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions. The aggregate for asphalt concrete (Leveling) shall conform to the 12.5 mm Maximum, Coarse grading specified in Section 39-2.03, "Aggregate," in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions.

If the Contractor selects the batch mixing method, asphalt concrete shall be produced by the automatic batch mixing method as provided in Section 39-5.03B, "Automatic Proportioning," in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions.

In addition to the aggregate requirements listed in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions, the combined aggregates shall conform to the following quality requirement when mixed with paving asphalt Grade AR-4000 in the amount of asphalt determined to be optimum by California Test 367:

Quality Requirement

Test	California Test	Requirement
Surface Abrasion	360, Method A	Loss not to exceed 15 grams

In addition to the requirements in Section 39-7.01, "Spreading Equipment," in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions, asphalt paving equipment shall be equipped with automatic screed controls and a sensing device or devices.

When placing asphalt concrete to lines and grades established by the Engineer, the automatic controls shall control the longitudinal grade and transverse slope of the screed. Grade and slope references shall be furnished, installed and maintained by the Contractor. Should the Contractor elect to use a ski device, the minimum length of the ski device shall be 9 m. The ski device shall be a rigid one piece unit and the entire length shall be utilized in activating the sensor.

When placing the initial mat of asphalt concrete on existing pavement, the end of the screed nearest the centerline shall be controlled by a sensor activated by a ski device not less than 9 m long. The end of the screed farthest from centerline shall be controlled by an automatic transverse slope device set to reproduce the cross slope designated by the Engineer.

When paving contiguously with previously placed mats, the end of the screed adjacent to the previously placed mat shall be controlled by a sensor that responds to the grade of the previously placed mat and will reproduce the grade in the new mat within a 3-mm tolerance. The end of the screed farthest from the previously placed mat shall be controlled in the same manner as when placing the initial mat.

Should the methods and equipment furnished by the Contractor fail to produce a layer of asphalt concrete conforming to the requirements, including straightedge tolerance, of Section 39-8.04, "Compacting," in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions, the paving operations shall be discontinued and the Contractor shall modify the equipment or methods, or furnish substitute equipment.

Should the automatic screed controls fail to operate properly during any day's work, the Contractor may use manual control of the spreading equipment for the remainder of that day, however, the equipment shall be corrected or replaced with alternative automatically controlled equipment conforming to the requirements in this section before starting another day's work.

In addition to the straightedge requirements in Section 39-8.04, "Compacting," in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions, asphalt concrete pavement shall conform to the surface tolerances specified herein.

The top surface of the uppermost layer of asphalt concrete surfacing shall be profiled, by the Contractor in the presence of the Engineer, using a California Profilograph or equivalent in accordance with California Test 526 and as specified in these special provisions. Prior to beginning profiles, the profilograph shall be calibrated in the presence of the Engineer. Profiles shall be made on the traveled way one meter from and parallel to each edge of traveled way and at the approximate location of the planned lane lines.

Pavement so profiled shall conform to the following Profile Index requirements:

1. Pavement on tangent alignment and pavement on horizontal curves having a centerline radius curve of 600 m or more shall have a Profile Index of 80 mm per kilometer or less for each 160-m section profiled.
2. Pavement on horizontal curves having a centerline radius curve of 300 m or more but less than 600 m including the pavement within the superelevation transition of such curves, and pavement thicker than 61-mm total thickness placed on existing surfacing, shall have a Profile Index of 160-mm per kilometer or less for each 160-m section profiled.
3. Pavement shall not have individual deviations in excess of 8 mm, as determined by California Test 526. The location of the profiles for determining deviations shall be designated by the Engineer.

Checking the following areas of pavement surface with the California Profilograph or equivalent will not be required:

1. Pavement on horizontal curves having a centerline radius curve of less than 300 m and pavement within the superelevation transition of such curves.
2. Pavement with a total thickness of 61 mm or less, or pavement with extensive grade correction which does not receive advance leveling operations as specified in Section 39-8.03, "Spreading," in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions, or where the edge of asphalt concrete conforms to curbs or gutters with a Profile Index greater than 80 mm per kilometer.
3. Pavement for ramps and connectors with steep grades and high rates of superelevation and short sections of city or county streets and roads.
4. Pavement within 15 m of a transverse joint that separates the pavement from an existing pavement not constructed under the contract.
5. All shoulders and miscellaneous areas.

The Contractor shall schedule paving operations such that final rolling of asphalt concrete pavement is completed and initial runs of the profilograph are completed prior to opening new pavement to public traffic. The scheduling of these operations shall be in consideration of the lane closure requirements specified in "Maintaining Traffic" of these special provisions. In the event that initial profiles are not made prior to opening the pavement to public traffic, the initial profilograph runs shall be made the next day that traffic control is permitted for the area to be profiled.

The top surface of the uppermost layer of asphalt concrete surfacing that does not meet all specified surface tolerances shall be brought within tolerance by abrasive grinding. Areas which have been abrasive ground shall receive a fog seal coat. Deviations in excess of 8 mm which cannot be brought into specified surface tolerances by abrasive grinding shall be corrected by either (1) removal and replacement or, (2) placing an overlay of asphalt concrete. The corrective method for each area shall be selected by the Contractor and shall be as approved by the Engineer prior to beginning the corrective work. Any replacement or overlay pavement not meeting specified tolerances shall be corrected by the methods specified above. All corrective work shall be at the Contractor's expense except that flagging costs will be paid for as provided in Section 12-2, "Flagging," of the Standard Specifications.

After abrasive grinding has been completed to reduce individual deviations in excess of 8 mm, additional grinding or corrections to the surface as specified above shall be performed as necessary to reduce the Profile Index of the pavement to the specified Profile Index value required for the area. The Contractor shall run profilograms of such areas that have received abrasive grinding or corrective work until the final profilograms indicate the Profile Index of the area is within the specified tolerance.

When abrasive grinding is used to bring the top surface of the uppermost layer of asphalt concrete surfacing within specified surface tolerances, additional abrasive grinding shall be performed as necessary to extend the area ground in each lateral direction so that the lateral limits of grinding are at a constant offset from, and parallel to the nearest lane line or pavement edge, and in each longitudinal direction so that the grinding begins and ends at lines normal to the pavement centerline, within any ground area. All ground areas shall be neat rectangular areas of uniform surface appearance.

Abrasive grinding shall conform to the requirements in the first paragraph and the last 4 paragraphs in Section 42-2.02, "Construction," of the Standard Specifications, except that the grinding residue shall be disposed of outside the highway right of way.

The original of final profilograms that indicate the pavement surface is within the Profile Index specified shall become the property of the State and shall be delivered to the Engineer prior to acceptance of the contract.

Full compensation for performing all profile checks for Profile Index and furnishing final profilograms to the Engineer, for performing all corrective work to the pavement surface including abrasive grinding, removing and replacing asphalt

concrete or placing asphalt concrete overlay to bring the surface within the tolerance specified shall be considered as included in the contract price paid per tonne for asphalt concrete and no separate payment will be made therefor.

The area to which paint binder has been applied shall be closed to public traffic. Care shall be taken to avoid tracking binder material onto existing pavement surfaces beyond the limits of construction.

A drop-off of more than 46 mm will not be allowed at any time between adjacent lanes open to public traffic.

With the exception of the deep excavation (cellular concrete area) between NT Station 115+80 and 120+60, where the existing pavement is to be widened by constructing a new structural section adjacent to the existing pavement, the new structural section, on both sides of the existing pavement, shall be completed to match the elevation of the edge of the existing pavement for the entire length of the project prior to spreading and compacting asphalt concrete over the adjacent existing pavement.

At drier-drum and continuous plants with cold feed control, Cleanness Value test samples will be obtained from the discharge of each coarse aggregate storage. An aggregate sampling device shall be provided which will provide a 25-kg sample of each coarse aggregate.

10-1.49 ASPHALT CONCRETE (MISCELLANEOUS AREAS)

Surfacing of miscellaneous areas with asphalt concrete shall conform to the provisions for miscellaneous areas in Section 39, "Asphalt Concrete," of the Standard Specifications and these special provisions.

Asphalt concrete placed in miscellaneous areas may be produced in accordance with the requirements for asphalt concrete placed on the traveled way in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions.

The amount of asphalt binder used in asphalt concrete placed in dikes, gutters, gutter flares, overside drains and aprons at the ends of drainage structures shall be increased one percent by mass of the aggregate over the amount of asphalt binder determined for use in asphalt concrete placed on the traveled way.

Aggregate for asphalt concrete dikes shall conform to the 9.5-mm maximum grading as specified in Section 39-2.02, "Aggregate," of the Standard Specifications.

In median areas adjacent to slotted median drains, each layer of asphalt concrete shall not exceed 25-mm, maximum compacted thickness.

The miscellaneous areas to be paid for at the contract price per square meter for place asphalt concrete (miscellaneous area) in addition to the prices paid for the materials involved shall be limited to the areas listed on the plans.

Asphalt concrete placed in miscellaneous areas will be paid for at the contract price per tonne for asphalt concrete specified in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions. Section 39-10.02, "Statistical Evaluation and Determination of Pay Factor," in Section 11-1, "Asphalt Concrete," elsewhere in these special provisions, shall not apply to asphalt concrete placed in miscellaneous areas. Payment for placing asphalt concrete in miscellaneous areas and dikes will be as specified in Section 39-8.02, "Payment," of the Standard Specifications.

10-1.50 REPLACE ASPHALT CONCRETE SURFACING

This work shall consist of removing existing asphalt concrete surfacing and underlying base and replacing the removed surfacing and base with new asphalt concrete as shown on the plans and in accordance with these special provisions.

The exact limits of asphalt concrete surfacing to be removed and replaced will be determined by the Engineer.

Existing asphalt concrete surfacing and underlying base material removed during a work period shall be replaced before the time the lane is to be opened to public traffic as designated in "Maintaining Traffic" of these special provisions.

The outline of the asphalt concrete surfacing to be removed shall be cut with a power-driven saw to a depth of not less than 46 mm before removing the surfacing. Surfacing and base shall be removed without damage to surfacing that is to remain in place. Damage to pavement which is to remain in place shall be repaired to a condition satisfactory to the Engineer, or the damaged pavement shall be removed and replaced with new asphalt concrete if ordered by the Engineer. Repairing or removing and replacing pavement damaged outside the limits of pavement to be replaced shall be at the Contractor's expense and will not be measured nor paid for.

Removed materials shall be disposed of outside the highway right of way in accordance with the provisions in Section 7-1.13, "Disposal of Material Outside the Highway Right of Way," of the Standard Specifications.

The material remaining in place, after removing asphalt concrete surfacing and base to the required depth, shall be graded to a plane, watered, and compacted. The finished surface of the remaining material shall not extend above the grade established by the Engineer.

Areas of the base material which are low as a result of over excavation shall be filled, at the Contractor's expense, with asphalt concrete.

Asphalt concrete used for replace asphalt concrete surfacing shall conform to the provisions in Section 39, "Asphalt Concrete," of the Standard Specifications.

At the Contractor's option asphalt concrete used for replace asphalt concrete surfacing may be produced in accordance with the provisions for asphalt concrete placed on the traveled way in Section 11-1, "Asphalt Concrete," of these special provisions.

The quantity of replace asphalt concrete surfacing to be paid for will be measured by the cubic meter. The volume to be paid for will be calculated on the basis of the dimensions shown on the plans adjusted by the amount of any change ordered by the Engineer.

The contract price paid per cubic meter for replace asphalt concrete surfacing shall include full compensation for furnishing all labor, materials (including asphalt concrete), tools, equipment, and incidentals, and for doing all the work involved in replacing asphalt concrete surfacing, complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.51 RUBBERIZED ASPHALT CONCRETE (TYPE G)

Rubberized asphalt concrete (Type G) shall consist of furnishing and mixing gap graded aggregate and asphalt-rubber binder and spreading and compacting the mixture. Type G rubberized asphalt concrete shall conform, except as otherwise provided, to the requirements specified for Type A asphalt concrete in Section 39, "Asphalt Concrete," of the Standard Specifications and these special provisions.

GENERAL.—The Contractor shall furnish samples of aggregate to the Engineer in conformance with the provisions in Section 39-3.03, "Proportioning," of the Standard Specifications.

Aggregate for Type G rubberized asphalt concrete shall be of such quality that the optimum amount of asphalt-rubber binder to be mixed with the aggregate, as determined by the Engineer in accordance with the requirements in California Test 367 (as amended below), shall be a minimum of 7.0 percent by mass of dry aggregate and a maximum of 9.0 percent by mass of dry aggregate. Aggregates which result in an optimum asphalt-rubber binder content of less than 7.0 percent or more than 9.0 percent by mass of dry aggregate shall not be used. The Engineer will determine the exact amount of asphalt-rubber binder to be mixed with the aggregate in accordance with the requirements in California Test 367, except as follows:

The specific gravity used in California Test 367, Section "B. Voids Content of Specimen", will be determined using California Test 308, Method A.

California Test 367, Section "C. Optimum Bitumen Content", is revised as follows:

1. Plot asphalt-rubber binder content versus void content for each specimen on Form TL-306 (Figure 3), and connect adjacent points with straight lines.
2. From Figure 3 select the theoretical asphalt-rubber binder content that has 5 percent voids.
3. Record the asphalt-rubber binder content in Step 2 as the Optimum Bitumen Content (OBC).
4. To establish a recommended range, use the Optimum Bitumen Content (OBC) as the high value and 0.3 percent less as the low value. Notwithstanding, the recommended range shall not extend below 7.0 percent nor shall the high value to establish the recommended range be above 9.0 percent. If the OBC is 7.0 percent, then there shall be no recommended range, and 7.0 percent shall be the recommended value.

Laboratory mixing and compaction shall be in accordance with the requirements of California Test 304, except that the mixing temperature of the aggregate shall be between 149°C and 163°C. The compaction temperature of the combined mixture shall be between 143°C and 149°C.

The rubberized asphalt concrete mixture, composed of the aggregate proposed for use and the optimum amount of asphalt-rubber binder as determined in accordance with the requirements in California Test 367 modified above, shall conform to the following quality requirements:

RUBBERIZED ASPHALT CONCRETE MIXTURE

Test Parameter	California Test	Requirement
Stabilometer Value, Minimum	304 and 366	23
Voids in Mineral Aggregate, Percent, Minimum	See Note	18

Note: Voids in mineral aggregate test shall be determined as described in Asphalt Institute Mix Design Methods for Asphalt Concrete (MS-2).

The asphalt-rubber binder content of the rubberized asphalt concrete (Type G) will be determined by extraction tests in accordance with the requirements in California Test 362, or will be determined in accordance with the requirements in California Test 379.

The Contractor shall furnish to the Engineer a Certificate of Compliance in accordance with the provisions in Section 6-1.07, "Certificates of Compliance" of the Standard Specifications for each material used in asphalt-rubber binder and the asphalt-rubber binder mixture. The Certificate of Compliance shall certify that the material conforms to the requirements in these special provisions. When requested by the Engineer, the Contractor shall also submit samples with the Certificate of Compliance. The Contractor shall provide the Engineer a Material Safety Data Sheet (MSDS) for each of the constituent components of the asphalt-rubber binder, for the completed mixture of asphalt-rubber binder and for the Type G rubberized asphalt concrete.

The Contractor shall provide a Certificate of Compliance for each truck load of crumb rubber modifier (CRM), paving asphalt and asphalt modifier delivered to the project. The Quality Control Program used by the manufacturer of each ingredient shall include a sampling and testing frequency as shown below.

CRM shall be tested, except for the grading requirement, at least once for every 225 tonnes with a minimum of once per project. CRM shall be tested for grading for every truck load delivered to the project.

Paving asphalt shall be tested at least once for every 180 tonnes of production with a minimum of once per project.

Asphalt modifier shall be tested at least once for every 23 tonnes of production with a minimum of once per project.

A copy of the laboratory test results for the test parameters specified in these special provisions for CRM, paving asphalt and asphalt modifier shall be submitted to the Engineer with the Certificate of Compliance for each truck load of individual material delivered to the project.

Certified volume or weight slips shall be delivered to the Engineer for the materials supplied.

PAVING ASPHALT.—The grade of paving asphalt to be used in the asphalt-rubber binder shall be AR-4000 and shall conform to the provisions in Section 92, "Asphalts," of the Standard Specifications and these special provisions.

The paving asphalt for use in asphalt-rubber binder shall be modified with an asphalt modifier.

ASPHALT MODIFIER.—The asphalt modifier will be a resinous, high flash point, aromatic hydrocarbon compound and shall conform to the following requirements:

ASPHALT MODIFIER

Test Parameter	ASTM Designation	Requirement
Viscosity, m^2/s ($x10^{-6}$) at 100°C	D 445	X ± 3*
Flash Point, CL.O.C., °C	D 92	207 min.
Molecular Analysis:		
Asphaltenes, percent by mass	D 2007	0.1 max.
Aromatics, percent by mass	D 2007	55 min.
* The symbol "X" is the viscosity of the asphalt modifier the Contractor proposes to furnish. The value "X" which the Contractor proposes shall be between the limits 19 and 36 and shall be submitted in writing to the Engineer. Any proposed change, requested by the Contractor, in the value "X" shall require a new asphalt-rubber binder design.		

The asphalt modifier shall be proportionately added to the paving asphalt at the production site where the asphalt-rubber binder is blended and reacted. Asphalt modifier shall be added at an amount of 2.5 to 6.0 percent by mass of the paving asphalt based on the recommendation of the asphalt-rubber binder supplier. The paving asphalt shall be at a temperature of not less than 190°C nor more than 226°C when the asphalt modifier is added. If the asphalt modifier is combined with the paving asphalt, before being blended with the CRM, the combined paving asphalt and asphalt modifier shall be mixed by circulation for a period of not less than 20 minutes. This premixing of asphalt modifier and the paving asphalt will not be required when the ingredients of the asphalt-rubber binder are proportioned and mixed simultaneously. Asphalt modifier and paving asphalt shall be measured for proportioning with meters conforming to the provisions in Section 9-1.01, "Measurement of Quantities," of the Standard Specifications.

CRUMB RUBBER MODIFIER (CRM).—Crumb rubber modifier (CRM) shall consist of a combination of scrap tire CRM and high natural CRM. The scrap tire CRM shall consist of ground or granulated rubber derived from any combination of automobile tires, truck tires or tire buffings. The high natural CRM shall consist of ground or granulated rubber derived from materials that utilize high natural rubber sources.

Steel and fiber separation may be accomplished by any method. Cryogenic separation, if utilized, shall be performed separately from and prior to grinding or granulating.

CRM shall be ground or granulated at ambient temperature. Cryogenically produced CRM particles which can pass through the grinder or granulator without being ground or granulated respectively, shall not be used.

CRM shall not contain more than 0.01-percent wire (by mass of CRM) and shall be free of other contaminants, except fabric. Fabric shall not exceed 0.05-percent by mass of CRM. The test and method for determining the percent by mass of wire and fabric is available at the Transportation Laboratory, Pavement Consulting Services Branch, Sacramento, California, Telephone (916) 227-7295, and will be furnished to interested persons upon request. A Certificate of Compliance certifying these percentages shall be furnished to the Engineer in accordance with the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications.

The length of any individual CRM particle shall not exceed 4.75 mm.

The CRM shall be sufficiently dry so as to be free flowing and not produce foaming when combined with the blended paving asphalt and asphalt modifier mixture. Calcium carbonate or talc may be added at a maximum amount of 3 percent by mass of CRM to prevent CRM particles from sticking together. The CRM shall have a specific gravity between 1.1 and 1.2 as determined by California Test 208. Scrap tire CRM and high natural CRM shall be delivered to the production site in

separate bags and shall be sampled and tested separately. CRM material shall conform to the following requirements as determined by ASTM Designation: D 297:

SCRAP TIRE CRUMB RUBBER MODIFIER

Test Parameter	Percent	
	Min.	Max.
Acetone Extract	6.0	16.0
Ash Content	—	8.0
Carbon Black Content	28.0	38.0
Rubber Hydrocarbon	42.0	65.0
Natural Rubber Content	22.0	39.0

HIGH NATURAL CRUMB RUBBER MODIFIER

Test Parameter	Percent	
	Min.	Max.
Acetone Extract	4.0	16.0
Rubber Hydrocarbon	50.0	—
Natural Rubber Content	40.0	48.0

The CRM for asphalt-rubber binder shall conform to the gradations specified below when tested in accordance with the requirements in ASTM Designation: C 136 amended as follows:

Split or quarter 100 grams ±5 grams from the CRM sample and dry to a constant mass at a temperature of not less than 57°C nor more than 63°C and record the dry sample mass. Place the CRM sample and 5.0 grams of talc in a 0.5-L jar. Seal the jar, then shake it by hand for a minimum of one minute to mix the CRM and the talc. Continue shaking or open the jar and stir until particle agglomerates and clumps are broken and the talc is uniformly mixed.

Place one rubber ball on each sieve. Each ball shall have a mass of 8.5 ±0.5 grams, have a diameter of 24.5 ±0.5 mm, and shall have a Shore Durometer "A" hardness of 50 ±5 in accordance with the requirements in ASTM Designation: D 2240. After sieving the combined material for 10 minutes ±1 minute, disassemble the sieves. Any material adhering to the bottom of a sieve shall be brushed into the next finer sieve. Weigh and record the mass of the material retained on the 2.36-mm sieve and leave this material (do not discard) on the scale or balance. Any observed fabric balls shall remain on the scale or balance and shall be placed together on the side of the scale or balance to prevent the fabric balls from being covered or disturbed when placing the material from finer sieves onto the scale or balance. The material retained on the next finer sieve (2.00-mm sieve) shall be added to the scale or balance. Weigh and record that mass as the accumulative mass retained on that sieve (2.00-mm sieve). Continue weighing and recording the accumulated masses retained on the remaining sieves until the accumulated mass retained in the pan has been determined. Prior to discarding the CRM sample, separately weigh and record the total mass of fabric balls in the sample.

Determine the mass of material passing the 75-µm sieve (or mass retained in the pan) by subtracting the accumulated mass retained on the 75-µm sieve from the accumulated mass retained in the pan. If the material passing the 75-µm sieve (or mass retained in the pan) has a mass of 5 grams or less, cross out the recorded number for the accumulated mass retained in the pan and copy the number recorded for the accumulated mass retained on the 75-µm sieve and record that number (next to the crossed out number) as the accumulated mass retained in the pan. If the material passing the 75-µm sieve (or mass retained in the pan) has a mass greater than 5 grams, cross out the recorded number for the accumulated mass retained in the pan, subtract 5 grams from that number and record the difference next to the crossed out number. The adjustment to the accumulated mass retained in the pan is made to account for the 5 grams of talc added to the sample. For calculation purposes, the adjusted total sample mass is the same as the adjusted accumulated mass retained in the pan. Determine the percent passing based on the adjusted total sample mass and record to the nearest 0.1 percent.

CRM GRADATIONS

Sieve Size	Scrap Tire CRM Percent Passing	High Natural CRM Percent Passing
2.36-mm	100	100
2.00-mm	98-100	100
1.18-mm	45-75	95-100
600-µm	2-20	35-85
300-µm	0-6	10-30
150-µm	0-2	0-4
75-µm	0	0-1

ASPHALT-RUBBER BINDER.—Asphalt-rubber binder shall consist of a mixture of paving asphalt, asphalt modifier and crumb rubber modifier.

At least 2 weeks before its intended use, the Contractor shall furnish to the Engineer 4 one-liter cans filled with the asphalt-rubber binder proposed for use on the project. The Contractor shall supply to the Engineer for approval a binder formulation and samples of the materials to be used in the asphalt-rubber binder, at least 2 weeks before construction is scheduled to begin. The binder formulations shall consist of the following information:

Paving Asphalt and Modifiers

(1) Source and grade of paving asphalt.
(2) Source and identification (or type) of modifiers used.
(3) Percentage of asphalt modifier by mass of paving asphalt.
(4) Percentage of the combined blend of paving asphalt and asphalt modifier by total mass of asphalt-rubber binder to be used.
(5) Laboratory test results for test parameters shown elsewhere in these special provisions.

Crumb Rubber Modifier (CRM)

(1) Source and identification (or type) of scrap tire and high natural CRM.
(2) Percentage of scrap tire and high natural CRM by total mass of the asphalt-rubber blend.
(3) If CRM from more than one source is used, the above information will be required for each CRM source used.
(4) Laboratory test results for test parameters shown elsewhere in these special provisions.

Asphalt-Rubber Binder

(1) Laboratory test results of the proposed blend for test parameters shown elsewhere in these special provisions.
(2) The minimum reaction time and temperature.

The method and equipment for combining paving asphalt, asphalt modifier and CRM shall be so designed and accessible that the Engineer can readily determine the percentages by mass for each material being incorporated into the mixture.

The proportions of the materials, by total mass of asphalt-rubber binder, shall be 80 ±2 percent combined paving asphalt and asphalt modifier, and 20 ±2 percent CRM. However, the minimum amount of CRM shall not be less than 18.0 percent. Lower values which are rounded up shall not be allowed. The CRM shall be combined at the production site and shall contain 75 ±2 percent scrap tire CRM and 25 ±2 percent high natural CRM, by mass.

The paving asphalt and asphalt modifier shall be combined into a blended mixture that is chemically compatible with the crumb rubber modifier to be used. The blended mixture is considered to be chemically compatible when it meets the requirements for asphalt-rubber binder (after reacting) found elsewhere in these special provisions.

The blended paving asphalt and asphalt modifier mixture, and the CRM shall be combined and mixed together at the production site in a blender unit to produce a homogeneous mixture.

The temperature of the blended paving asphalt and asphalt modifier mixture shall be not less than 190°C nor more than 226°C when the CRM is added. The combined materials shall be reacted for a minimum of 45 minutes after incorporation of the CRM at a temperature of not less than 190°C nor more than 218°C. The temperature shall not be higher than 6°C below the actual flash point of the asphalt-rubber binder.

After reacting, the asphalt-rubber binder shall conform to the following requirements:

ASPHALT-RUBBER BINDER			
Test Parameter	ASTM Test Method	Requirement	
		Min.	Max.
Cone Penetration @ 25°C, 1/10 mm	D 217	25	70
Resilience @ 25°C, Percent rebound	D 3407	18	—
Field Softening Point, °C	D 36	52	74
Viscosity @ 190°C, Pa • s ($x10^{-3}$)	See Note	1500	4000
NOTE: The viscosity test shall be conducted using a hand held Haake Viscometer Model VT-02 with Rotor 1, 24 mm in depth x 53 mm in height, or equivalent, as determined by the Engineer. The accuracy of the viscometer shall be verified by comparing the viscosity results obtained with the hand held viscometer to 3 separate calibration fluids of known viscosities ranging from 1000 to 5000 Pa • s ($x10^{-3}$). The viscometer will be considered accurate if the values obtained are within 300 Pa • s ($x10^{-3}$) of the known viscosity. The known viscosity value shall be based on the fluid manufacturers standard test temperature or the test temperature versus viscosity correlation table provided by the fluid manufacturer. Viscometers used on the project shall be verified to be accurate. The test method for determining the viscosity of asphalt-rubber binder using a hand held viscometer is available at the Transportation Laboratory, Pavement Consulting Services Branch, Sacramento, California, Telephone (916) 227-7108. The accuracy verification results shall be provided to the Engineer and shall be certified by a Certificate of Compliance. The Certificate of Compliance shall be furnished to the Engineer in accordance with the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications.			

The Contractor shall provide a Haake Viscometer, or equivalent, at the production site during the combining of asphalt-rubber binder materials. The Contractor shall take viscosity readings of asphalt-rubber binder from samples taken from the feed line connecting the storage and reaction tank to the asphalt concrete plant. Readings shall be taken at least every hour with no less than-one reading for each batch of asphalt-rubber binder. The Contractor shall log these results including time and asphalt-rubber binder temperature, and a copy of the log shall be submitted to the Engineer on a daily basis. As determined by the Engineer, the Contractor shall either notify the Engineer at least 15 minutes prior to each test or provide the Engineer a schedule of testing times.

The reacted asphalt-rubber binder shall be maintained at a temperature of not less than 190°C nor more than 218°C.

If any of the material in a batch of asphalt-rubber binder is not used within 4 hours after the 45-minute reaction period, heating of the material shall be discontinued. Any time the asphalt-rubber binder cools below 190°C, and is then reheated, shall be considered a reheat cycle. The total number of reheat cycles shall not exceed 2. The material shall be uniformly reheated to a temperature of not less than 190°C nor more than 218°C prior to use. Additional scrap tire CRM may be added to the reheated binder and reacted for a minimum of 45 minutes. The cumulative amount of additional scrap tire CRM shall not exceed 10 percent of the total binder mass. Reheated asphalt-rubber binder shall conform to the requirements for asphalt-rubber binder.

EQUIPMENT FOR PRODUCTION OF ASPHALT-RUBBER BINDER.—The Contractor shall utilize the following equipment for production of asphalt-rubber binder:

1. An asphalt heating tank equipped to heat and maintain the blended paving asphalt and asphalt modifier mixture at the necessary temperature before blending with the CRM. This unit shall be equipped with a thermostatic heat control device and a temperature reading device and shall be accurate to within ±3°C and shall be of the recording type.
2. A mechanical mixer for the complete, homogeneous blending of paving asphalt, asphalt modifier, and CRM. Paving asphalt and asphalt modifier shall be introduced into the mixer through meters conforming to the provisions of Section 9-1.01, "Measurement of Quantities," of the Standard Specifications. The blending system shall be capable of varying the rate of delivery of paving asphalt and asphalt modifier proportionate with the delivery of CRM. During the proportioning and blending of the liquid ingredients, the temperature of paving asphalt and the asphalt modifier shall not vary more than ±14°C. The paving asphalt feed, the asphalt modifier feed and CRM feed, shall be equipped with devices by which the rate of feed can be determined during the proportioning operation. Meters used for proportioning individual ingredients shall be equipped with rate-of-flow indicators to show the rates of delivery and resettable totalizers so that the total amounts of liquid ingredients introduced into the mixture can be determined. The liquid and dry ingredients shall be fed directly into the mixer at a uniform and controlled rate. The rate of feed to the mixer shall not exceed that which will permit complete mixing of the materials. Dead areas in the mixer, in which the material does not move or is not sufficiently agitated, shall be corrected by a reduction in the volume of material or by other adjustments. Mixing shall continue until a homogeneous mixture of uniformly distributed and properly blended asphalt-rubber binder of unchanging appearance and consistency is produced. The Contractor shall provide a safe sampling device capable of delivering a representative sample of the completed asphalt-rubber binder of sufficient size to permit the required tests.
3. An asphalt-rubber binder storage tank equipped with a heating system furnished with a temperature reading device to maintain the proper temperature of the asphalt-rubber binder and an internal mixing unit capable of maintaining a homogeneous mixture of paving asphalt, asphalt modifier and CRM.

The equipment shall be approved by the Engineer prior to use.

AGGREGATE.—The aggregate for Type G rubberized asphalt concrete shall conform to the following grading and shall meet the quality requirements specified for Type A asphalt concrete in Section 39-2.02, "Aggregate," of the Standard Specifications, except as follows:

1. California Test 211, Los Angeles Rattler loss at 500 revolutions shall be 40 percent maximum.
2. California Test 205, Section D, definition of a crushed particle is revised as follows: "Any particle having 2 or more fresh mechanically fractured faces shall be considered a crushed particle".
3. The swell and moisture vapor susceptibility requirements shall not apply.

The symbol "X" in the following table is the gradation which the Contractor proposes to furnish for the specific sieve.

Aggregate Grading Requirements
Percentage Passing
12.5-mm maximum

Sieve Size	Limits of Proposed Gradation	Operating Range	Contract Compliance
19-mm	—	100	100
12.5-mm	—	90-100	90-100
9.5-mm	83-87	X±5	X±7
4.75-mm	33-37	X±5	X±7
2.36-mm	18-22	X±4	X±5
600-µm	8-12	X±4	X±5
75-µm	—	2-7	0-8

PROPORTIONING, SPREADING AND COMPACTING.—When batch type asphalt concrete plants are used to produce Type G rubberized asphalt concrete, the asphalt-rubber binder and mineral aggregate shall be proportioned by mass.

If the Contractor selects the batch mixing method, asphalt concrete shall be produced by the automatic batch mixing method as provided in Section 39-3.03A(2), "Automatic Proportioning," of the Standard Specifications.

When continuous mixing type asphalt concrete plants are used to produce Type G rubberized asphalt concrete, the asphalt-rubber binder shall be proportioned by an asphalt meter of the mass flow, Coriolis effect type. The meter shall have been Type-approved by the Division of Measurement Standards prior to the start of production. The meter shall be calibrated in accordance with the requirements in California Test 109. The meter shall be interfaced with the existing continuous mixing plant controller in use on the asphalt concrete plant.

Type G rubberized asphalt concrete shall be placed only when the atmospheric and pavement surface temperatures are 13°C or above.

When the atmospheric and pavement surface temperature is 18°C or higher, the following shall apply:

1. The temperature of the aggregate shall not be greater than 163°C at the time the asphalt-rubber binder is added to the aggregate.
2. Type G rubberized asphalt concrete shall be spread at a temperature of not less than 138°C nor more than 163°C, measured in the mat directly behind the paving machine.
3. The first coverage of initial or breakdown compaction shall be performed when the temperature of the Type G rubberized asphalt concrete is not less than 135°C, and all breakdown compaction shall be completed before the temperature of the Type G rubberized asphalt concrete drops below 121°C.

When the atmospheric or pavement surface temperature is below 18°C, the following shall apply:

1. The temperature of the aggregate shall not be less than 149°C nor more than 163°C at the time the asphalt-rubber binder is added to the aggregate.
2. The Contractor shall cover the loads of Type G rubberized asphalt concrete with tarpaulins. The tarpaulins shall completely cover the exposed Type G rubberized asphalt concrete until the Type G rubberized asphalt concrete has been completely transferred into the asphalt concrete paver hopper or deposited on the roadbed.
3. Type G rubberized asphalt concrete shall be spread at a temperature of not less than 143°C nor more than 163°C, measured in the mat directly behind the paving machine.
4. The first coverage of initial or breakdown compaction shall be performed when the temperature of the Type G rubberized asphalt concrete is not less than 138°C, and all breakdown compaction shall be completed before the temperature of the Type G rubberized asphalt concrete drops below 127°C.

Pneumatic tired rollers shall not be used to compact Type G rubberized asphalt concrete.

In addition to the requirements in Section 39-5.01, "Spreading Equipment," of the Standard Specifications, asphalt paving equipment shall be equipped with automatic screed controls and a sensing device or devices.

When placing asphalt concrete to the lines and grades established by the Engineer, the automatic controls shall control the longitudinal grade and transverse slope of the screed. Grade and slope references shall be furnished, installed and maintained by the Contractor. Should the Contractor elect to use a ski device, the minimum length of the ski device shall be 9 m. The ski device shall be a rigid one piece unit and the entire length shall be utilized in activating the sensor.

When placing the initial mat of asphalt concrete on existing pavement, the end of the screed nearest the centerline shall be controlled by a sensor activated by a ski device not less than 9 m long. The end of the screed farthest from centerline shall be controlled by an automatic transverse slope device set to reproduce the cross slope designated by the Engineer.

When paving contiguously with previously placed mats, the end of the screed adjacent to the previously placed mat shall be controlled by a sensor that responds to the grade of the previously placed mat and will reproduce the grade in the new mat within a 3-mm tolerance. The end of the screed farthest from the previously placed mat shall be controlled in the same manner as when placing the initial mat.

Should the methods and equipment furnished by the Contractor fail to produce a layer of asphalt concrete conforming to the requirements, including straightedge tolerance, of Section 39-6.03, "Compacting," of the Standard Specifications, the

paving operations shall be discontinued and the Contractor shall modify the equipment or methods, or furnish substitute equipment.

Should the automatic screed controls fail to operate properly during any day's work, the Contractor may use manual control of the spreading equipment for the remainder of that day, however, the equipment shall be corrected or replaced with alternative automatically controlled equipment conforming to the requirements in this section before starting another day's work.

The area to which paint binder has been applied shall be closed to public traffic. Care shall be taken to avoid tracking binder material onto existing pavement surfaces beyond the limits of construction.

A drop-off of more than 46 mm will not be allowed at any time between adjacent lanes open to public traffic.

Shoulders or median borders adjacent to a lane being paved shall be surfaced prior to opening the lane to traffic.

Alternative compacting equipment as specified in Section 39-6.03, "Compacting," of the Standard Specifications shall be used to compact the Type G rubberized asphalt concrete.

Traffic shall not be allowed on the Type G rubberized asphalt concrete until final rolling operations have been completed and sand has been applied to the surface.

Sand shall be spread on the surface of Type G rubberized asphalt concrete at a rate of 0.5 to 1.0 kg/m^2. The exact rate will be determined by the Engineer. When ordered by the Engineer excess sand shall be removed from the pavement surface by sweeping. Sand shall be free from clay or organic material. Sand shall conform to the fine aggregate grading provisions in Section 90-3.03, "Fine Aggregate Grading," of the Standard Specifications.

If the finished surface of the asphalt concrete on the Route 92 traffic lanes does not meet the specified surface tolerances, it shall be brought within tolerance by either (1) abrasive grinding (with fog seal coat on the areas which have been ground), (2) removal and replacement, or (3) placing an overlay of asphalt concrete. The method will be selected by the Engineer. The corrective work shall be at the Contractor's expense.

If abrasive grinding is used to bring the finished surface to specified surface tolerances, additional grinding shall be performed as necessary to extend the area ground in each lateral direction so that the lateral limits of grinding are at a constant offset from, and parallel to the nearest lane line or pavement edge, and in each longitudinal direction so that the grinding begins and ends at lines normal to the pavement centerline, within any ground area. All ground areas shall be neat rectangular areas of uniform surface appearance. Abrasive grinding shall conform to the requirements in the first paragraph and the last 4 paragraphs in Section 42-2.02, "Construction," of the Standard Specifications.

MEASUREMENT AND PAYMENT.—Rubberized asphalt concrete (Type G) will be measured and paid for by the tonne in the same manner specified for asphalt concrete in Section 39-8, "Measurement and Payment," of the Standard Specifications.

Full compensation for furnishing and spreading sand on the rubberized asphalt concrete surface and for sweeping and removing excess sand from the pavement surface shall be considered as included in the contract price paid per tonne for rubberized asphalt concrete (Type G) and no separate payment will be made therefor.

10-1.52 CONCRETE PAVEMENT

Portland cement concrete pavement shall conform to the provisions in Section 40, "Portland Cement Concrete Pavement," of the Standard Specifications and these special provisions.

Transverse weakened plane joints across portland cement concrete shoulders shall be continuous with such joints across the traveled way. Longitudinal joints shall be constructed between portland cement concrete shoulders and adjacent traffic lanes, and tie bars shall be installed at such joints as provided herein.

Tie bars shall be installed at longitudinal joints between portland cement concrete shoulders and adjacent traffic lanes and at transverse contact joints, as shown on the plans. Tie bars shall be deformed reinforcing steel bars conforming to the specifications of ASTM Designation: A 615/A 615M, Grade 300 or 400, ASTM Designation: A 616/A 616M, Grade 350 or 400, or ASTM Designation: A 706/A 706M, and shall be epoxy-coated as specified in Section 52-1.02B, "Epoxy-coated Bar Reinforcement," of the Standard Specifications.

The joint detail shown on the plans for transverse and longitudinal joints, including the foam backer rod and silicone joint sealant shown on the plans, shall not apply.

10-1.53 PILING

Piling shall conform to the provisions in Section 49, "Piling," of the Standard Specifications, and these special provisions.

Foundation recommendations are included in the "Information Handout" available to the Contractor as provided for in Section 2-1.03, "Examination of Plans, Specifications, Contract, and Site of Work," of the Standard Specifications.

Attention is directed to "Welding Quality Control" of these special provisions.

Attention is directed to "Public Safety," of these special provisions. Before performing any pile handling or pile installation operation at any location that is closer than the length of the pile being handled or installed to the edge of any area open to public traffic or public use, the Contractor shall submit to the Engineer, as provided in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications, a detail plan of the measures that will be employed to provide for the safety of traffic and the public.

The second paragraph in Section 49-1.03, "Determination of Length," of the Standard Specifications is amended to read:

At the Contractor's option, the Contractor may conduct additional foundation investigation, including installing and axial load testing additional non-production indicator piling. The Engineer shall approve locations of additional foundation testing. The Contractor shall notify the Engineer at least 5 working days prior to beginning additional foundation investigation.

Additional foundation investigation shall be completed prior to requesting revised specified pile tip elevations or modification to the installation methods specified herein. Revisions to specified tip elevations and modifications to the specified installation methods will be subject to the provisions of Section 5-1.14, "Cost Reduction Incentive."

Modification to the specified installation methods and specified pile tip elevation will not be considered at locations where lateral load demands control design pile tip elevations or when the plans state that specified pile tip elevation shall not be revised.

The pile structural capacity design is based on the nominal strength as defined in Caltrans Bridge Design Specifications (Article 8.1.3) or the nominal resistance as defined in the LRFD Bridge Design Specifications (Article 1.3.2.1). The nominal resistance of the pile, as shown on the plans, is the design capacity required to resist the factored axial load demands.

Indicator compression pile load testing shall conform to the requirements of ASTM Designation: D 1143. The acceptance criteria for compression pile load testing shall be as follows:

The pile shall sustain the first compression test load applied which is equal to the nominal compression resistance, as shown on the plans, with no more than 13 mm total vertical movement at the top of the pile measured relative to the top of the pile prior to the start of compression load testing.

Indicator tension pile load testing shall conform to the requirements of ASTM Designation: D 3689. The loading apparatus described as "Load Applied to Pile by Hydraulic Jack(s) Acting at One End of Test Beam(s) Anchored to the Pile" shall not be used. The acceptance criteria for tension pile load testing shall be as follows:

The pile shall sustain the first tension test load applied which is equal to the nominal tension resistance, as shown on the plans, with no more than 13 mm total vertical movement at the top of the pile measured relative to the top of the pile prior to the start of tension load testing.

Indicator piling shall be removed in conformance with the requirements in Section 15-4.02, "Removal Methods," and the remaining holes shall be backfilled with earth or other suitable material approved by the Engineer.

For driven piling, the Contractor shall furnish piling of sufficient length to obtain both the specified tip elevation and design load shown on the plans or specified in the special provisions. For cast-in-drilled-hole concrete piling, the Contractor shall construct piling of such length to develop the compression nominal resistance and to obtain the specified tip elevation shown on the plans or specified in the special provisions.

The fifth paragraph in Section 49-1.04, "Load Test Piles," of the Standard Specifications is amended to read:

Load test anchorages in piles used as anchor piles shall conform to the following requirements:

High strength threaded steel rods shall conform to the provisions for bars in Section 50-1.05, "Prestressing Steel," except Type II bars shall be used.

High strength steel plates shall conform to the requirements in ASTM Designation: A 709, Grade 50.

Anchor nuts shall conform to the provisions in the second paragraph in Section 50-1.06, "Anchorages and Distribution."

The eighth, ninth and tenth paragraphs in Section 49-1.04, "Load Test Piles," of the Standard Specifications are amended to read:

Should the Engineer fail to complete the load tests within the time specified in the special provisions and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in load testing of piles, the delay will be considered a right of way delay as specified in Section 8-1.09, "Right of Way Delays."

The Contractor shall furnish labor, materials, tools, equipment, and incidentals as required to assist the Engineer in the installation, operation and removal of State-furnished steel load test beams, State-furnished jacks, bearing plates, drills, and other test equipment. This work will be paid for as extra work as provided in Section 4-1.03D.

The first and second paragraphs in Section 49-1.05, "Driving Equipment," of the Standard Specifications are amended to read:

49-1.05 Driving Equipment.—Driven piles shall be installed with impact hammers that are approved in writing by the Engineer. Impact hammers shall be steam, hydraulic, air, or diesel hammers. Impact hammers shall develop sufficient energy to drive the piles at a penetration rate of not less than 3 mm per blow at the specified bearing value.

Vibratory hammers shall not be used for installation of piles, unless otherwise shown on the plans or specified in the special provisions.

Hammers with an external combustion engine that are not single action, shall have a transducer that records ram velocity.

Double acting diesel hammers with internal combustion engines shall have a transducer that records bounce chamber pressure.

For hammers with no visual way of observing the ram stroke, a printed readout showing hammer energy during driving operation shall be provided to the Engineer by the Contractor.

The fifth paragraph in Section 49-1.05, "Driving Equipment," of the Standard Specifications is deleted.

At the option of the Contractor, vibratory hammers may be used to install the precast prestressed sheet piling at Toll Plaza Retaining Wall to elevation –3.0.

Difficult pile installation is anticipated at San Mateo-Hayward Bridge, Toll Plaza retaining Wall due to the presence of rock slope protection (rip-rap) up to 1 meter in diameter along the wall alignment, and may also be present within the upper 3 meters of the embankment . Excavation of this material will be necessary prior to driving sheet piles. At the Contractor's option, pre-trenching may be used to facilitate the installation of the sheet pile. Trench should not be wider than 305 mm and not deeper than elevation –3 meter.

The third paragraph in Section 49-4.04, "Steel Shells," of the Standard Specifications is amended to read:

Steel shells shall conform to the provisions for steel pipe piles specified in Section 49-5, "Steel Piles."

Section 49-5.01, "Description," of the Standard Specifications is amended to read:

49-5.01 Description.—Steel piles shall include structural shape piles and pipe piles. Structural shape steel piles shall be of the rolled section shown on the plans or of the section specified in the special provisions and shall be structural steel conforming to the specifications of ASTM Designation: A 36/A 36M, or at the option of the Contractor, structural steel conforming to the specifications of ASTM Designation: A 572/A 572M.

Steel pipe piling shall conform to the following requirements:

1. Piles shall be of the nominal diameter and the nominal wall thickness as the pipe piles shown on the plans unless otherwise specified in the special provisions.

2. The carbon equivalency (CE) as defined in AWS D 1.1, Section XI5.1, shall not exceed 0.45.

3. The sulfur content shall not exceed 0.05 percent.

4. Piles shall conform to any additional requirements in the special provisions, including but not limited to, tolerances for diameter, edge alignment, end match marking, roundness, and straightness, that are required in order to conform with steel pile splice welding and welding inspection provisions.

5. Steel pipe pile seams shall be complete penetration welds and shall conform to the requirements of AWS D1.1 and any additional amendments to AWS D1.1 listed herein and in the special provisions. Incomplete penetration welds and defective welds of steel pipe piles shall be repaired or restored to achieve complete joint penetration groove welds.

6. Steel pipe piles that are less than 360 mm in diameter shall conform to the specifications of ASTM Designation: A 252, Grade 2 or 3, and steel pipe piles that are 360 mm and greater in diameter shall conform to the specifications of ASTM Designation: A 252, Grade 3, as amended by the above requirements.

Steel piles shall not be joined by welded lap splicing.

The manufacturer or fabricator of steel piling shall furnish a Certificate of Compliance stating that the piling being supplied conforms to these specifications and to the special provisions. The Certificate of Compliance shall include test reports for tensile, chemical, and any specified nondestructive tests. Samples for testing shall be taken from the base metal, steel, coil or from the manufactured or fabricated piling.

Section 49-5.02, "Splicing," of the Standard Specifications is amended to read:

49-5.02 Splicing.—Steel pile splices shall conform to the requirements of AWS D 1.1 and the special provisions. Structural shape steel piling splices shall be complete joint penetration groove welds. Steel pipe pile splices that are made at a permanent manufacture or fabrication facility, and that are made prior to furnishing the Certificate of Compliance shall be complete penetration welds. Steel pipe pile splices that are made in the field shall be complete joint penetration groove welds.

Ends of steel pipe piling to be spliced that have been damaged during driving shall be removed to a sound and uniform section conforming to the tolerances for diameter, edge alignment and roundness required to meet the steel pile splice welding requirements. Pipe ends shall be field cut using automated guided cutting equipment. Manual flame cutting shall not be used.

Alternative "X" type piles shall have a dimension, T, not less than 355 mm

REDRIVING.--Piles at Industrial Boulevard Undercrossing (Bridge No. 33-0370), and Mt. Eden Overhead (Bridge No. 33-0092 R/L) which do not attain the required bearing value when the pile tip has reached the specified tip elevation shall be allowed to stand for a "set period" without driving. The "set period" shall be at least 24 to 48 hours unless bearing has been obtained sooner. After the required "set period" has elapsed, 2 piles or 10 percent of such piles in a footing, whichever is greater, shall be redriven. The Engineer will designate which piles are to be redriven. Redriving shall consist of operating the driving hammer at full rated energy on the pile and then measuring the bearing value of the pile.

If the required bearing value has been attained for each pile designated to be redriven, then the remaining piles in that footing shall be considered satisfactory and further driving will not be required. If redriving said designated piles demonstrates that the required bearing value has not been attained, all piles in that footing shall be redriven until the required bearing value has been reached.

Full compensation for redriving and for conforming to the requirements for "set period" and any delays in connection therewith shall be considered as included in the contract unit price paid for driving the piles involved and no separate payment will be made therefor.

Holes shall be predrilled through existing concrete footings for the installation of cast-in-place concrete piles at the locations shown on the plans.

Drilling through the existing reinforced concrete bridge footings shall be done in a manner which preserves as much as possible of the bottom area of the existing footings in a structurally sound, load supporting condition. Drilling methods and equipment shall be approved by the Engineer before starting the predrilling.

CAST-IN-DRILLED-HOLE CONCRETE PILES

Cast-in-drilled-hole concrete piling shall conform to the provisions in Section 49-4, "Cast-In-Place Concrete Piles," of the Standard Specifications and these special provisions.

The sixth sentence of the sixth paragraph of Section 49-4.03, "Drilled Holes," of the Standard Specifications is amended to read:

Casing may be vibrated or hammered when required to assist in removal of the casing from the hole, to prevent lifting of the reinforcement, and to prevent concrete contamination.

Materials

Cast-in-drilled-hole concrete piles 600 mm in diameter or larger may be constructed by excavation and depositing concrete under slurry.

Concrete deposited under slurry shall have a nominal penetration equal to or greater than 90 mm. Concrete shall be proportioned to prevent excessive bleed water and segregation.

Concrete deposited under slurry shall contain not less than 400 kg of cement per cubic meter.

At the Contractor's option, the Contractor may use either the 12.5-mm maximum combined aggregate grading or the 9.5-mm maximum combined aggregate grading. The grading requirements for the 12.5-mm maximum coarse aggregate and the 9.5-mm maximum coarse aggregate are shown in the following table:

Sieve Sizes	Percentage Passing Primary Aggregate Nominal Size			
	12.5 mm x 4.75 mm		9.5 mm x 2.36 mm	
	Operating Range	Contract Compliance	Operating Range	Contract Compliance
19 mm	100	100		
12.5 mm	82 - 100	80 - 100	100	
9.5 mm	X ± 15	X ± 22	X ± 15	X ± 20
4.75 mm	0 - 15	0 - 18	0 - 25	0 - 28
2.36 mm	0 - 6	0 - 7	0 - 6	0 - 7

In the table above, the symbol X is the gradation which the Contractor proposes to furnish for the 9.5-mm sieve size.

The gradation proposed by the Contractor for the 12.5-mm x 4.75-mm primary aggregate or for the 9.5-mm x 2.36-mm primary aggregate shall be within the following percentage passing limits:

Primary Aggregate Nominal Size	Sieve Sizes	Limits of Proposed Gradation
12.5 mm x 4.75 mm	9.5 mm	40 - 78
9.5 mm x 2.36 mm	9.5 mm	50 - 85

The combined aggregate grading for the 12.5-mm x 4.75-mm primary aggregate nominal size or for the 9.5-mm x 2.36-mm primary aggregate nominal size shall be within the following limits:

Grading Limits of Combined Aggregate		
	Percentage Passing	
Sieve Sizes	12.5-mm Max.	9.5-mm Max.
19 mm	100	100
12.5 mm	90 - 100	90 - 100
9.5 mm	55 - 86	50 - 100
4.75 mm	45 - 63	45 - 63
2.36 mm	35 - 49	35 - 49
1.18 mm	25 - 37	25 - 37
600 µm	15 - 25	15 - 25
300 µm	5 - 15	5 - 15
150 µm	1 - 8	1 - 8
75 µm	0 - 4	0 - 4

Construction

The Contractor shall submit a placing plan to the Engineer for approval prior to producing the test batch for cast-in-drilled-hole concrete piling and at least 10 working days prior to constructing piling. The plan shall include complete description, details, and supporting calculations as listed below:

Requirements for all cast-in-drilled hole concrete piling:

1. Concrete mix design, certified test data, and trial batch reports.
2. Drilling methods and equipment.
3. Proposed method for casing installation and removal when necessary.
4. Plan view drawing of pile showing reinforcement and inspection pipes, if required.
5. Methods for placing, positioning and supporting bar reinforcement.
6. Methods and equipment for accurately determining the depth of concrete and actual and theoretical volume placed, including effects on volume of concrete when any casings are withdrawn.

Additional requirements when concrete is placed under slurry:

7. Concrete batching, delivery, and placing systems including time schedules and capacities therefor. Time schedules shall include the time required for each concrete placing operation at each pile.
8. Concrete placing rate calculations. When requested by the Engineer, calculations shall be based on the initial pump pressures or static head on the concrete and losses throughout the placing system, including anticipated head of slurry and concrete to be displaced.
9. Suppliers test reports on the physical and chemical properties of the slurry and any proposed slurry chemical additives including Material Safety Data Sheet.
10. Slurry testing equipment and procedures.
11. Removal and disposal of excavation, slurry, and contaminated concrete, including methods and rates of removal.
12. Slurry agitating, recirculating, and cleaning methods and equipment.

In addition to compressive strength requirements, the consistency of the concrete to be deposited under slurry shall be verified before use by producing a batch to be tested. The test batch shall be produced and delivered to the job under conditions and in time periods similar to those expected during the placement of concrete in the piles. Concrete for the test batch shall be placed in an excavated hole or suitable container of adequate size to allow testing in conformance with California Test 533. Depositing of test batch concrete under slurry will not be required. The test batch shall demonstrate that the proposed concrete mix design achieves both the specified nominal penetration and a penetration of at least 50 mm after twice the time required for each concrete placing operation at each pile, as submitted in the placing plan, has elapsed. The time period shall begin at the start of placement. The concrete shall not be vibrated or agitated during the test period. Upon

completion of testing, the concrete shall be disposed of in conformance with the provisions of Section 7-1.13, "Disposal of Material Outside the Highway Right of Way," of the Standard Specifications.

Concrete deposited under slurry shall not be vibrated until all temporary casing is removed and concrete contaminated with soil, slurry, or other materials is removed. Concrete deposited under slurry shall be vibrated in the upper 2 m of the pile.

The concrete deposited under slurry shall be carefully placed in a compact, monolithic mass and by a method that will prevent washing of the concrete. Placing concrete shall be a continuous operation lasting not more than the time required for each concrete placing operation at each pile, as submitted in the placing plan, unless otherwise approved in writing by the Engineer. The concrete shall be placed with concrete pumps and delivery tube system of adequate number and size to complete the placing of concrete in the time specified. The delivery tube system shall consist of one of the following:

1. A tremie tube or tubes, each of which are at least 250 mm in diameter, fed by one or more concrete pumps.
2. One or more concrete pump tubes, each fed by a single concrete pump.

The delivery tube system shall consist of watertight tubes with sufficient rigidity to keep the ends always in the mass of concrete placed. If only one delivery tube is utilized to place the concrete, the tube shall be placed near the center of the drilled hole. Multiple tubes shall be uniformly spaced in the hole. Internal bracing for the steel reinforcing cage shall accommodate the delivery tube system. Tremies shall not be used for piles without space for a 250-mm tube.

Spillage of concrete into the slurry during concrete placing operations shall not be allowed. Delivery tubes shall be capped with a water tight cap, or plugged above the slurry level with a good quality, tight fitting, moving plug that will expel the slurry from the tube as it is charged with concrete. The cap or plug shall be designed to be released as the tube is charged. The pump discharge or tremie tube shall extend to the bottom of the hole before charging the tube with concrete. After charging the delivery tube system with concrete, the flow of concrete through a tube shall be induced by slightly raising the discharge end. During concrete placement, the tip of the delivery tube shall be maintained to prevent reentry of the slurry into the tube. Until at least 3 m of concrete has been placed, the tip of the delivery tube shall be within 150 mm of the bottom of the drilled hole, and then the embedment of the tip shall be maintained at least 3 m below the top surface of the concrete. Rapid raising or lowering of the delivery tube shall not be permitted. If the seal is lost or the delivery tube becomes plugged and must be removed, the tube shall be withdrawn, the tube cleaned, the tip of the tube capped to prevent entrance of the slurry, and the operation restarted by pushing the capped tube 3 m into the concrete and then reinitiating the flow of concrete.

When slurry is used, the slurry level shall be maintained within 300 mm of the top of the drilled hole.

When slurry is used, a fully operational standby concrete pump, adequate to complete the work in the time specified, shall be provided at the site during concrete placement.

A log of the placing of the concrete in each drilled hole shall be maintained by the Contractor when concrete is deposited under slurry. The log shall show the pile location, tip elevation, dates of excavation and concrete placement, total quantity of concrete deposited, length and tip elevation of any casing, and details of any hole stabilization method and materials used. The log shall include a 215 mm x 280 mm sized graph of the concrete placed versus depth of hole filled. The graph shall be plotted continuously throughout placing of concrete. The depth of drilled hole filled shall be plotted vertically with the pile tip oriented at the bottom and the quantity of concrete shall be plotted horizontally. Readings shall be made at least at each 1.5 m of pile depth, and the time of the reading shall be indicated. The graph shall be labeled with the pile location, tip elevation, cutoff elevation, and the dates of excavation and concrete placement. The log shall be delivered to the Engineer within one working day of completion of placing concrete in the pile.

After placing reinforcement and prior to placing concrete in the drilled hole, if drill cuttings settle out of slurry, as determined by the Engineer, the bottom of the drilled hole shall be cleaned.

If temporary casing is used, concrete placed under slurry shall be maintained at a level at least 1.5 m above the bottom of the casing. The withdrawal of casings shall not cause contamination of the concrete with slurry.

Material resulting from using slurry shall be disposed of in conformance with the provisions in Section 7-1.13, "Disposal of Material Outside the Highway Right of Way," of the Standard Specifications.

Any pile which has been increased in diameter as provided above will be paid for at the contract price per meter for the size of cast-in-drilled-hole concrete piling shown on the plans at that location.

Inspection Pipes for Acceptance Testing

Vertical inspection pipes shall be provided in all cast-in-drilled-hole concrete piles that are 600 mm in diameter or larger, except when the holes are dry or when the holes are dewatered without the use of temporary casing to control the groundwater.

Inspection pipes shall be Schedule 40 polyvinyl chloride pipe with a nominal inside diameter of 50 mm. Each inspection pipe shall be capped top and bottom and shall have watertight couplers to provide a clean, dry and unobstructed 50-mm diameter clear opening from 1.0 m above the pile cutoff down to the bottom of the reinforcing cage.

If the Contractor drills the hole below the specified tip elevation, the Contractor shall extend the reinforcement and the inspection pipes to 75 mm clear of the bottom of the drilled hole.

Inspection pipes shall be placed around the pile, inside the outermost spiral or hoop reinforcement, and 75 mm clear of the vertical reinforcement, at a uniform spacing not exceeding 840 mm measured along the circle passing through the centers of inspection pipes. A minimum of 2 inspection pipes per pile shall be used. When the vertical reinforcement is not bundled and each bar is not more than 26 mm in diameter, inspection pipes may be placed 50 mm clear of the vertical reinforcement.

The inspection pipes shall be placed to provide the maximum diameter circle that passes through the centers of the inspection pipes while maintaining the clear spacing required herein. The pipes shall be installed in straight alignment, parallel to the main reinforcement, and securely fastened in place to prevent misalignment during installation of the reinforcement and placing of concrete in the hole.

The Contractor shall log the location of the inspection pipe couplers with respect to the plane of pile cut off, and these logs shall be delivered to the Engineer upon completion of the placement of concrete in the drilled hole.

After placing concrete and before requesting acceptance tests, each inspection pipe shall be tested by the Contractor in the presence of the Engineer by passing a 48.3-mm diameter rigid cylinder 610 mm long through the complete length of pipe. If the 48.3-mm diameter rigid cylinder fails to pass any of the inspection pipes, the Contractor shall attempt to pass a 32.0-mm diameter rigid cylinder 1.375 m long through the complete length of those pipes in the presence of the Engineer. If an inspection pipe fails to pass the 32.0-mm diameter cylinder, the Contractor shall immediately fill all inspection pipes in the pile with water.

The Contractor shall replace each inspection pipe that does not pass the 32.0-mm diameter cylinder with a 50.8-mm diameter hole cored through the concrete for the entire length of the pile. Cored holes shall be located as close as possible to the inspection pipes they are replacing, no more than 150 mm inside the reinforcement, and coring shall not damage the pile reinforcement. Cored holes shall be made with a double wall core barrel system utilizing a split tube type inner barrel. Coring with a solid type inner barrel will not be allowed. Coring methods and equipment shall provide intact cores for the entire length of the pile concrete. The coring operation shall be logged by an Engineering Geologist or Civil Engineer licensed in the State of California and experienced in core logging. Coring logs shall include complete descriptions of inclusions and voids encountered during coring, and shall be delivered to the Engineer upon completion. Concrete cores shall be preserved, identified with the exact location the core was recovered from within the pile, and made available for inspection by the Engineer.

Acceptance tests of the concrete will be made by the Engineer, without cost to the Contractor. Acceptance tests will evaluate the homogeneity of the placed concrete. Tests will include gamma-gamma logging. Tests may also include crosshole sonic logging and other means of inspection selected by the Engineer. The Contractor shall not conduct operations within 8.0 m of the gamma-gamma logging operations. The Contractor shall separate reinforcing steel as necessary to allow the Engineer access to the inspection pipes to perform gamma-gamma logging or other acceptance testing. After requesting acceptance tests, and providing access to said piling, the Contractor shall allow 15 working days for the Engineer to conduct these tests if the 48.3-mm diameter cylinder passed all inspection pipes, and 20 working days if only the 32.0-mm diameter cylinder passed all inspection pipes. Should the Engineer fail to complete such tests within the time allowance, and if in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in inspection, the delay will be considered a right of way delay as specified in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

All inspection pipes and cored holes in a pile shall be dewatered and filled with grout after notification by the Engineer that the pile is acceptable. Placement and removal of water in the inspection pipes shall be at the Contractors expense. Grout shall conform to the requirements in Section 50-1.09, "Bonding and Grouting," of the Standard Specifications. The inspection pipes and holes shall be filled using grout tubes that extend to the bottom of the pipe or hole or into the grout already placed.

If acceptance testing performed by the Engineer determines that a pile does not meet the requirements of the specifications, then that pile will be rejected and all depositing of concrete under slurry or concrete placed using temporary casing for the purpose of controlling groundwater shall be suspended until written changes to the methods of pile construction are approved in writing by the Engineer.

The Contractor shall submit to the Engineer for approval a mitigation plan for repair, supplementation, or replacement for each rejected cast-in-drilled-hole concrete pile, and this plan shall conform to the provisions in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications. Prior to submitting this mitigation plan, the Engineer will hold a repair feasibility meeting with the Contractor to discuss the feasibility of repairing rejected piling. The Engineer will consider the size of the defect, the location of the defect, and the design information and corrosion protection considerations for the pile. This information will be made available to the Contractor, if appropriate, for the development of the mitigation plan. If the Engineer determines that it is not feasible to repair the rejected pile, the Contractor shall not include repair as a means of mitigation and shall proceed with the submittal of a mitigation plan for replacement or supplementation of the rejected pile.

If the Engineer determines that a pile does not require mitigation due to structural, geotechnical, or corrosion concerns, the Contractor may elect to not repair anomalies found during acceptance testing of that pile. For the unrepaired pile, no payment will be made for the length of pile affected by the anomaly, as determined by the Engineer. Pile mitigation plans shall include the following:

1. The designation and location of the pile addressed by the mitigation plan.
2. A review of the structural, geotechnical, and corrosion design requirements of the rejected pile.
3. A step by step description of the mitigation work to be performed, including drawings if necessary.
4. An assessment of how the proposed mitigation work will address the structural, geotechnical, and corrosion design requirements of the rejected pile.
5. Methods for preservation or restoration of existing earthen materials.
6. A list of affected facilities, if any, with methods and equipment for protection of these facilities during mitigation.

7. The State assigned contract number, bridge number, full name of the structure as shown on the contract plans, District-County-Route-Kilometer Post , and the Contractor's (and Subcontractor's if applicable) name on each sheet.
8. A list of materials, with quantity estimates, and personnel, with qualifications, to be used to perform the mitigation work.
9. The seal and signature of an engineer who is licensed as a Civil Engineer by the State of California.

For rejected piles to be repaired, the Contractor shall submit a pile mitigation plan that contains the following additional information:

1. An assessment of the nature and size of the anomalies in the rejected pile.

2. Provisions for access for additional pile testing if required by the Engineer.

For rejected piles to be replaced or supplemented, the Contractor shall submit a pile mitigation plan that contains the following additional information:

1. The proposed location and size of additional piling.
2. Structural details and calculations for any modification to the structure to accommodate the replacement or supplemental piling.

All provisions for cast-in-drilled-hole concrete piling shall apply to replacement piling.

The Contractor shall allow the Engineer 15 working days to review the mitigation plan after a complete submittal has been received.

Should the Engineer fail to review the complete pile mitigation submittal within the time specified, and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in reviewing the pile mitigation plan, an extension of time commensurate with the delay in completion of the work thus caused will be granted in conformance with the provisions in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

When repairs are performed, the Contractor shall submit to the Engineer a mitigation report within 10 days of completion of the repair. This report shall state exactly what repair work was performed and quantify the success of the repairs relative to the submitted mitigation plan. The mitigation report shall be stamped and signed by an engineer that is licensed as a Civil Engineer by the State of California. The mitigation report shall show the State assigned contract number, bridge number, full name of the structure as shown on the contract plans, District-County-Route-Kilometer Post, and the Contractor (and Subcontractor if applicable) name on each sheet. The Engineer shall be the sole judge as to whether a mitigation proposal is acceptable, the mitigation efforts are successful, and to whether additional repairs, removal and replacement, or construction of a supplemental foundation is required.

SLURRY.--Slurry shall be commercial quality mineral or synthetic drilling slurry and shall conform the requirements of these special provisions

Water for slurry shall conform to the requirements in Section 90-2.03, "Water," of the Standard Specifications and these special provisions. Natural ground water in the drilled hole may be used for slurry when approved by the Engineer.

Slurry shall not weaken the bond between the concrete and both the reinforcement and the foundation material at the sides of the excavation.

The Contractor shall sample and test all slurry in the presence of the Engineer, unless otherwise directed. The date, time, names of the persons sampling and testing the slurry, and results of the tests shall be recorded and shall be approved by the Engineer before concrete is placed. A copy of slurry test results shall be delivered to the Engineer at the completion of each pile.

Mineral.--Mineral slurry shall be mixed and thoroughly hydrated in slurry tanks, and slurry shall be sampled from the slurry tanks and tested before placement in the drilled hole.

Slurry shall be recirculated or continuously agitated in the drilled hole to maintain the specified properties.

Recirculation shall include removal of drill cuttings from the slurry before discharging the slurry back into the drilled hole. When recirculation is used, the slurry shall be sampled and tested at least every 2 hours after beginning its use until tests show that the samples taken form the slurry tank and from near the bottom of the hole have consistent specified properties. Subsequently, slurry shall be sampled at least twice per shift as long as the specified properties remain consistent.

Slurry that is not recirculated in the drilled hole shall be sampled and tested at least every two hours after beginning its use. The slurry shall be sampled midheight and near the bottom of the hole. Slurry shall be recirculated when tests show that the samples taken from midheight and near the bottom of the hole do not have consistent specified properties.

Slurry shall also be sampled and tested prior to final cleaning of the bottom of the hole and again just prior to placing concrete. Samples shall be taken from midheight and near the bottom of the hole. Cleaning of the bottom of the hole and placement of the concrete shall not start until tests show that the samples taken from midheight and near the bottom of the hole have consistent specified properties.

Mineral slurry shall be tested for conformance to the requirements shown in the following table:

MINERAL SLURRY		
PROPERTY	REQUIREMENT	TEST
Density (kg/m^3) - before placement in the drilled hole - during drilling - prior to final cleaning - immediately prior to placing concrete	 1030* to 1110* 1030* to 1200*	 Mud Weight (Density) API 13B-1 Section 1
Viscosity (seconds/liter) bentonite attapulgite	 29 to 53 29 to 42	Marsh Funnel and Cup API 13B-1 Section 2.2
pH	8 to 10.5	Glass Electrode pH Meter or pH Paper
Sand Content (percent) - prior to final cleaning - immediately prior to placing concrete	 less than or equal to 4.0	Sand API 13B-1 Section 5
*When approved by the Engineer, slurry may be used in salt water, and the allowable densities may be increased up to 32 kg/m^3. Slurry temperature shall be at least 4 degrees Celsius when tested.		

Any caked slurry on the sides or bottom of hole shall be removed before placing reinforcement. If concrete is not placed immediately after placing reinforcement, the reinforcement shall be removed and cleaned of slurry, the sides of the drilled hole cleaned of caked slurry, and the reinforcement again placed in the hole for concrete placement.

Synthetic.--Synthetic slurries shall be used in conformance with the manufacturer's recommendations and these special provisions. The following synthetic slurries may be used:

PRODUCT	MANUFACTURER
SlurryPro CDP	KB Technologies Ltd. Suite 216 735 Broad Street Chattanooga, TN 37402 (800) 525-5237
Super Mud	PDS Company 8140 East Rosecrans Ave. Paramount, CA 90723 (310) 634-8180

Inclusion of a synthetic slurry on the above list may be obtained by meeting the Department's requirements for synthetic slurries. The requirements can be obtained from the Office of Structure Design, P.O. Box 942874, Sacramento, CA 94274-0001.

Synthetic slurries listed may not be appropriate for a given site.

Synthetic slurries shall not be used in holes drilled in primarily soft or very soft cohesive soils as determined by the Engineer.

A manufacturer's representative, as approved by the Engineer, shall provide technical assistance for the use of their product, shall be at the site prior to introduction of the synthetic slurry into a drilled hole, and shall remain at the site until released by the Engineer.

Synthetic slurries shall be sampled and tested at both mid-height and near the bottom of the drilled hole. Samples shall be taken and tested during drilling as necessary to verify the control of the properties of the slurry. Samples shall be taken and tested when drilling is complete, but prior to final cleaning of the bottom of the hole. When samples are in conformance with the requirements shown in the following tables for each slurry product, the bottom of the hole shall be cleaned and any loose or settled material removed. Samples shall be obtained and tested after final cleaning and just prior to placing concrete.

SlurryPro CDP synthetic slurries shall be tested for conformance to the requirements shown in the following table:

SLURRYPRO CDP KB Technologies Ltd.		
PROPERTY	REQUIREMENT	TEST
Density (kg/m^3) - during drilling - prior to final cleaning - just prior to placing concrete	less than or equal to 1075* less than or equal to 1025*	Mud Weight (Density) API 13B-1 Section 1
Viscosity (seconds/liter) - during drilling -prior to final cleaning - just prior to placing concrete	53 to 127 less than or equal to 74	Marsh Funnel and Cup API 13B-1 Section 2.2
pH	6 to 11.5	Glass Electrode pH Meter or pH Paper
Sand Content (percent) - prior to final cleaning - just prior to placing concrete	less than or equal to 0.5	Sand API 13B-1 Section 5
*When approved by the Engineer, slurry may be used in salt water, and the allowable densities may be increased up to 32 kg/m^3. Slurry temperature shall be at least 4 degrees Celsius when tested.		

Super Mud synthetic slurries shall be tested for conformance to the requirements shown in the following table:

SUPER MUD PDS Company		
PROPERTY	REQUIREMENT	TEST
Density (kg/m^3) - prior to final cleaning - just prior to placing concrete	less than or equal to 1025*	Mud Weight (Density) API 13B-1 Section 1
Viscosity (seconds/liter) - during drilling - prior to final cleaning - just prior to placing concrete	34 to 64 less than or equal to 64	Marsh Funnel and Cup API 13B-1 Section 2.2
pH	8 to 10.0	Glass Electrode pH Meter or pH Paper
Sand Content (percent) - prior to final cleaning -just prior to placing concrete	less than or equal to 0.5	Sand API 13B-1 Section 5
*When approved by the Engineer, slurry may be used in salt water, and the allowable densities may be increased up to 32 kg/m^3. Slurry temperature shall be at least 4 degrees Celsius when tested.		

Water.--At the option of the Contractor water may be used as slurry when casing is used for the entire length of the drilled hole.

Water slurry shall be tested for conformance to the requirements shown in the following table:

WATER SLURRY		
PROPERTY	REQUIREMENT	TEST
Density (kg/m^3) - prior to final cleaning - just prior to placing concrete	1017 *	Mud Weight (Density) API 13B-1 Section 1
Sand Content (percent) - prior to final cleaning -just prior to placing concrete	less than or equal to 0.5	Sand API 13B-1 Section 5
*When approved by the Engineer, salt water slurry may be used, and the allowable densities may be increased up to 32 kg/m^3.		

MEASUREMENT AND PAYMENT (PILING).—Measurement and payment for the various types and classes of piles shall conform to the provisions in Sections 49-6.01, "Measurement," and 49-6.02, "Payment," of the Standard Specifications and these special provisions.

Payment for cast-in-place concrete piling will be as provided in Section 49-6.02, "Payment," of the Standard Specifications except that, when the diameter of cast-in-place concrete piling is shown on the plans as 600-mm or larger, reinforcement in the piling, except cast-in-drilled hole piling, will be paid for as bar reinforcing steel (bridge).

The sixth paragraph in Section 49-6.02, "Payment," of the Standard Specifications is amended to read:

> If precast prestressed concrete piling or steel pipe piling is manufactured or fabricated more than 480 air line kilometers from both Sacramento and Los Angeles, additional shop inspection expenses will be sustained by the State. Whereas it is and will be impractical and extremely difficult to ascertain and determine the actual increase in such expenses, it is agreed that payment to the Contractor for furnishing piling of the types shown in the Engineer's Estimate will be reduced $5000 for each manufacture or fabrication site located more than 480 air line kilometers from both Sacramento and Los Angeles and an additional $3000 ($8000 total) for each manufacture or fabrication site located more than 4800 air line kilometers from both Sacramento and Los Angeles.

Full compensation for excavating the rock slope protection or pretrenching to facilitate the installation of the precast prestress sheet pile shall be considered as included in the contract price paid per unit for drive prestressed concrete sheet pile and no additional compensation will be allowed therefor.

Full compensation for drilling holes through existing concrete footings for installing cast-in-place concrete piling, and disposing of material resulting from drilling shall be considered as included in the contract price paid per unit for Cast-in-place concrete piling of the size shown on the plans and no additional compensation will be allowed therefor.

Full compensation for slurry, depositing concrete under slurry, test batches, inspection pipes, filling inspection holes and pipes with grout, drilling oversized cast-in-drilled-hole concrete piling, filling cave-ins and oversized piles with concrete, and redrilling through concrete shall be considered as included in the contract prices paid per meter for cast-in-drilled-hole concrete piling of the sizes listed in the Engineer's Estimate and no additional compensation will be allowed therefor.

10-1.54 PILING (TOLL BOOTH)

Piling (Toll Booth) shall conform to the provisions in Section 49, "Piling," of the Standard Specifications, and these special provisions. Section 49-1.08, "Bearing Value and Penetration," of the Standard Specifications shall not apply

Foundation recommendations are included in the "Information Handout" available to the Contractor as provided for in Section 2-1.03, "Examination of Plans, Specifications, Contract, and Site of Work," of the Standard Specifications.

Attention is directed to "Public Safety," of these special provisions. Before performing any pile handling or pile installation operation at any location that is closer than the length of the pile being handled or installed to the edge of any area open to public traffic or public use, the Contractor shall submit to the Engineer, as provided in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications, a detail plan of the measures that will be employed to provide for the safety of traffic and the public.

Piles shall be driven to the length specified on the plans.

The first and second paragraphs in Section 49-1.05, "Driving Equipment," of the Standard Specifications are amended to read:

49-1.05 Driving Equipment.—Driven piles shall be installed with impact hammers that are approved in writing by the Engineer. Impact hammers shall be steam, hydraulic, air, or diesel hammers. Impact hammers shall develop sufficient energy to drive the piles at a penetration rate of not less than 3 mm per blow at the specified bearing value.

Vibratory hammers shall not be used for installation of piles, unless otherwise shown on the plans or specified in the special provisions.

Hammers with an external combustion engine that are not single action, shall have a transducer that records ram velocity.

Double acting diesel hammers with internal combustion engines shall have a transducer that records bounce chamber pressure.

For hammers with no visual way of observing the ram stroke, a printed readout showing hammer energy during driving operation shall be provided to the Engineer by the Contractor.

The fifth paragraph in Section 49-1.05, "Driving Equipment," of the Standard Specifications is deleted.

Predrilled Holes

Piles shall be driven in over sized drilled holes in conformance with the provisions in Section 49-1.06, "Predrilled Holes," of the Standard Specifications at the locations and to the corresponding bottom of hole elevations listed in the following table:

Location	Length of Predrilled Hole
Tunnel	1.5 m below tunnel invert
Columns	3 m below bottom of footing
Concrete slab extension	5 m below bottom of footing

Full compensation for predrilling, filling the space around piles driven in predrilled holes with sand or pea gravel, and disposing of material resulting from jetting, drilling, and predrilling shall be considered as included in the contract unit price paid for driving the piles involved, and no additional compensation will be allowed therefor.

MEASUREMENT AND PAYMENT (PILING TOLL BOOTH)

Measurement and payment for the various types and classes of piles shall conform to the provisions in Sections 49-6.01, "Measurement," and 49-6.02, "Payment," of the Standard Specifications and these special provisions.

The sixth paragraph in Section 49-6.02, "Payment," of the Standard Specifications is amended to read:

If precast prestressed concrete piling or steel pipe piling is manufactured or fabricated more than 480 air line kilometers from both Sacramento and Los Angeles, additional shop inspection expenses will be sustained by the State. Whereas it is and will be impractical and extremely difficult to ascertain and determine the actual increase in such expenses, it is agreed that payment to the Contractor for furnishing piling of the types shown in the Engineer's Estimate will be reduced $5000 for each manufacture or fabrication site located more than 480 air line kilometers from both Sacramento and Los Angeles and an additional $3000 ($8000 total) for each manufacture or fabrication site located more than 4800 air line kilometers from both Sacramento and Los Angeles.

10-1.55 PRESTRESSING CONCRETE

Prestressing concrete shall conform to the provisions in Section 50, "Prestressing Concrete," of the Standard Specifications and these special provisions.

The first paragraph in Section 50-1.02, "Drawings," of the Standard Specifications is amended to read:

The Contractor shall submit to the Office of Structure Design (OSD) for approval in accordance with the provisions in Section 5-1.02, "Plans and Working Drawings," working drawings of the prestressing system proposed for use. For initial review, 6 sets of the drawings shall be submitted for railroad bridges and 4 sets shall be submitted for other structures. After review, between 6 and 12 sets, as requested by the Engineer, shall be submitted to OSD for final approval and for use during construction.

The sixth paragraph in Section 50-1.02, "Drawings," of the Standard Specifications is amended to read:

At the completion of each structure on the contract, one set of reduced prints on 75-g/m^2 (minimum) bond paper, 279 mm x 432 mm in size, of the corrected original tracings of all working drawings for each structure shall be furnished to the Engineer. Reduced prints of drawings which are common to more than one structure shall be submitted for each structure. An index prepared specifically for the drawings for each structure containing sheet numbers and titles shall be included on the first reduced print in the set for each structure. Reduced prints for each structure shall be arranged in the order of drawing numbers shown in the index.

The seventh paragraph in Section 50-1.08, "Prestressing," of the Standard Specifications is amended to read:

Each jack used to stress tendons shall be equipped with either: (1) two pressure gages or (2) one pressure gage and a load cell, at the option of the Contractor. The jack body shall be permanently marked with the ram area. Each pressure gage shall be fully functional and have an accurately reading dial at least 150 mm in diameter. The jack and each gage shall be calibrated as a unit with the cylinder extension in the approximate position that it will be at final jacking force. The load cell, if used, shall be calibrated and shall be provided with an indicator which may be used to determine the prestressing force in the tendon. The range of the load cell shall be such that the lower 10 percent of the manufacturer's rated capacity will not be used in determining the jacking stress. The jacking equipment calibration procedure shall be as follows:

Each jack used to stress tendons, which are permanently anchored at 25 percent or more of the specified minimum ultimate tensile strength of the prestressing steel, shall be calibrated by the Transportation Laboratory within one year prior to use and after each repair, unless otherwise directed. The Contractor shall be responsible for:

1. scheduling the calibration of the jacking equipment with the Transportation Laboratory, telephone (916) 227-7251;
2. verifying that the jack and supporting systems are complete, with proper components, and are in good operating condition;
3. mechanically calibrating the gages with a dead weight tester or other approved means prior to calibration of the jacking equipment by the Transportation Laboratory,
4. providing sufficient labor, equipment, and material to install and support the jacking and calibration equipment and to remove the equipment after the calibration is complete, and;
5. plotting the calibration results.

Each jack used to stress tendons, which are permanently anchored at less than 25 percent of the specified minimum ultimate tensile strength of the prestressing steel, shall be calibrated by a private laboratory approved by the Transportation Laboratory within 6 months prior to use and after each repair, unless otherwise directed.

10-1.56 CONCRETE STRUCTURES

Portland cement concrete structures shall conform to the provisions in Section 51, "Concrete Structures," of the Standard Specifications and these special provisions.

Drainage Inlets located in areas to be paved or surfaced shall be, at the option of the Contractor, cast-in-place structures or combined precast and cast-in-place structures, provided the structures in place substantially conform to cast-in-place construction as specified in the Standard Specifications. Where combined precast and cast-in-place structures are used, the top portion of the structure surrounding the inlet frame and grate, or manhole cover, shall be of cast-in-place construction. No individual structure shall be constructed to final grade until the paving or surfacing has been completed immediately adjacent to the structure.

Shotcrete shall not be used as an alternative construction method for reinforced concrete members unless otherwise specified.

The first sentence of the tenth paragraph in Section 51-1.05, "Forms," of the Standard Specifications is amended to read:

Form panels for exposed surfaces shall be plywood conforming to or exceeding the requirements of U.S. Product Standard PS 1 for Exterior B-B (Concrete Form) Class I Plywood or any material which will produce a smooth uniform concrete surface substantially equal to that which would result from the use of that plywood.

The third paragraph in Section 51-1.15, "Drains in Walls," of the Standard Specifications is amended to read:

In addition to the drain holes and weep holes specified in the preceding paragraph, holes approximately 75 mm in diameter for relief of hydrostatic pressure shall be provided at the bottom of walls, immediately above the footing, at approximately 4500-mm centers.

Materials for access opening covers in soffits of new cast-in-place concrete box girder bridges shall conform to the provisions for materials in Section 75-1.03, "Miscellaneous Bridge Metal," of the Standard Specifications.

Concrete.—Concrete used in the construction of the slabs at Toll Plaza Retaining Wall shall be fast setting early strength concrete type, and shall contain not less than 400 kg of cement per cubic meter and shall conform Section 51, "Concrete Structures," of the Standard Specifications and these special provisions

The Contractor shall submit to the Engineer for approval 1) an optional design mix for fast setting early strength concrete or 2) fast-setting hydraulic cement concrete or 3) concrete with a non-chloride Type C chemical admixture, and shall conform to the following requirements:

The Contractor shall schedule his operations so that the pavement and base materials removed during a work period shall be replaced, in that same work period, with fast-setting early strength concrete. In the event the existing pavement and base materials are removed and the Contractor is unable, as determined by the Engineer, to construct, finish and cure the new slab by the time the lane is to be opened to public traffic, the excavation shall be filled with a temporary roadway structural section as specified in this section.

The Contractor shall provide, at the job site, a sufficient standby quantity, as determined by the Engineer, of asphalt concrete and aggregate base for construction of a temporary roadway structural section. The temporary structural section shall be maintained, and later removed as a first order of work when the Contractor is able to construct and cure the slab within the prescribed time limit. The temporary structural section shall consist of 90-mm thick layer of asphalt concrete over aggregate base.

At least 10 days prior to use, the Contractor shall submit in writing to the Engineer a proposed mix design that shall include the proposed aggregate gradings, the mix proportions of cement and aggregate, the type and amount of chemical admixture, the maximum time allowed between batching and placing concrete, the range of temperature over which this mix design is effective (10° C maximum) and any special conditions (including water temperature) or instructions. The Contractor may submit more than one mix design to include the temperature range that is anticipated during placement of concrete.

Fast-Setting Hydraulic Cement Concrete

Fast-setting hydraulic cement concrete shall be cured for at least 6 hours and meets the minimum strength requirements of "Evaluation and Acceptance" in this section prior to the time the lane is to be opened to public traffic as designated in "Maintaining Traffic" of these special provisions

Material for fast-setting hydraulic cement concrete shall conform to the provisions in Section 40, "Portland Concrete Pavement," of the Standard Specifications and these special provisions.

The combined aggregate grading used in fast-setting hydraulic cement concrete shall be either the 37.5-mm maximum or the 25-mm maximum grading.

The cement for fast-setting hydraulic cement concrete shall be a hydraulic cement as defined in ASTM Designation: C219 and shall conform to the following requirements:

Property	Test Method	Requirement
Contraction in Air	California Test 527 w/C Ratio = 0.39±0.010	0.053% (max)
Mortar Expansion in Water	ASTM: C1038	0.04% (max)
Soluble Chloride	California Test 422	0.05% (max)
Soluble Sulfates	California Test 417	0.30% (max)
Thermal Stability	ASTM: C109	90% (min)*
Compressive Strength @ 3 hours	ASTM: C109	13.8 MPa
Compressive Strength @ 3 days	ASTM: C109	27.5 MPa

* Comparison of compressive strength cubes cured 7 days in water at 23°C followed by 7 days in water at 90°C to that of cubes cured 14 days in water at 23°C.

The provisions in Section 90-1.01, "Description," and of the Standard Specifications shall not apply to fast-setting hydraulic cement concrete except as follows:

The mix proportions for fast-setting hydraulic cement concrete shall be as recommended by the manufacturer of the hydraulic cement.

The provisions in Section 90-2.01, "Portland Cement," and of the Standard Specifications shall not apply except as follows:

Hydraulic cement shall be protected from exposure to moisture until used. Adequate facilities shall be provided to assure that hydraulic cement meeting the requirements of these special provisions will be kept separate from other cement in order to prevent any but the specified hydraulic cement from entering the work. Safe and suitable facilities for sampling hydraulic cement shall be provided at the weigh hopper, or in the feed line immediately in advance of the hopper.

Hydraulic cement furnished without a Certificate of Compliance shall not be used in the work.

A Type C accelerating chemical admixture approved by the Engineer and conforming to the requirements in Section 90-4, "Admixtures," of the Standard Specifications may be used. The Type C accelerating chemical admixture, if used, shall be included in all the testing for the requirements listed in the table above. In addition to the accelerating chemical admixtures listed on the Department's current list of approved brands of admixtures, citric acid or borax may be used. All chemical admixtures shall be blended into fast-setting hydraulic cement concrete in liquid form.

In addition, the Contractor shall furnish samples of the hydraulic cement, aggregates and chemical admixture proposed for use in fast-setting hydraulic cement concrete in the quantity ordered by the Engineer. The Contractor shall develop and furnish both flexural strength and compressive strength gain curves for each proposed mix design for fast-setting hydraulic cement concrete measured at intervals of 3 hours, 6 hours, 8 hours, 24 hours, 7 days and 28 days. Strength gain curves may be developed from laboratory prepared samples. Fast-setting hydraulic cement concrete may be laboratory prepared and shall include all admixtures. 10 days in advance of revising the mix proportions for the fast-setting hydraulic cement concrete to be used in the work, the Contractor shall submit in writing to the Engineer a copy of the proposed mix design. Before using fast-setting hydraulic cement concrete, the mix designs shall be approved by the Engineer.

Fast-setting hydraulic cement concrete shall develop a flexural strength (modulus of rupture) of not less than 5.85 MPa not more than 6 hours after completion of final finishing and also shall develop a flexural strength (modulus of rupture) of not less 25 MPa 28 days after completion. The modulus of rupture shall be considered to be the average of the test results of 3 beam specimens determined by California Test 523 or AASHTO T97.

The penetration requirement in Section 90-6.06, “Amount of Water and Penetration,” of the Standard Specifications shall not apply.

The provisions in Section 40-1.015, “Cement Content” of the Standard Specifications shall not apply to fast-setting hydraulic cement concrete.

Fast-Setting Cement Concrete Using Non-Chloride Type C Chemical Admixture

The removal of forms, which support the dead load of the cantilevered section of the retaining wall, shall not begin until at least 5 days or until the concrete has sufficient strength to support its deal load and other loads applied on it by the Contractor operations.

Portland cement for use in concrete using a non-chloride Type C chemical admixture shall be Type II Modified, Type II Prestress, or Type III. Type II Modified and Type III cement shall conform to the provisions in Section 90-2.01, "Portland Cement," of the Standard Specifications. Type II Prestress cement shall conform to the requirements of Type II Modified cement, except the mortar containing the portland cement to be used and Ottawa sand, when tested in conformance with California Test 527, shall not contract in air more than 0.053-percent.

The non-chloride Type C chemical admixture shall be approved by the Engineer and conform to the requirements of ASTM Designation: C 494 and Section 90-4, "Admixtures," of the Standard Specifications.

The concrete with non-chloride Type C chemical admixture shall be prequalified prior to placement in conformance with the provisions for prequalification of concrete specified by compressive strength in Section 90-9.01, "General," of the Standard Specifications and the following:

Immediately after fabrication of the 5 test cylinders, the cylinders shall be stored in a temperature medium of 21 ± 1.5°C until the cylinders are tested.

The 6-hour average strength of the 5 test cylinders shall not be less than 5.85 MPa. No more than 2 test cylinders shall have a strength of less than 5.5 MPa.

Slabs shall be cured with pigmented curing compound (1) in conformance with the provisions for curing structures in Section 90-7.01B, "Curing Compound Method," of the Standard Specifications. The minimum curing period as specified elsewhere in this section, shall be considered to begin at the start of discharge of the last truck load of concrete to be used in the slab. Fogging of the surface with water after the curing compound has been applied will not be required. Should the film of curing compound be damaged from any cause before the slab is opened to public traffic, the damaged portion shall be repaired immediately with additional compound, at the Contractor's expense. Any damage to the curing compound after the slab is opened to public traffic shall not be repaired.

If the ambient temperature is below 18°C during the curing period, an insulating layer or blanket shall cover the surface. The insulation layer or blanket shall have an R-value rating given in the table below. At the Contractor's option, a heating tent may be used in lieu of or in combination with the insulating layer or blanket:

Temperature range during curing period	R-value, minimum
13°C to 18°C	1
7°C to 13°C	2
4°C to 7°C	3

The top surface of the slabs at Toll Plaza Retaining Wall shall be finished in conformance with the provisions in Section 51-1.17, "Finishing Bridge Decks," of the Standard Specifications. The finished top surface shall not vary more than 6 mm from the lower edge of a 3.6-m straightedge placed parallel with the centerline. Edges of slabs shall be edger finished.

Tests to determine the coefficient of friction of the final textured surface will be made only if the Engineer determines by visual inspection that the final texturing may not have produced a surface having the specified coefficient of friction. Any tests to determine the coefficient of friction will be made after the approach slab is opened to public

traffic, but not later than 5 days after concrete placement. The coefficient of friction will be measured by California Test 342. Any portions of completed concrete surfaces that are found to have a coefficient of friction less than 0.35 shall be ground or grooved parallel to the center line in conformance with the requirements for bridge decks of Section 42, "Groove and Grind Pavement," of the Standard Specifications.

FALSEWORK

Falsework shall be designed and constructed in conformance with the requirements in Section 51-1.06, "Falsework," of the Standard Specifications and these special provisions.

Attention is directed to "Railroad Relations and Insurance" of these special provisions for additional requirements for falsework over railroads.

In addition to the requirements in Section 51-1.06A, "Falsework Design and Drawings," of the Standard Specifications, the following requirements shall apply:

The time to be provided for the Engineer's review of the working drawings for specific structures, or portions thereof, shall be as follows:

Structure or Portion of Structure	Review Time - Weeks
MT. EDEN OVERHEAD (Bridge No. 33-0092 R/L)	11

The fourth and fifth sentences of the eighteenth paragraph of Section 51-1.06A, "Falsework Design and Drawings," of the Standard Specifications are deleted.

The fifth paragraph of Section 51-1.06A(1), "Design Loads," of the Standard Specifications is amended to read:

The minimum horizontal load to be allowed for wind on heavy-duty steel shoring or steel pipe column falsework having a vertical load carrying capacity exceeding 130 kN per leg or column shall be the sum of the products of the wind impact area, shape factor, and the applicable wind pressure value for each height zone. The wind impact area is the total projected area of all the elements in the tower face or falsework bent normal to the direction of the applied wind. The shape factor shall be taken as 2.2 for heavy-duty shoring and 1.0 for pipe column falsework. Wind pressure values shall be determined from the following table:

Height Zone (Meters above ground)	Wind Pressure Value (Pa)	
	Shores or Columns Adjacent to Traffic	At Other Locations
0-9	960	720
9-15	1200	960
15-30	1440	1200
over 30	1675	1440

The first 2 sentences of the sixth paragraph of Section 51-1.06A(1), "Design Loads," of the Standard Specifications are amended to read:

The minimum horizontal load to be allowed for wind on all other types of falsework, including falsework supported on heavy-duty shoring or pipe column falsework, shall be the sum of the products of the wind impact area and the applicable wind pressure value for each height zone. The wind impact area is the gross projected area of the falsework and any unrestrained portion of the permanent structure, excluding the areas between falsework bents or towers where diagonal bracing is not used.

The last paragraph under "Timber" in the second paragraph of Section 51-1.06A(2), "Design Stresses, Loadings, and Deflections," of the Standard Specifications is amended to read:

Timber connections shall be designed in conformance with the procedures, stresses and loads permitted in the Falsework Manual as published by the Department of Transportation, Division of Structures, Office of Structure Construction.

The first entry following the third paragraph under "Steel" in the second paragraph of Section 51-1.06A(2), "Design Stresses, Loadings, and Deflections," of the Standard Specifications is amended to read:

Compression, flexural.... $\frac{83{,}000}{Ld/bt}$ MPa, but not to exceed 152 MPa for unidentified steel or steel conforming to ASTM Designation: A 36/A 36M nor 0.6Fy for other identified steel.

The third paragraph of Section 51-1.06B, "Falsework Construction," of the Standard Specifications is amended to read:

When falsework is supported on piles, the piles shall be driven and the actual bearing value assessed as specified in Section 49, "Piling."

For falsework piles with a calculated loading capacity greater than 900 kN, the Contractor shall conduct dynamic monitoring of pile driving and conduct penetration and bearing analyses based on a wave equation analysis. Said analysis shall be signed by an engineer who is registered as a Civil Engineer in the State of California and submitted to the Engineer prior to completion of falsework erection.

Temporary crash cushion modules, as shown on the plans and conforming to the provisions in "Temporary Crash Cushion Module," of these special provisions, shall be installed at the approach end of temporary railings which are located less than 4.6 m from the edge of a traffic lane. For two-way traffic openings, temporary crash cushion modules shall be installed at the departing end of temporary railings which are located less than 1.8 m from edge of a traffic lane.

Prior to erecting any falsework bent within 6 m horizontally from the centerline of the railroad track, temporary collision posts shall be installed. The collision posts shall be located 4 m clear of the centerline of the track or at the same clearance provided by the falsework, whichever is less, and approximately 30 m from each end of the falsework bent. The collision posts shall consist of unspliced 7.5-m long steel HP 250 x 62 sections set vertically with their webs parallel to the track in 1.8-m deep concrete-backfilled holes. The diameter of the holes shall be not less than 150 mm larger than the greatest dimension of the post cross section. The concrete shall be Class 3 or minor concrete conforming to the provisions in Section 90, "Portland Cement Concrete," of the Standard Specifications. Alternative steel sections may be used if the section moduli are not less than those of an HP 250 x 62 section and the thickness of the metal is not less than 10 mm. Collision posts shall remain in place until the falsework has been removed and then the posts shall be removed to at least 300 mm below the ground line. Full compensation for furnishing, installing and removing temporary collision posts shall be considered as included in the price paid per cubic meter for the cast-in-place concrete supported by the falsework, and no additional compensation will be allowed therefor.

DECK CLOSURE POURS.—Where deck closure pour is shown on the plans, reinforcement protruding into the closure space and forms for the closure pour shall conform to the following:

During the time of placement of concrete in the deck, other than for the closure pour itself, reinforcing steel which protrudes into the closure space shall be completely free from any connection to the reinforcing steel, concrete, or other attachments of the adjacent structure, including forms. The reinforcing steel shall remain free of any connection for a period of not less than 24 hours following completion of the pour.

Forms for the closure pour shall be supported from the superstructure on both sides of the closure space.

ELASTOMERIC BEARING PADS.—Elastomeric bearing pads shall conform to the provisions in Section 51-1.12H, "Elastomeric Bearing Pads," of the Standard Specifications and these special provisions.

The fifth paragraph of Section 51-1.12H(1), "Plain and Fabric Reinforced Elastomeric Bearings," of the Standard Specifications is amended to read:

The peel strength test will be performed after immersing the sample in water for a minimum of 10 days. The bond between elastomer and fabric shall be such that when a sample is tested for separation, it shall have a minimum peel strength of 5.3 kN/m when tested in accordance with California Test 663.

The last 2 sentences of the tenth paragraph of Section 51-1.12H(1), "Plain and Fabric Reinforced Elastomeric Bearings," of the Standard Specifications are amended to read:

Pads shall be available for sampling at least 4 weeks in advance of intended use. All sample pads for testing shall be furnished by the Contractor at the Contractor's expense.

The fifth subparagraph of the first paragraph of Section 51-1.12H(2), "Steel Reinforced Elastomeric Bearings," of the Standard Specifications is amended to read:

One sample bearing shall be furnished to the Engineer from each lot of bearings to be furnished for the contract. Samples shall be available at least 3 weeks in advance of intended use. The sample bearing shall be one of the following:

Bearing Pad Thickness as Shown on the Plans	Sample Bearing
50 mm or less	Smallest complete bearing shown on the plans
Greater than 50 mm	* 57 ± 3 mm thick sample not less than 200 mm x 305 mm in plan and cut by the manufacturer from the center of one of the thickest complete bearings

* The sample bearing plus remnant parts of the complete bearing shall be furnished to the Engineer

MEASUREMENT AND PAYMENT.--Measurement and payment for concrete in structures shall conform to the provisions in Sections 51-1.22, "Measurement," and 51-1.23, "Payment," of the Standard Specifications and these special provisions.

Full compensation for furnishing and installing access opening covers in soffits of new cast-in-place box girder bridges shall be considered as included in the contract price paid per cubic meter for structural concrete, bridge, and no separate payment will be made therefor.

10-1.57 PRECAST PRESTRESSED CONCRETE BRIDGE MEMBERS

Precast prestressed concrete members shall conform to the provisions in Section 51, "Concrete Structures," of the Standard Specifications and these special provisions.

The anticipated deflection and method of accommodation of deflection of precast prestressed concrete girders, prior to the time the deck concrete is placed, shall be shown on the working drawings as provided in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications. The deflection shall include the following:

1. Anticipated upward deflection caused by the prestressing forces.
2. Downward deflection caused by the dead load of the girder.
3. Deflection caused by the creep and shrinkage of the concrete for the time interval between the stressing of the girders and the planned placement of the deck.

Such deflection shall be substantiated by calculations that consider the ages of the girder concrete at the time of stressing and the Contractor's planned placement of the deck. All deflection calculations shall be based on the concrete producer's estimate of the modulus of elasticity at the applicable concrete age.

Adjustments to accommodate girder deflections, which occur prior to the time the deck concrete is placed, may include revisions in bearing seat elevations, but any such adjustments shall be limited by the following conditions:

A. The minimum permanent vertical clearance under the structure as shown on the plans shall not be reduced.
B. The profile grade and cross slope of the deck shall not be changed.
C. A minimum of 25 mm of deck slab concrete between the top of the precast girders and the deck slab reinforcement shall be maintained.

Girders with unanticipated girder deflection and which cannot comply with conditions A, B and C will be rejected in accordance with the provisions in Section 6-1.04, "Defective Materials."

Adjustments to accommodate girder deflections will not be considered a change in dimensions. Full compensation for any increase in the cost of construction, including any increase in the quantity of deck or bearing seat concrete, resulting from adjustments to accommodate girder deflections shall be considered as included in the contract price paid for the various items of work involved, and no additional compensation will be allowed therefor.

Temporary lateral bracing shall be provided for girders located over the roadway, at the Industrial Boulevard Undercrossing bridge. The bracing shall be installed at each end of each girder, except notched ends, prior to the release of the erection equipment from the girder and shall remain in place until 2 days after the concrete diaphragms have been placed. The bracing shall be adequate to prevent overturning of the girders prior to completion of the work and as a minimum shall be capable of resisting a lateral force of 720 Pa of girder side area applied laterally in either direction to the top of the girder. Girder erection shall not be started until the temporary lateral bracing proposed for use by the Contractor has been approved by the Engineer.

10-1.58 CLOSURE WALLS

Concrete closure walls used to curtain off open areas at bridges shall conform to the details shown on the plans and the requirements in Section 51, "Concrete Structures," of the Standard Specifications and these special provisions.

The closure walls shall be constructed in accordance with either Alternative 1, 2 or 3 as shown on the plans, at the option of the Contractor.

For prestressed concrete panels, the provisions in Section 50-1.02, "Drawings," of the Standard Specifications shall not apply. The Contractor shall submit 2 sets of working drawings for the prestressed concrete panels to the Engineer for use in

administering the contract. The drawings shall show the panel dimensions, materials, prestressing methods, tendon arrangement and working stresses, including any addition or rearrangement of reinforcing steel from that shown on the plans. Prestressing steel shall be placed not more than 150 mm from the edges of the panel and spaced at not more than 450-mm centers between the edge tendons.

Anchorage details consisting of bars and bolts shall conform to the provisions in Section 75-1.03, "Miscellaneous Bridge Metal," of the Standard Specifications. The bolt holes shall conform to the provisions in Section 55-3.14A, "Bolt Holes," of the Standard Specifications, except that punching of the holes to full size will not be permitted, and the finished hole sizes shall be as shown on the plans.

Precast panels shall have an exposed surface matching the adjacent cast-in-place concrete.

The quantity of concrete closure wall will be measured by the square meter on the outside surface of the completed panels.

The contract price paid per square meter for concrete closure wall shall include full compensation for furnishing all labor, materials (including reinforcement, prestressing steel, and anchorages), tools, equipment, and incidentals, and for doing all the work involved in constructing the closure wall complete in place, including erecting precast panels, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.59 STRUCTURE APPROACH SLABS (TYPE EQ)

This work shall consist of constructing reinforced concrete approach slabs in accordance with the details shown on the plans, the provisions in Section 51, "Concrete Structures," of the Standard Specifications and these special provisions.

APPROACH SLABS:

Concrete for use in approach slabs— shall contain not less than 400 kilograms of cement per cubic meter.

Miscellaneous steel parts and all steel components of abutment ties including plates, nuts, washers, and rods shall conform to the provisions in Section 75-1.03, "Miscellaneous Bridge Metal," of the Standard Specifications. Any bar reinforcement that has an unbonded portion shall be galvanized in accordance with the provisions in Section 75-1.05, “Galvanizing,” of the Standard Specifications.

Except as listed herein, structure approach slabs shall be cured for not less than 5 days prior to opening to traffic, unless, at the option of the Contractor, the structure approach slabs are constructed using concrete with a non-chloride Type C chemical admixture.

Portland cement for use in concrete using a non-chloride Type C chemical admixture shall be Type II Modified, Type II Prestress, or Type III. Type II Modified and Type III cement shall conform to the provisions in Section 90-2.01, "Portland Cement," of the Standard Specifications. Type II Prestress cement shall conform to the requirements of Type II Modified cement, except the mortar containing the portland cement to be used and Ottawa sand, when tested in accordance with California Test 527, shall not contract in air more than 0.053-percent.

The non-chloride Type C chemical admixture, approved by the Engineer, shall conform to the requirements of ASTM Designation: C 494 and Section 90-4, "Admixtures," of the Standard Specifications.

The concrete with non-chloride Type C chemical admixture shall be prequalified prior to placement in accordance with the provisions for prequalification of concrete specified by compressive strength in Section 90-9.01, "General," of the Standard Specifications and the following:

Immediately after fabrication of the 5 test cylinders, the cylinders shall be stored in a temperature medium of 21 ± 1.5°C until the cylinders are tested.

The 6-hour average strength of the 5 test cylinders shall not be less than 5.85 MPa. No more than 2 test cylinders shall have a strength of less than 5.5 MPa.

The top surface of approach slabs shall be finished in conformance with the provisions in Section 51-1.17, "Finishing Bridge Decks," of the Standard Specifications. Edges of slabs shall be edger finished.

Approach slabs shall be cured with pigmented curing compound (1) in accordance with the provisions for curing structures in Section 90-7.01B, "Curing Compound Method," of the Standard Specifications.

Structure approach slabs constructed using concrete with a non-chloride Type C chemical admixture shall be cured for not less than 6 hours prior to opening to traffic. The curing period shall be considered to begin at the start of discharge of the last truck load of concrete to be used in the slab.

If the ambient temperature is below 18°C during the curing period for approach slabs using concrete with a non-chloride Type C chemical admixture, an insulating layer or blanket shall cover the surface. The insulation layer or blanket shall have an R-value rating given in the table below. At the Contractor's option, a heating tent may be used in lieu of or in combination with the insulating layer or blanket.

Temperature range during curing period	R-value, minimum
13°C to 18°C	1
7°C to 13°C	2
4°C to 7°C	3

JOINTS.--Hardboard and expanded polystyrene shall conform to the provisions in Section 51-1.12D, "Sheet Packing, Preformed Pads and Board Fillers," of the Standard Specifications.

Type AL joint seals shall conform to the provisions in Section 51-1.12F, "Sealed Joints" of the Standard Specifications. The sealant may be mixed by hand-held power-driven agitators and placed by hand methods.

The pourable seal between the steel angle and concrete barrier shall conform to the requirements for Type A and AL seals in Section 51-1.12F(3), "Materials and Installation," of the Standard Specifications. The sealant may be mixed by hand-held power-driven agitators and placed by hand methods. Immediately prior to placing the seal, the joint shall be thoroughly cleaned, including abrasive blast cleaning of the concrete surfaces, so that all foreign material and concrete spillage are removed from all joint surfaces. Joint surfaces shall be dry at the time the seal is placed.

MEASUREMENT AND PAYMENT.--Structural concrete, approach slab (Type EQ) will be measured and paid for in accordance with the provisions in Sections 51-1.22, "Measurement," and 51-1.23, "Payment," of the Standard Specifications and these special provisions.

Full compensation for miscellaneous bridge metal, pourable seals and miscellaneous bridge metal, shall be considered as included in the contract price paid per cubic meter for structural concrete, approach slab (Type EQ) and no additional compensation will be allowed therefor.

10-1.60 STRUCTURE APPROACH SLABS (TYPE R)

Structure approach slabs (Type R) shall consist of removing portions of existing pavement including asphalt concrete surfacing and constructing new reinforced concrete approach slabs at structure approaches as shown on the plans and in accordance with these special provisions.

GENERAL

The thickness shown on the plans for structure approach slabs is the minimum thickness. The thickness will vary depending on the thickness of the pavement and base materials removed.

The Contractor shall establish a grade line for new approach slabs by setting stringlines on each side of the proposed approach slab. The stringlines shall start approximately 30 m from the structure and extend approximately 15 m onto the structure. The stringlines shall then be adjusted as necessary to provide a smooth profile grade for the new approach slab. The profile grade will be subject to the approval of the Engineer.

The Contractor shall schedule his operations so that the pavement and base materials removed during a work period shall be replaced, in that same work period, with approach slab concrete that shall be cured for at least 6 hours prior to the time the lane is to be opened to public traffic as designated in "Maintaining Traffic" of these special provisions. In the event the existing pavement and base materials are removed and the Contractor is unable, as determined by the Engineer, to construct, finish and cure the new approach slab by the time the lane is to be opened to public traffic, the excavation shall be filled with a temporary roadway structural section as specified in this section, "Structure Approach Slabs (Type R)."

TEMPORARY ROADWAY STRUCTURAL SECTION

The Contractor shall provide, at the job site, a sufficient standby quantity, as determined by the Engineer, of asphalt concrete and aggregate base for construction of a temporary roadway structural section where existing approaches to structures are being replaced. The temporary structural section shall be maintained, and later removed as a first order of work when the Contractor is able to construct and cure the approach slab within the prescribed time limit. The temporary structural section shall consist of 90-mm thick layer of asphalt concrete over aggregate base.

The aggregate base for the temporary structural section shall conform to the requirements specified under "Aggregate Base (Approach Slab)" in these special provisions.

The asphalt concrete for the temporary structural section shall be produced from commercial quality aggregates and asphalt binder. The grading of the aggregate shall conform to the 19-mm maximum medium grading in Section 39-2.02, "Aggregate," of the Standard Specifications and the asphalt binder shall conform to the requirements of liquid asphalt SC-800 in Section 93, "Liquid Asphalts," of the Standard Specifications. The amount of asphalt binder to be mixed with the aggregate shall be approximately 0.3-percent less than the optimum bitumen content as determined by California Test 367.

Aggregate base and asphalt concrete for the temporary structural section shall be spread and compacted by methods that will produce a well-compacted, uniform base, free from pockets of coarse or fine material and a surfacing of uniform smoothness, texture, and density. The aggregate base may be spread and compacted in one layer and the asphalt concrete may be spread and compacted in one layer. The finished surface of the asphalt concrete shall not vary more than 15 mm from the lower edge of a 3.6-m straightedge placed parallel with the centerline and shall match the elevation of the existing concrete pavement and structure along the joints between the existing pavement and structure and the temporary surfacing.

The material from the removed temporary structural section shall be disposed of outside the highway right of way in conformance with Section 7-1.13 of the Standard Specifications except that removed aggregate base may be stockpiled at the job site and reused for construction of another temporary structural section. When no longer required, standby material or stockpiled material for construction of temporary structural sections shall be removed and disposed of outside the right of way in conformance with said Section 7-1.13.

REMOVING EXISTING PAVEMENT AND BASE MATERIALS

The outlines of excavations in asphalt concrete shall be cut on a neat line to a minimum depth of 75 mm with a power-driven concrete saw or wheel-type rock cutting excavator before any asphalt concrete material is removed. Removing existing base materials shall be in accordance with the provisions in "Earthwork," elsewhere in these special provisions. Such excavations shall be permanently or temporarily backfilled to conform to the grade of adjacent pavement prior to opening the lane to public traffic. Surplus excavated material may be used as temporary backfill material.

Regardless of the type of equipment used to remove concrete within the sawed outline, the surface of the concrete to be removed shall not be impacted within 0.5-m of the pavement to remain in place. Removing existing pavement and base materials shall be performed without damage to the adjacent structure or pavement that is to remain in place. Damage to the structure or to pavement that is to remain in place shall be repaired to a condition satisfactory to the Engineer. Damaged pavement shall be removed and replaced with new concrete pavement if ordered by the Engineer. Repairing damage to structures or repairing or removing and replacing damaged pavement outside the limits of structure approach slabs shall be at the Contractor's expense.

Materials removed shall be disposed of outside the highway right of way in conformance with the provisions in Section 7-1.13 of the Standard Specifications.

The base material remaining in-place, after removing the existing pavement and base materials to the required depth, shall be graded uniformly, watered, and compacted. The finished surface of the base material at any point shall not extend above the grade approved by the Engineer.

AGGREGATE BASE (APPROACH SLAB)

The aggregate base (approach slab) for temporary structural section shall be produced from commercial quality aggregates consisting of broken stone, crushed gravel or natural rough-surfaced gravel, and sand, or any combination thereof. The grading of the aggregate base shall conform to the 19-mm maximum grading specified in Section 26-1.02A, "Class 2 Aggregate Base," of the Standard Specifications.

STRUCTURE APPROACH SLAB

Reinforced concrete approach slabs shall conform to the provisions for approach slabs in Section 51, "Concrete Structures," of the Standard Specifications and these special provisions.

Concrete for use in approach slabs shall contain not less than 400 kg of cement per cubic meter.

Miscellaneous steel parts and all steel components of abutment ties including plates, nuts, washers, and rods shall conform to the provisions in Section 75-1.03, "Miscellaneous Bridge Metal," of the Standard Specifications.

Approach slab concrete that requires a minimum curing period of 6 hours shall be constructed using a non-chloride Type C chemical admixture. Mineral admixture will not be required in this concrete.

Portland cement for use in concrete using a non-chloride Type C chemical admixture shall be Type II Modified, Type II Prestress, or Type III. Type II Modified and Type III cement shall conform to the provisions in Section 90-2.01, "Portland Cement," of the Standard Specifications. Type II Prestress cement shall conform to the requirements of Type II Modified cement, except the mortar containing the portland cement to be used and Ottawa sand, when tested in conformance with California Test 527, shall not contract in air more than 0.053-percent.

The non-chloride Type C chemical admixture shall be approved by the Engineer and conform to the requirements of ASTM Designation: C 494 and Section 90-4, "Admixtures," of the Standard Specifications.

The concrete with non-chloride Type C chemical admixture shall be prequalified prior to placement in conformance with the provisions for prequalification of concrete specified by compressive strength in Section 90-9.01, "General," of the Standard Specifications and the following:

Immediately after fabrication of the 5 test cylinders, the cylinders shall be stored in a temperature medium of 21 ± 1.5°C until the cylinders are tested.

The 6-hour average strength of the 5 test cylinders shall not be less than 5.85 MPa. No more than 2 test cylinders shall have a strength of less than 5.5 MPa.

Building paper shall be commercial quality No. 30 asphalt felt.

Polyvinyl chloride (PVC) conduit used to encase the abutment tie rod shall be of commercial quality.

Bar reinforcement or abutment tie rods in drilled holes shall be bonded in conformance with the provisions for drilling and bonding dowels in Section 83-2.02D(1), "General," of the Standard Specifications.

The top surface of approach slabs shall be finished in conformance with the provisions in Section 51-1.17, "Finishing Bridge Decks," of the Standard Specifications. The finished top surface shall not vary more than 6 mm from the lower edge of a 3.6-m straightedge placed parallel with the centerline. Edges of slabs shall be edger finished.

The surface of the approach slab will not be profiled and the Profile Index requirements shall not apply.

Approach slabs shall be cured with pigmented curing compound (1) in conformance with the provisions for curing structures in Section 90-7.01B, "Curing Compound Method," of the Standard Specifications. The minimum curing period as specified elsewhere in this section-"Structure Approach Slabs (Type R)" shall be considered to begin at the start of discharge of the last truck load of concrete to be used in the slab. Fogging of the surface with water after the curing compound has been applied will not be required. Should the film of curing compound be damaged from any cause before the approach slab

is opened to public traffic, the damaged portion shall be repaired immediately with additional compound, at the Contractor's expense. Any damage to the curing compound after the approach slab is opened to public traffic shall not be repaired.

If the ambient temperature is below 18°C during the curing period, an insulating layer or blanket shall cover the surface. The insulation layer or blanket shall have an R-value rating given in the table below. At the Contractor's option, a heating tent may be used in lieu of or in combination with the insulating layer or blanket:

Temperature range during curing period	R-value, minimum
13°C to 18°C	1
7°C to 13°C	2
4°C to 7°C	3

Tests to determine the coefficient of friction of the final textured surface will be made only if the Engineer determines by visual inspection that the final texturing may not have produced a surface having the specified coefficient of friction. Any tests to determine the coefficient of friction will be made after the approach slab is opened to public traffic, but not later than 5 days after concrete placement. The coefficient of friction will be measured by California Test 342. Any portions of completed concrete surfaces that are found to have a coefficient of friction less than 0.35 shall be ground or grooved parallel to the center line in conformance with the requirements for bridge decks of Section 42, "Groove and Grind Pavement," of the Standard Specifications.

JOINTS

Hardboard and expanded polystyrene shall conform to the provisions in Section 51-1.12D, "Sheet Packing, Preformed Pads and Board Fillers," of the Standard Specifications.

Type AL joint seals shall conform to the provisions in Section 51-1.12F, "Sealed Joints" of the Standard Specifications. The sealant may be mixed by hand-held power-driven agitators and placed by hand methods.

The pourable seal between the steel angle and concrete barrier shall conform to the requirements for Type A and AL seals in Section 51-1.12F(3), "Materials and Installation," of the Standard Specifications. The sealant may be mixed by hand-held power-driven agitators and placed by hand methods. Immediately prior to placing the seal, the joint shall be thoroughly cleaned, including abrasive blast cleaning of the concrete surfaces, so that all foreign material and concrete spillage are removed from all joint surfaces. Joint surfaces shall be dry at the time the seal is placed.

MEASUREMENT AND PAYMENT

Structural concrete, approach slab (Type R) will be measured and paid for in conformance with the provisions in Sections 51-1.22, "Measurement," and 51-1.23, "Payment," of the Standard Specifications and these special provisions.

Full compensation for removing and disposing of portions of existing pavement including asphalt concrete surfacing, miscellaneous metal, type AL joint seals and pourable seals shall be considered as included in the contract price paid per cubic meter for structural concrete, approach slab (Type R) and no separate payment will be made therefor.

Full compensation for furnishing, stockpiling and disposing of standby material for construction of temporary structural sections; and for constructing, maintaining, removing and disposing of temporary structural sections shall be considered as included in the contract price paid per cubic meter for structural concrete, approach slab (Type R) and no separate payment will be made therefor.

Full compensation for drilling and bonding of bar reinforcement or abutment tie rods shall be considered as included in the contract price paid per cubic meter for structural concrete, approach slab (Type R) and no separate payment will be made therefor.

10-1.61 PAVING NOTCH EXTENSION

This work shall consist of extending existing paving notches in conformance with the details shown on the plans and in conformance with these special provisions.

Concrete for the paving notch extensions shall conform to the requirements for structure approach slab concrete of these special provisions.

At least 12 hours shall elapse between the time of placing concrete for the paving notch extension and placing concrete for the structure approach slab.

The construction joint between the paving notch extension and the existing abutment shall conform to the provisions for horizontal construction joints in Section 51-1.13, "Bonding," of the Standard Specifications. Concrete shall be placed in the spalled portions of the existing paving notch concurrently with the concrete for the paving notch extension.

Bar reinforcing steel shall conform to the provisions in Section 52, "Reinforcement," of the Standard Specifications.

Structure excavation and backfill shall conform to the provisions in Section 19-3, "Structure Excavation and Backfill," of the Standard Specifications.

Drilling of holes and bonding of reinforcing steel dowels shall conform to the provisions for drilling and bonding dowels in Section 83-2.02D(1), "General," of the Standard Specifications.

The quantity of concrete for paving notch extension will be measured by the cubic meter. The volume to be paid for will calculated from the dimensions shown on the plans or other dimensions that may be ordered in writing by the Engineer.

The contract price paid per cubic meter for paving notch extension shall include full compensation for furnishing all labor, materials (including concrete for the paving notch spalled areas), tools, equipment, and incidentals, and for doing all the work involved in constructing the paving notch extension, complete in place, including structure excavation and backfill, reinforcement, and drilling and bonding dowels, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.62 SOUND WALL

DESCRIPTION

This work shall consist of constructing sound walls of masonry block. Sound walls shall be supported on concrete barriers as shown on the plans.

The Contractor shall submit 2 sets of elevation and plan layout drawings to the Engineer, as provided in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications. The drawings shall be to scale and shall show the proposed top and bottom elevation lines. The top and bottom elevation lines shown on the plans are minimum and they shall be fully contained in the proposed layout drawings. The drawings shall include, within the limits shown on the plans, the panel sizes, pile spacing, post spacing, footing steps, aesthetic features, locations of expansion joints and access gates. The Contractor shall allow two weeks after complete drawings are submitted for review.

SOUND WALL (MASONRY BLOCK)

Sound wall (masonry block), consisting of a reinforced hollow unit masonry block stem, shall conform to the provisions in Sections 19, "Earthwork," 52, "Reinforcement," and 90, "Portland Cement Concrete," of the Standard Specifications and these special provisions.

Sound wall masonry unit stems shall be constructed with joints of portland cement mortar. Wall stems shall be constructed with hand laid block. Wall stems shall not be constructed with preassembled panels.

Concrete for sound wall footings, pile caps and grade beams, if required, shall conform to the provisions in Section 90-10, "Minor Concrete," of the Standard Specifications.

Reinforcing bars shall conform to the requirements in ASTM Designation: A 706/A 706M.

Concrete masonry units shall be hollow, load bearing, conforming to the requirements in ASTM Designation: C 90, lightweight or medium weight classification, Type II. Standard or open end units may be used. Open end units, if used, shall not reduce the spacing of the bar reinforcement as shown on the plans.

The masonry units shall be nominal size and texture and of uniform color. All masonry units shall be made by the same manufacturer.

Samples of each type of masonry block acceptable for use in this project are available for inspection for the purpose of determining the color, texture and aggregate types. These samples are located at Caltrans District Office, Landscape Architecture Branch, 111 Grand Avenue, Oakland, California, 94623, Room 1137 8-D. Samples inspection hours are from 7:00 a.m. to 5:00 p.m., Monday through Friday except legal holidays.

When high strength concrete masonry units with f'm=17.2 MPa are shown on the plans, the high strength masonry units shall have a minimum compressive strength of 26 MPa based on net area. Each high strength concrete masonry unit shall be identified with a groove embedded in an interior corner. The groove shall extend from a mortar surface for a length of about 50 mm and shall have a depth of about 5 mm.

Expansion joint filler shall conform to the requirements in ASTM Designation: D 1751 or ASTM Designation: D 2000 2AA-805.

Portland cement mortar shall be colored to match the units. Coloring shall be chemically inert, fade resistant mineral oxide or synthetic type.

Portland cement for wall stems shall conform to the provisions in Section 90-2.01, "Portland Cement," of the Standard Specifications.

Hydrated lime shall conform to the requirements in ASTM Designation: C 207, Type S.

Mortar sand shall be commercial quality.

Mortar for laying masonry units shall consist, by volume, of one part portland cement, 0 to 0.5 parts of hydrated lime, and 2.25 to 3 parts of mortar sand. Sufficient water shall be added to make a workable mortar. Each batch of mortar shall be accurately measured and thoroughly mixed. Mortar shall be freshly mixed as required. Mortar shall not be retempered more than one hour after mixing.

Prepackaged mortar materials and mortar containing admixtures may be used when approved in writing by the Engineer, provided the mortar shall not contain more than 0.05 percent soluble chlorides in conformance with California Test 422 or 0.25 percent soluble sulfates, as SO_4, in conformance with California Test 417.

Prior to laying masonry units using prepackaged mortar materials or mortar containing admixtures, the Contractor shall submit to the Engineer the proposed sources of the materials together with test data from an independent testing laboratory for mortar tested in conformance with California Test 551. The test data shall be from specimens having a moist cure, except, the sample shall not be immersed in lime water. The average 28-day compressive strength of the mortar shall be not less than 17.2 MPa.

At the option of the Contractor, grout for filling masonry units may be proportioned either by volume or mass. Grout shall contain only enough water to cause it to flow and fill the voids without segregation. The maximum amount of free

water shall not exceed 0.7 times the weight of the cement for regular strength masonry. The maximum amount of free water shall not exceed 0.6 times the mass of the cement for high strength masonry.

Grout proportioned by volume for regular strength masonry shall consist of at least one part portland cement and 4.5 parts aggregate. Grout proportioned by volume for high strength masonry shall consist of at least one part portland cement and 3.5 parts aggregate. Aggregate volumes shall be based on a loose, air-dry condition.

Grout proportioned by mass for regular strength masonry shall contain at least 325 kilograms of portland cement per cubic meter. Grout proportioned by mass for high strength masonry shall contain at least 400 kilograms of portland cement per cubic meter.

Construction of reinforced concrete masonry unit wall stems with portland cement mortar joints shall conform to the following:

Concrete masonry unit construction shall be true and plumb in the lateral direction and shall conform to the grade shown on the plans in the longitudinal direction. Bond beam units or recesses for horizontal reinforcement shall be provided.

Mortar joints shall be approximately 10 mm wide. Walls and cross webs forming cells to be filled with grout shall be full bedded in mortar to prevent leakage of grout. All head and bed joints shall be solidly filled with mortar for a distance in from the face of the wall or unit not less than the thickness of the longitudinal face shells. Head joints shall be shoved tight.

Mortared joints around cells to be filled shall be placed so as to preserve the unobstructed vertical continuity of the grout filling. Any overhanging mortar or other obstruction or debris shall be removed from the inside of such cells.

Reinforcement shall be securely held in position at top and bottom with either wire ties or spacing devices and at intervals not exceeding 192 bar diameters prior to placing any grout. Wire shall be 16-gage or heavier. Wooden, aluminum, or plastic spacing devices shall not be used.

Splices in vertical reinforcement will be allowed only where shown on the plans.

Only those cells containing reinforcement shall be filled solidly with grout. All grout in the cells shall be consolidated at the time of placement by vibrating, and reconsolidated after excess moisture has been absorbed, but before plasticity is lost. Slicing with a trowel is not acceptable.

Walls shall be constructed in 1.2-m maximum height lifts. Grouting of each lift shall be completed before beginning masonry unit construction for the next lift. The top course of each lift shall consist of a bond beam.

A construction joint is required at the top of the top course to permit placement of the mortar cap. The mix design for the mortar cap shall be as approved by the Engineer.

Construction joints shall be made when the placing of grout, in grout filled cells, is stopped for more than one hour. The construction joint shall be 12 mm below the top of the last course filled with grout.

Bond beams shall be continuous. The top of unfilled cells under horizontal bond beams shall be covered with metal or plastic lath.

When fresh masonry joins masonry that is partially or totally set, the contact surface shall be cleaned, roughened and lightly wetted.

Surfaces of the concrete on which the masonry walls are to be placed shall be roughened and cleaned, exposing the stone aggregate, and shall be flushed with water and allowed to dry to a surface dry condition immediately prior to laying the masonry units.

Where masonry unit cutting is necessary, all cuts shall be made with a masonry saw to neat and true lines. Masonry units with cracking or chipping of the finished exposed surfaces will not be acceptable.

Masonry shall be protected as specified for concrete structures in Section 90-8, "Protecting Concrete," of the Standard Specifications and these special provisions.

During erection, all cells shall be kept dry in inclement weather by covering partially completed walls. The covering shall be waterproof fabric, plastic or paper sheeting, or other approved material. Wooden boards and planks are not acceptable as covering materials. The covering shall extend down each side of masonry walls approximately 0.6-m.

Splashes, stains or spots on the exposed faces of the wall shall be removed.

MEASUREMENT AND PAYMENT.--Sound walls (barrier), of the types designated in the Engineer's Estimate, will be measured by the square meter of walls supported on barriers from the top of the barrier to the upper elevation line and length of wall.

The contract prices paid per square meter for sound wall (barrier) shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in constructing the sound wall complete in place, including all supports (except retaining walls, barriers and barrier supports), anchorages, excavation, backfill, reinforcement and grade beams, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer. Retaining walls, concrete barrier and barrier supports supporting sound walls (barrier) will be measured and paid for as separate items of work.

10-1.63 DRILL AND BOND DOWELS

Drilling and bonding dowels shall conform to the details shown on the plans, the provisions in Section 83-2.02D(1), "General," of the Standard Specifications and these special provisions.

Dowels shall conform to the provisions for bar reinforcement in "Reinforcement" elsewhere in these special provisions.

If reinforcement is encountered during drilling before the specified depth is attained, the Engineer shall be notified. Unless the Engineer approves coring through the reinforcement, the hole will be rejected and a new hole, in which reinforcement is not encountered, shall be drilled adjacent to the rejected hole to the depth shown on the plans.

Coring will not be allowed at Mount Eden Overhead, Bridge No. 33-0092 R/L.

Unless otherwise provided, dowels to be bonded into drilled holes will be paid for as bar reinforcing steel (bridge).

Unless otherwise provided, drilling and bonding dowels will be measured and paid for by the meter determined by the number and the required depth of holes as shown on the plans, or as ordered by the Engineer.

The contract price paid per meter for drill and bond dowel shall include full compensation for furnishing all labor, materials (except reinforcing steel dowels), tools, equipment, and incidentals, and for doing all the work involved in drilling the holes, including coring through reinforcement when approved by the Engineer, and bonding the dowels, complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.64 CORE CONCRETE

Coring concrete shall consist of coring holes through reinforced concrete bridge members as shown on the plans and in conformance with the requirements in these special provisions.

For cored holes greater than 3000 mm in length, the following shall apply:

Prior to coring, the Contractor shall submit, in accordance with Section 5-10.2, "Plans and Working Drawings," of the Standard Specifications, the methods and equipment to be used in the coring operations.

The deviation in alignment of cored holes from that shown on the plans shall not be more than 13 mm per 3000 mm of cored hole length with a maximum deviation of not more than 75 mm.

Immediately after coring, the concrete cores shall be identified by the Contractor with a description of the core locations and submitted to the Engineer for inspection. When reinforcement is cut, coring operations shall be terminated, and the Contractor shall submit to the Engineer for approval, the procedure proposed to repair the cut reinforcement and to prevent further cutting of reinforcement.

The holes shall be cored by methods that will not shatter or damage the concrete adjacent to the holes.

Water for core drilling operations shall be from the local domestic water supply or shall not contain more than 1000 parts per million of chlorides as Cl, nor more than 1300 parts per million of sulfates as SO_4, nor shall it contain any impurities in a sufficient amount to cause discoloration of the concrete or produce etching of the surface.

Water from core drilling operations shall not be permitted to fall on public traffic, to flow across shoulders or lanes occupied by public traffic, or to flow into gutters or other drainage facilities.

Coring concrete will be measured and paid for by the meter as core concrete of the sizes listed in the Engineer's Estimate. The cored concrete will be measured along the centerline of the hole without deduction for expansion joints.

The contract price paid per meter for core concrete of the sizes listed in the Engineer's Estimate shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all work involved in coring the holes, including control of water from core drilling and repairing any damaged reinforcement, as shown on the plans, as specified in these special provisions, and as directed by the Engineer.

10-1.65 SEALING JOINTS

Joints in concrete bridge decks and joints between concrete structures and concrete approach slabs shall be sealed in conformance with the details shown on the plans, the provisions in Section 51, "Concrete Structures," of the Standard Specifications and these special provisions.

Where polyurethane seals are shown on the plans, silicone sealant conforming to these specifications may be used.

The second through fourth paragraphs of Section 51-1.12F, "Sealed Joints," of the Standard Specifications are amended to read:

Type A and AL joint seals shall consist of a groove in the concrete which is filled with field mixed and placed polyurethane or silicone sealant.

Type B joint seals shall consist of a groove in the concrete which is filled with a preformed elastomeric joint seal.

Joint seal assemblies shall consist of metal or metal and elastomeric assemblies which are anchored or cast into a recess in the concrete over the joint.

The type of seal to be used for the Movement Rating (MR) shown on the plans shall be as follows:

MR	Seal Type
15 mm	Type A or Type B
30 mm	Type A (silicone only) or Type B
> 30 mm and ≤ 50 mm	Type B
> 50 mm	joint seal assembly

The first and second paragraphs of Section 51-1.12F(3), "Materials and Installation," of the Standard Specifications are amended to read:

(a) Type A and AL Seal.— The sealant shall consist of a 2 component polyurethane sealant, which will withstand up to ±25 percent movement, or a 2 component silicone sealant, which will withstand up to ±50 percent movement.

Polyurethane and silicone sealants shall be tested in accordance with California Test 435. The sealants shall conform to State Specification 8030-61J-01 and the following requirements:

SPECIFICATION	REQUIREMENT
Modulus at 150 percent elongation	35-520 kPa
Width of sealant after 7 days extension and one hour recovery	17 mm, max.
Condition 24 hours after notching	Notched or loss of bond 6 mm, max.
Condition of water immersed specimen at 7 days	Notched or loss of bond 6 mm, max.
Condition of specimen when tested in accordance with ASTM Designation: G 53 using FS 40 UV-B bulbs for a minimum of 25 cycles. The cycle shall be 4 hours UV exposure at 60°C and 4 hours condensate exposure at 40°C	No more than slight checking or cracking.
Grease cone penetration	4.5 - 12.0 mm.

State Specifications for polyurethane and silicone sealants may be obtained from the Transportation Laboratory.

Section 51-1.12F(3)(a), "Type A and AL Seal," of the Standard Specifications is amended by adding the following paragraphs after paragraph 8:

A Certificate of Compliance, accompanied by a certified test report, shall be furnished for each batch of polyurethane and silicone sealant in conformance with the provisions in Section 6-1.07, "Certificates of Compliance."

Samples of the two components, not less than one liter each, from each batch of sealant shall be submitted to the Transportation Laboratory. In addition, samples of manufacturer required primers, not less than one liter each, shall be submitted. The samples shall be furnished for testing, with the Certificate of Compliance, 30 days in advance of proposed use.

When ordered by the Engineer, a joint seal larger than called for by the Movement Rating shown on the plans shall be furnished and installed. Payment to the Contractor for furnishing the larger seal and for saw cutting the increment of additional depth of groove required will be determined as provided in Section 4-1.03, "Changes," of the Standard Specifications.

The fifth subparagraph of the second paragraph of Section 51-1.12F(3) (b), "Type B Seal," of the Standard Specifications is amended to read:

The seal shall be furnished full length for each joint with no more than one shop splice in any 18-m length of seal.

One field splice per joint may be made at locations and by methods approved by the Engineer. The seals are to be manufactured full length for the intended joint, then cut at the approved splice section and rematched before splicing. The Contractor shall submit splicing details, prepared by the joint seal manufacturer, to the Engineer for approval prior to beginning splicing work.

The Contractor shall demonstrate the adequacy of the procedures to be used in the work before installing seals in the joints.

Shop splices and field splices shall have no visible offset of exterior surfaces, and shall show no evidence of bond failure.

Saw cutting of grooves will not be required at existing joints that are to be sealed with Type A joint seal unless ordered by the Engineer. The Contractor shall make saw cuts as ordered by the Engineer and the saw cutting will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications.

10-1.66 DRILL AND PRESSURE GROUT DOWELS

Drilling and pressure grouting dowels into concrete shall consist of drilling holes through concrete members, placing dowels, and filling holes with hydraulic-cement grout, as shown on the plans and in conformance with the requirements in these special provisions.

Dowels to be placed in the drilled holes shall conform to the provisions for bar reinforcement in "Reinforcement" elsewhere in these special provisions.

Unless otherwise specified, dowels to be pressure grouted into drilled holes will be paid for as bar reinforcing steel (bridge).

If reinforcement is encountered during drilling before the specified depth is attained, the Engineer shall be notified. Unless the Engineer approves coring through the reinforcement, the hole will be rejected and a new hole, in which reinforcement is not encountered, shall be drilled adjacent to the rejected hole to the depth shown on the plans.

Concrete areas and steel surfaces to be in contact with the grout shall be cleaned of all loose or foreign material that would in any way prevent bonding and shall be flushed with water and allowed to dry to a surface dry condition immediately prior to grouting.

Grout shall be a non-shrink grout, premixed package blend of portland cement, graded silica sand and water reducing, plasticizing and time release expansion agents, which conforms to the requirements of ASTM Designation: C 1107 and provides a minimum 35 MPa compressive strength at 28 days when tested by California Test 551. The grout shall be mixed in accordance with the manufacturer's recomendations. Water shall comply with the provisions in Section 90-2.03, "Water".

Admixtures shall not contain more than 500 parts per million of chlorides as Cl, when tested by California Test 422, and shall not contain more than 2500 parts per million of sulfates as SO_4, when tested by California Test 417.

After dowel placement, ends of the drilled hole containing the dowel shall be sealed, with one vent tube and an injection feed tube. The tubes shall be placed in the hole in a manner which will allow the air to vent and the hole to be completely filled with grout. Sufficient pressure shall be achieved to ensure that the hole is free of voids. Grout shall be pumped through the holes and continually wasted until no visible slugs or other visible evidence of water or air are ejected.

Grout and water shall not be permitted to flow into any waterway, on to public traffic, across shoulders or lanes occupied by public traffic, or into gutters or other drainage facilities.

Drilling and pressure grouting dowels into concrete will be measured and paid for by the linear meter as drill and pressure grout. The drilled hole through concrete will be measured along the centerline of the hole without deduction for expansion joints.

The contract price paid per linear meter for drill and pressure grout dowel shall include full compensation for furnishing all labor, materials (including grout), tools, equipment, and incidentals, and for doing all work involved in drilling the holes, placing dowels in the holes, and pressure grouting the holes, as shown on the plans, as specified in these special provisions, and as directed by the Engineer.

10-1.67 REFINISHING BRIDGE DECKS

Surfaces of bridge decks that are exposed when existing railings, curbs, or sidewalks are removed shall be prepared and refinished flush with the adjoining deck surface with portland cement concrete or rapid setting concrete, at the option of the Contractor, in conformance with the requirements in these special provisions.

The exact area to be refinished will be designated by the Engineer.

Attention is directed to "Public Safety" of these special provisions.

When work is being performed within 3 m of a traffic lane or performed over traffic, dust and residue from deck preparation and cleaning shall be removed or controlled by vacuum, water spray, or shield methods approved by the Engineer.

Concrete shall be removed without damage to concrete that is to remain in place. Damage to concrete which is to remain in place shall be repaired to a condition satisfactory to the Engineer.

The concrete in deck areas to be refinished shall be removed to a depth of approximately 20 mm below the adjoining deck surface. A 20 mm deep saw cut shall be made along the perimeter of areas prior to removing concrete.

Existing areas of the deck more than 20 mm below the adjoining deck surface shall be prepared by removing not less than 6 mm of surface material to expose sound aggregates.

Concrete removal may be done by abrasive blast cutting, abrasive sawing, impact tool cutting, machine rotary abrading, or other methods, all to be approved by the Engineer. Cut areas shall be cleaned free of dust and all other loose and deleterious materials by brooming, abrasive blast cleaning and high pressure air jets. Equipment shall be fitted with suitable traps, filters, drip pans or other devices to prevent oil or other deleterious matter from being deposited on the deck.

Existing reinforcement, exposed during the removal of concrete, that is to remain in place shall be protected from damage.

Steel dowels shall be cut off flush with the existing concrete or cut off at the bottom of concrete removal, whichever is lower. Patching around or over dowels in sound concrete will not be required. Existing voids around dowels, where

refinishing is not required, shall be chipped back to sound concrete, the dowels removed 25 mm below the finished surface and the hole filled with rapid setting concrete.

Refinishing isolated high areas in the existing deck may be accomplished by cutting the concrete down to be flush with the plane of the adjoining deck surface by abrasive sawing, grinding, impact tool cutting, or other methods subject to the approval of the Engineer. When grinding is performed to bring the deck concrete flush with the adjoining deck surface, the resulting surface shall have a coefficient of friction of not less than 0.35 as determined by California Test 342.

PORTLAND CEMENT CONCRETE

An epoxy adhesive shall be applied to the surfaces to be refinished before placing the portland cement concrete. Immediately prior to applying the adhesive, the area to receive the adhesive shall be cleaned by abrasive blasting and blown clean by compressed air to remove dust and any other loose material. The area to be covered shall be surface dry and the ambient temperature shall be 10°C or above when the adhesive is applied.

The epoxy adhesive shall be furnished and applied in conformance with the provisions in Sections 95-1, "General," and 95-2.03, "Epoxy Resin Adhesive for Bonding New Concrete to Old Concrete," of the Standard Specifications. Whenever the ambient temperature is below 18°C, Type II epoxy shall be used. The exact rate of applying epoxy adhesive shall be as directed by the Engineer. The adhesive shall be worked onto the surface with stiff brushes or equal.

Portland cement concrete used to fill prepared areas shall conform to the provisions in Section 90, "Portland Cement Concrete," of the Standard Specifications and the following:

The concrete shall contain a minimum of 400 kilograms of portland cement per cubic meter.

The amount of free water used in concrete shall not exceed 166 kg/m^3.

The aggregate shall contain between 50 and 55 percent fine aggregate and the remainder shall be pea gravel. The grading of pea gravel shall be such that 100 percent passes the 12.5 mm screen and not more than 5 percent passes the 1.18 mm sieve, unless a larger size is ordered by the Engineer.

Admixtures shall be furnished and used if directed by the Engineer.

Immediately after depositing on the newly placed adhesive, the portland cement concrete shall be thoroughly consolidated until all voids are filled and free mortar appears on the surface and then struck off to the required grade.

Concrete shall be cured as provided in Section 90-7.03, "Curing Structures," of the Standard Specifications.

No loads of any kind shall be applied to the portland cement concrete for at least 7 days after placing, unless otherwise permitted by the Engineer.

RAPID SETTING CONCRETE

The concrete used to fill the prepared areas shall be a high-strength material consisting of either magnesium phosphate concrete, modified high alumina based concrete, or portland cement based concrete. Magnesium phosphate concrete shall conform to the requirements for magnesium phosphate concrete in Section 83-2.02D(1), "General," of the Standard Specifications and these special provisions. Modified high alumina based concrete and portland cement based concrete shall be water activated and shall conform to the requirements for single component (water activated) magnesium phosphate concrete in Section 83-2.02D(1), "General," of the Standard Specifications and the following:

A clean uniform rounded aggregate filler may be used to extend the concrete. The moisture content of the aggregate shall not exceed 0.5 percent. Grading of the aggregate shall conform to the following:

Sieve Size	Percentage Passing
12.5 mm	100
1.18 mm	0-5

The amount of aggregate filler shall conform to the manufacturer's recommendation, but in no case shall the concrete strengths be less than that specified for magnesium phosphate concrete in Section 83-2.02D(1), "General," of the Standard Specifications.

Mixing of components of dual component (with a prepackaged liquid activator) magnesium phosphate shall be by complete units, supplied by the manufacturer. Portions of units shall not be used. Water shall not be added to dual component magnesium phosphate.

Immediately prior to applying the rapid setting concrete, the surface shall be dry and blown clean by compressed air to remove accumulated dust and any other loose material. If the surface becomes contaminated at any time prior to placing the concrete, the surface shall be cleaned by abrasive blasting. The surface temperature of the areas to be covered shall be 4°C or above when the concrete is applied. Methods proposed to heat said surfaces are subject to approval by the Engineer. The surface for the magnesium phosphate concrete shall be dry. The surfaces for modified high alumina based concrete or portland cement based concrete may be damp but not saturated.

Magnesium phosphate concrete shall not be mixed in containers or worked with tools containing zinc, cadmium, aluminum or copper. Modified high alumina based concrete shall not be mixed in containers or worked with tools containing aluminum.

Retempering concrete will not be permitted. Finishing tools that are cleaned with water shall be thoroughly dried before working the concrete.

When placing concrete on slopes exceeding 5 percent, the Engineer may require the Contractor to provide a flow controlled modified material.

Modified high alumina based concrete and portland cement based concrete shall be cured in conformance with the provisions in Section 90-7.01B, "Curing Compound Method," of the Standard Specifications. Magnesium phosphate concrete shall not be cured.

Unless otherwise permitted in writing by the Engineer, traffic shall not be permitted on the new concrete until at least 24 hours after final set.

FINISHING REQUIREMENTS

In advance of curing operations, the surface of the concrete shall be textured by brooming with a stiff bristled broom or by other suitable devices which will result in uniform scoring. Brooming shall be performed transversely. The operation shall be performed at a time and in a manner to produce a hardened surface having a uniform texture and a coefficient of friction of not less than 0.35 as determined by California Test 342.

Refinished surfaces that are found to have a coefficient of friction less than 0.35 shall be ground or grooved by the Contractor at his expense in conformance with the applicable requirements in Section 42, "Groove and Grind Pavement," of the Standard Specifications.

In the longitudinal direction, refinished surfaces shall not vary more than 6 mm from the lower edge of a 3.6 m straightedge. The refinished surface shall be flush with the existing adjoining surface.

MEASUREMENT AND PAYMENT

No adjustment of compensation will be made for any increase or decrease in the quantity of refinish bridge deck, regardless of the reason for such increase or decrease. The provisions in Section 4-1.03B, "Increased or Decreased Quantities," of the Standard Specifications shall not apply to the contract item of refinish bridge deck.

The quantity in square meters of refinish bridge deck to be paid for will be determined from the lengths and widths of the refinished areas, measured horizontally, plus 0.02-m^2 for patching around each dowel.

The contract price paid per square meter for refinish bridge deck shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in refinishing areas of the existing bridge deck (including cutting steel dowels), complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.68 REINFORCEMENT

Reinforcement shall conform to the provisions in Section 52, "Reinforcement," of the Standard Specifications and these special provisions.

The first paragraph of Section 52-1.02A, "Bar Reinforcement," of the Standard Specifications is amended to read:

52-1.02A Bar Reinforcement.—Reinforcing bars shall be low-alloy steel deformed bars conforming to the requirements in ASTM Designation: A 706/A 706M, except that deformed or plain billet-steel bars conforming to the requirements in ASTM Designation: A 615/A 615M, Grade 300 or 420, may be used as reinforcement in the following 5 categories:

1. Slope and channel paving;
2. Minor structures;
3. Sign and signal foundations (pile and spread footing types);
4. Roadside rest facilities; and
5. Concrete barrier Type 50 and Type 60 series and temporary railing.

Deformations specified in ASTM Designation: A 706/A 706M will not be required on bars used as spiral or hoop reinforcement in structures and concrete piles.

Section 52-1.02C, "Welded Wire Fabric," of the Standard Specifications is amended to read:

52-1.02C Welded Wire Fabric.—Welded wire fabric shall be either plain or deformed conforming to the requirements in ASTM Designation: A 185 or ASTM Designation: A 497, respectively.

The last paragraph of Section 52-1.07, "Placing," of the Standard Specifications is amended to read:

Whenever a portion of an assemblage of bar reinforcing steel that is not encased in concrete exceeds 6 m in height, the Contractor shall submit to the Engineer for approval, in accordance with the provisions in Section 5-1.02, "Plans and Working Drawings," working drawings and design calculations for the temporary support system to be used. The working drawings and design calculations shall be signed by an engineer who is registered as a Civil Engineer in the State of California. The temporary support system shall be designed to resist all expected loads and shall be adequate to prevent collapse or overturning of the assemblage. If the installation of forms or other work requires revisions to or

temporary release of any portion of the temporary support system, the working drawings shall show the support system to be used during each phase of construction. The minimum horizontal wind load to be applied to the bar reinforcing steel assemblage, or to a combined assemblage of reinforcing steel and forms, shall be not less than 960 Pa on the gross projected area of the assemblage.

The first paragraph of Section 52-1.08, "Splicing," of the Standard Specifications is amended to read:

52-1.08 Splicing.—Splicing of reinforcing bars shall be by lapping, butt welding, mechanical butt splicing, or mechanical lap splicing, at the option of the Contractor. Reinforcing bars Nos. 43 through 57 shall not be spliced by lapping.

The length of any type of ultimate mechanical butt splice shall not exceed 10 times the bar diameter of the bar to be spliced.

The sixth paragraph of Section 52-1.08, "Splicing," of the Standard Specifications is amended to read:

Except when otherwise specified, mechanical lap splicing shall conform to the details shown on the plans, the requirements for mechanical butt splices as specified in this Section 52-1.08, and Sections 52-1.08C, "Mechanical Butt Splices," 52-1.08D, "Qualification of Welding and Mechanical Splicing," and 52-1.08E, "Job Control Tests," and the following:

The mechanical lap splice shall be a unit consisting of a sleeve, in which the reinforcing bars are positioned, and a wedge driven through holes in the sleeve and between the reinforcing bars. The mechanical lap splice shall only be used for splicing non-epoxy-coated deformed reinforcing bars Nos. 13, 16 and 19.

The eighth and ninth paragraphs of Section 52-1.08, "Splicing," of the Standard Specifications are amended to read:

Unless otherwise shown on the plans or approved by the Engineer, splices in adjacent reinforcing bars at any particular section shall be staggered. The minimum distance between staggered lap splices or mechanical lap splices shall be the same length required for a lapped splice in the largest bar. The minimum distance between staggered butt splices shall be 600 mm. Distances shall be measured between the midpoints of the splices along a line which is centered between the axes of the adjacent bars.

Completed butt splices shall develop a minimum tensile strength, based on the nominal bar area, of 430 MPa for ASTM Designation: A 615/A 615M, Grade 300 bars, and 550 MPa for ASTM Designation: A 615/A 615M, Grade 420 and ASTM Designation: A 706/A 706M bars. If butt splices are made between 2 bars of dissimilar strengths, the minimum required tensile strength for the splice shall be that required for the weaker bar.

The second sentence of the eleventh paragraph of Section 52-1.08, "Splicing," of the Standard Specifications is amended to read:

Job control tests shall be made on sample splices representing each lot of mechanical butt splices as provided in Section 52-1.08E, "Job Control Tests."

The third and fourth paragraphs of Section 52-1.08A, "Lapped Splices," of the Standard Specifications are amended to read:

Where ASTM Designations: A 615/A 615M, Grade 420 or A 706/A 706M reinforcing bars are required, the length of lapped splices shall be as follows: Reinforcing bars No. 25, or smaller, shall be lapped at least 45 diameters of the smaller bar joined, and reinforcing bars Nos. 29, 32 and 36 shall be lapped at least 60 diameters of the smaller bar joined, except when otherwise shown on the plans.

Where ASTM Designation: A 615/A 615M, Grade 300 reinforcing bars are permitted, the length of lapped splices shall be as follows: Reinforcing bars No. 25, or smaller, shall be lapped at least 30 diameters of the smaller bar joined, and reinforcing bars Nos. 29, 32 and 36 shall be lapped at least 45 diameters of the smaller bar joined, except when otherwise shown on the plans.

Section 52-1.08B, "Butt Welded Splices," of the Standard Specifications is amended to read:

52-1.08B Butt Welded Splices.—Butt welded splices in reinforcing bars shall be complete joint penetration butt welds conforming to the requirements in AWS D1.4, and the requirements of these specifications and the special provisions.

At the option of the Contractor, shop produced resistance butt welds, that are produced by a fabricator who is approved by the Transportation Laboratory, may be used. These welds shall conform to the requirements of these specifications and the special provisions.

Only the joint details and dimensions as shown in Figure 3.2, "Direct Butt Joints," of AWS D 1.4-92, shall be used for making complete joint penetration butt welds of bar reinforcement. Split pipe backing shall not be used.

Material used as backing for complete joint penetration butt welds of bar reinforcement shall be a flat plate conforming to the requirements in ASTM Designation: A 709/A 709M, Grade 36[250]. The flat plate shall be 6 mm thick with a width, as measured perpendicular to the axis of the bar, equal to the nominal diameter of the bar, and a length which does not exceed twice the nominal diameter of the bar. The flat plate backing shall be fitted tightly to the bar with the root of the weld centered on the plate. Any bar deformation or obstruction preventing a tight fit shall be ground smooth and flush with the adjacent surface. Tack welds used to fit backing plates shall be within the weld root area so that they are completely consumed by the finished weld. Backing plates shall not be removed.

Butt welds shall be made with multiple weld passes using a stringer bead without an appreciable weaving motion. The maximum stringer bead width shall be 2.5 times the diameter of the electrode and slagging shall be performed between each weld pass. Weld reinforcement shall not exceed 4 mm in convexity.

Before any electrodes or flux-electrode combinations are used, the Contractor, at the Contractor's expense, shall furnish certified copies of test reports for all the pertinent tests specified in AWS A5.1, AWS A5.5, AWS A5.18 or AWS A5.20, whichever is applicable, made on electrodes or flux-electrode combinations of the same class, brand and nearest specified size as the electrodes to be used. The tests may have been made for process qualification or quality control, and shall have been made within one year prior to manufacture of the electrodes and fluxes to be used. The report shall include the manufacturer's certification that the process and material requirements were the same for manufacturing the tested electrodes and the electrodes to be used. The forms and certificates shall be as directed by the Engineer.

Electrodes for manual shielded metal arc welding of ASTM Designation: A 615/A 615M, Grade 420 bars shall conform to the requirements in AWS A5.5 for E9018-M or E10018-M electrodes.

Electrodes for manual shielded metal arc welding of ASTM Designation: A 706/A 706M bars shall conform to the requirements of AWS A5.5 for E8016-C3 or E8018-C3 electrodes.

Solid and composite electrodes for semiautomatic gas metal-arc and flux-cored arc welding of Grade 300 reinforcing bars shall conform to the requirements of AWS A5.18 for ER70S-2, ER70S-3, ER70S-6 or ER70S-7 electrodes; or AWS A5.20 for E70T-1, E70T-5, E70T-6 or E70T-8 electrodes.

Electrodes for semiautomatic welding of ASTM Designation: A 615/A 615M, Grade 420 and ASTM Designation: A 706/A 706M bars shall produce a weld metal deposit with properties conforming to the requirements of Section 5.3.4 of AWS D1.1-96 for ER80S-Ni1, ER80S-Ni2, ER80S-Ni3, ER80S-D2, E90T1-K2 and E91T1-K2 electrodes.

Reinforcing bars shall be preheated for a distance of not less than 150 mm on each side of the joint prior to welding.

For all welding of ASTM Designation: A 615/A 615M, Grade 300 or Grade 420 bars, the requirements of Table 5.2, "Minimum Preheat and Interpass Temperatures," of AWS D1.4-92 are superseded by the following:

> The minimum preheat and interpass temperatures shall be 200°C for Grade 300 bars and 300°C for Grade 420 bars. Immediately after completing the welding, at least 150 mm of the bar on each side of the splice shall be covered by an insulated wrapping to control the rate of cooling. The insulated wrapping shall remain in place until the bar has cooled below 90°C.

When welding different grades of reinforcing bars, the electrode shall conform to Grade 300 bar requirements and the preheat shall conform to the Grade 420 bar requirements.

In the event that any of the specified preheat, interpass and post weld cooling temperatures are not met, all weld and heat affected zone metal shall be removed and the splice rewelded.

Welding shall be protected from air currents, drafts, and precipitation to prevent loss of heat or loss of arc shielding. The method of protecting the welding area from loss of heat or loss of arc shielding shall be subject to approval by the Engineer.

Reinforcing bars shall not be direct butt spliced by thermite welding.

The first paragraph of Section 52-1.08C, "Mechanical Butt Splices," of the Standard Specifications is amended to read:

52-1.08C Mechanical Butt Splices.—Mechanical butt splices shall be the sleeve-filler metal type, the sleeve-threaded type, the sleeve-swaged type, the sleeve-filler grout type, the sleeve-lockshear bolt type, the two-part sleeve-forged bar type, or the two-part sleeve-friction bar type, at the option of the Contractor.

The third paragraph of Section 52-1.08C, "Mechanical Butt Splices," of the Standard Specifications is amended to read:

The total slip of the reinforcing bars within the splice sleeve after loading in tension to 200 MPa and relaxing to 20 MPa shall not exceed the following, measured between gage points clear of the splice sleeve: 250 µm for reinforcing bars No. 43, or smaller, or 750 µm for reinforcing bars No. 57.

The following is added after the third paragraph of Section 52-1.08C, "Mechanical Butt Splices," of the Standard Specifications:

Slip requirements shall not apply to mechanical lap splices.

The fourth subparagraph of the last paragraph of Section 52-1.08C, "Mechanical Butt Splices," of the Standard Specifications is amended to read:

4. A statement that the splicing systems and materials used in accordance with the manufacturer's procedures will develop not less than the minimum tensile strengths, based on the nominal bar area, of 430 MPa for ASTM Designation: A 615/A 615M, Grade 300 bars and 550 MPa for ASTM Designations: A 615/A 615M, Grade 420 and A 706/A 706M bars, and will comply with the total slip requirements and the other requirements in these specifications.

Section 52-1.08C(5), "Sleeve-Extruded Mechanical Butt Splices," of the Standard Specifications is amended to read:

52-1.08C(5) Sleeve-Lockshear Bolt Mechanical Butt Splices.—The sleeve-lockshear bolt type of mechanical butt splices shall consist of a seamless steel sleeve, 2 serrated steel strips welded to the inside of the sleeve, center hole with centering pin, and bolts that are tightened until the bolt heads shear off and the bolt ends are embedded in the reinforcing bars.

52-1.08C(6) Two-Part Sleeve-Forged Bar Mechanical Butt Splices.—The two-part sleeve-forged bar type of mechanical butt splices shall consist of a shop machined two-part threaded steel sleeve that interlocks 2 hot-forged reinforcing bars ends. The forged bar ends may be either shop produced or field produced.

52-1.08C(7) Two-Part Sleeve-Friction Bar Mechanical Butt Splices.—The two-part sleeve-friction bar type of mechanical butt splices shall consist of a shop machined two-part threaded steel sleeve whose ends are friction welded, in the shop, to the reinforcing bars ends.

The fourth paragraph of Section 52-1.08D, "Qualification of Welding and Mechanical Splicing," of the Standard Specifications is amended to read:

Each operator qualification test for mechanical splices shall consist of 2 sample splices. Each mechanical splice procedure test shall consist of 2 sample splices.

For sleeve-filler, sleeve-threaded, sleeve-lockshear bolt and two-part sleeve friction bar mechanical butt splices, all sample splices shall be made on the largest reinforcing bar size to be spliced by the procedure or operator being tested except that No. 43 bars may be substituted for No. 57 bars.

For sleeve-swaged and two-part sleeve-forged mechanical butt splices, and mechanical lap splices, all sample splices shall be made on the largest reinforcing bar size of each deformation pattern to be spliced by the procedure or operator being tested. When joining new reinforcing bars to existing reinforcement, the qualification test sample bars shall be made using only the deformation patterns of the new reinforcement to be joined.

Section 52-1.08E, "Job Control Tests," of the Standard Specifications is amended to read:

52-1.08E Job Control Tests.—When mechanical butt splices, shop produced complete joint penetration butt welded splices, or shop produced resistance butt welded splices are used, the Contractor shall furnish job control tests from a local qualified testing laboratory. A job control test shall consist of the fabrication, under conditions used to produce the splice, and the physical testing of 3 sample splices for each lot of 150 splices.

A lot of mechanical butt splices is defined as 150, or fraction thereof, of the same type of mechanical butt splices used for each combination of bar size and bar deformation pattern that is used in the work.

A lot of shop produced complete joint penetration butt welded splices, or shop produced resistance butt welded splices, is defined as 150, or fraction thereof, of the same type of welds used for each combination of bar size and bar deformation pattern that is used in the work.

When joining new reinforcing bars to existing reinforcement, the job control test shall be made using only the deformation patterns of the new reinforcement to be joined.

A sample splice shall consist of a splice made at the job site to connect two 760 mm, or longer, bars using the same splice materials, position, location, and equipment, and following the same procedures as are being used to make splices in the work. Shorter sample splice bars may be used if approved by the Engineer.

Sample splices shall be made and tested in the presence of the Engineer or the Engineer's authorized representative.

Sample splices shall be suitably identified with weatherproof markings prior to shipment to the testing laboratory.

For sleeve-threaded mechanical butt splices, the reinforcing bars to be used for job control tests shall be fabricated on a random basis during the cutting of threads on the reinforcing bars of each lot and shipped to the job site with the material they represent.

For shop produced complete joint penetration butt welds, shop produced resistance butt welded splices and all types of mechanical butt splices, except the sleeve-threaded type, the Engineer will designate when samples for job control tests are to be fabricated, and will determine the limits of the lot represented by each job control test.

Should the average of the results of tests made on the 3 sample splices or should more than one sample splice in any job control test fail to meet the requirements for splices, all splices represented by that test will be rejected in accordance with the provisions in Section 6-1.04, "Defective Materials," of the Standard Specifications. This rejection shall prevail unless the Contractor, at the Contractor's expense, obtains and submits evidence, of a type acceptable to the Engineer, that the strength and quality of the splices in the work are acceptable.

Section 52-1.08F, "Nondestructive Splice Tests," of the Standard Specifications is amended to read:

52-1.08F Nondestructive Splice Tests.—All required radiographic examinations of complete joint penetration butt welded splices shall be performed by the Contractor in accordance with the requirements of AWS D 1.4 and these specifications.

Prior to radiographic examination, welds shall meet the requirements of Section 4.4, "Quality of Welds," of AWS D1.4-92.

Radiographic examinations shall be performed on 25 percent of all complete joint penetration butt welded splices from a production lot. The size of a production lot will be a maximum of 100 splices. The Engineer will select the splices which will compose the production lot and also the splices within each production lot to be radiographically examined.

Should more than 12 percent of the splices which have been radiographically examined in any production lot be defective, an additional 25 percent of the splices, selected by the Engineer from the same production lot, shall be radiographically examined. Should more than 12 percent of the cumulative total of splices tested from the same production lot be defective, all remaining splices in the lot shall be radiographically examined.

Additional radiographic examinations performed due to the identification of defective splices shall be at the Contractor's expense.

All defects shall be repaired in accordance with the requirements of AWS D1.4.

Radiographic examinations will not be required for either shop produced complete joint penetration butt welds or shop produced resistance butt welded splices of No. 25 or smaller bars used as spiral or hoop reinforcement.

In addition to radiographic examinations performed by the Contractor, any mechanical or welded splice may be subject to inspection or nondestructive testing by the Engineer. The Contractor shall provide sufficient access facilities in the shop and at the jobsite to permit the Engineer or his agent to perform the inspection or testing.

The Contractor shall notify the Engineer in writing 48 hours prior to performing any radiographic examinations.

The radiographic procedure used shall conform to the requirements of ASME Boiler and Pressure Vessels Code, Section V, Article 2 and the following:

Two exposures shall be made for each complete joint penetration butt welded splice. For each of the two exposures, the radiation source shall be centered on each bar to be radiographed. The first exposure shall be made with the radiation source placed at zero degrees from the top of the weld and perpendicular to the weld root and identified with a station mark of "0." When obstructions prevent a zero degree placement of the radiation source for the first exposure, and when approved in writing by the Engineer, the source may be rotated, around the centerline of the reinforcing bar, a maximum of 25 degrees. The second exposure shall be at 90 degrees to the "0" station mark and shall be identified with a station mark of "90."

For field produced complete joint penetration butt welds, no more than one weld shall be radiographed during one exposure. For shop produced complete joint penetration butt welds, if more than one weld is to be radiographed during one exposure, the angle between the root line of each weld and the direction to the radiation source shall be not less than 65 degrees.

Radiographs shall be made by either X-ray or gamma ray. Radiographs made by X-ray or gamma rays shall have densities of not less than 2.3 nor more than 3.5 in the area of interest. A tolerance of 0.05 in density is allowed for densitometer variations. Gamma rays shall be from the iridium 192 isotope and the emitting specimen shall not exceed 4.45 mm in the greatest diagonal dimension.

The radiographic film shall be placed perpendicular to the radiation source at all times; parallel to the root line of the weld unless source placement determines that the film must be turned; and as close to the root of the weld as possible.

The minimum source to film distance shall be maintained so as to insure that all radiographs maintain a maximum geometric unsharpness of 0.020 at all times, regardless of the size of the reinforcing bars.

Penetrameters shall be placed on the source side of the bar and perpendicular to the radiation source at all times. One penetrameter shall be placed in the center of each bar to be radiographed, perpendicular to the weld root, and adjacent to the weld. Penetrameter images shall not appear in the weld area.

When radiography of more than one weld is being performed per exposure, each exposure shall have a minimum of one penetrameter per bar, or 3 penetrameters per exposure. When 3 penetrameters per exposure are used, one penetrameter shall be placed on each of the 2 outermost bars of the exposure, and the remaining penetrameter shall be placed on a centrally located bar.

An allowable weld buildup of 4 mm may be added to the total material thickness when determining the proper penetrameter selection. No image quality indicator equivalency will be accepted. Wire penetrameters or penetrameter blocks shall not be used.

Penetrameters shall be sufficiently shimmed using a radiographically identical material. Penetrameter image densities shall be a minimum of 2.0 and a maximum of 3.6.

All radiographic film shall be Class 1, regardless of the size of reinforcing bars.

Radiographs shall be free of film artifacts and processing defects, including, but not limited to, streaks, scratches, pressure marks, or marks made for the purpose of identifying film or welding indications.

Each splice shall be clearly identified on each radiograph and the radiograph identification and marking system shall be established between the Contractor and the Engineer before radiographic inspection begins. Film shall be identified by lead numbers only; etching, flashing, or writing in identifications of any type will not be permitted. Each piece of film identification information shall be legible and shall include, as a minimum, the following information: Contractor's name, date, name of nondestructive testing firm, initials of radiographer, contract number, part number, and weld number. The letter "R" and repair number shall be placed directly after the weld number to designate a radiograph of a repaired weld.

Radiographic film shall be developed within a time range of one minute less to one minute more than the film manufacturer’s recommended maximum development time. Sight development will not be allowed.

Processing chemistry shall be done with a consistent mixture and quality, and processing rinses and tanks shall be clean to ensure proper results. Records of all developing processes and any chemical changes to the developing processes shall be kept and furnished to the Engineer upon request. The Engineer may request, at any time, that a sheet of unexposed film be processed in the presence of the Engineer to verify processing chemical and rinse quality.

All radiographs shall be interpreted and graded by a Level II or Level III technician who is qualified in accordance with the American Society for Nondestructive Testing's Recommended Practice No. SNT-TC-1A. The results of these interpretations shall be recorded on a signed certification and a copy kept with the film packet.

Technique sheets prepared in accordance with ASME Boiler and Pressure Vessels Code, Section V, Article 2 Section T-291 shall also contain the developer temperature, developing time, fixing duration and all rinse times.

All radiographic envelopes shall have clearly written on the outside of the envelope the following information: name of the Contractor's Quality Control Manager (QCM), name of the nondestructive testing firm, name of the radiographer, date, contract number, complete part description, and all included weld numbers or a report number, as detailed in the Contractor's Quality Control Plan (QCP). In addition, all innerleaves shall have clearly written on them the part description and all included weld numbers, as detailed in the Contractor's QCP.

The third paragraph of Section 52-1.10, "Measurement," of the Standard Specifications is amended to read:

The lap of bars for all splices, including splices shown on the plans where a continuous bar is used, will be measured for payment. The mass calculated shall be based upon the following table:

BAR REINFORCING STEEL

Deformed Bar Designation Number	Mass Kilogram Per Meter	Nominal Diameter, Millimeters
10	0.560	9.5
13	0.994	12.7
16	1.552	15.9
19	2.235	19.1
22	3.042	22.2
25	3.973	25.4
29	5.060	28.7
32	6.404	32.3
36	7.907	35.8
43	11.38	43.0
57	20.24	57.3

Note: Bar numbers approximate the number of millimeters of the nominal diameter of the bars. The nominal diameter of a deformed bar is equivalent to the diameter of a plain round bar having the same mass per meter as the deformed bar.

EPOXY-COATED PREFABRICATED REINFORCEMENT

Bar reinforcement to be epoxy-coated shall conform to the ASTM Designation and grade required or permitted by Section 52-1.02A, "Bar Reinforcement," for the location or type of structure involved. The coated bar reinforcement shall conform to the requirements in ASTM Designation: A 934/A 934M except as provided herein.

Wire reinforcement to be epoxy-coated shall conform to the ASTM Designation and grade required or permitted by Section 52-1.02D, "Reinforcing Wire and Plain Bars," for the location or type of structure involved. The coated wire reinforcement shall conform to the provisions for Class A, Type 2 coating of ASTM Designation: A 884/A 884M except as provided herein.

Appendices X1 and X2, "Guidelines For Job-Site Practices," of ASTM Designation: A 884/A 884M and A 934/A 934M, respectively, shall apply except as provided herein. The term "shall" shall replace the term "should" in these appendices. Section X1.2 of Appendix X1 and Section X2.2 of Appendix X2 shall not apply.

All coatings shall be purple or gray in color.

Except for field welding of butt splices, all welding of reinforcement shall be complete prior to epoxy coating the reinforcement.

Bending of epoxy-coated reinforcement after the coating has been applied will not be allowed.

When any portion of a reinforcing bar or wire requires epoxy coating, the entire bar or wire shall be coated; except, when the bar or wire is spliced outside of the limits of epoxy coating shown on the plans, epoxy coating will not be required on the portion of bar or wire beyond the splice.

Within areas where epoxy-coated reinforcement is required, tie wire and bar chairs or other metallic devices used to secure or support the reinforcement shall be plastic-coated or epoxy-coated to prevent corrosion of the devices or damage to the coated reinforcement.

Prior to coating, the Contractor shall furnish to the Transportation Laboratory a representative 110 g sample from each batch of epoxy coating material to be used. Each sample shall be packaged in an airtight container identified with the manufacturer's name and batch number.

Two 700-mm long samples of coated bar or wire reinforcement from each size and from each load shipped to the jobsite shall be furnished to the Engineer for testing. These samples shall be representative of the material furnished. These samples, as well as any additional random samples taken by the Engineer, may be tested for specification compliance. Additional sampling, and all tests performed by the Engineer, may be performed at any location deemed appropriate by the Engineer. Failure of any sample to meet the requirements of the specifications will be cause for rejection.

If any bar tested for coating thickness or for adhesion of coating fails to meet the requirements for coated bars in Section 9 of ASTM Designation: A 934/A 934M, 2 retests on random samples taken from bars represented by the failed test will be conducted for each failed test. If the results of both retests meet the specified requirements, the coated bars represented by the samples may be certified as meeting the test requirements.

If any wire reinforcement tested for coating thickness or for flexibility fails to meet the requirements for coated wire in Section 8 of ASTM Designation: A 884/A 884M, 2 retests on random samples taken from wire represented by the failed test will be conducted for each failed test. If the results of both retests meet the specified requirements, the coated wire represented by the samples may be certified as meeting the test requirements.

Epoxy-coated reinforcement shall be covered with an opaque polyethylene sheeting or other suitable protective material to protect the reinforcement from exposure to sunlight, salt spray and weather. For stacked bundles, the protective covering shall be draped around the perimeter of the stack. The covering shall be adequately secured; however, it should allow for air circulation around the reinforcement to prevent condensation under the covering. Epoxy-coated reinforcement shall not be stored within 300 meters of ocean or tidal water for more than 2 months.

All visible damage to coatings caused by shipping, handling or installation shall be repaired as required for repairing coating damaged prior to shipment as specified in ASTM Designation: A 934/A 934M for bar reinforcement or ASTM Designation: A 884/A 884M for wire reinforcement. When the extent of coating damage prior to repair exceeds 2 percent of the bar or wire surface area in any 300-mm length, repair of the bar or wire will not be allowed and the coated bar or wire will be rejected.

The patching material and process shall be suitable for field application. The patching material shall be prequalified as required for the coating material and shall be either identified on the container as a material compatible with the reinforcement coating, or shall be accompanied by a Certificate of Compliance certifying that the material is compatible with the reinforcement coating. Damaged areas shall be patched in conformance with the patching material manufacturer's recommendations.

Except for lap splices, all splices for epoxy-coated reinforcement shall be coated with a corrosion protection covering that is on the Department's list of approved products. The covering shall be installed in conformance with the manufacturer's recommendations and as directed by the Engineer. The list is available from the Transportation Laboratory.

A Certificate of Compliance conforming to the provisions in Section 6-1.07, "Certificates of Compliance," shall be furnished for each shipment of epoxy-coated bar or wire reinforcement certifying that the coated bars or wire conform to the requirements in ASTM Designation: A 934/A 934M for bars or A 884/A 884M for wire and Section 52-1.02B, "Epoxy-coated Bar Reinforcement." This Certificate of Compliance shall include all the certifications specified in ASTM Designation: A 934/A 934M for bars or ASTM Designation: A 884/A 884M for wire and a statement that the coating material has been prequalified by acceptance testing performed by the Valley Forge Laboratories, Inc., Devon, Pennsylvania.

Any portion of bar or wire reinforcement extending beyond the limits for epoxy coated reinforcement shown on the plans will be measured and paid for as bar reinforcing steel (bridge).

10-1.69 SHOTCRETE

Shotcrete shall conform to the provisions in Section 51, "Concrete Structures," and Section 53, "Shotcrete," of the Standard Specifications and these special provisions.

Shotcrete operations shall completely encase all reinforcement and other obstructions shown on the plans. Exceptional care shall be taken to properly encase the reinforcement and other obstructions with shotcrete.

Attention is directed to the section, "Order of Work," elsewhere in these special provisions regarding furnishing preconstruction shotcrete test panels.

Except for finish coats, shotcrete shall be applied by the wet-mix process only.

Finish coats, applied by the dry-mix process, may be used only when approved by the Engineer.

Shotcrete shall have a minimum compressive strength of 22.5 MPa at 28 days or as shown on the plans, whichever is greater. No shotcrete work shall be performed prior to verification by the Engineer of the required compressive strength.

Splicing of reinforcing bars No. 22 or larger in shotcrete shall be by butt splicing only.

The Contractor shall be responsible for obtaining and testing all required preconstruction and production test cores. All coring and testing shall be at the Contractor's expense and performed in the presence of the Engineer, unless otherwise directed. The Engineer shall be notified a minimum of 24 hours prior to the Contractor performing any coring or testing operations.

All cores shall be obtained and tested for compressive strength in accordance with the specifications of ASTM Designation: C 42. Cores used for determining compressive strength shall not contain any bar reinforcement or other obstructions. The testing shall be performed at an independent testing facility approved by the Engineer. A copy of the test results shall be furnished to the Engineer within 5 days following completion of testing.

All test panels shall become the property of the Contractor and shall be disposed of as provided in Section 7-1.13, "Disposal of Material Outside the Highway Right of Way," of the Standard Specifications.

PRECONSTRUCTION REQUIREMENTS.--Prior to performing shotcrete work, the Contractor shall construct at least 2 preconstruction shotcrete test panels for each mixture being considered unless otherwise specified.

The nozzleperson shall have a minimum of 3000 hours experience as a nozzleperson on projects with a similar application.

At least 10 working days prior to constructing any shotcrete test panels, the Contractor shall submit to the Engineer for approval, a Quality Control Plan (QCP) for the proposed method of shotcrete placement. The plan shall include the following:

1. The number and qualifications of nozzlepersons available to place shotcrete, the number of nozzlepersons on-site at any time during the shotcrete placement, description of their work schedule, and the procedures for avoiding fatigue of any nozzleperson.
2. The proposed method of placing shotcrete, including, but not limited to, application rates, details of any proposed construction joints and their locations, and methods for achieving the required thickness and surface finish.
3. The procedure for curing shotcrete surfaces.
4. The description of a debris containment system, to be used during the cleaning of bar reinforcing steel and concrete and placing of shotcrete, as required to provide for public safety.

The Engineer shall have 10 working days to review and approve the QCP submittal after a complete plan has been received. No construction of shotcrete test panels shall be performed until the QCP is approved by the Engineer. Should the Engineer fail to complete the review within this time allowance, and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in approving the QCP, the delay will be considered a right of way delay as specified in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

Preconstruction shotcrete test panels shall be constructed by the nozzlepersons and application crew scheduled to do the work, using equipment, materials, mixing proportions, ambient temperatures and procedures proposed for the work. The preconstruction shotcrete test panels shall conform to the following:

One shotcrete test panel, of the size determined by the Contractor, shall be unreinforced and shall have 3 cores taken from it and tested for compressive strength. The compressive strength shall be the average strength of the 3 cores, except that, if any core should show evidence of improper coring, the core shall be discarded and the compressive strength shall be the average strength of the remaining cores. The test panel shall be identified and submitted to the Engineer with the test results including a description of the mixture, proportions, and ambient temperature.

One shotcrete test panel shall have the same (1) thickness, (2) bar size and amount of bar reinforcement or other obstructions and (3) positioning of bar reinforcement or obstructions as the most heavily reinforced section of shotcrete to be placed. The test panel shall be square with the length of the sides equal to at least 3 times the thickness of the most heavily reinforced section of shotcrete to be placed, but not less than 750 millimeters. After a minimum 7 days of cure,

the test panel shall be broken by the Contractor, in the presence of the Engineer, into pieces no larger than 250 millimeters in greatest dimension. The surfaces of the broken pieces shall be dense and free of laminations and sand pockets, and shall verify that the bar reinforcement or other obstructions are completely encased.

Both test panels shall be cured under conditions similar to the actual work.

At the option of the Contractor, cores to be used for determining the compressive strength may be taken from the reinforced test panel described above in lieu of making a separate unreinforced test panel as described above. The compressive strength shall be the average strength of the 3 cores, except that, if any core should show evidence of improper coring or contains bar reinforcement or other obstructions, the core shall be discarded and the compressive strength shall be the average strength of the remaining cores. If cores are taken from the reinforced test panel, the panel shall not be broken into pieces, as described above, until it has cured for a minimum of 14 days.

The requirements for constructing preconstruction shotcrete test panels may be eliminated, when approved by the Engineer, if a test panel report and certified compressive strength test data are furnished from a State highway project with a similar application of approximately equal thickness, including similar amounts and placement of reinforcement or other obstructions. The nozzleperson, proposed to be used, shall have constructed the test panel described in the test panel report. The test panel report shall list the names of the application crew, equipment used, materials, mixing proportions, ambient temperatures and procedures used to make the test panels. The certified compressive strength test data shall be for cores taken from the same test panels.

PLACING.--An air blowpipe shall be used during shotcrete placement to remove rebound, overspray and other debris from the areas to receive shotcrete.

Construction joints shall be tapered, and shall conform to the provisions in Sections 51-1.13, “Bonding,” of the Standard Specifications.

All overspray and rebound shall be removed prior to final set and before placement of shotcrete on adjacent surfaces.

Rebound or any other material which has already exited the nozzle shall not be reused.

Shotcrete shall be cured in accordance with the provisions of Section 90-7.03, “Curing Structures,” of the Standard Specifications.

When a finish coat is to be used, all loose, uneven or excess material, glaze, and rebound shall be removed by brooming, scraping, or other means and the surface left scarified. Any surface deposits which take a final set shall be removed by abrasive blasting. Prior to placing the finish coat, the receiving surface shall be washed down with an air-water blast.

Shotcrete extending into the space shown on the plans for cast-in-place concrete shall be removed.

TESTING AND ACCEPTANCE.--At least 3 production shotcrete test cores shall be taken from each 30 square meters or portion thereof of shotcrete placed each day. The cores shall be 76-millimeters in diameter. The location where cores are to be taken will be designated by the Engineer. Test cores shall be identified by the Contractor and a description of the core location and mixture, including proportions, shall be submitted to the Engineer with the test cores, immediately after coring. Cored holes shall be filled with mortar in accordance with the requirements of Section 51-1.135, “Mortar,” of the Standard Specifications.

Upon receipt of the cores, the Engineer will perform a visual examination to determine acceptance, as described below. Within 48 hours after receipt, the Engineer will return the cores to the Contractor for compressive strength testing.

The compressive strength test shall be performed using the shotcrete production test cores described above. The compressive strength shall be the average strength of the 3 cores, except that, if any core should show evidence of improper coring, the core shall be discarded and the compressive strength shall be the average strength of the remaining cores.

The basis of acceptance for production shotcrete test cores shall be (1) that the core is dense and free of laminations and sand pockets, and shows that the reinforcement or other obstructions are completely encased and (2) the same as specified for test cylinders in the fourth and fifth paragraphs of Section 90-9.01, "General," of the Standard Specifications.

If any production test core shows signs of defective shotcrete as described in (1) above, the shotcrete represented by such test core will be rejected, unless the Contractor, at the Contractor’s expense, obtains and submits evidence acceptable to the Engineer that the strength and quality of the shotcrete placed in the work are acceptable.

The surface finish of the shotcrete shall conform to the provisions of Section 51-1.18, "Surface Finishes," of the Standard Specifications.

MEASUREMENT AND PAYMENT.--Full compensation for the Quality Control Plan, constructing and breaking test panels, furnishing and testing cores and patching cored holes shall be considered as included in the contract price paid per cubic meter for shotcrete and no additional compensation will be allowed therefor.

10-1.70 SIGN STRUCTURES

Sign structures and foundations for overhead signs shall conform to the provisions in Section 56-1, "Overhead Sign Structures," of the Standard Specifications and these special provisions.

Paragraph 3 in Section 56-1.01, "Description," of the Standard Specifications is amended to read:

Before commencing fabrication of sign structures, the Contractor shall submit 2 sets of working drawings to the Engineer in conformance with the provisions in Section 5-1.02, "Plans and Working Drawings." The working drawings shall include sign panel dimensions, span lengths, post heights, anchorage layouts, proposed splice locations, a snugging and tensioning pattern for anchor bolts and high strength bolted connections, and details for permanent steel anchor bolt templates. The drawings shall be supplemented by a written quality control program listing methods, equipment, and personnel necessary to satisfy the requirements specified herein and in the special provisions.

Working drawings shall be 559 mm x 864 mm or 279 mm x 432 mm in size and each drawing and calculation sheet shall include the sign structure type and reference as shown on the contract plans, District-County-Route, and contract number.

The Engineer shall have 20 working days to review the sign structure working drawings after a complete submittal has been received. No fabrication or installation of sign structures shall be performed until the working drawings are approved in writing by the Engineer.

Should the Engineer fail to complete the review within the time allowance and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in reviewing the sign structure working drawings, the delay will be considered a right of way delay in conformance with the provisions in Section 8-1.09, "Right of Way Delays."

Section 56-1.02C, "Bolts, Nuts and Washers," of the Standard Specifications is amended to read:

56-1.02C Bolts, Nuts, and Washers.—Bolts, nuts, and washers for use in sign structures shall conform to the provisions in Section 55-2, "Materials."
A permanent steel template shall be used to maintain the proper anchor bolt spacing.
One top nut, one leveling nut, and two washers shall be provided for the upper threaded portion of each anchor bolt.

Paragraph 1 in Section 56-1.03, "Fabrication," of the Standard Specifications, with the exception of the title, is deleted.
Paragraph 3 in Section 56-1.03, "Fabrication," of the Standard Specifications is amended by adding the following:

Surfaces of base plates which are to come in contact with concrete, grout, or washers and leveling nuts shall be flat to within 3 mm tolerance in 305 mm, and to within 5 mm tolerance overall. Faying surfaces of plates in high-strength bolted connections including flange surfaces of field splices, chord joints, and frame junctures, and contact surfaces of plates used for breakaway slip base assemblies shall be flat to within 2 mm tolerance in 305 mm, and within 3 mm tolerance overall.

Thermally cut holes made in tubular members of sign supports, other than holes in base and flange plates, shall initially be made a minimum of 2 mm undersized, and then be mechanically enlarged by reaming or grinding to the final required size and shape. All edges shall have a surface roughness of not greater than 6.35 μm. Round holes may be drilled to the exact final diameter. No holes shall be made in members unless the holes are shown on the plans or are approved in writing by the Engineer.

Paragraphs 6 through 13 in Section 56-1.03, "Fabrication," of the Standard Specifications are amended to read:

High-strength bolted connections, where shown on the plans, shall conform to the provisions in Section 55-3.14, "Bolted Connections," except that only fastener assemblies consisting of a high-strength bolt, nut, hardened washer and direct tension indicator shall be used.

High-strength fastener assemblies, and any other bolts, nuts, and washers attached to sign structures shall be zinc-coated by the mechanical deposition process.

An alternating snugging and tensioning pattern for anchor bolts and high-strength bolted splices shall be used. Once tensioned, high-strength fastener components and DTI's shall not be reused.

For bolt diameters less than 10 mm, the diameter of the bolt hole shall be not more than 0.80-mm larger than the nominal bolt diameter. For bolt diameters greater than or equal to 10 mm, the diameter of the bolt hole shall be not more than 1.6 mm larger than the nominal bolt diameter.

Sign structures shall be fabricated into the largest practical sections prior to galvanizing.

Ribbed sheet metal panels for box beam closed truss sign structures shall be fastened to the truss members by cap screws or bolts as shown on the plans, or by 4.76 mm stainless steel blind rivets conforming to Industrial Fasteners Institute, Standard IFI-114, Grade 51. The outside diameter of the large flange rivet head shall be not less than 15.88 mm in diameter. Web splices in ribbed sheet metal panels may be made with similar type blind rivets of a size suitable for the thickness of material being connected.

Any spalling or chipping of concrete structures shall be repaired by the Contractor at the Contractor's expense.

Overhead sign supports shall have an aluminum identification plate permanently attached near the base, adjacent to the traffic side on one of the vertical posts, using either stainless steel rivets or stainless steel screws. As a minimum, the information on the plate shall include the name of the manufacturer, the date of manufacture and the contract number.

Section 56-1.04, "Welding," of the Standard Specifications is amended to read:

56-1.04 Welding.— Welding, nondestructive testing (NDT) of welds, and acceptance and repair criteria for NDT of steel overhead sign structure members shall conform to the requirements of AWS D1.1 and the special provisions. Steel members used for overhead sign structures shall receive NDT in conformance with AWS D1.1 and the following:

Weld Location	Weld Type	Minimum Required NDT
Welds for butt joint welds in tubular sections, nontubular sections, and posts	CJP groove weld with backing ring	100% UT or RT
Longitudinal seam welds*	PJP groove weld	25% MT
	CJP groove weld	100% UT or RT
Welds for base plate, flange plate, or end cap to post or mast arm	CJP groove weld	25% UT or RT
	Fillet weld	25% MT
* Longitudinal seam welds shall have 60% minimum penetration, except that within 150 mm of any circumferential weld, longitudinal seam welds shall be CJP groove welds.		

A written procedure approved by the engineer shall be used when performing UT on material less than 8 mm thick. Contoured shoes shall be used when performing UT on round tubular sections under 1270 mm in diameter.

When less than 100 percent of a weld is specified for NDT, and if defects are found during this inspection, additional NDT shall be performed. This additional NDT shall be performed on 25% of the total weld for all similar welds, as determined by the Engineer, produced for sign structures in the project. If any portion of the additional weld inspected is found defective, 100% of all similar welds produced for sign structures in the project, as determined by the Engineer, shall be tested. Circumferential welds and base plate to post welds may be repaired only one time without written permission from the Engineer.

Tubular sign structures shall be galvanized and not painted.

PAYMENT

Full compensation for furnishing anchor bolt templates and for testing of welds, shall be considered as included in the contract lump sum price paid per kilogram for furnish sign structure and no additional compensation will be allowed therefor.

10-1.71 ROADSIDE SIGNS

Roadside signs shall be installed at the locations shown on the plans or where directed by the Engineer, and shall conform to the provisions in Section 56-2, "Roadside Signs," of the Standard Specifications and these special provisions.

The first three paragraphs of Section 56-2.02B, "Wood Posts," of the Standard Specifications are amended to read:

The grades and species allowed for wood posts, 90 mm x 90 mm in size, are select heart redwood; No. 1 heart structural redwood (1050f); No. 2 heart structural redwood (900f); No. 1 structural light framing Douglas fir, free of heart center; No. 1 structural light framing Hem-Fir, free of heart center; or No. 1 structural light framing Southern yellow pine, free of heart center. The grades and species allowed for wood posts, 90 mm x 143 mm in size, are select heart grade redwood; select heart structural grade redwood (1100f); No. 1 heart structural redwood (950f); No. 2 structural joists and planks, Douglas fir, free of heart center; No. 1 structural joists and planks Hem-Fir, free of heart center; or No. 2 structural joists and planks Southern yellow pine. The grades and species allowed for wood posts larger than 90 mm x 143 mm in size are select heart redwood; No. 1 heart structural redwood (950f); No. 1 posts and timbers (also known as No. 1 structural) Douglas fir, free of heart center; select structural posts and timbers Hem-Fir, free of heart center; or No. 1 timbers Southern yellow pine, free of heart center.

Posts shall be graded in conformance with the provisions in Section 57-2, "Structural Timber." Sweep shall not exceed 25 mm in 3.0 m.

Before preservative treatment, the moisture content of Douglas fir, Hem-Fir, and Southern yellow pine posts shall be not more than 25 percent as measured at the midpoint of the post in the outer 25 mm, using an approved type of moisture meter, in conformance with the requirements of ASTM Designation: D 4444.

10-1.72 INSTALL SIGN OVERLAY

Sign overlays shall be installed on existing signs, as shown on the plans, and in conformance with these special provisions.

Sign overlay panels will be furnished by the State as provided under "Materials" of these special provisions.

Self plugging blind rivets for installing sign overlays shall have a 4.8-mm x 15.9-mm shank. A No.-10 drill shall be used for drilling the rivet holes. If the overlay is not pre-punched, maximum rivet spacing shall be 400 mm.

The contract price paid per square meter for install sign overlay shall include full compensation for furnishing all labor, materials (except sign overlays), tools, equipment, and incidentals, and for doing all the work involved in installing sign overlay panels on existing signs (including fastening hardware), as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.73 TIMBER STRUCTURES

The first paragraph in Section 57-1.02A, "Structural Timber and Lumber," of the Standard Specifications is amended to read:

> Structural timber and lumber shall be of the following species: Douglas fir, Hem-Fir, redwood, or Southern yellow pine, as shown on the plans or as specified in the specifications.

Section 57-2.01, "Description," of the Standard Specifications is amended to read:

> Douglas fir timber shall be the species "Pseudotsuga menziesii"; redwood shall be the species "Sequoia sempervirens"; Hem-Fir shall be one of the species "Abies magnifica," "Abies grandis," "Abies procera," "Abies amabillis," "Abies concolor," or "Tsuga heterophylla"; and Southern yellow pine shall be one of the several species recognized by the Southern Pine Inspection Bureau.

The second paragraph in Section 57-2.02, "Grading Rules and Requirements," of the Standard Specifications is amended to read:

> Douglas fir and Hem-Fir shall be graded in conformance with the requirements of the current standard grading and dressing rules of the West Coast Lumber Inspection Bureau, or the current standard grading rules of the Western Wood Products Association.

10-1.74 ALTERNATIVE PIPE

Alternative pipe culverts shall conform to the provisions in Section 62, "Alternative Culverts," of the Standard Specifications and these special provisions.

SPIRAL RIB PIPE.--Spiral rib pipe shall meet the requirements for corrugated steel pipe in Section 66, "Corrugated Metal Pipe," of the Standard Specifications, except for profile and fabrication requirements.

Spiral rib pipe shall, at the option of the Contractor, consist of either one rectangular rib spaced midway between seams or 2 rectangular ribs and one half-circle rib equally spaced between seams. All ribs shall be continuous helical ribs that project outward from the surface of the pipe. On single rib pipe the rib shall be 19 mm wide by 19 mm high and the maximum rib pitch shall be 191 mm. On the two rectangular rib with one half-circle rib pipe, rectangular ribs shall be 19 mm wide by 25 mm high. The half-circle rib diameter shall be 13 mm and shall be spaced midway between the rectangular ribs and the maximum rib pitch shall be 292 mm. Rib pitch measured at right angles to the direction of the ribs may vary ±13 mm.

Corrugated steel spiral rib pipe shall be fabricated by a continuous helical lock seam fabricated in accordance with the requirements in Section 66-3.03C(1), "Fabrication by Continuous Lock Seam," of the Standard Specifications.

Coupling bands for spiral rib pipe shall conform to the requirements in Section 66-1.07, "Coupling Bands," of the Standard Specifications. Any coupling band shown on the plans or approved by the Engineer in accordance with Section 61-1.02, "Performance Requirements for Culvert and Drainage Pipe Joints," of the Standard Specifications, for use on a pipe corrugation of 68 mm x 13 mm for corrugated metal pipe may be used on spiral rib pipe having 68 mm x 13 mm rerolled annular ends. The width of band (W) for hat bands for pipe sizes larger than 1200 mm in diameter shall be 95 mm.

Concrete backfill for alternative culverts shall be constructed in accordance with the requirements in Section 66-1.045, "Concrete Backfill," of the Standard Specifications and will be measured and paid for in accordance with the provisions in Section 66-4, "Measurement and Payment," of the Standard Specifications and the following:

> The quantity of concrete backfill to be paid for, regardless of the kind of culvert and wall thickness of the culvert installed, will be based on the dimensions shown on the plans and the installation of corrugated steel pipe, except that when reinforced concrete pipe is designated as the only kind of culvert allowed for the installation of an alternative culvert, the quantity of concrete backfill to be paid for at that installation, regardless of the kind of culvert and wall thickness of the culvert installed, will be based on the dimensions shown on the plans and the installation of reinforced concrete pipe with the least wall thickness shown in AASHTO Designation: M 170M for the Class of pipe designated.

10-1.75 CORRUGATED METAL PIPE

Corrugated steel pipe and slotted steel pipe culverts shall conform to the provisions in Section 66, "Corrugated Metal Pipe," of the Standard Specifications and these special provisions.

Corrugated steel pipe shall be fabricated from zinc-coated steel sheet.

The first paragraph in Section 66-1.03, "Protective Coatings, Linings and Pavings," of the Standard Specifications is amended to read:

> 66-1.03 Protective Coatings, Linings and Pavings.—When required by the special provisions or designated in the Engineer's Estimate, pipes shall be protected with bituminous coating, bituminous lining or have the invert paved with bituminous material or coated with polymerized asphalt. Moisture, dirt, oil, unbonded or incompatible paint, grease, alkalies or other foreign matter shall be removed from the surface to be coated before the coating material is applied.

Section 66-1.03, "Protective Coatings, Linings and Pavings," of the Standard Specifications is amended by adding the following paragraphs after the eighth paragraph:

> Polymerized asphalt invert coating shall be applied in conformance with the requirements in ASTM Designation: A 849 for "Invert Paved Type with Polymer Material (Class P)," except that polymerized asphalt coatings shall be applied by immersion to a minimum thickness of 1.3 mm above the crests and troughs of the corrugations of the interior and exterior invert including pipe ends. Polymerized asphalt material shall conform to the "Requirements for Polymer Coating" contained in ASTM Designation: A 742/A 742M, and the following:
>
> > Polymerized asphalt shall be hot-applied thermoplastic material containing a minimum of 7.0 percent styrene-butadiene-styrene block copolymer.
> >
> > There shall be not more than 6.4 mm undercutting or delamination from the scribe when a minimum 300 mm by 300 mm coupon cut from the coated pipe is exposed for 1000 hours in accordance with the requirements in ASTM Designation: B 117. Cut edges shall be sealed by dipping in a sample of the polymerized asphalt coating heated to the manufacturer's recommended application temperature. There shall be no corrosion or delamination from the sealed edges following exposure as specified.

The last paragraph in Section 66-1.03, "Protective Coatings, Linings and Pavings," of the Standard Specifications is amended to read:

> Damaged protective coatings, linings and invert paving shall be repaired by the Contractor at the Contractor’s expense. Bituminous material conforming to the requirements in AASHTO Designation: M 190 or other materials approved by the Engineer shall be used to repair damaged bituminous coatings; asphalt mastic material conforming to the requirements in AASHTO Designation: M 243 shall be used to repair damaged asphalt mastic coatings; and tar base material conforming to the provisions of AASHTO Designation: M 243 shall be used to repair damaged polymeric coatings. The repair of damaged polymerized asphalt coatings shall conform to the requirements in ASTM Designation: A 762, Section 11, "Repair of Damaged Coatings."

Section 66-3.06, "Damaged Aluminum Coatings," of the Standard Specifications is amended to read:

> 66-3.06 Damaged Aluminum Coatings.—In lieu of the requirements in AASHTO Designation: M 36/M 36M, damaged aluminum coatings shall be repaired as provided for damaged galvanizing in Section 75-1.05, "Galvanizing," or Section 66-3.05, "Damaged Galvanizing."

10-1.76 ALTERNATIVE PIPE UNDERDRAINS

Alternative pipe underdrains shall conform to the requirements in Section 68-1, "Underdrains," of the Standard Specifications for the kind of alternative pipe underdrain installed and these special provisions.

100 mm plastic pipe shown on sheet C-31 of the plans is an underdrain system and is to be measured and paid for as an underdrain per Section 68-1, "Underdrains," of the Standard Specifications.

Permeable material shall be Class 3.

The percentage composition by mass of Class 3 permeable material in place shall conform to the following grading:

Grading Requirements

Sieve Sizes	Percentage Passing
37.5-mm	100
25-mm	90-100
19-mm	40-100
9.5-mm	0-50
4.75-mm	0-15
2.36-mm	0-5

Class 3 permeable material shall have a Durability Index of not less than 40.

Not less than 90 percent by mass of Class 3 permeable material shall be crushed particles as determined by California Test 205.

10-1.77 CHECK VALVES

Check valves shall conform to the following special provisions.

The check valve shall be designed for gravity flow and 1.0 meter of back pressure measured from the center of the valve to the highest water level. The valve shall provide for free outflow, but prevent backflow. The check valve cuff area shall be 100% of the mating pipe port size and shall be designed for a slip-on type connection to the size and type of pipe specified. The check valve saddle area shall taper from a round cuff to a flared “duckbill” configuration at the valve bill.

The check valve shall be furnished complete with check sleeve, clamps, and fasteners.

The check valve sleeve shall be one piece neoprene construction with synthetic fabric reinforcement, and shall be manufactured using a steam vulcanization process. Neoprene shall conform to the specifications of ASTM Designation: D-2000 BC410/414. Ethylene-Propylene (EPDM) wrapping shall be vulcanized over the fabric reinforcement on the outside of the sleeve.

Clamps, holding bolts, nuts, and fasteners shall be manufactured of stainless steel conforming to the specifications of ASTM Designation: A240, Type 316. Holding bolts and nuts shall conform to ASTM Designations F 738, and F 836, respectively. The quantity of clamps and holding bolts, and the method of installation, shall be as recommended by the manufacturer and approved by the Engineer.

If the mating pipe has a corrugated exterior surface, rubber filler strips supplied by the check valve manufacturer shall be used to fill the voids between the corrugations and the valve cuff to provide a watertight seal. The filler strips shall be of sufficient length to wrap completely around the pipe a minimum of 3 revolutions.

The Contractor shall be responsible for providing the check valve manufacturer with the pipe material type, corrugation dimensions if applicable, and the actual outside diameter of the mating pipe.

The check valve shall be installed per the manufacturer’s recommendations and as directed by the Engineer. The outer surface of the check sleeve shall be free of gouges. The manufacturers name, plant location, valve size, and serial number shall be bonded to the check valve exterior surface.

The contract unit price for check valves shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, including holding bolts, and for doing all the work involved in installing the various sizes of check valves, complete in place, as shown on the plans, and as specified by the manufacturer and these special provisions, and as directed by the Engineer.

10-1.78 PERMEABLE MATERIAL (BLANKET)

Permeable material (blanket) shall be constructed in accordance with the details shown on the plans and these special provisions.

Permeable material for permeable material (blanket) shall be Class 3 and shall conform to the provisions of Section 68-1, "Underdrains," of the Standard Specifications, except for payment.

The percentage composition by mass of Class 3 permeable material in place shall conform to the following grading:

Grading Requirements

Sieve Sizes	Percentage Passing
37.5-mm	100
25-mm	90-100
19-mm	40-100
9.5-mm	0-50
4.75-mm	0-15
2.36-mm	0-5

Class 3 permeable material shall have a Durability Index of not less than 40.

Not less than 90 percent by mass of Class 3 permeable material shall be crushed particles as determined by California Test 205.

Filter fabric for use with permeable material (blanket) shall conform to the requirements for filter fabric for underdrain trenches in Section 88, "Engineering Fabrics," of the Standard Specifications and the following:

The subgrade to receive the filter fabric, immediately prior to placing, shall conform to the compaction and elevation tolerance specified for the material involved.

Filter fabric shall be handled and placed in accordance with the manufacturer's recommendations.

The fabric shall be aligned and placed in a wrinkle-free manner.

Adjacent borders of the fabric shall be overlapped from 300 to 450 mm or stitched. The preceding roll shall overlap the following roll in the direction the material is being spread or shall be stitched. When the fabric is joined by stitching, it shall be stitched with yarn of a contrasting color. The size and composition of the yarn shall be as recommended by the fabric manufacturer. The stitches shall number 5 to 7 per 25 mm of seam.

Within 24 hours after the filter fabric has been placed, it shall be covered with the planned thickness of permeable material or aggregate subbase material as shown on the plans.

During spreading and compaction of the permeable material and aggregate subbase material, a minimum of 150 mm of the material shall be maintained between the fabric and the Contractor's equipment. Where embankment material is to be placed on the filter fabric, a minimum of 450 mm of embankment material shall be maintained between the fabric and the Contractor's equipment. Equipment or vehicles shall not be operated or driven directly on the filter fabric.

Driving or operating vehicles directly on top of permeable material (blanket) shall be kept to the absolute minimum necessary to place, grade, and compact the permeable material.

Surfaces to receive permeable material which are lower than the grade established by the Engineer may be filled with permeable material. Volumes of permeable material so placed will not be included in the volume calculated for payment.

Permeable material for permeable material (blanket) shall be delivered as uniform mixtures and shall be deposited in layers or windrows.

Segregation shall be avoided and the permeable material shall be free from pockets of coarse or fine material. The permeable material, after spreading, shall be shaped to such thickness that after compaction, the completed permeable material (blanket) shall conform to the required cross section and the grade for thickness within the tolerance herein specified.

The thickness of permeable material (blanket) shall be not less than the planned thickness and shall not exceed the planned thickness by more than 30 mm.

Permeable material (blanket) shall be compacted using a 2-axle, steel-tired tandem roller weighing not less than 7.3 tonnes. At least one coverage shall be made with the roller. A pass shall be one movement of a roller in either direction. A coverage shall be as many passes as are necessary to cover the entire width of the placed permeable material.

During periods of inclement weather or when conditions are such that contamination of the permeable material could occur, as determined by the Engineer, permeable material (blanket) shall be covered and there shall be no further placement of the permeable material until conditions are such that proper placement of permeable material (blanket) can be resumed.

Permeable material (blanket) will be measured and paid for by the cubic meter. Quantities of permeable material to be paid for as permeable material (blanket) will be determined from the dimensions shown on the plans or such other dimensions as may be ordered in writing by the Engineer, and permeable material (blanket) constructed in excess of these dimensions will not be paid for.

The contract price paid per cubic meter for permeable material (blanket) shall include full compensation for furnishing all labor, materials (including filter fabric), tools, equipment, and incidentals, and for doing all the work involved in constructing permeable material (blanket) and placing filter fabric, complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.79 OVERSIDE DRAINS

Asphalt concrete overside drains shall conform to the provisions in Section 69, "Overside Drains," of the Standard Specifications.

10-1.80 MISCELLANEOUS FACILITIES

Alternative flared end sections and corrugated steel pipe tee dissipaters shall conform to the provisions in Section 70, "Miscellaneous Facilities," of the Standard Specifications.

10-1.81 SLOPE PROTECTION

Slope protection shall conform to the provisions in Section 72, "Slope Protection," of the Standard Specifications.

Rock slope protection fabric shall be woven or nonwoven type fabric, Type A or Type B, at the option of the Contractor.

10-1.82 GABIONS (PVC)

Gabions shall be constructed as shown on the plans and in conformance with the provisions in Section 72, "Slope Protection," of the Standard Specifications and these special provisions.

Gabions shall consist of wire mesh, box-shaped or mattress-shaped baskets that are filled on the project site with hard, durable rock. The individual wires shall have a polyvinyl chloride (PVC) coating.

Standard gabion sizes and the overall plan and profile dimensions of the gabion structures shall be as shown on the plans. The width, height or length of the standard gabions shall not vary more than 5 percent from the dimensions specified in these special provisions or as shown on the plans.

At the Contractor's option, either twisted mesh or welded mesh may be used, except as shown in Table 1 and Table 2 of these special provisions. For each standard gabion, the same mesh style shall be used for the base, front, ends, back, diaphragms, and lid panels. Each standard gabion shall be divided into one meter long cells by diaphragm panels.

Empty gabions shall be assembled individually and joined successively. Individual gabion mesh panels (base, front, ends, back, diaphragms, and lid) and successive gabions shall be assembled so that the strength and flexibility along the joints is comparable to a single panel.

MATERIALS

All materials for the gabions and gabion assembly shall conform to the provisions in these special provisions. Each shipment of gabions to the project site shall be accompanied by a Certificate of Compliance conforming to the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications.

Mesh

Individual wires of either the twisted mesh style or the welded mesh style shall conform to the definitions and requirements in ASTM Designation: A641/A641 M .

Mattress-style gabions that are 0.3–m and 0.5–m high shall be manufactured from either 11-gage (3.05 mm) welded mesh or 12-gage (2.69 mm) twisted mesh. Cubical celled gabions that are one meter high by one meter wide shall be made from 12-gage (2.69 mm) twisted mesh or welded mesh gages between 11–gage (3.05 mm) and 9–gage (3.76 mm) , inclusive.

Table 1	
CUBICAL-CELLED FACILITIES	
USA WIRE GAGE	MESH STYLE
12	Twisted Mesh
11 Min to 9 Max	Welded Mesh

Table 2	
MATTRESS-STYLE FACILITIES	
USA WIRE GAGE	MESH STYLE
12	Twisted Mesh
11	Welded Mesh

GABION MESH MATERIAL PROPERTIES

Characteristic	Test Designation	Requirement
Minimum tensile strength	ASTM A370	410 MPa
Wire Size	USA Steel Wire Gage	12
Wire Diameter (Minimum)	ASTM A641/A641 M ASTM A641/A641 M	2.69 mm 2.59 mm
Galvanizing, Zinc	ASTM A641/A641 M, Class 3 and ASTM A90 / A90M	230 g/m^2
Wire Size	USA Steel Wire Gage	11
Wire Diameter (Minimum)	ASTM A641/A641 M ASTM A641/A641 M	3.05 mm 2.95 mm
Galvanizing, Zinc	ASTM A641/A641 M, Class 3 and ASTM A90 / A90M	240 g/m^2
Wire Size	USA Steel Wire Gage	9
Wire Diameter (Minimum)	ASTM A641/A641 M ASTM A641/A641 M	3.76 mm 3.66 mm
Galvanizing, Zinc	ASTM A641/A641 M, Class 3 and ASTM A90 / A90M	270 g/m^2

Twisted-mesh wires shall form a uniform hexagonal pattern and shall be formed with a nonraveling twist. The area of the hexagonal opening shall not exceed the dimensions shown on the plans. Twisted-mesh gabion panels shall be manufactured from 12-gage (2.69 mm) wires with 10-gage (3.43 mm) selvage wires.

Welded-mesh wires shall form a grid pattern as shown on the plans. Welds shall be made by resistance welding. Welds and panels shall conform to the requirements in ASTM Designation: A185, except weld shears shall be 2.7 kN for 11-gage (3.05 mm) wires and 3.6 kN for 9-gage (3.76 mm) wires. Resistance welding after coating the wire with zinc will be acceptable, so that there are no large splashes, flakes or flashes of zinc at the weld.

Polyvinyl Chloride (PVC) Coating

External coating shall consist of a nonconductive material, primarily polyvinyl chloride (PVC). Mesh wire, standard tie wires, standard spiral binders, internal connecting wires, preformed stiffeners, and selvage wire shall be coated with the PVC material after the zinc coating is applied in conformance with the manufacturers specifications.

The PVC coating shall be evaluated by infrared (IR) spectral scan. The scan must closely match those of tested known acceptable products already on file at the Transportation Laboratory.

The minimum thickness of PVC which covers the wire shall be 0.38-mm, measured radially at any cross-section transverse to the wire length.

The PVC coating shall be complete by visual inspection. There shall be no nicks, cuts, holidays or abraded areas in the PVC coating of the mesh. Minor cuts, nicks, and other minor imperfections due to manufacturing, will be permitted along selvage-wrapped edges of twisted mesh. PVC will not be required to coat the ends of either style of mesh where the PVC has been trimmed along wire or panel edges during the normal manufacturing process.

PVC coating shall be resistant to degradation by ultraviolet (UV) radiation. A suitable, UV-resistant additive shall be blended with the PVC. This additive shall be identified on the Certificate of Compliance.

The color of the PVC shall be black. The color shall be resistant to fading when exposed to natural sunlight.

Joints

Standard tie wire and standard spiral binder shall conform to the definitions and requirements in ASTM Designation: A641/A641 M and shall conform to the following provisions:

Minimum Tensile Strength	ASTM A370	410 MPa
Tie Wire		
Wire Size (Minimum)	USA Steel Wire Gage	13.5
Wire Diameter (Minimum)	ASTM A641/A641 M ASTM A641/A641 M	2.19 mm 2.09 mm
Zinc Coating	ASTM A641/A641 M, Class 3 and ASTM A90 / A90M	220 g/m^2
Spirals		
Wire Size (Maximum)	USA Steel Wire Gage	9
Wire Diameter (Minimum)	ASTM A641/A641 M ASTM A641/A641 M	3.76 mm 3.66 mm
Zinc Coating	ASTM A641/A641 M, Class 3 and ASTM A90 / A90M	270 g/m^2

Spiral binders shall have a 75 mm to 85 mm separation between continuous, successive loops.

Alternative fasteners shall have the configurations, wire diameters, and other dimensions shown on the plans. Alternative fasteners shall conform to the definitions and requirements in ASTM Designation: A313/A313 M for "Stainless Steel Spring Wire" and shall be Tensile Type 302, Class 1.

Internal Connecting Wire

Internal connecting wires shall be 13.5-gage (2.19 mm) minimum. Each wire shall conform to the minimum requirements for standard tie wire in these special provisions and shall be installed in conformance with the provisions in these special provisions and as shown on the plans. Alternatively, at the Contractor's option, preformed stiffeners may be substituted for internal connecting wires. Preformed stiffener wire shall meet the requirements specified for standard tie wire and shall be installed in conformance with these special provisions and the manufacturers recommendations.

Rock Slope Protection Fabric

Rock slope protection fabric for use with gabions shall conform to the provisions in Section 88–1.04, "Rock Slope Protection Fabric," of the Standard Specifications and these special provisions.

Where gabions are used for downdrains, woven tape fabric shall be used in place of the rock slope protection fabric. The woven tape fabric shall conform to the requirements in ASTM Designation: D 4491, with a maximum permittivity of 0.10 per second.

Rock

Rock for filling gabions, which are greater than or equal to 0.5-m in height, shall vary in size and shall conform to the following:

Screen Size (mm)	Percent Passing
305	100
102	0-5

Rock for filling gabions, which are equal to 0.3-m in height, shall vary in size and shall conform to the following:

Screen Size (mm)	Percent Passing
203	100
102	0-5

Rock shall conform to the following:

Test	California Test No.	Requirement
Apparent Specific Gravity	206	1.5 max.
Absorption	206	20% max.
Durability Index	229	52 min.

The maximum unit mass of a rock-filled gabion shall be1050 kg/m^3. Verification of the 1050 kg/m^3 shall be performed when ordered by the Engineer. Verification shall be performed on the smallest standard gabion size to be used on the project. The rock supplied for the project shall be used for verification. Filling shall be done using the same method intended for actual construction. The mass of a rock-filled gabion shall be determined using available certified scales. The volume for calculating the unit mass shall be determined on the theoretical volume of the standard gabion which is rock-filled and weighed.

GRADING, EXCAVATION AND BACKFILL

Areas where gabions are to be placed shall be constructed to the lines and grades shown on the plans, as specified elsewhere in these special provisions, and as determined by the Engineer.

ROCK SLOPE PROTECTION FABRIC PLACEMENT

Rock slope protection fabric shall be placed in conformance with the provisions in Section 72-2.025, “Rock Slope Protection Fabric”. Rock slope protection fabric shall be placed on the subgrade, backslope, and sides of excavations. If earth fill is to be placed over the gabions, rock slope protection fabric shall be placed on top of the gabions, under the earth fill.

CONSTRUCTION

Gabions shall be assembled individually as empty units. Each gabion shall be manufactured with the necessary panels, properly spaced and secured, so that the panels can be rotated into position at the construction site with no additional tying of the rotation joint. The panels and diaphragms shall be rotated into position and joined along the vertical edges.

For twisted mesh, the joint shall be constructed using alternating double and single half hitches (locked loops) of 13.5-gage (2.19 mm) standard tie wire at 100-mm nominal spacing. Joints shall not be constructed with simple spiraling (looping without locking) of the standard tie wires.

When standard tie wire is used as a joint connector for welded-mesh, the joint shall be constructed using alternating double and single half hitches (locked loops) in every mesh opening along the joint. When 9-gage (3.76 mm) spiral binders are used, the spiral shall be placed so that the spiral binder passes through each mesh opening along the joint. Both ends of all 9-gage (3.76 mm) spiral binders shall be crimped to secure the spiral in place.

Temporary fasteners may be used to hold panels wherever gabion-to-gabion joints will be constructed. Temporary fasteners may remain in place.

At the Contractor’s option, interlocking fasteners or overlapping fasteners may be used for assembly of either the twisted mesh or welded mesh gabions. A fastener shall be placed in each mesh opening along the joint (a minimum of 10 fasteners per meter).

ASSEMBLY OF SUCCESSIVE GABIONS (GABION-TO-GABION JOINTS)

Empty gabions shall be set in place. Individually constructed empty gabions shall then be joined successively to the next empty gabion with 13.5-gage (2.19 mm) tie wire or 9-gage (3.76 mm) standard spiral binder before filling the gabion with rock. The 13.5-gage (2.19 mm) standard tie wire or 9-gage (3.76 mm) standard spiral binder shall secure, in one pass, all selvage or end wires of the panels of all adjacent gabions along the joint.

When forming successive gabion-to-gabion joints with alternative fasteners, there shall be one alternative fastener in each mesh opening. The alternative fastener shall contain and secure all the wires along the joint.

Gabion baskets shall be joined along the front, back, and ends, including the tops and bottoms of the adjacent gabions.

ASSEMBLY OF MULTIPLE LAYERED GABIONS

Multi-layered gabion configurations shall be stepped and staggered as shown on the plans or as designated by the Engineer.

When constructing multi-layered gabion configurations, each layer of gabions shall be joined to the underlying layer along the front, back, and ends.

ASSEMBLY OF SHEAR KEY GABIONS

Shear key gabions, or counterforts, shall be spaced as shown on the plans. Shear key gabions shall be tied to adjacent gabions in the manner specified for "Assembly of Successive Gabions (Gabion-to-Gabion Joints)" of these special provisions.

ASSEMBLY OF TRANSITIONAL GABIONS

To match the geometry of the planned gabion configuration, or to meet specific conditions, panels shall be folded, cut, or retied as shown on the plans or as directed by the Engineer.

FILLING WITH ROCK

Before filling each gabion with rock, all kinks and folds in the wire fabric shall be straightened and all successive gabions shall be properly aligned.

Rock shall be placed in the gabions to provide proper alignment, avoid bulges in the wire mesh, and provide a minimum of voids. All exposed rock surfaces shall have a smooth and neat appearance. Sharp rock edges shall not project through the wire mesh.

Internal connecting wires or preformed stiffeners shall be used to produce a flat, smooth external surface, when constructing with 0.5-m high or one meter high gabions. If the Engineer determines that there is excessive bulging or dimpling of the outside panels, the unit shall be reconstructed at the Contractor's expense.

When filling one meter high gabions, rock shall be placed in 2 nominal 0.33-m layers to allow placement of the 13.5-gage (2.19 mm) internal connecting wires. The wires shall be fastened as shown on the plans. Alternatively, preformed stiffeners may be installed at the one-third points in conformance with the recommendations of the manufacturer, to produce a smooth external surface.

When filling 0.5-m high gabions, one nominal 0.25-m layer of rock shall be placed to allow placement of a set of internal connecting wires or preformed stiffeners. The configuration of wires shall be similar to those used on the one meter high gabions, except there shall be only one set of internal connecting wires instead of the 2 sets of internal connecting wires or preformed stiffeners.

The last layer of rock shall slightly overfill the gabions so that the lid will rest on rock when the lid is closed.

CLOSURE OF LIDS

Lids shall be tied along the front, ends, and diaphragms in conformance with the provisions in "Assembly of Successive Gabions (Gabion-to-Gabion Joints)" of these special provisions.

MEASUREMENT

Gabions will be measured by the cubic meter as determined from the dimensions shown on the plans.

PAYMENT

The contract price paid per cubic meter for gabions (pvc) shall include full compensation for furnishing all labor, materials (including rock and rock slope protection fabric), tools, equipment, and incidentals, and for doing all the work involved in constructing gabions, complete, in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by he Engineer.

10-1.83 MISCELLANEOUS CONCRETE CONSTRUCTION

Minor concrete (colored paving), and concrete gutter lining around existing inlets to be adjusted or modified, shall conform to the provisions in Section 73, "Concrete Curbs and Sidewalks," and Section 90-10, "Minor Concrete" of the Standard Specifications and these special provisions.

If the Contractor elects to use the curing compound method for curing the concrete for minor concrete (colored paving), the curing compound shall be curing compound (6) as specified in Section 90-7.01B, "Curing Compound Method," of the Standard Specifications.

The curing compound shall be applied in a manner that will provide a complete coating of all exposed faces of the concrete surface.

The Beige color for colored paving is available for review by prospective bidders at the office of the Department of Transportation, Office of Landscape Architecture, 111 Grand Avenue, Oakland, Ca 94612. Portland cement concrete closely conforming to the color specified for colored paving are available through commercial concrete sources.

A sample of sufficient size, of the colored paving, to demonstrate the color, including integral color sealer and finishing , shall be submitted to the Engineer for written approval.

Colored paving shall not be placed on the project prior to approval by the Engineer of the samples prepared and submitted by the Contractor.

The respective type and color of concrete for colored paving shall be placed at the locations shown on the plans, struck off and compacted until a layer of mortar is brought to the surface. The concrete shall be screeded to the required grade and cross section and floated to a uniform surface.

The broom finish shall be applied to the surfaces while the concrete is still in the plastic stage of set.

The contract price paid per cubic meter for minor concrete (gutter lining or colored paving) shall include full compensation for furnishing all labor, materials (including welded wire fabric), tools, equipment, and incidentals, and for doing all the work involved in constructing gutter lining or colored paving, complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.84 MISCELLANEOUS IRON AND STEEL

Miscellaneous iron and steel shall conform to the provisions in Section 75, "Miscellaneous Metal," of the Standard Specifications and these special provisions.

Inlets, frames, and grates shown on the plans from NT Line Sta. 109+00 to 150+00 (Drainage System #1 through #35, inclusive) shall be galvanized per Section 75-1.05 of the Standard Specifications.

10-1.85 METAL (BARRIER MOUNTED SIGN)

Metal (barrier mounted sign) shall conform to the provisions for miscellaneous bridge metal in Section 75, "Miscellaneous Metal," of the Standard Specifications and these special provisions.

Attention is directed to “Welding Quality Control” elsewhere in these special provisions.

Cast-in-place inserts shall be the ferrule loop type.

All metal parts of anchorage devices shall be fabricated from stainless steel conforming to the requirements of ASTM Designation: A 276, Type 304 or 316.

10-1.86 MISCELLANEOUS METAL (BRIDGE)

Miscellaneous metal (bridge) shall conform to the provisions for miscellaneous bridge metal in Section 75, "Miscellaneous Metal," of the Standard Specifications and these special provisions.

Attention is directed to "Welding Quality Control" of these special provisions.

Unless otherwise specified, materials shall conform to the following specifications:

MATERIAL	SPECIFICATION
Steel bars, plates and shapes	ASTM Designation: A 36/A 36M or A 575, A 576 (AISI or M Grades 1016 through 1030 except Grade 1017)
Other parts for general applications	Commercial quality
Steel fastener components for general applications:	
Bolts and studs	ASTM Designation: A 307
Headed anchor bolts	ASTM Designation: A 307, Grade B, including S1 supplementary requirements
Nonheaded anchor bolts	ASTM Designation: A 307, Grade C, including S1 supplementary requirements and S1.6 of AASHTO Designation: M 314 supplementary requirements or AASHTO Designation: M 314, Grade 36 or 55, including S1 supplementary requirements
High-strength bolts and studs which include threaded rods and high-strength nonheaded anchor bolts	ASTM Designation: A 449, Type 1
Nuts	ASTM Designation: A 563, including Appendix X1[(a)]
Washers	ASTM Designation: F 844
Components of high-strength steel fastener assemblies for use in structural steel joints:	
Bolts	ASTM Designation: A 325, Type 1
Tension control bolts	ASTM Designation: F 1852, Type 1
Nuts	ASTM Designation: A 563, including Appendix X1[(a)]

Hardened washers Direct tension indicators	ASTM Designation: F 436, Type 1, Circular, including S1 supplementary requirements ASTM Designation: F 959, Type 325, zinc-coated
Stainless steel fasteners for general applications: Bolts, screws, nuts and studs which include threaded rods and nonheaded anchor bolts Washers	Alloys 304 or 316 ASTM Designation: F 593 or F 738M ASTM Designation:A 240 and ANSI B 18.22M
Carbon-steel castings	ASTM Designation: A 27/A 27M, Grade 65-35 [450-240], Class 1
Malleable iron castings	ASTM Designation: A 47, Grade 32510 or A 47M, Grade 22010
Gray iron castings	ASTM Designation: A 48, Class 30B
Ductile iron castings	ASTM Designation: A 536, Grade 65-45-12
Cast iron pipe	Commercial quality standard soil
Steel pipe	Commercial quality welded
(a) Zinc-coated nuts that will be tightened beyond snug or wrench tight shall be furnished with a dry lubricant conforming to Supplementary Requirement S2 in ASTM Designation: A 563.	

Miscellaneous metal (bridge) shall consist of the miscellaneous bridge metal items listed in Section 75-1.03, "Miscellaneous Bridge Metal," of the Standard Specifications, and the following:

End steel plate, high strength rod and hardware.
Standard Pipe Sleeve

Section 75-1.03, "Miscellaneous Bridge Metal," of the Standard Specifications is amended by adding the following paragraph after paragraph 3:

High-strength bolted connections shall conform to the provisions for high-strength steel fasteners and for bolted connections in Section 55, "Steel Structures."

Self-tapping screws shall be hex-head, stainless steel or monel metal, installed in holes drilled to fit the self-tapping screws.

For drainage piping NPS 8 or smaller, which is: (1) enclosed in a box girder cell and exposed for a length not greater than 6 m, or (2) encased in concrete, the Contractor shall have the option of substituting polyvinyl chloride (PVC) plastic pipe, with the same diameter and minimum bend radius as shown on the plans, for welded steel pipe. The support spacing for PVC plastic pipe shall be 2.5 m, maximum. The polyvinyl chloride (PVC) plastic pipe shall be Schedule 40 conforming to the requirements of ASTM Designation: D 1785. If polyvinyl chloride (PVC) plastic pipe is substituted for welded steel pipe, the quantity of drainage piping will be computed on the basis of the dimensions and details shown on the plans and no change in the quantities to be paid for will be made because of the use by the Contractor of polyvinyl chloride (PVC) plastic pipe.

Cast-in-place inserts shall be the ferrule loop type.

All metal parts of anchorage devices shall be fabricated from stainless steel conforming to the requirements of ASTM Designation: A 276, Type 304 or 316.

10-1.87 CHAIN LINK FENCE

Chain link fence shall be Types CL-0.9, CL-1.2 and CL-1.8 and shall conform to the provisions in Section 80, "Fences," of the Standard Specifications.

10-1.88 METAL BEAM GUARD RAILING

Metal beam guard railing shall conform to the provisions in Section 83-1, "Railings," of the Standard Specifications and these special provisions.

Attention is directed to "Order of Work" of these special provisions.

Line posts and blocks shall be wood.

The ninth, eleventh and twelfth paragraphs in Section 83-1.02B, "Metal Beam Guard Railing," of the Standard Specifications are amended to read:

The grades and species of wood posts and blocks shall be No. 1 timbers (also known as No. 1 structural) Douglas fir or No. 1 timbers Southern yellow pine. Wood posts and blocks shall be graded in conformance with the provisions in Section 57-2, "Structural Timber," except allowances for shrinkage after mill cutting shall in no case exceed 5 percent of the American Lumber Standards minimum sizes, at the time of installation.

Wood posts and blocks shall be pressure treated after fabrication as provided in Section 58, "Preservative Treatment of Lumber, Timber and Piling," with creosote, creosote coal tar solution, creosote-petroleum solution (50-50), pentachlorophenol in hydrocarbon solvent, copper naphthenate, ammoniacal copper arsenate, or ammoniacal copper zinc arsenate. In addition to the preservatives listed above, Southern yellow pine may also be pressure treated with chromated copper arsenate. When other than one of the creosote processes is used, blocks shall have a minimum retention of 6.4 Kg/m^3, and need not be incised.

If copper naphthenate, ammoniacal copper arsenate, chromated copper arsenate, or ammoniacal copper zinc arsenate is used to treat the wood posts and blocks, the bolt holes shall be treated as follows:

Before the bolts are inserted, bolt holes shall be filled with a grease, recommended by the manufacturer for corrosion protection, which will not melt or run at a temperature of 65°C.

Metal beam guard rail elements and any required backup plates, terminal sections, end sections, and return sections shall conform to the requirements of Type 2 W-Beam as shown in AASHTO Designation: M 180.

TERMINAL SYSTEM (TYPE ET).—Terminal system (Type ET) shall be furnished and installed as shown on the plans, and as specified in these special provisions.

Terminal system (Type ET) shall be an ET-2000 (4-tube system) extruder terminal as manufactured by Syro, Inc., a Trinity Industries Company, and shall include all the items detailed for terminal system (Type ET) shown on the plans.

Arrangements have been made to insure that any successful bidder can obtain the ET-2000 (4-tube system) extruder terminal from the manufacturer, Syro, Inc., a Trinity Industries Company, P.O. Box 99, 950 West 400S, Centerville, UT 84014, Telephone (800) 772-7976. The price quoted by the manufacturer for the ET-2000 (4-tube system) extruder terminal, FOB Centerville, Utah is $1,305.00, not including sales tax.

The above price will be firm for all orders placed on or before June 30, 2000, provided delivery is accepted within 90 days after the order is placed.

The Contractor shall provide the Engineer with a Certificate of Compliance from the manufacturer in conformance with the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications. The Certificate of Compliance shall certify that terminal systems (Type ET) conform to the contract plans and specifications, conform to the prequalified design and material requirements, and were manufactured in conformance with the approved quality control program.

The terminal system (Type ET) shall be installed in conformance with the manufacturer's installation instructions and these requirements. At the Contractor's option, steel foundation tubes with soil plates attached, shall be either driven, with or without pilot holes, or placed in drilled holes. Any space around the steel foundation tubes shall be backfilled with selected earth, free of rock, placed in layers approximately 100 mm thick and each layer shall be moistened and thoroughly compacted. The wood terminal posts shall be inserted into the steel foundation tubes by hand and shall not be driven. Before the wood terminal posts are inserted, the inside surfaces of the steel foundation tubes to receive the wood posts shall be coated with a grease which will not melt or run at a temperature of 65°C or less. The edges of the wood terminal posts may be slightly rounded to facilitate insertion of the post into the steel foundation tubes.

Surplus excavated material remaining after the terminal system (Type ET) has been constructed shall be disposed of in a uniform manner along the adjacent roadway as directed by the Engineer.

The terminal system will be measured and paid for by the unit as terminal system (Type ET). The quantity will be determined from actual count in place in the completed work.

The contract unit price paid for terminal system (Type ET) shall include full compensation for furnishing all labor, materials, tools, equipment and incidentals, and for doing all work involved in furnishing and installing the terminal system (Type ET), complete in place, including excavation, backfill and disposal of surplus material and connecting the terminal system to new or existing barrier and railing, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

TERMINAL SYSTEM (TYPE SRT).—Terminal system (Type SRT) shall be furnished and installed as shown on the plans, and as specified in these special provisions.

Terminal system (Type SRT) shall be a SRT-350 Slotted Rail Terminal as manufactured by Syro, Inc., a Trinity Industries Company, and shall include all the items detailed for terminal system (Type SRT) shown on the plans.

Arrangements have been made to insure that any successful bidder can obtain the SRT-350 Slotted Rail Terminal from the manufacturer, Syro, Inc., a Trinity Industries Company, P.O. Box 99, 950 West 400S, Centerville, UT 84014, Telephone (800) 772-7976. The price quoted by the manufacturer for the SRT-350 Slotted Rail Terminal, FOB Centerville, Utah is $ 865.00, not including sales tax.

The above price will be firm for orders placed on or before June 30, 2000 provided delivery is accepted within 90 days after the order is placed.

The Contractor shall provide the Engineer with a Certificate of Compliance from the manufacturer in conformance with the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications. The Certificate of Compliance shall certify that terminal systems (Type SRT) conform to the contract plans and specifications, conform to the prequalified design and material requirements and were manufactured in conformance with the approved quality control program.

The terminal system (Type SRT) shall be installed in conformance with the manufacturer's installation instructions and these requirements. At the Contractor's option, steel foundation tubes with soil plates attached, shall be either driven, with or without pilot holes, or placed in drilled holes. Any space around the steel foundation tubes shall be backfilled with selected earth, free of rock, placed in layers approximately 100 mm thick and each layer shall be moistened and thoroughly compacted. Wood terminal posts shall be inserted into the steel foundation tubes by hand. Before the wood terminal posts are inserted, the inside surfaces of the steel foundation tubes to receive the wood posts shall be coated with a grease which will not melt or run at a temperature of 65°C or less. The edges of the wood terminal posts may be slightly rounded to facilitate insertion of the post into the steel foundation tubes.

Surplus excavated material remaining after the terminal system (Type SRT) has been constructed shall be disposed of in a uniform manner along the adjacent roadway as directed by the Engineer.

The quantity of terminal systems (Type SRT) will be measured as units determined from actual count in place in the completed work.

The contract unit price paid for terminal system (Type SRT) shall include full compensation for furnishing all labor, materials, tools, equipment and incidentals, and for doing all work involved in furnishing and installing terminal system (Type SRT), complete in place, including excavation, backfill and disposal of surplus material and connecting the terminal system to new or existing metal beam guard railing, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

TERMINAL SYSTEM (TYPE CAT).—Terminal system (Type CAT) shall be furnished and installed as shown on the plans, and as specified in these special provisions.

Terminal system (Type CAT) shall be a CAT-350 Crash Cushion Attenuating Terminal as manufactured by Syro, Inc., a Trinity Industries Company, and shall include all the items detailed for terminal system (Type CAT) shown on the plans.

Excluding the terminal system (Type CAT) backup, arrangements have been made to insure that any successful bidder can obtain the CAT-350 Crash Cushion Attenuating Terminal from the manufacturer, Syro, Inc., a Trinity Industries Company, P.O. Box 99, 950 West 400S, Centerville, UT 84014, Telephone (800) 772-7976. The price quoted by the manufacturer for the CAT-350 Crash Cushion Attenuating Terminal, FOB Centerville, Utah is $ 3000.00, not including sales tax.

The above price will be firm for orders placed on or before June 30, 2000, provided delivery is accepted within 90 days after the order is placed.

The Contractor shall provide the Engineer with a Certificate of Compliance from the manufacturer in conformance with the provisions in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications. The Certificate of Compliance shall certify that the terminal system (Type CAT) conforms with the contract plans and specifications, conforms to the prequalified design and material requirements, and was manufactured in conformance with the approved quality control program.

The terminal system (Type CAT) shall be installed in conformance with the manufacturer's installation instructions and these requirements. At the Contractor's option, steel foundation tubes with soil plates attached, shall be either driven, with or without pilot holes, or placed in drilled holes. Any space around the steel foundation tubes shall be backfilled with selected earth, free of rock, placed in layers approximately 100 mm thick and each layer shall be moistened and thoroughly compacted. Wood posts shall be inserted into the steel foundation tubes by hand. Before the wood posts are inserted, the inside surfaces of the steel foundation tubes to receive the wood posts shall be coated with a grease which will not melt or run at a temperature of 65°C or less. The edges of the wood posts may be slightly rounded to facilitate insertion of the post into the steel foundation tubes.

Surplus excavated material remaining after the terminal system (Type CAT) has been constructed shall be disposed of in a uniform manner along the adjacent roadway as directed by the Engineer.

Terminal system (Type CAT) will be measured as units determined from actual count in place in the completed work.

The contract unit price paid for terminal system (Type CAT) shall include full compensation for furnishing all labor, materials, tools, equipment and incidentals, and for doing all the work involved in furnishing and installing the terminal system (Type CAT), complete in place, including excavation, backfill and disposal of surplus material, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-1.89 CHAIN LINK RAILING

Chain link railing shall conform to the provisions in Section 83-l, "Railings," of the Standard Specifications and these special provisions.

The chain link fabric shall be 9-gage (3.76 mm), Type IV, Class B, bonded vinyl coated fabric, conforming to the requirements in AASHTO Designation: M 181.

The strength of the bond between the coating material and steel of the bonded vinyl coated chain link fabric shall be equal to or greater than the cohesive strength of the polyvinyl chloride (PVC) coating material.

10-1.90 CONCRETE BARRIER

Concrete barriers shall conform to the provisions in Section 83-2, "Barriers," of the Standard Specifications and these special provisions.

The provisions of the third paragraph in Section 83-2.02D(4), "Finishing," of the Standard Specifications, as amended herein, shall not apply.

Concrete barrier markers shall be as specified in "Approved Traffic Products" of these special provisions. At the locations designated on the plans, concrete barrier markers shall be cemented to the barrier in conformance with the manufacturer's recommendations.

The subparagraphs in the second paragraph in Section 83-2.02D(l), "General," of the Standard Specifications are amended to read:

a. For concrete barriers Type 50 series and Type 60 series, the top shall not vary more than 6 mm from the edge of the straightedge and the faces shall not vary more than 12 mm from the edge of the straightedge.
b. For concrete barriers other than Type 50 series and Type 60 series, both the top and faces shall not vary more than 6 mm from the edge of the straightedge.

The eighth paragraph in Section 83-2.02D(l), "General," of the Standard Specifications is amended to read:

Granular material for backfill between the 2 walls of concrete barriers (Type 50E), (Type 60E), (Type 60GE), and (Type 60SE), as shown on the plans, shall be placed without compaction.

The first, second, third, and eighth paragraphs in Section 83-2.02D(2), "Materials," of the Standard Specifications are amended to read:

83-2.02D(2) Materials.—Type 50 and Type 60 series concrete barriers shall be constructed of minor concrete conforming to the provisions in Section 90-10, "Minor Concrete," except as follows:

a. The maximum size of aggregate used for extruded or slip-formed concrete barriers shall be at the option of the Contractor, but in no case shall the maximum size be larger than 37.5 mm nor smaller than 9.5 mm.
b. If the 9.5-mm maximum size aggregate grading is used to construct extruded or slip-formed concrete barriers, the cement content of the minor concrete shall be not less than 400 kg/m^3.

Concrete barriers other than Type 50 and Type 60 series shall be constructed of Class 2 concrete conforming to the provisions in Section 90, "Portland Cement Concrete."

The concrete paving between the tops of the 2 walls of concrete barrier (Type 50E), (Type 60E), (Type 60GE), and (Type 60SE) and the optional concrete slab at the base between the 2 walls of concrete barrier (Type 50E), (Type 60E), (Type 60GE), and (Type 60SE) shall be constructed of minor concrete conforming to the provisions of Section 90-10, "Minor Concrete," except that the minor concrete shall contain not less than 300 kg of cement per cubic meter.

Granular material for backfill between the 2 walls of concrete barrier (Type 50E), (Type 60E), (Type 60GE), and (Type 60SE) shall be earthy material suitable for the purpose intended, having no rocks, lumps or clods exceeding 37.5 mm in greatest dimension.

The first and second paragraphs in Section 83-2.02D(3), "Construction Methods," of the Standard Specifications are amended to read:

83-2.02D(3) Construction Methods.—Type 50 series and Type 60 series concrete barriers shall be constructed by either the "cast-in-place with fixed forms" method or the "extrusion or slip-form" method or a combination thereof, at the Contractor’s option.

Concrete barriers other than Type 50 series and Type 60 series shall be constructed by the "cast-in-place with fixed forms" method.

Section 83-2.02D(4), "Finishing," of the Standard Specifications is amended to read:

83-2.02D(4) Finishing.—The surface finish of concrete barriers Type 50 series and Type 60 series, prior to the application of the curing compound, shall be free from surface pits larger than 25 mm in diameter and shall be given a final soft brush finish with strokes parallel to the line of the barriers. Finishing with a brush application of grout will not be permitted.

To facilitate finishing, fixed forms for cast-in-place concrete barriers Type 50 series and Type 60 series, shall be removed as soon as possible after the concrete has set enough to maintain the shape of the barrier without support.

Not less than 7 days after placing, exposed surfaces of concrete barriers, Type 50 series and Type 60 series, shall receive a light abrasive blast finish so that a uniform appearance is achieved.

The final surface finish of concrete barriers other than Type 50 series and Type 60 series shall be Class 1 Surface Finish as specified in Section 51-1.18B, "Class 1 Surface Finish." Alternative final surface finish methods proposed by the Contractor shall be submitted in writing and shall not be used unless approved by the Engineer.

Section 83-2.02D(5), "Curing," of the Standard Specifications is amended to read:

83-2.02D(5) Curing.—Exposed surfaces of concrete barriers shall be cured with the non-pigmented curing compound (6) as provided in Section 90-7.01B, "Curing Compound Method." At the Contractor's option, the formed surfaces of concrete barriers, which are on bridges or walls but which do not support soundwalls, may be cured as provided in Section 90-7.01D, "Forms-In-Place Method," except the forms shall be retained in place for a minimum period of 12 hours after the concrete has been placed. When curing Type 50 series and Type 60 series concrete barriers, curing compound shall be applied by a mechanical sprayer capable of applying the curing compound to at least one entire side and the top of the concrete barrier in one application at a uniform rate of coverage. The spray shall be adequately protected against wind.

The ninth and tenth paragraphs in Section 83-2.03, "Measurement," of the Standard Specifications are amended to read:

Concrete barriers, except (Type 50E), (Type 60E), (Type 60GE) and (Type 60SE), will be measured along the top of the barrier.

Concrete barriers (Type 50E), (Type 60E), (Type 60GE), and (Type 60SE) will be measured once along the center-line between the 2 walls of the barrier.

The fourth paragraph in Section 83-2.04, "Payment," of the Standard Specifications is amended to read:

The contract prices paid per meter for concrete barrier of the type or types listed in the Engineer's Estimate shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in constructing the concrete barriers, complete in place, including bar reinforcing steel, steel dowels and drilling and bonding dowels in structures, hardware for steel plate barrier, miscellaneous metal, concrete barrier markers, excavation, backfill (including concrete paving, granular material and concrete slab used as backfill in concrete barriers (Type 50E), (Type 60E), (Type 60GE), and (Type 60SE)), and disposing of surplus material and for furnishing, placing, removing and disposing of the temporary railing for closing the gap between existing barrier and the concrete barrier being constructed, as shown on the plans, as specified in these specifications and the special provisions, and as directed by the Engineer.

Type 25A concrete barriers will be measured and paid for as concrete barrier (Type 25).

Type 60A (Modified) concrete barriers will be measured and paid for as concrete barrier (Type 60A).

Bar reinforcing steel for use in concrete barriers shall conform to the provisions in "Epoxy-coated Reinforcement," elsewhere in these special provisions.

At the Contractor's option, epoxy-coated bar reinforcement for use in concrete barriers conforming to the following requirements may be used:

Bar reinforcement to be epoxy-coated shall conform to the ASTM Designation and grade required or permitted by Section 52-1.02A, "Bar Reinforcement," for the location or type of structure involved. The epoxy-coated bar reinforcement shall conform to the provisions of ASTM Designation: A 775/A 775M, except as provided herein. Fabrication and jobsite handling of the epoxy-coated bar reinforcement shall conform to the provisions of ASTM Designation: D 3963/D 3963M, except as provided herein.

Appendix X1, "Guidelines For Job-Site Practices," of ASTM Designation: A 775/A 775M shall apply except as provided herein. The term "shall" shall replace the term "should" in this appendix. Section X1.2 shall not apply.

Coatings shall be light green in color.

Except for field welding of butt splices, all welding of reinforcement shall be complete prior to epoxy coating the reinforcement.

When any portion of a reinforcing bar requires epoxy coating, the entire bar shall be coated.

Within areas where epoxy-coated reinforcement is required, tie wire and bar chairs or other metallic devices used to secure or support the reinforcement shall be plastic-coated or epoxy-coated to prevent corrosion of the devices or damage to the coated reinforcement.

Prior to coating, the Contractor shall furnish to the Transportation Laboratory a representative 110 g sample from each batch of epoxy coating material to be used. Each sample shall be packaged in an airtight container identified with the manufacturer's name and batch number.

Two 700-mm long samples of coated bar reinforcement from each size and from each load shipped to the jobsite shall be furnished to the Engineer for testing. These samples shall be representative of the material furnished. These

samples, as well as any additional random samples taken by the Engineer, may be tested for specification compliance. Such additional sampling, and all tests performed by the Engineer, may be performed at any location deemed appropriate by the Engineer. Failure of any sample to meet the requirements of the specifications will be cause for rejection.

If any bar tested for coating thickness or for flexibility of coating fails to meet the requirements for coated bars in Section 8 of ASTM Designation: A 775/A 775M, 2 retests on random samples taken from bars represented by the failed test will be conducted for each failed test. If the results of both retests meet the specified requirements, the coated bars represented by the samples may be certified as meeting the test requirements.

Epoxy-coated reinforcement shall be covered with an opaque polyethylene sheeting or other suitable protective material to protect the reinforcement from exposure to sunlight, salt spray and weather. For stacked bundles, the protective covering shall be draped around the perimeter of the stack. The covering shall be adequately secured; however, it should allow for air circulation around the reinforcement to prevent condensation under the covering. Epoxy-coated reinforcement shall not be stored within 300 meters of ocean or tidal water for more than 60 days.

All visible damage to the coatings caused by shipping, handling or installation shall be repaired as required for repairing coating damaged prior to shipment as specified in ASTM Designation: A 775/A 775M. When the extent of coating damage prior to repair exceeds 2 percent of the bar surface area in any 300-mm length, repair of the bar will not be allowed and the coated bar will be rejected.

The patching material and process shall be suitable for field application. The patching material shall be prequalified as required for the coating material and shall be either identified on the container as a material compatible with the bar reinforcement coating, or shall be accompanied by a Certificate of Compliance conforming to Section 6-1.07, "Certificates of Compliance," of the Standard Specifications certifying that the material is compatible with the bar reinforcement coating. Damaged areas shall be patched in accordance with the patching material manufacturer's recommendations. If damage to a bar occurs during field bending the area shall be patched immediately with the prequalified patching material.

Except for lap splices, splices for epoxy-coated bar reinforcement shall be coated with a corrosion protection covering that is on the Department's list of approved products. The covering shall be installed in accordance with the manufacturer's recommendations and as directed by the Engineer. The list is available from the Office of Materials Engineering and Testing Services, 5900 Folsom Boulevard, Sacramento, CA 95819, telephone (916) 227-7000.

A Certificate of Compliance conforming to the provisions in Section 6-1.07, "Certificates of Compliance," shall be furnished for each shipment of epoxy-coated bar reinforcement certifying that the coated bars conform to the requirements of ASTM Designation: A 775/A 775M and Section 52-1.02B, "Epoxy-coated Bar Reinforcement." The Certificate of Compliance shall include all the certifications specified in ASTM Designation: A 775/A 775M and a statement that the coating material has been prequalified by acceptance testing performed by the Valley Forge Laboratories, Inc., Devon, Pennsylvania.

10-1.91 DOUBLE THRIE BEAM BARRIER

Double thrie beam barrier shall conform to the provisions in Section 83-2, "Barriers," of the Standard Specifications and these special provisions.

Attention is directed to "Order of Work" of these special provisions.

Rail elements and any required backup plates, terminal sections, terminal connectors, and return sections shall conform to the requirements of Type 2 Thrie Beam in AASHTO Designation: M 180.

10-1.92 CRASH CUSHION, SAND FILLED

Sand filled crash cushions shall be furnished and installed as shown on the plans, and as specified in the Standard Specifications and these special provisions.

A sand filled crash cushion shall consist of a grouping of sand filled modules.

At the Contractor's option, modules for use in sand filled crash cushions shall be either Energite III Inertial Modules, Fitch Inertial Modules manufactured after March 31, 1997, or equal:

Energite III Inertial Modules manufactured by Energy Absorption Systems, Inc. One East Wacker Drive, Chicago, IL 60601-2076, Telephone 1-312-467-6750, FAX 1-800-770-6755.

Distributor (Northern): Traffic Control Service, Inc. 8585 Thys Court, Sacramento, CA 95828 Telephone 1-800-884-8274, FAX 1-916-387-9734

Distributor (Southern): Traffic Control Service, Inc. 1881 Betmor Lane, Anaheim, CA 92805 Telephone 1-800-222-8274, FAX 1-714-937-1070.

Fitch Inertial Modules National Distributor: Roadway Safety Service, Inc. 1050 North Rand Road, Wauconda, IL 60084, Telephone 1-800-426-0839, FAX 1-847-487-9820.

Distributor: Singletree Sales Company 1533 Berger Drive, San Jose, CA 95112 Telephone 1-800-822-7735, FAX 1-408-287-1929.

Modules contained in the crash cushion shall be of the same type at each location. The color of modules shall be the standard yellow color as furnished by the vendor, with black lids. The exterior components of the modules shall be formulated or processed to resist deterioration from ambient ultraviolet rays. The modules shall exhibit good workmanship free from structural flaws and objectionable surface defects.

The Contractor shall provide the Engineer with a Certificate of Compliance from the manufacturer in accordance with the provisions of Section 6-1.07, "Certificates of Compliance," of the Standard Specifications. The Certificate of Compliance shall certify that the crash cushions comply with the contract plans and specifications, conform to the prequalified design and material requirements, and were manufactured in accordance with the approved quality control program.

Sand for filling the modules shall be clean washed concrete sand of commercial quality. At the time of placing in the modules, the sand shall contain not more than 7 percent water, as determined by California Test 226.

Modules placed on bridge decks shall be provided with positioning blocks fastened to the deck surface. Positioning blocks shall be shaped as segments of a ring and placed along the inner or outer periphery of the module wall. A minimum of 2 blocks, a minimum of one-sixth of a ring in length shall be provided for each module. Positioning blocks and fasteners shall be of a material that is corrosion and water resistant.

Module cylinders shall be filled with sand in accordance with the manufacturer's directions, and to the sand capacity in kilograms for each module as shown on the plans.

Lids shall be securely attached as recommended by the manufacturer.

A Type R or P marker panel shall be attached to the front of the crash cushion as shown on the plans, when the closest point of crash cushion array is within 3.6 m of the traveled way. The marker panel, when required, shall be firmly fastened to the crash cushion with commercial quality hardware or by other methods approved by the Engineer.

Sand filled crash cushions, regardless of the number of modules required in each sand filled crash cushion, will be measured and paid for by the unit as crash cushion, sand filled. The quantity to be paid for will be determined from actual count of the units in place in the completed work.

The contract unit price paid for crash cushion, sand filled shall include full compensation for furnishing all labor, materials (including sand and marker panels), tools, equipment and incidentals, and for doing all the work involved in furnishing and installing the crash cushion complete in place, as shown on the plans, as specified in these special provisions, and as directed by the Engineer.

10-1.93 THERMOPLASTIC TRAFFIC STRIPES AND PAVEMENT MARKINGS

Thermoplastic traffic stripes (traffic lines) and pavement markings shall conform to the provisions in Section 84, "Traffic Stripes and Pavement Markings," of the Standard Specifications and these special provisions.

The second and third sentences of Section 84-2.02, "Materials," of the Standard Specifications are amended to read:

> Glass beads to be applied to the surface of the molten thermoplastic material shall conform to the requirements of State Specification 8010-004 (Type II).
>
> State Specifications for thermoplastic material and glass beads may be obtained from the Transportation Laboratory, 5900 Folsom Boulevard, Sacramento, CA 95819-4612, Telephone 916-227-7289.

Thermoplastic material for traffic stripes shall be applied at a minimum thickness of 2.03 mm.

At the option of the Contractor, permanent striping tape as specified in "Approved Traffic Products" of these special provisions, may be placed instead of the thermoplastic traffic stripes and pavement markings specified herein, except that 3M, "Stamark" Series A320 Bisymetric Grade, manufactured by the 3M Company, shall not be used. Pavement tape, if used, shall be installed in conformance with the manufacturer's specifications. If pavement tape is placed instead of thermoplastic traffic stripes and pavement markings, the pavement tape will be measured and paid for as thermoplastic traffic stripe and thermoplastic pavement marking.

10-1.94 PAINT TRAFFIC STRIPES AND PAVEMENT MARKINGS

Painting traffic stripes (traffic lines) and pavement markings shall conform to the provisions in Section 84, "Traffic Stripes and Pavement Markings," of the Standard Specifications and these special provisions.

The subparagraphs of the first paragraph in Section 84-3.02, "Materials," of the Standard Specifications are amended to read:

	State Specification No.
Solvent Borne, Acrylic Copolymer Traffic Line.—White, Yellow and Black	PT-170-A
Water Borne, Traffic Line.—White, Yellow and Black	8010-20A

The second and third paragraphs in Section 84-3.02, "Materials," of the Standard Specifications are amended to read:

> Glass beads shall conform to State Specification 8010-004 (Type II).
>
> State Specifications for traffic paint and glass beads may be obtained from the Transportation Laboratory, 5900 Folsom Boulevard, Sacramento, CA 95819-4612, Telephone 916-227-7289.

At the option of the Contractor, permanent striping tape as specified in "Approved Traffic Products" of these special provisions, may be placed instead of the painted traffic stripes and pavement markings specified herein, except that 3M, "Stamark" Series A320 Bisymetric Grade, manufactured by the 3M Company, shall not be used. Pavement tape, if used, shall be installed in conformance with the manufacturer's specifications. If pavement tape is placed instead of painted traffic stripes and pavement markings, the pavement tape will be measured and paid for as paint traffic stripe and paint pavement marking of the number of coats designated in the Engineer's Estimate.

10-1.95 PAVEMENT MARKERS

Pavement markers shall conform to the provisions in Section 85, "Pavement Markers," of the Standard Specifications and these special provisions.

Attention is directed to "Traffic Control System For Lane Closure" in these special provisions regarding the use of moving lane closures during placement of pavement markers with bituminous adhesive.

SECTION 10-2. HIGHWAY PLANTING AND IRRIGATION SYSTEMS

10-2.01 GENERAL

The work performed in connection with highway planting and irrigation systems shall conform to the provisions in Section 20, "Erosion Control and Highway Planting," of the Standard Specifications and these special provisions.

The Contractor shall notify the Engineer not less than 72 hours prior to requiring initial access to the existing irrigation controllers. When the Engineer determines that access to the controllers is required at other times, arrangements will be made to provide this access.

PROGRESS INSPECTIONS.--Progress inspections will be performed by the Engineer for completed highway planting and irrigation system work at designated stages during the life of the contract.

Progress inspections will not relieve the Contractor of his responsibility for installation in accordance with the special provisions, plans and Standard Specifications. Work within an area shall not progress beyond each stage until the inspection has been completed; corrective work has been performed; and the work is approved, unless otherwise permitted by the Engineer.

The requirements for progress inspections will not preclude additional inspections of work by the Engineer at any time during the life of the contract.

The Contractor shall notify the Engineer in writing, at least 4 working days prior to completion of the work for each stage of an area and shall allow a minimum of 3 working days for the inspection.

Progress inspections will be performed at the following stages of work:

PRESSURE TESTING OF PIPELINES.--During pressure testing of the pipelines on supply side of control valves.

TESTING OF CONDUCTORS.--During testing of low voltage conductors.

10-2.02 EXISTING HIGHWAY IRRIGATION FACILITIES

In addition to the provisions in Section 20, "Erosion Control and Highway Planting," of the Standard Specifications, the work performed in connection with the various existing highway irrigation system facilities shall conform to the provisions in Section 15, "Existing Highway Facilities," of the Standard Specifications and these special provisions.

Existing irrigation facilities shown on the plans or specified in these special provisions to be removed or relocated shall remain in place until their use, as determined by the Engineer, is no longer required.

Existing irrigation facilities that are to remain, or are to be maintained or relocated as part of this contract, shall be protected from damage. If the Contractor's operations damage the existing irrigation facilities, the Contractor shall, at the Contractor's expense, repair or replace the damaged facilities as follows:

Repair or replacement of damaged facilities shall be completed within 10 working days of the damage.

Replaced irrigation facilities shall be new, and of equal or better quality than the damaged facility. Replacement irrigation facilities shall be compatible with the irrigation systems to remain.

After repair or replacement of the facilities is complete, the Contractor shall demonstrate to the Engineer that the repaired or replaced facilities operate properly. When remote control valves are repaired or replaced, the valves shall be tested with the irrigation controller in the automatic mode.

10-2.02A CHECK AND TEST EXISTING IRRIGATION FACILITIES

Existing irrigation facilities that are to remain or be relocated, and that are within areas where clearing and grubbing or earthwork operations are to be performed, shall be checked for missing or damaged components and proper operation prior to performing the operations. Existing irrigation facilities outside of work areas that are affected by the construction work shall also be checked for proper operation.

The Contractor shall submit a written list of existing irrigation system deficiencies to the Engineer within 5 working days after checking the existing facilities.

Deficiencies found during checking existing irrigation facilities shall be corrected by the Contractor as directed by the Engineer. Corrective work ordered by the Engineer will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications.

Repairs to the existing irrigation facilities ordered by the Engineer after checking and testing the facilities, and any further repairs required thereafter as ordered by the Engineer, except as otherwise provided under "Existing Highway Irrigation Facilities" elsewhere in these special provisions, will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications.

Full compensation for checking and testing existing irrigation facilities, including testing existing backflow preventers, shall be considered as included in the contract price paid per meter for NPS 3 supply line (Bridge) and no additional compensation will be allowed therefor.

10-2.02B REMOVE EXISTING IRRIGATION FACILITIES

Existing irrigation facilities to be removed, shall be removed and disposed of, except for facilities that are more than 150 mm below finished grade may be abandoned in place. Removed facilities shall be disposed of outside the highway right of way in accordance with the provisions in Section 7-1.13 of the Standard Specifications.

Immediately after disconnecting an existing irrigation facility to be removed or abandoned from an existing facility to remain, the remaining facility shall be capped.

Full compensation for removing and disposing of existing irrigation facilities, and abandoning existing irrigation facilities, shall be considered as included in the contract price paid per meter for extend 200 mm conduit and no separate payment will be made therefor.

10-2.02C RELOCATE IRRIGATION CONTROLLER

Relocating existing irrigation controller shall be as shown on the plans or as specified in these special provisions.

Relocate irrigation controller shall consist of relocating the existing controller, controller enclosure and controller enclosure cabinet; constructing concrete pad; and furnishing and installing anchor bolts, electrical conduits, including control and neutral conductors and electrical power conductors. Conduits for control and neutral conductors and electrical power conductors shall terminate in separate new or relocated pull boxes located within 1.5 m of the new concrete pads.

Relocate existing electrical power (irrigation) for the irrigation controllers shall conform to the requirements specified under "Electrical Service (Irrigation)" elsewhere in these special provisions.

Existing irrigation facilities, designated on the plans to be relocated, that are, in the opinion of the Engineer, unsuitable for the purpose intended, shall be replaced in accordance with the provisions in Section 15-2.05, "Reconstruction," of the Standard Specifications.

After irrigation facilities have been relocated, the Contractor shall demonstrate to the Engineer that the relocated facilities function properly.

The contract unit price paid for relocate irrigation controller shall include full compensation for furnishing all labor, materials, tools, equipment and incidentals and for doing all the work involved in relocate irrigation controller as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-2.03 IRRIGATION SYSTEMS

Irrigation systems shall be furnished and installed in accordance with the provisions in Section 20-5, "Irrigation Systems," of the Standard Specifications, except materials containing asbestos fibers shall not be used.

Attention is directed to the requirements specified under "Obstructions" elsewhere in these special provisions, regarding work over or adjacent to existing underground facilities. Excavation for proposed irrigation facilities shall not be started until the existing underground facilities have been located.

Materials for irrigation systems, unless otherwise specified, shall be commercial quality.

PIPE AND CONDUIT ANCHORS.--Anchors used to attach galvanized steel pipe supply lines and sprinkler control conduits (electrical conduits) to the exterior surfaces of concrete structures and walls shall consist of metal straps and anchoring devices. Metal straps shall be made of steel and shall be fabricated to the details and dimensions shown on the plans. Anchoring devices shall consist of resin capsule anchor bolts or mechanical expansion stud anchors. Anchors shall conform to the provisions in Section 75-1.03, "Miscellaneous Bridge Metal," of the Standard Specifications.

Holes for anchorage devices shall conform to the following:

1. Holes shall not be drilled closer than 150 mm to the edge of a concrete structure.
2. Holes shall be drilled with drills approved by the Engineer.

3. Holes shall be relocated if reinforcing steel is encountered. Abandoned holes shall be filled with portland cement concrete mortar conforming to the requirements in Section 51-1.135, "Mortar," of the Standard Specifications.
4. Holes shall be drilled to a minimum depth of 8 times the diameter of the anchor bolt or stud anchor.

Full compensation for anchors and for attaching galvanized steel pipe supply lines and sprinkler control conduits shall be considered as included in the contract price paid per meter for 75 mm galvanized steel pipe (supply line) and no additional compensation will be allowed therefor.

10-2.03A PIPE

STEEL PIPE.--75 mm galvanized steel pipe (supply line) installed between existing water meter and existing 100 mm asbestos cement water pipe, at NT Line Station 126+58.8 (Lt) as shown on the plans, shall be installed not less than 1000 mm below finished grade, measured to the top of the pipe.

Prior to performing any work on the 75 mm galvanized steel pipe (supply line), the Contractor shall obtain a City of Hayward Excavation Permit for the work, and shall coordinate the 75 mm galvanized steel pipe (supply line) work with the City of Hayward Public Works Department's associated water meter replacement work at the same location.

PLASTIC PIPE.--Plastic pipe supply lines shall be polyvinyl chloride (PVC) 1120 or 1220 pressure rated pipe with minimum pressure ratings (PR) as shown on the plans.

Schedule 40 plastic pipe supply lines shall conform to the requirements of ASTM Designation: D 1785.

Plastic pipe supply lines less than 50 mm in diameter shall have solvent cemented type joints. Primers shall be used on the solvent cemented type joints.

Plastic pipe supply lines installed in conduit shall have a minimum pressure rating (PR) of 315.

Solvent cement for plastic pipe supply lines shall conform to the requirements of the local Air Quality Management District.

SECTION 10-3. SIGNALS, LIGHTING AND ELECTRICAL SYSTEMS

10-3.01 DESCRIPTION

Modifying traffic signals, lighting, sign illumination and traffic operations system (TOS) shall conform to the provisions in Section 86, "Signals, Lighting and Electrical Systems," of the Standard Specifications and these special provisions.

TOS shall consist of:

1. Changeable message sign (CMS) system
2. Closed circuit television (CCTV) system
3. Hub systems

Sprinkler control conduit is included in the following structures:

INDUSTRIAL BOULEVARD UNDERCROSSING, Bridge Number 33-0370.
MT. EDEN OVERHEAD, Bridge Number 33-0092 R/L.
Traffic signal work is to be performed at the following location:

Industrial Boulevard and Sleepy Hollow Avenue

10-3.02 ABBREVIATIONS

The following abbreviations are added to those listed in Section 1-1.02, "Abbreviations," of the Standard Specifications:

BNC	Bayonet Nut Connector
CCD	Charge Coupled Device
CCITT	International Telegraph and Telephone Consultative Committee
DLCI	Data Link Control Identifier
FCC	Federal Communications Commission
GFCI	Ground Fault Current Interrupter
IDC	Insulation Displacement Connector
IPCEA	Insulated Power Cable Engineering Association
ISDN	Integrated Services Digital Network

NEC	National Electric Code
NTSC	National Television Systems Committee
OSHA	Occupational Safety and Health Administration
OSI	Open Systems Interconnect
PVC	Polyvinyl Chloride
REA	Rural Electrification Administration
SDLC	Serial Data Link Control
TIA	Telecommunications Industry Association
TELCO	Telephone company

10-3.03 CODES

All work performed and material installed on the contract shall conform to Section 86-1.02 "Regulations and Code" and the following codes:

1. California Administrative Code, Title 24, Part 3 Basic Electrical Regulations
2. National Fire Protection Association Standards.
3. All local ordinances.
4. National Electrical Codes (NEC)

10-3.04 STANDARDS

The materials to be furnished and work to be performed shall conform to Section 86-1.02 "Regulations and Code" and the following standards subject to the modifications and additional requirements in these special provisions:

REA Standard, "Fully Color-Coded, Polyethylene-Insulated, Polyethylene-Jacketed Telephone Cable," shall apply to telephone communication conductors and cables.

IPCEA No. A-61-402, NEMA WC-5, "Thermoplastic-Insulated Wire and cable for the Transmission and Distribution of Electrical Energy," shall apply to high voltage cable and 600-V class conductors.

10-3.05 EQUIPMENT LIST AND DRAWINGS

The Contractor shall submit within 60 calendar days after approval of the contract, a complete list of equipment which he proposes to install, manufacturer's catalog information, shop drawings of custom fabricated units and such other data as required by the Engineer.

Drawings submitted by the Contractor shall be approximately the same size as the contract plans (600 mm x 900 mm).

The list shall include all items identified on the plans or in these special provisions by the Manufacturer's designation. The list shall be complete as to the manufacturer's name, catalog number, address, and telephone number. The catalog information shall contain information such as physical size, weight, rating and such additional data as may be required by the Engineer. All data submitted shall be clearly identified by the name of the project and shall be made in quadruplicate.

The list of equipment to be submitted by the Contractor shall include but is not limited to the following:

1. Cables, conductors and conduits.
2. Electrical equipment for changeable message sign system, closed circuit television system, and Hub systems.
3. Electrical components.

The Contractor shall furnish the equipment list and drawings in a manual. The manuals shall include, but shall not be limited to the following information:

(a) Specifications
(b) Design characteristics
(c) General operation theory
(d) Function of all controls
(e) Trouble shooting procedure (diagnostic routine)
(f) Block circuit diagram
(g) Geographical layout of components
(h) Schematic diagrams
(i) List of replaceable component parts with stock numbers

The equipment manuals shall be in addition to any instructions or parts lists packed with or attached to the equipment when delivered.

Manuals and other data shall be printed on heavy, first quality paper, 212.5 mm x 275 mm size with standard 3-hole punching. Drawings and diagrams shall be reduced to 212.5 mm x 275 mm or 275 mm x 425 mm. Where reduction is not practicable, large drawings shall be folded separately and placed in envelopes which are bound into the manuals. Each envelope shall bear suitable identification on the outside.

Three preliminary copies of each manual, temporarily bound in heavy paper covers bearing suitable identification, shall be submitted to the Engineer for approval prior to the date of shipment of the equipment. After approval by the Engineer, 3 final copies of each operation and maintenance manual shall be prepared and delivered to the Engineer no later than 30 days prior to placing the equipment in operation. Final manuals and all parts lists and information shall be assembled in a heavy duty, permanent, 3-ring or 3-post binders. As much as possible, material shall be assembled and bound in the same order as specified, and each volume shall have a table of contents and suitable index tabs.

All material shall be marked with project identification; inapplicable information shall be marked out or deleted.

Installation of equipment will not be considered complete until all required manuals and data have been received.

10-3.06 COST BREAK-DOWN

The Contractor shall furnish to the Engineer a cost break-down for each contract lump sum item of work described in this Section 10-3.

The Contractor shall determine the quantities required to complete the work shown on the plans. The quantities and values shall be included in the cost break-down submitted to the Engineer for approval. The Contractor shall be responsible for the accuracy of the quantities and values used in the cost break-down submitted for approval.

No adjustment in compensation will be made in the contract lump sum prices paid for the various electrical work items due to any differences between the quantities shown in the cost break-down furnished by the Contractor and the quantities required to complete the work as shown on the plans and as specified in these special provisions.

The sum of the amounts for the units of work listed in the cost break-down for electrical work shall be equal to the contract lump sum price bid for the work. Overhead and profit shall be included in each individual unit listed in the cost break-down, however, costs for traffic control system shall not be included. Bond premium, temporary construction facilities, plant and other items will not be paid for under the various electrical work items and shall be included in the mobilization bid item for the entire project.

The cost break-down shall be submitted to the Engineer for approval within 15 days after the contract has been approved. The cost break-down shall be approved, in writing, by the Engineer before any partial payment for the items of electrical work will be made.

At the Engineer's discretion the approved cost break-down may be used to determine partial payments during the progress of the work and as the basis of calculating the adjustment in compensation for the item or items of electrical work due to changes ordered by the Engineer. When an ordered change increases or decreases the quantities of an approved cost break-down, the adjustment in compensation may be determined at the Engineer's discretion in the same manner specified for increases and decreases in the quantity of a contract item of work in accordance with Section 4-1.03B, "Increased or Decreased Quantities," of the Standard Specifications.

The cost breakdown shall, as a minimum, include the following items:

ELECTRICAL SYSTEM COST BREAK-DOWN SAMPLE

Contract No. 04-045034

Bid Item: _____

UNIT DESCRIPTION	UNIT	QUANTITY	VALUE/ UNIT	TOTAL LABOR	TOTAL AMOUNT
CONDUIT - List by Each size and Installation Method	M				
PULL BOXES - Each Type	EA				
TRENCH AND BACKFILL - List by Depth and Ground Conditions (W/O Conduit)	M				
JACKING - List by Size (W/O Conduit)	M				
CONDUCTORS AND CABLES - Each Type	M				
LUMINAIRES - Each Type	EA				
STANDARDS AND POLES - List by Each Type	EA				
FOUNDATIONS - Each Type	EA				
SERVICE EQUIPMENT ENCLOSURE	EA				
TESTING	LS				
TRAINING	LS				
DOCUMENTATION	LS				

TOTAL: _____

10-3.07 MAINTAINING EXISTING AND TEMPORARY ELECTRICAL SYSTEMS

Traffic signal system shutdowns shall be limited to periods between the hours of 9:00 a.m. and 3:00 p.m.

10-3.08 CONDUIT

Conduit to be installed underground shall be Type 1 or Type 3 unless otherwise specified. Detector termination conduits shall be Type 3 or Type 4.

The conduit in a foundation and between a foundation and the nearest pull box shall be Type 1.

Conduit sizes shown on the plans and specified in the Standard Specifications and these special provisions are referenced to metallic type conduit. When rigid non-metallic conduit is required or allowed, the nominal equivalent industry size shall be used as shown in the following table:

Size Designation for Metallic Type Conduit	Equivalent Size for Rigid Non-metallic Conduit
21	20
27	25
41	40
53	50
63	65
78	75
103	100

When a standard coupling cannot be used for joining Type 1 conduit, a UL listed threaded union coupling, as specified in the third paragraph in Section 86-2.05C, "Installation," of the Standard Specifications, or a concrete-tight split coupling or concrete-tight set screw coupling shall be used.

When Type 3 conduit is placed in a trench (not in pavement or under portland cement concrete sidewalk), after the bedding material is placed and the conduit is installed, the trench shall be backfilled with commercial quality concrete, containing not less than 250 kg of portland cement per cubic meter, to not less than 100 mm above the conduit before additional backfill material is placed.

After conductors have been installed, the ends of conduits terminating in pull boxes, and in service and controller cabinets shall be sealed with an approved type of sealing compound.

At locations where conduit is required to be installed under pavement and existing underground facilities require special precautions, as described in "Obstructions" of these special provisions, conduit shall be placed by the "Trenching in Pavement Method" as specified in Section 86-2.05C, "Installation," of the Standard Specifications.

At other locations where conduit is required to be installed under pavement and if delay to any vehicle will not exceed 5 minutes, conduit may be installed by the "Trenching in Pavement Method."

Pull ropes for use when installing cables in Type 3 conduit shall consist of a flat, woven, lubricated, soft-fiber polyester tape with a minimum tensile strength of 8000 N and shall have printed sequential measurement markings at least every meter.

At the option of the Contractor, the final 0.6-m of conduit entering a pull box in a reinforced concrete structure may be Type 4.

Conduit between irrigation controller enclosure and adjacent service pull box shall be metric trade size 41.

10-3.09 PULL BOXES

Grout shall not be placed in bottom of new or existing pull boxes.

10-3.10 TRAFFIC PULL BOXES

Traffic pull boxes and covers shall have a vertical proof-load strength of 111 kN. The 111 kN load shall be distributed through a 229-mm x 229-mm x 51-mm steel plate according to Federal Specification RR-F-621e. This load shall be placed anywhere on the box and cover for a period of one minute without causing any cracks or permanent deformations.

The No. 3 1/2(T) and No. 5(T) pull boxes shall be reinforced with a galvanized Z-bar welded frame and cover similar to that shown on the plans for No. 6(T) pull boxes. Frames shall be anchored to the boxes by means of 6-mm x 57-mm long concrete anchors. Four concrete anchors shall be provided for the No. 3 1/2(T) pull box, one placed in each corner. Six concrete anchors shall be provided for each No. 5(T) and No. 6(T) pull box, one placed in each corner and one placed near the middle of each of the longer sides.

Hold down screws shall be 9-mm hex flange cap screws of Type 316 stainless steel. The nut shall be zinc plated carbon steel and shall be made vibration resistant with a wedge ramp at the root of the thread. The nut shall be spot welded to the underside of, or fabricated with, the galvanized Z-bar pull box frame.

Steel covers shall be countersunk approximately 6 mm to accommodate the bolt head. The bolt head shall not extend more than 3 mm above the top of the cover when tightened down. A 6-mm tapped hole and brass bonding screw shall be provided.

The opening of traffic pull boxes shall have the following dimensions:

Pull Box Type	Width (±25 mm)	Length (±25 mm)
No. 3 1/2(T)	270 mm	430 mm
No. 5(T)	330 mm	600 mm
No. 6(T)	430 mm	760 mm

Concrete placed around and under traffic pull boxes as shown on the plans shall contain a minimum of 325 kg of portland cement per cubic meter.

After the installation of traffic pull boxes, the steel covers shall be installed and kept bolted down during periods when work is not actively in progress at the pull box. When placing the steel cover for the final time, the cover and the Z-bar frame shall be cleaned of all debris and securely tightened down.

10-3.11 CONDUCTORS, CABLES AND WIRING

Splices shall be insulated by "Method B".

Conductors shall be stranded copper and shall meet the following specifications unless noted otherwise on the plans or other sections of these special provisions:

1. Conductors Type TW as specified in UL Standard 83 shall not be used.
2. Power tray cables shall be UL listed Type TC with THHN conductors in dry locations and THWN conductors in wet locations, and shall meet applicable UL Standard 83 and UL Standard 1277 specifications.

10-3.11A CLOSED CIRCUIT TELEVISION CABLES

Television control (TVC) cable shall consist of 15 No. 18 conductors, unshielded and with an outer jacket. Each conductor shall have a minimum of 16 tinned copper stands with a minimum of 400 µm insulation. Individual conductor insulation shall be chrome PVC with a nominal thickness of 1 mm. The outside diameter of the jacket shall not exceed 14 mm.

Color code for TVC cable shall be:

1. Black
2. White
3. Red
4. Green
5. Orange
6. Blue
7. White/ Black
8. Red/ Black
9. Green/ Black
10. Orange/ Black
11. Blue/ Black
12. Black/ White
13. Red/ White
14. Green/ White
15. Blue/ White

Television power (TVP) conductors shall be 3 No. 14 (120 VAC, AC-, equipment ground) individually insulated, stranded copper conductors in conformance with Section 86-2.08, "Conductors" of the Standard Specifications. The conductors shall be color coded black, white, and green respectively.

Television control power (TVCP) cable shall consist of 12 No. 18 conductors, unshielded and with an outer jacket. Each conductor shall have a minimum of 16 tinned copper strands with a minimum of 400 µm insulation. Individual conductor insulation shall be polyvinyl chloride (PVC), rated for 300 V (see color code below). The jacket shall be chrome PVC with a nominal thickness of 1 mm. The outside diameter of the jacket shall not exceed 12 mm.

Color code for TVCP cable shall be:

1. Black
2. White
3. Red
4. Green
5. Orange
6. Blue

7. White/ Black
8. Red/ Black
9. Green/ Black
10. Orange/ Black
11. Blue/ Black
12. Black/ White

Television video (TVL) cable shall consist of an RG-6/U coaxial cable. Each cable shall be provided with a solid No. 18 copper clad steel center conductor and shall conform to the following requirements:

Electrical	TVL
Capacitance (picofarads/m nominal)	54.1
Impedance (ohms-nominal)	75
Velocity of propagation (nominal)	84%
D.C. loop resistance (ohms/100 m)	11.7

Attenuation at 20˚C:

Frequency (MHz)	TVL (Nominal dB/ 100 m)
5.0	1.90
30	3.64
108	6.40

Physical Specifications	TVL Nominal O.D. (mm)
Copper-clad steel center conductor	1.00
Foam polyethylene dielectric	4.57
Sealed APA tape with 1.6 mm overlap	4.75
Woven aluminum braid	5.39
Sealed APA tape with 1.6 mm overlap	5.49
Woven aluminum braid	6.12
Flooding compound	
PVC outer jacket	7.55

(APA = Aluminum polyolefin and aluminum with adhesive)

TVL cable shall be terminated with BNC plug connector at both ends.

COAXIAL CABLE CONNECTORS (TVL COAXIAL CABLES).--Coaxial cable connectors for attaching Type TVL coaxial cable shall meet the following requirements:

1. Electrical:

Impedance	75 Ω nominal
Return loss	30 dB minimum (5 MHz to 300 MHz)
Rated working voltage	500 V rms

2. Mechanical:

Type of construction	Integral sleeve BNC
Method of attachment	Crimp-crimp
Composition	Bodies - alloy Finish - chromate conversion, silver plating, or other corrosion resistant metal

3. Environmental:

Temperature	-10˚C to +50˚C
Moisture	Weather resistance design

The mating connector for TVL cable in junction box shall be provided. The center contact of this jack shall be beryllium copper.

TESTING.--Testing of TVL cables and connectors shall be performed in accordance with provisions in Section 86-2.14B, "Field Testing" of the Standard Specifications and these special provisions.

Cable lengths found to have faults shall be replaced and retested. The removed faulty cable shall be disposed of by the Contractor.

Prior to the beginning of work, each length of coaxial cable shall be tested for attenuation and faults to ensure compliance with specifications contained herein using a time domain reflectometer (TDR). For the purpose of these special provisions, a fault in a long length of cable is defined by one or more of the following:

a. Return loss measurements indicating that attenuation exceeds 3 dB at 5 MHz to 30 MHz in a portion of cable less than 3 m long.
b. A return loss measurement indicating that there is a short in the cable.
c. A return loss measurement indicating a cut or open circuit in the cable.
d. A visual inspection which reveals exposure of or damage to the cable shielding.

10-3.11B TELEPHONE CABLE

The telephone cable (TC) shall consist of 6 pairs of No. 19 solid copper conductors. Conductors shall be twisted in pairs. Each conductor shall be insulated with a high molecular weight, heat stabilized, color coded polyethylene material. The insulation shall be 440 µm nominal.

Color code for TC cable shall be as follows:

1. White/Blue
2. White/Orange
3. White/Green
4. White/Brown
5. White/Gray
6. Red/Blue

The core shall be protected by a non-hygroscopic polyester film with a single longitudinally applied 120 µm thick corrugated copper shield (or 190 µm thick plastic coated aluminum shield). A moisture barrier of petrolatum-polyethylene compound shall be applied over the core tape and over and under the cable shield to fill all cable interstices.

The cable shall be provided with an outer jacket of extruded, black, high molecular weight, heat stabilized polyethylene material. The outer jacket shall have a thickness of 1.5 mm nominal. The outer diameter of the cable shall be 15.25 mm maximum.

Splices will not be allowed, except where shown on the plans.

All conductors shall be terminated inside the telephone demarcation cabinet and the controller cabinet as shown on the plans. All connections from the TBO terminal block to the 8-position connecting block shall be via a cable consisting of 2 pairs of No. 22 solid conductors and shall meet the same specifications as the TC cable.

10-3.11C ASSORTED CABLES AND OUTLETS

DATA CABLE.--The data cables shall be a 100 Ω extended unshielded twisted pair (UTP) communications cables, and shall conform to EIA/TIA-568A Category 5, EIA/TIA TSB-36, and NEMA low loss extended frequency requirements.

The data cables shall consist of No. 24 AWG solid copper conductors. Each conductor shall be insulated with a high molecular weight, heat stabilized and color coded polyethylene material with a minimum thickness of 8-mils.

The cable core shall contain the designated number of pairs cabled together and each pair shall be twisted together with varying lays to reduce crosstalk.

The outer jacket shall be made of extruded, black, high molecular weight, heat stabilized PVC material. The cable shall include a nylon ripcord under the jacket. The maximum DC resistance shall be.0886 Ω/meter at 20° C. The mutual capacitance shall be 44.3 pF/m nominal. The characteristic impedance shall be 100 Ω ± 15 percent from 1 MHz to 100 MHz.

The cable shall meet or exceed the following electrical transmission requirements:

Frequency (MHz)	Attenuation (Max.) (dB/100m)	Power Sum Near End Crosstalk (NEXT) Worst Pair (dB)
0.772	1.8	64
1	2.1	62
4	4.3	53
10	6.6	47
16	8.2	44
20	9.2	42
31.25	11.8	40
62.50	17.1	35
100	21.9	32

Frequency (MHz)	Structural Return Loss (SRL) Worst Pair (dB)
0.772	N/A
1	23
4	23
10	23
16	23
20	23
31.25	21
62.50	18
100	16

The data cables shall be color coded up to the designated number of pairs as follows:

PAIR	COLOR CODE (T--R)
1	White/Blue--Blue/White
2	White/Orange--Orange/White
3	White/Green--Green/White
4	White/Brown--Brown/White
5	White/Gray--Gray/White
6	Red/Blue--Blue/Red
7	Red/Orange--Orange/Red
8	Red/Green--Green/Red
9	Red/Brown--Brown/Red
10	Red/Gray--Gray/Red
11	Black/Blue--Blue/Black
12	Black/Orange--Orange/Black
13	Black/Green--Green/Black
14	Black/Brown--Brown/Black
15	Black/Gray--Gray/Black
16	Yellow/Blue--Blue/Yellow
17	Yellow/Orange--Orange/Yellow
18	Yellow/Green--Green/Yellow
19	Yellow/Brown--Brown/Yellow
20	Yellow/Gray--Gray/Yellow
21	Violet/Blue--Blue/Violet
22	Violet/Orange--Orange/Violet
23	Violet/Green--Green/Violet
24	Violet/Brown--Brown/Violet
25	Violet/Gray--Gray/Violet

Distribution Data Cable (DDC).-- The DDC shall consist of 25 pairs of conductors.

Service Distribution Data Cable (SDDC).--The SDDC shall consist of 12 pairs of conductors.

Outside Plant Distribution Data Cable (OPDDC).--The OPDDC shall consist of 25 pairs and have two cable jackets. The inner jacket shall be made of extruded, black, high molecular weight, heat stabilized PVC material. The outer jacket shall

have a minimum thickness of 60-mils and be made of black polyethylene (PE). The cable shall include a nylon ripcord under the jackets. Pulling tension shall be reduced when installing the OPDDC by using a water based lubricant.

SERIAL CABLE.--The serial cable shall be a NEC type CM cable and shall meet UL-1581 test requirements. The serial cable shall have six conductors. Each conductor shall be No. 24 AWG, stranded tinned copper with good crush resistance and light weight insulation. The core shall be covered with an overall aluminum-polyester foil plus 65 percent minimum tinned copper braid shield for 100 percent shield coverage. The cable shall have an outer chrome PVC jacket. The cable shall have a No. 24 AWG stranded tinned copper drain wire. The nominal capacitance between conductors shall be 29.4 pF/m , and between one conductor and other conductors connected to the shield shall be 72 pF/m .

The type of connector at each end of the cable shall be as required by the equipment, as shown on the plans, or as specified in these special provisions.

DUAL TONE MULTI-FREQUENCY (DTMF) OUTLET.--The DTMF outlet shall be a modular duplex surface mount jack. The module assembly shall consist of a common base equipped with two modular telephone jacks (4 conductors, 4 positions), associated integral screw type terminal blocks and a common snap-on cover. The module shall be furnished with both mounting screws and adhesive strip for mounting.

DTMF CABLE.--The DTMF cable shall be a NEC Type CM cable. The cable shall consist of two twisted pairs of No. 24 AWG solid tinned copper conductors. Each conductor shall be insulated with a PVC material with a minimum thickness of 10-mils. The outer jacket shall be made of PVC material with a minimum thickness of 32-mils.

CABLE MANAGEMENT.--Cable ties, wire mounting devices and fixed diameter clamps shall be used in the closets, Caltrans telephone closet (CTC), distribution equipment cabinets (DECs), and console to avoid physical interference between cables and adjacent equipment. Wire management brackets shall be used every two feet at the cabinets and console to route cable. Cables and wire management system components shall allow equipment to be removed from cabinets without physical interference and keep terminal blocks clearly visible.

INSTALLATION PRACTICES.--Installation of equipment for DTMF services shall be compliant with EIA-570. Installation of equipment for integrated services digital network (ISDN) and frame relay services shall be compliant with EIA-568, EIA-569, and TIA-607.

10-3.12 SERVICE

Continuous welding of exterior seams in service equipment enclosures is not required.

Type III service equipment enclosures shall be the aluminum type.

Circuit breakers shall be the cable-in/cable-out type, mounted on non-energized clips. All circuit breakers shall be mounted vertically with the up position of the handle being the "ON" position.

Each service shall be provided with up to 2 main circuit breakers which shall disconnect ungrounded service entrance conductors. Where the "Main" circuit breaker consists of 2 circuit breakers as shown on the plans or required in the special provisions, each of the circuit breakers shall have a minimum interrupting capacity of 10 000 A, rms.

Dead front panel or panels, and corresponding exterior door, shall be hinged on one side and shall be openable without the use of tools.

A barrier type terminal block rated for 40 A, minimum, shall be provided in each service equipment enclosure. The terminal block shall have a minimum of 12 positions with terminals rated at Size No. 8 or larger, to accept the field wires indicated on the plans. Field wires shall be terminated using crimped, insulated loop connectors.

Type H service shall consist of a conduit and conductors with length and size as required by the serving utility company.

The neutral conductor shall run from the service equipment enclosure to the controller cabinet without splicing to any other neutral conductor.

The bottom of the lowest circuit breaker shall be 600 mm minimum above the bottom of the service equipment enclosure.

10-3.13 SIGN DISCONNECTS

Sign disconnects shall be fused switches.

10-3.14 PANELBOARDS

The Contractor shall install at locations shown on the plans approved panelboards of the type indicated and specified in these special provisions. The panelboards shall be designed, manufactured, assembled and tested in accordance with all applicable NEC, NEMA and UL standards.

Interiors shall be completely factory assembled with bolt-on devices. They shall be designed such that switching and protective devices can be replaced without disturbing adjacent units and without removing the main bus connectors.

Unless otherwise noted on the plans, provide full size insulated neutral bars. Bus bar taps for panels with single pole branches shall be arranged for sequence phasing of the branch circuit devices. Neutral busing shall have a suitable lug for each outgoing feeder requiring a neutral connection.

The main bus bars shall be copper plated and sized in accordance with UL standards. A standard ground bus bar shall be included in all panels.

Enclosures shall be made from galvanized steel and shall meet NEMA Type 2 requirements. Removable blank ends and interior mounting studs with adjustable nuts shall be provided.

Trims shall be supplied with a hinged door over all circuit breaker handles. Doors in panelboard trims shall not uncover any live parts. Doors shall have semi-flush cylinder lock and catch assembly. Distribution panelboard trims shall cover all live parts. Switching device handles shall be accessible.

Surfaces of the trim assembly shall be cleaned, primed and have a gray ANSI 61 polyester powder finish coat paint. Surface trims shall be the same height and width as the enclosure. Flush trims shall overlap the enclosure on all sides.

Engraved nameplates shall be fixed to panelboards for each panel section. A directory card with clear plastic cover shall be supplied, mounted on the inside of each door.

The short circuit rating of the assembled panelboard shall not be less than 22,000 A rms symmetrical fully rated. The main and branch circuit breakers shall be as specified for circuit breakers in Section 86-2.11, "Service" of the Standard Specifications, and as required on the plans, except that breakers shall be a minimum of 100 A frame size.

In existing panelboards, provide new circuit breakers, where required. Provide mounting hardware, bus straps, and related materials for proper circuit breaker installation. Add entries to the directory card, reflecting the changes made under the contract.

Entries for load phase current, unbalanced load neutral current, and breaker size in the panelboard schedules, if provided on the plans, shall be updated.

MAIN PANELBOARD.--The main panelboard shall be rated for 225 A, 3 phase, 4 wire, 120/208 VAC service. The minimum number of branch circuits including spaces shall be thirty.

UNINTERRUPTIBLE POWER SYSTEM (UPS) PANELBOARD.--The UPS panelboard shall be rated for 100 A, 3 phase, 4 wire, 120/208 VAC service. The minimum number of branch circuits including spaces shall be eighteen.

10-3.15 ENCLOSED CIRCUIT BREAKER

The main disconnect for the traffic operations system (TOS) HUB building shall be an assembled enclosed circuit breaker with electrical characteristics and ratings as specified on the plans. The assembled enclosed circuit breaker shall meet UL and NEC requirements for service entrance equipment and shall consist of a circuit breaker enclosure, an operating handle mechanism, a ground bus, and a circuit breaker including frame, trip, and terminals. The circuit breaker enclosure shall meet NEMA 12 requirements. The circuit breaker shall meet the "Molded Case Circuit Breakers" requirements of these special provisions.

The door of the enclosure shall be hinged. The operating handle shall be front operated, shall have a visible red vinyl sleeve, and shall be padlockable in the "OFF" position. The cover of the enclosure shall be interlocked with the handle mechanism to prevent opening the cover with the circuit breaker in the "ON" position. A factory installed front cover nameplate shall record catalog number and maximum ratings.

MOLDED CASE CIRCUIT BREAKERS.--The molded case circuit breakers shall be designed, manufactured, assembled and tested in accordance with UL 489 and NEMA AB-1 standards.

All electrical circuits in HUB building shall be protected by molded case circuit breakers which shall be operated by a toggle-type handle and shall have a quick-make, quick-break over-center switching mechanism that is mechanically trip-free from the toggle-handle so that the contacts cannot be held closed against short circuits and abnormal currents. All poles shall be so constructed that they open, close and trip simultaneously. The ON and OFF positions shall be clearly marked on the cover of the circuit breaker.

The circuit breaker frames shall employ high strength, molded-polyester, glass-reinforced cases and covers. The breaker frame shall have legible, tamper-proof nameplates containing maximum frame ampere ratings, maximum voltage ratings and interrupting ratings in accordance with UL standards. All breaker frame sizes shall have external means for manually tripping the breaker and exercising the mechanism and trip latch member.

All three phase circuit breakers shall have digital solid state, ambient temperature insensitive tripping. The breaker frames above 100 A shall use field installed, UL listed rating plugs to establish the ampere rating and shall be suitable for reverse feed (i.e., no line/load markings). The breakers shall accurately sense sinusoidal current waveforms by continuously sampling each phase throughout every cycle. The breaker frame sizes shall have a single, customer-adjustable, instantaneous pickup knob to set the instantaneous response for all poles. A frame shall reject a rating plug not intended for use in it.

The circuit breaker ratings shall be as specified on the plans.

10-3.16 EQUIPMENT ENCLOSURES (HUB BUILDING)

Equipment enclosures shall be UL-listed for indoor use and shall meet NEMA Type 1 enclosure requirements.

Enclosures shall be fabricated from 16 gauge sheet steel and hot-dip galvanized after fabrication. Enclosures shall have flat removable covers fastened with stainless steel screws and shall have gray enamel finish.

10-3.17 NAMEPLATES

Nameplates shall be installed on equipment as shown on the plans.

Nameplates shall be laminated phenolic plastic, black front and back with white core. Lettering shall be etched through the outer covering, indicating the function of device or assembly unit. The character size shall be 1/4 inch, unless noted otherwise.

The nameplates shall be fastened to the enclosures' exterior surface using stainless steel rivets or stainless steel screws.

10-3.18 EQUIPMENT GROUNDING

All noncurrent-carrying metal parts of fixed, portable, and mobile equipment and associated fences, housings, enclosures, and supporting structures shall be grounded. Grounding methods shall meet the requirements of NEC Article 250.

10-3.19 NUMBERING ELECTRICAL EQUIPMENT

The placement of numbers on electrical equipment will be done by others.

10-3.20 CAMERA UNIT

Each camera unit shall consist of a camera, lens, environmental housing and camera unit cable assembly. The camera, lens and environmental housing, combined, shall not weigh more than 6.8 kg. Each camera unit shall be interchangeable with the existing camera units already installed on various traffic operations system (TOS) projects without changes or adjustments to either the system or the camera unit.

Each camera unit shall be assembled, inspected, and tested in accordance with these special provisions prior to delivery to the job site. Installation, operations and maintenance manuals shall also be submitted at the time of delivery.

Applicable Documents - The following documents of the U.S. Military Specification (MIL-SPEC), Underwriters' Laboratory, Inc. (UL), Electronics Industries Association (EIA) Standards, and other Standards form parts of the specification to the extent specified in these standards. In the event of a conflict between the content of this section and the content of the specification, the standards defined in this section shall supersede.

Military Specification Documents	
MIL-I-45208A	Inspection System Requirements, Dec. 16, 1963
MIL-C-45662	Calibration System Requirements, June 10, 1980
MIL-STD-416A	Electromagnetic Interface Characteristics Requirements for Equipment, Subsystems & Systems, Aug. 1, 1968
MIL-E-5400T	Electronic Equipment, Airborne General Specification
MIL-STD-810	Environmental Test Methods, 19 July 1983
MIL-C-5541	Chemical Conversion Coatings on Aluminum Alloys, June 3, 1970

Underwriters' Laboratory, Inc. and other documents	
UL-796	Printed Circuit Boards
EIA-170A	Electrical Performance Standards Color Television Studio Facilities
EIA RS-330	Electrical Performance Standards for Closed Circuit Television (CCTV) Camera 525/60 Interlaced

CAMERA.--

Approved Camera - The cameras listed below have been approved by the Engineer. Further information such as the source of the cameras can be provided upon demand. Any camera submitted by the Contractor that is not one of the approved cameras listed below shall be stringently compared to the specifications set forth in these special provisions before it is approved by the Engineer.

Approved Cameras:

Cohu 8240
Sony SSC-C374
Burle/Philips TC293C-Z0P2X596

1. General -

a. Each camera shall be self-contained and designed for continuous unattended 24-hour operation.
b. Camera performance shall meet or exceed EIA-170A standards.
c. Each camera shall have automatic sensitivity and black level control so that it operates without further adjustment when illumination ranges from the minimum specified herein to that of full daylight.
d. Each active electronic device within the camera shall be solid-state.
e. Each camera shall have a switch selectable electronic shutter. The shutter speed shall range from 1/60 of a second (off) to 1/10,000 of a second in 8 steps. The control of the electronic shutter rate shall be accessible through a side panel opening. Remote on/off control of the shutter shall be accomplished via an output connector on the rear of the environmental housing.

2. Light sensitivity - At a scene with 50 percent light reflectivity and with light level on the scene measured to be 10 lux and greater, each camera with its 10:1 zoom lens shall generate a picture on the monitor specified in these special provisions which resolves all 10 shades of gray on the EIA Television Test Chart. Not every camera shall be required to resolve the same gray scale when the light level on the same scene is measured to be less than one lux.

3. Electrical Power - Each camera shall operate when the applied voltage is between 95 to 130 VAC, 60 Hz ±0.3 Hz, single phase. The power consumption of camera shall not exceed 10 W. All camera circuitry shall be powered by a regulated, over-voltage protected DC power supply contained in the camera.

4. Synchronization -

a. Each camera shall generate synchronization signals by means of a single integrated circuit.
b. Each camera shall operate with the internal EIA-170A crystal as the sync reference source with the field rate of 59.94 Hz.
c. Each camera shall have the capability of synchronizing to an external input source.
d. The synchronization signal at the video output shall conform to EIA-170A.

5. Scanning - Each camera's mode of scanning shall be two-to-one interlaced at 525 lines, 60 fields per second, as specified by EIA-170A.

6. Video Processing -

a. The video output level shall be maintained to within 3 dB for changes in scene brightness of 0.17 to 109 600 cd/m^2 using a motorized iris lens having a transmission range of F/1.2 to F/1200.
b. Each camera shall have a black-level control system.
c. Each camera shall have an automatic white (color) balance control to maintain proper color rendition by automatically referencing to white areas of the scene. Manual control of the white balance shall be accessible through side panel or remotely controlled via the rear connector on the environmental enclosure.
d. Automatic gain control (AGC). The switch selectable fixed gain response shall maintain the output video level to 90 percent video when the light level on the image sensor falls too low to maintain full video output. The video out will be maintained at 90 percent peak-average setting to optimize video output under varying lighting conditions. The controls for the on/off and peak average adjustments shall be accessible through the side panel of the camera.
e. One 75 Ω (±1 percent) source-terminated, single-ended video output jack shall be provided.
f. An adjustable white clipper shall be provided to limit highlight brightness.
g. The video output level shall be 1.0 V peak-to-peak composite (0.7 V video, 0.3 V sync) signal, polarity black negative, across a 75 Ω load impedance.
h. The minimum signal-to-noise ratio shall be 50 dB at 25°C.

7. Imager -

a. Each camera shall be designed to use a 12.7 or 16.9 mm format solid-state, color, interline transfer charged-coupled device (CCD) imager.
b. The imager shall have a minimum resolution of 768 horizontal by 493 vertical active picture elements.
c. When provided with 6.89 lux of highlight illumination on the environmental housing window, the camera shall provide the following resolution with AGC off:

i. Horizontal center resolution shall be 460 TV lines minimum.
ii. Vertical resolution shall be 350 TV lines minimum in the center and all corners simultaneously.

8. Mechanical -

a. All camera circuitry shall be on a printed circuit board which shall be removable and replaceable using no more than common hand tools.
b. Test points and adjustment identifications shall be etched or silk-screened on the boards.
c. Each camera shall be self-supporting and operable when removed from the environmental housing and shall have a lens adapter ring which accommodates a standard 16 mm C-mount. A back focal distance adjustment shall be provided.
d. Each camera shall utilize the rigidity of the environmental housing for strength against shock and vibration.

9. Environmental Operation -

a. Each camera within its environmental housing shall withstand the following requirements:

i. Operate over an ambient temperature range on -10°C to +50°C with the use of heaters.
ii. Peak random vibration of 5 g from 60 to 1000 Hz, with camera in operation.
iii. Shock up to 15 g in all axes under non-operating conditions.
iv. Exposure to sand, dust, fungus, and salt atmosphere as per MIL-E-5400T paragraphs 3.2.24.7, 3.2.24.8, 3.2.24.9, and 3.2.24.10.
v. Up to 100 percent relative humidity as per MIL-E-5400T, paragraph 3.2.24.2.

10. Picture Quality -

a. The quality of the picture shall be such that most objects in low light levels can be distinguished without excess interference from undesirable picture attributes.
b. Undesirable picture attributes include blooming, transfer smear, vertical register shifting.

LENS.--

1. General -

a. Each lens shall be fully compatible with the camera provided.
b. Each lens shall have a maximum aperture of no less than F/1.2, and a minimum range of F/1.2 to F/1200.
c. Each lens shall be a 16 mm C-mount.
d. Each lens shall have a minimum zoom range of 10 to 1. The lens focal length for a 16.9-mm CCD camera shall be shall be 10-100 mm, 10.5-105 mm or 11-110 mm. The lens focal length for a 12.7 mmCCD camera shall be shall be 8-80 mm. A 1.5-X extender shall be included with the lens.
e. Each lens shall be equipped with a motorized zoom and focus control.
f. Each lens shall have an automatic, motor-driven iris with manual override. The iris shall be controlled directly through the camera in automatic mode and from the camera control unit in the manual mode. The automatic iris shall provide continuous aperture adjustments of the lens as determined by the amount of light reaching the camera imager. The power supply and electronics for iris motor shall be contained within the environmental housing.
g. Each lens shall be fitted with an intra-spot neutral density filter.
h. Each lens must be able to respond to the following inputs from camera control unit (CCU) for lens control:

Focus near	+9.0VDC @ 100 mA
Focus far	-9.0VDC @ 100 mA
Zoom in	+9.0VDC @ 100 mA
Zoom out	-9.0VDC @ 100 mA
Iris closed	+9.0VDC @ 100 mA
Iris open	-9.0VDC @ 100 mA

i. The travel time for the upper limit stop to the lower limit stop for zoom and focus shall be from 5 seconds minimum to 10 seconds maximum.
j. Each lens shall have the capability of at least 10 pre-positioning positions.

2. Environmental Operation -

a. Each lens within its pressurized environmental enclosure shall withstand the following requirements:

i. Operate over an ambient temperature range on -10°C to +50°C with the use of heaters.

ii. Peak random vibration of 5 g from 60 to 1000 Hz, with lens in operation.

iii. Shock up to 15 g in all axes under non-operating conditions.

iv. Exposure to sand, dust, fungus, and salt atmosphere as per MIL-E-5400T paragraphs 3.2.24.7, 3.2.24.8, 3.2.24.9, and 3.2.24.10.

v. Up to 100 percent relative humidity as per MIL-E-5400T, paragraph 2.24.2.

ENVIRONMENTAL HOUSING.--

1. General -

a. Each camera and lens shall be mounted in a sealed, cylindrical, environmental housing which shall not exceed 165 mm in diameter and 560 mm in length.

b. The housing shall be fabricated of seamless aluminum tubing Type 6061-T6 and shall be finished with heat reflecting, weather resisting enamel. The rear plate shall be fabricated of Type 6061-T6 aluminum.

c. The front of the housing shall be closed with a clear optically flat glass or Lexan window.

d. The housing shall include a sun shroud which shall be strapped to the housing bindings in such a manner as to minimize heat conduction by maintaining air space between shroud and housing. The shroud shall shield the entire top portion of the housing from the sun, and shall extend a minimum of 76 mm in front of the housing's glass plate.

e. The housing shall not interfere with the widest viewing angle of the lens.

f. The housing shall include a thermostatically controlled heating pad rated at 115 VAC, 100 W maximum.

g. The backplate, which is an integral part of the housing, shall have a nameplate attached bearing the manufacturer's camera assembly model number and serial number.

h. Camera connector -

i. The camera connector shall be designed to MIL-C-28462 Series 1 specifications with solder contacts. It shall be PT-07C-20-39 P, the male counterpart to PT-06A-20-39 S SR.

ii. The connector shall be made of the same shell material and pin arrangement as the PT-06A-20-39 S SR connector on the end of the wiring harness.

iii. The pinout of the connector shall be as shown on the plans.

iv. Each camera connector shall be mounted on the rear plate with an airtight connection and a watertight seal for each conductor on its connecting cable.

i. Each camera shall be designed for operating with the housing purged of air and filled with dry nitrogen to 35 kPa.

j. Valves -

i. The housing shall have two valves, both on the rear end plate of the housing.

ii. One valve shall be a standard Schrader valve. It shall have a tube running from the valve to the front end of the housing.

iii. The other valve shall be an air escape/pressure relief valve preset to 140 kPa.

iv. The two valves shall be arranged such that filling with dry nitrogen from the Schrader valve will force gas to flow from front end of the housing to the rear end and vent through the air escape valve.

v. The leakage rate with both valves closed and the housing pressurized to 70 kPa, shall leak less than 7 kPa per year, and less than 0.7 kPa per month.

k. The enclosure shall be factory pressurized to 35 kPa with dry nitrogen.

l. The notation "CAUTION PRESSURIZED" shall be printed on the rear plate of the enclosed.

m. A safety pressure release bolt shall be incorporated such that the enclosure may not be opened without first releasing the internal pressure.

n. A low-pressure (LP) sensor with related circuitry shall be provided to send an LP alarm (dry contact closure) via the camera connector. The LP sensor threshold shall be preset to activate the LP alarm at 14 kPa.

o. Two units of desiccant shall be placed inside the housing but will not obstruct the camera view or operation.

p. A humidity level indicator strip with discrete indications for 10, 20 and 30 percent humidity, shall be installed inside the housing in a position that allows inspection, while facing directly, through the window without obstructing the view of the camera at the widest viewing angle of the lens.

2. Mounting Base -

a. A mounting base shall surround the enclosure to provide a tamper-resistant attachment of the environmental housing to the pan/tilt unit and securely hold the housing in either an upright or inverted position. The combined height of camera housing and mounting base shall be less than 254 mm. The mounting base attachment shall be as shown on the plans.

b. All fasteners and nuts used in attaching the mounting base to the environmental housing shall be of grade 18-8 stainless steel.

c. Each camera unit shall be provided with three stainless steel hex head bolts to secure the camera unit to the pan/tilt unit.

CAMERA UNIT CABLE ASSEMBLY.--

1. General - Each camera unit cable assembly shall consist of box mounting socket connector and a wiring harness.

2. Box Mounting Socket Connector -

a. Each box mounting socket connector shall be MS-3102E-28-21S-F80.
b. One full set of crimp contacts shall be provided with each box mounting socket connector.
c. The pinout of the connector shall be as shown on the plans.

3. Wiring Harness -

a. The camera unit cable assembly wiring harness shall be 3 m long and shall be COHU AC27E or other cable approved by the Engineer. The coaxial cables' impedance shall be 75 Ω. One end of each wiring harness shall be type PT06A-20-39S (SR) and shall mate with the camera connector. It shall have a 90-degree end bell, Bendix 10-25997-203 or equal, for strain relief. The end bell shall be positioned such that the opening is 180 degrees from the main key on the connector. The other end of each wiring harness shall be type MS-3106E-28-21P and shall mate with the box mounting socket connector.

b. All connectors shall be potted with an appropriate potting compound for environmental sealing.

c. All connectors' pinout shall be as shown on the plans.

10-3.21 PAN/TILT UNIT

1. General --

a. Each pan/tilt unit, with camera unit attached, shall pan 355 degrees minimum in either direction, and tilt 60 degrees minimum from horizontal position to either up or down positions.

b. Each pan/tilt unit shall be capable of minimum ten preset positions for each pan and tilt operation.

2. Electrical --

a. Each pan/tilt unit shall operate with input voltage of 115 VAC, 60 Hz ±0.3 Hz.

b. Each pan/tilt unit shall not exceed 200 W power consumption.

c. The motors shall be impedance overload protected, two phase induction type, rated for continuous duty and shall be instantaneous reversing.

d. The limit switches shall be rated 5 A, 10 million cycles and with external adjustments.

e. Each pan/tilt unit shall be provided with box-mounting type connector.

f. One mating connector with full set of crimp contacts and strain relief shall be provided with the box-mounting connector.

3. Mechanical --

a. The maximum load shall be 45 kg at 127 mm from tilt table to center of gravity in both upright and inverted position.

b. Each pan/tilt unit shall have maximum dimensions of 400 mm high, 355 mm wide and 230 mm deep.

c. Each pan/tilt unit shall not exceed the maximum weight of 27 kg.

d. Each pan/tilt unit shall be constructed in aluminum casting and plate. All internal parts shall be corrosion protected.

e. Each pan/tilt shall have textured semi-gloss beige enamel finish.

f. All bearings shall be heavy duty ball or roller bearing.

g. All gears shall be hardened steel.

h. All gasket seals shall be designed for all weather protection.

i. Each pan/tilt unit shall have a mounting hole pattern as shown on the plans.

j. Each pan/tilt unit shall have a minimum pan torque rating of 34 N·m and an instantaneous minimum tilt torque of 68 N·m.

k. The pan speed at no load shall be 6 to 8 degrees per second, nominal.

l. The tilt speed at no load shall be 3 to 4 degrees per second, nominal.

4. Environmental --

a. Ambient operating temperature shall be from -10°C to 50°C.

b. It shall be able to withstand vibration of 760 µm total excursion from 5 to 30 Hz and a peak random vibration of 5 g from 30 to 1000 Hz.

c. It shall be able to withstand shock up to 15 g in all axes under non-operating conditions, conforming to MIL-E-5400R, Para. 3.2.24.6.

d. It shall be able to operate in atmospheres up to 95 percent relative humidity, conforming to MIL-E-5400R, Para 3.2.24.4.

e. It shall be able to withstand exposure to sand, dust, fungus, and salt atmosphere, conforming to MIL-E-5400R, Para 3.2.24.7, 3.2.24.8 and 3.2.24.9.

5. Pan and tilt unit mounting --The nuts and bolts securing the pan and tilt unit to the camera platform shall be hex head and made of stainless steel. Before each bolt is fastened, a locking type coating shall be applied to the threads. The coating shall lock the bolts and nuts in place making it impossible to turn bolt or nut without tools. This coating shall last and be effective through at least 10 insertions and withdrawals of the bolt or nut.

10-3.22 CAMERA JUNCTION BOX

The camera junction box (JCB) shall be constructed as shown on the plans and described in these special provisions. All seams shall be continuously welded. All JCB mounting Hex head stainless steel nuts and bolts shall be 6 mm □-20 x 25 mm. Steel surfaces on which JCB is to be mounted shall be drilled and tapped 6 mm □-20 thread. Each JCB shall be fabricated from 14 gauge sheet steel. Two 8-32 threaded studs for terminal strip mounting shall be welded on the bottom of the box as shown on the plans. After fabrication the JCB shall be hot-dip galvanized.

A twenty position terminal block with No. 8 lugs and cover shall be mounted on the bottom of the box. Permanent terminal position markings shall be used. A laminated wiring schematic shall be permanently attached to the inside of box cover showing wiring from the camera unit box mounting connector to the terminal block.

10-3.23 INTEGRATED SERVICES DIGITAL NETWORK TERMINAL ADAPTER UNIT

1. General --

a. Each integrated services digital network terminal adapter (ISDN TA) unit shall provide an interface between data terminal equipment (DTE) such as the video encoder unit (VEU) or video decoder unit (VDU) and ISDN.

b. Each ISDN TA unit shall include an internal network termination Type 1 device (NT1) interface and include Terminal Adapter functionality. No external NT1 shall be allowed.

c. A prototype unit is not acceptable.

d. Each ISDN TA unit shall be of the current standard production units.

e. Each ISDN TA unit shall be new and not previously used.

f. The Contractor shall provide a manual for each unit supplied.

2. Qualifying specifications --

a. Operation --

i. Each ISDN TA shall support ISDN basic rate interface (BRI) service and shall provide three digital channels capable of simultaneous data and voice transmission via a standard telephone line. The three channels shall include two Bearer (B) channels at 64 kbps and one Data (D) channel at 16 kbps (2B+D).

ii. Leased operation shall support 1B (64 kbps) or 2B (128 kbps) clear channel synchronous configurations.

iii. At data rates over 64 kbps, the BONDING delay equalization protocol shall synchronize data over the two B channels.

b. Data rates --

i. Synchronous: 2.4, 4.8, 9.6, 19.2, 38.4, 48, 56, 64, 112, 128 kbps; selectable.

ii. Asynchronous: 0.3, 1.2, 2.4, 4.8, 9.6, 19.2, 38.4, 57.6, 115.2 kbps; selectable.

c. Interoperability -- The ISDN TA unit shall support communications with public switched 56 kbps services and switched 56 kbps channel service unit/data service unit (CSU/DSU) as well as other ISDN TA, ISDN terminal equipment, and BONDING compatible inverse multiplexers.

d. D-Channel switch compatibility -- AT&T 5ESS, NTI DMS-100, National ISDN-1.

e. Dialing --Dialing shall be supported in the following ways:

i. Manually from a front panel keypad.

ii. Automatically from up to ten stored numbers.

iii. Automatically through an RS-366 parallel dial port.

iv. Dialing over the DTE interface using the asynchronous AT command set.
v. V.25 bis in-band dialing over the DTE interface using V.25 bis commands.

f. DTE interface -- The ISDN TA shall provide both EIA-530A and V.35 interfaces. The interface to be used shall be selectable. A three meter male/female EIA-530 interface cable shall be provided with each ISDN TA.

g. Network interface -- Network termination shall be designed into the ISDN unit thereby eliminating the need for an external NT1. Connection to the network shall be made by a telephone company provided 2-wire and/or 4-wire 2B1Q U-interface which is connected directly to an eight-pin RJ45 modular jack on the rear panel of the ISDN TA unit. The ISDN TA unit shall provide two RJ45 modular jacks, where one jack shall be designated for dial-up ISDN and the other jack designated for leased ISDN. The dial-up ISDN and leased ISDN operational modes maybe integrated on a single RJ45 jack if the operational modes are user selectable.

h. Local control facilities --Local operator control of all essential features of the ISDN TA unit shall be accomplished by the use of necessary discrete front panel controls.

i. Remote control facilities -- Remote configuration and control of the ISDN TA unit shall be possible using the AT command set in-band over the DTE interface. Remote call setup and termination shall also be possible using V.25 bis in-band dialing.

j. Diagnostics requirements -- Each ISDN TA unit shall be able to perform a variety of tests that allow problems to be identified and isolated. Testing shall be supported manually from the front panel, or in-band from either the network provider or distant end unit. Internal error checking shall be available for both the local and a remote activated digital loopback.

k. Physical --

i. Each ISDN TA unit shall be secured and mounted on a shelf assembly.

ii. Each shelf assembly shall provide for a minimum of four mounting screws in order to mount the shelf assembly in 89 mm (2 rack units) of EIA-310 rack space.

l. Electrical --

i. Power input voltage shall be 115 VAC ±10 percent, 60 Hz.
ii. Power dissipation shall not be greater than 8 W.

m. Environmental --

i. Operating temperature: 0°C to 50°C, minimum.
ii. Storage temperature: -20°C to 70°C, minimum.
iii. Relative humidity: 0 to 95 percent, non-condensing.

10-3.24 CAMERA CONTROL UNIT

1. General -- Each part of each camera control unit (CCU) shall be electrically and physically interchangeable with the like part in any other CCU furnished under this contract.

2. Mechanical --

a. Each CCU shall mount in 133 mm of EIA-310 rack space with a maximum depth of 356 mm.

b. The front panel shall be white gloss color Number 17886 as per Federal Standard Color Chart 595a.

c. The front and rear panel lettering shall be black color Number 17986 as per Federal Standard Color Chart 595a.

d. A high-impedance panel jack BNC (Bayonet Nut connector) connector shall be installed on the front panel as shown on the plans. This connector shall provide video input to a test monitor without affecting the remainder of the CCTV system. This connector shall be directly connected to the video input on the rear panel.

e. A glass type, size 6.35 mm x 31.75 mm (AG) slow blow fuse shall be installed on the front panel. The fuse shall be replaceable from the outside of the front panel.

f. Switches shall protrude no more than 25 mm from the front panel and shall be mounted as shown on the plans.

g. The rear panel connectors shall be mounted as shown on the plans and shall meet the following requirements:

i. Connectors C1-C3 shall be of the following type or equivalent:

C1	AMP 206430-1
C2	AMP 206043-1
C3	AMP 206306-1

ii. The pin and socket contacts for connectors C1-C3 shall be constructed with brass contact body material and with stainless steel spring that are sub-plated with 1.27 µm nickel and plated with 0.762 µm gold. Pin diameter shall be 1.575 mm. Contact size shall be 16.

iii. Each C1, C2 and C3 connector shall use the AMP No. 601105-1 or No. 91002-1 contact insertion and the AMP No. 305183 contact extraction tool.

iv. One mating connector with a full set crimp contacts and strain relief shall be supplied with connectors C1, C2 and C3.

v. The connectors C4 and C5 shall be a DB-25 socket connector.

h. Serial cable assemblies (SCA1 and SCA2) with length of 3 m shall be provided to mate with C4 and C5, respectively.

i. Pin and socket contacts for DB-25 connectors shall be copper alloy body; finished with 0.762 µm gold over 1.27 µm nickel.

j. The rear and front panel BNC connectors shall be of copper material with bright nickel (tarnish resistant) finish for the body and silver finish for the contact.

k. Each printed circuit board shall be vertically installed.

l. Each LED shall be equal to Hewlett Packard High Intensity Red Untinted, Non-diffused LED (Part Number HLMP-D105). Each LED shall be mounted as shown on the plans.

m. A front panel on/off switch shall turn the CCU on/off and shall also control AC power to the rear panel power output connector (C1). The indicator used for AC power shall be green when energized.

n. One coaxial cable labeled "AVO" (Analog Video Output) terminated with BNC plug connectors on each end shall be provided. This cable shall be RG-59/U with overall length of one meter.

3. Electrical --

a. Each CCU shall have auto-iris override.

b. Each CCU shall have circuitry to detect the absence and presence of video sync on its video input. Each CCU shall also have circuitry to monitor the low-pressure alarm contact closure from the camera unit. A local/remote control switch shall be provided to override the lens and pan/tilt controls through C4 when the switch is in local mode. When in local mode, the local control alarm shall be active.

Alarm status shall be constantly monitored and updated. Upon receipt of a "status query" message, the CCU shall send alarm status message with data as follows:

"0"	None of the alarms active.
"1"	Local Control (LC) alarm active.
"2"	Low Pressure (LP) alarm active.
"3"	LP/LC alarms active.
"4"	Video Sync Absence (VSA) alarm active.
"5"	VSA/LC alarms active.
"6"	VSA/LP alarms active.
"7"	VSA/LP/LC alarms active.

The front panel alarm light shall be lit if any the alarms are active.

c. Each CCU shall have circuitry for a source character generator. The source character generator shall display 16 alphanumeric characters superimposed on the video image. Each character shall be 28 TV lines high and shall be derived from a standard 5 x 7 dot matrix. The programmed characters shall be stored in a non-volatile memory. Upon receipt of "Set ID" message, the CCU shall position from the camera ID in the video image as follows:

"1"	Upper 15% limit of the left viewing area
"2"	Upper 15% limit of the right viewing area
"3"	Lower 15% limit of the left side viewing area
"4"	Lower 15% limit of the right side viewing area

The characters shall be superimposed on the video signal using non-additive mixing techniques.

d. Each CCU shall be designed to prevent simultaneous operation of pan right/left, tilt up/down, zoom in/out, focus near/far or iris open/close.

e. Each CCU shall have power supply(ies) for camera zoom, focus, motors, control and interface circuits. The voltage for zoom, focus and iris shall be selectable internally by one jumper for 12.0 VDC, 9.0 VDC or 5.0 VDC at 100 mA. The CCU shall be pre-configured with the voltage jumper select set to 9.0 VDC. The operation of zoom, focus and iris shall be as follows:

Zoom in	+VDC
Zoom out	-VDC
Focus near	+VDC
Focus far	-VDC
Iris close	+VDC
Iris open	-VDC

f. The maximum power consumption for the CCU shall not exceed 450 W. Power consumption of equipment attached to pin 1 of connector C1 shall not exceed 100 W. Power consumption of equipment attached to pin 12 of connector C2 shall not exceed 200 W.

g. Each CCU shall have eight independently operating 24 VDC relays (options 1 to 8). Each relay shall be single pole, double throw (SPDT), with contacts rated 1.25 A at 120 VAC.

h. Each CCU shall be capable of a minimum of ten presets and capable of controlling camera units and pan/tilt units equipped with pre-positioning feedback potentiometers. Each CCU shall have circuitry to filter out any electrical noise interference on each of the pre-positioning feedback voltage signal for the camera unit and pan/tilt unit.

i. A system reset switch shall be a momentary-pushbutton type and be mounted on the front panel to function as external reset input to the microprocessor. System reset shall not cause existing pan/tilt and lens positions to change. System reset shall be executed without requiring the operator to hold the momentary-pushbutton for more than one second.

j. The front panel of the camera control unit shall have LEDs and switches to provide the following control functions as shown on the plans.

Function	Hardware	Indicator
Zoom (In/Off/Out)	(ON)-OFF-(ON)	2 LED
Focus (Near/Off/Far)	(ON)-OFF-(ON)	2 LED
Pan (Left/Off/Right)	(ON)-OFF-(ON)	2 LED
Tilt (Up/Off/Down)	(ON)-OFF-(ON)	2 LED
Iris (Open/Off/Close)	(ON)-OFF-(ON)	2 LED
Iris override (Manual/Auto)	ON-OFF	1 LED
Option 1 (On/Off)	ON-OFF	1 LED
Option 2 (On/Off)	ON-OFF	1 LED
Option 3 (On/Off)	ON-OFF	1 LED
Option 4 (On/Off)	ON-OFF	1 LED
Option 5 (On/Off)	ON-OFF	1 LED
Option 6 (On/Off)	ON-OFF	1 LED
Option 7 (On/Off)	ON-OFF	1 LED
Option 8 (On/Off)	ON-OFF	1 LED
Alarm (On/Off)	ON-OFF	1 LED
Control (Local/Remote)	ON-OFF	
Reset	(ON)-OFF (momentary pushbutton)	

k.

CCU connector assignments	
C1	4 contact connector
C2	14 contact connector
C3	37 contact connector
C4, C5	DB-25 connectors

C1 -- AC Power	
Position	Function
1	AC +
2	AC -
3	Equipment Ground
4	NA

C2 -- Pan/Tilt			
Pos.	Function	Pos.	Function
1	Pan right	8	Pan feedback
2	Pan left	9	Tilt feedback
3	AC-	10	Preset -VDC
4	Tilt up	11	NA
5	Tilt down	12	AC+
6	AC-	13	AC-
7	Preset +VDC	14	Ground

C3 -- Camera			
Pos.	Function	Pos.	Function
1	Zoom	20	Option 3 N.O.
2	Z/F/I Common	21	Option 3 Common
3	Focus	22	Option 3 N.C.
4	Iris	23	Option 4 N.O.
5	Iris Override Common	24	Option 4 Common
6	Iris Override	25	Option 4 N.C.
7	Preset +VDC	26	Option 5 N.O.
8	Zoom Preset feedback	27	Option 5 Common
9	Focus Preset feedback	28	Option 5 N.C.
10	Preset -VDC	29	Option 6 N.O.
11	LP alarm	30	Option 6 Common
12	LP alarm	31	Option 6 N.C.
13	NA	32	Option 7 N.O.
14	Option 1 N.O.	33	Option 7 Common
15	Option 1 Common	34	Option 7 N.C.
16	Option 1 N.C.	35	Option 8 N.O.
17	Option 2 N.O.	36	Option 8 Common
18	Option 2 Common	37	Option 8 N.C.
19	Option 2 N.C.		

Note:
N.O. = Normally open
N.C. = Normally closed
NA = Not Available

C4, C5 -- Serial communication ports to and from external device.			
Pos.	Function	Pos.	Function
1	NA	14	NA
2	Transmit Data	15	NA
3	Receive Data	16	NA
4	NA	17	NA
5	NA	18	NA
6	NA	19	NA
7	Signal Ground	20	NA
8	NA	21	NA
9	NA	22	NA
10	NA	23	NA
11	NA	24	NA
12	NA	25	NA
13	NA		

Serial cables			
SCA1		SCA2	
DB-25 Pin	DB-25 Pin	DB-25 Pin	DB-25 Socket
2	2	2	2
3	3	3	3
7	7	7	7

4. Environmental --
   a. Each CCU shall operate in an ambient temperature environment of -10°C to 50°C.
   b. Each CCU shall conform to MIL-STD-810D-516.1 and MIL-STD-810D-514.1 shock and vibration test.

5. CCU messages --
   a. Each CCU shall communicate through the C4 serial port with the following communication message codes:

DIREC-TION	MESSAGE	CHARACTER		DATA
		1ST CODE	2ND CODE	
Transmit	Alarm status	A	space	"0"-"7"
Receive	Status query	Q	space	NONE
Receive	Pan stop	p	space	NONE
Receive	Tilt stop	t	space	NONE
Receive	Zoom stop	z	space	NONE
Receive	Focus stop	f	space	NONE
Receive	Iris stop	i	space	NONE
Receive	Pan left	L	space	NONE
Receive	Pan right	R	space	NONE
Receive	Tilt up	U	space	NONE
Receive	Tilt down	D	space	NONE
Receive	Zoom in	I	space	NONE
Receive	Zoom out	O	space	NONE
Receive	Focus near	N	space	NONE
Receive	Focus Far	F	space	NONE
Receive	Iris open	J	space	NONE
Receive	Iris close	K	space	NONE
Receive	Iris manual	M	space	NONE
Receive	Iris auto	m	space	NONE
Receive	Set ID word	C	"1"-"4"	16-ASCII char.
Receive	Home position 0-9	H	"0"-"9"	NONE
Receive	Home position program 0-9	P	"0"-"9"	NONE
Receive	Option on 1-8	S	"1"-"8"	NONE
Receive	Option off 1-8	s	"1"-"8"	NONE
Receive	Enter Echo mode	E	space	NONE
Receive	Exit Echo mode	^]C	This sequence is not in a communication packet	

   b. After receiving the "enter echo" command the CCU shall pass all characters from C5 to C4 and C4 to C5. The CCU shall disable all camera movement.
   c. When the "exit echo" mode sequence is received on C4, the CCU shall return to normal operation.

6. Serial communications protocol--
   a. The communication protocol shall consist of 8 data bits, 1 stop bit and no parity.
   b. Communication handshaking shall use XON/XOFF.
   c. The communication packet shall contain the following items: ADDRESS, CODE, DATA, CHECKSUM, CR. The packet is sent as a string of ASCII printable characters. The ADDRESS, which has its $80 bit set in order to signal the start of the packet. The CHECKSUM is generated by Exclusive-ORing the ADDRESS, CODE, and DATA. The communication byte count shall be as follows:

ADDRESS	1
CODE	2
DATA	≥0
CHECKSUM	2
CR	1

The receiver will compute the CHECKSUM. If the computed CHECKSUM is correct the receiver will send ACK, otherwise the receiver will send NAK.

10-3.25 VIDEO ENCODER UNIT

1. General --
   a. A prototype of the video encoder unit (VEU) is not acceptable.
   b. All equipment shall be off the shelf production units.
   c. All equipment shall be new and not previously used.
   d. The Contractor shall provide a manual per each unit ordered.

2. Acceptable equipment -- The VEU shall be compatible and interoperable with the existing video decoder unit (VDU), Enerdyne Technologies Model DEC1000R5. No other VEU/VDU pairs shall be different or deviate from other pairs.

3. Qualifying specifications --
   a. Video encoding --

      i. The VEU shall replicate the adaptive digital video standard (ADVS) for digital compression and transmission of video images.
      ii. The VEU shall implement frame sensitive algorithms, joint photograhic experts group (JPEG) to perform frame updating. Motion sensitive algorithms motion picture experts group (MPEG) shall not be allowed.
      iii. The VEU shall be compatible with integrated services digital network (ISDN) basic rate interface at 128 kbps and shall comply with bandwidth on demand interoperability group (BONDING) protocol.
      iv. The VEU shall be compatible with Switched-56 digital service at 56 kbps.
      v. The VEU shall be compatible with advanced digital network (ADN) service at 56 kbps.
      vi. The VEU shall be compatible with T1 service at 1.544 Mbps.

   b. Physical -- The physical size of the VEU shall be as follows:

Weight	9 kg, maximum
Height	135 mm, maximum
Width	483 mm, maximum
Depth	300 mm, maximum

   c. Mounting --
      i. The VEU shall be mountable in a standard EIA-310 equipment rack.
      ii. Each VEU shall be designed and mounted in such a way that it is easily accessible for maintenance.

   d. Primary power input and output requirements --

      i. All electrical power distribution, service and wiring components shall be UL listed or equivalent and meet the requirements of the national electric code as well as these special provisions.
      ii. Power input shall be 100 to 130 VAC at 60 Hz ±3 Hz.
      iii. Maximum power requirement shall be 25 W at 120 VAC.

   e. Local control facilities --

      i. Local operator control of all essential features of the VEU shall be accomplished by the use of necessary discrete front panel controls and/or switches. Each VEU shall have a front panel status display.
      ii. The VEU shall store operator set default parameters in EEPROM to retain system configuration after loss of power. These parameters shall be loaded into volatile RAM during operation where they may be modified by operator set operational parameters.
      iii. The VEU shall have a front panel status display.

f. Remote control facilities --

i. The control/status ports shall be EIA-232 with selectable data rates of 1200 to 9600 bps and the connector shall be a DB-25 type.

ii. The control/status ports shall provide telephone dialing, remote and local diagnostics testing, and system configuration.

iii. The control/status port shall provide selection of any of the video inputs.

iv. The control/status port shall provide in-band dialing for all interfaces using the AT and/or V.25 bis command set.

v. The control/status port of the VEU shall override in-band control of the VEU via the VDU.

vi. The control/status port shall provide selection of all network interface data rates and/or services.

vii. The control/status port shall provide user selectable video resolution. Minimum resolution settings shall be 560 (high), 280 (standard), and 140 (low) pixels per line.

viii. The control/status port shall provide user selection of 480 lines interlaced (frame mode) or 240 lines non-interlaced (field mode).

ix. The control/status port shall provide user selection of the compression algorithm quantization levels (Q-factor).

x. The control/status port shall provide cropping of the encoded video image at minimum of 104 percent (overscan), 100 percent, 85 percent, and 63 percent.

xi. The control/status port shall provide control over image attributes including, but not limited to, color hue, tint, and saturation. Section of monochrome or color of the VEU digitized video stream shall also be possible.

g. Video interface requirements --

i. The VEU shall be capable of both color and black/white video operation without modification to the hardware. Minimum motion video resolution shall be 560 pixels x 240 lines for the 525 lines, National Television Systems Committee (NTSC) standard, 60 Hz, composite input. Motion handling capability shall be up to 30 frames per second.

ii. The video interface formats shall be the following:

Composite	525 line NTSC 60 Hz
Component	Y/C: Luminance / Chrominance

iii. The video input for the VEU shall be compatible with EIA-170 at 75 Ω impedance with Bayonet Nut Connector (BNC) type connectors and shall be provided as a minimum the following:

3 video inputs composite
3 video input Y/C

All video inputs shall be software selectable from the front panel on the VEU and from the control/status port.

h. Network interface requirements --

i. The VEU shall have hardware and software selectable network bit rates with the following as a minimum: 56 kbps, 64 kbps, 112 kbps, 128 kbps, 384 kbps, 768 kbps and 1.544 Mbps (T1).

ii. The VEU shall have three network interfaces: EIA-530A, DS-1 (T1 service), and V.35.

iii. The VEU shall transmit compressed and digitized video at a minimum of 95 percent of the available channel bandwidth, whenever bandwidth is not used by telephone, camera controls and or remote diagnostics.

i. Diagnostic and alarm requirements --

i. The VEU shall have self-diagnostic features display on the front panel.

ii. The VEU shall blank video on loss of video input.

iii. The VEU shall regain video lost due to excessive temperature when the temperature has reduced to 69°C.

j. Telephone interface --

i. The VEU shall provide an RJ11 telephone jack for voice communication.

ii. The VEU shall provide 16 kbps bandwidth for telephony within the bandwidth allocated for video only when bandwidth is needed for telephone.

k. Camera control interface --

i. The camera control interface shall provide a half-duplex clear channel for camera control and status user data with the following requirements:

The port shall communicate at a user selectable data rate from 1200 to 9600 bps, asynchronous.
The port shall be EIA-232, with a Type DB-25 connector.

ii. The VEU shall provide bandwidth for camera control within the bandwidth allocated for video only when bandwidth is needed for camera control/status data transmission.

l. Environmental --

i. Minimum operating temperature range shall be from -40°C to 70°C ambient, with guaranteed start up at -10°C. An environmental housing with air conditioning will not be allowed.
ii. Operating humidity shall be from 0 to 95 percent, non-condensing.

4. Accessory items -- The following VEU equipment accessory items shall be supplied by the Contractor as specified in these special provisions.

a. Service and Operations manual describing the video VEU as ordered and in accordance with these special provisions.
b. EIA-530A cable assembly shall be provided to connect the VEU to the integrated services digital network terminal adapter (ISDN TA) unit with basic rate interface that is specified elsewhere in these special provisions.

10-3.26 CAMERA STATION

GENERAL.--
The Contractor shall furnish and install the following closed circuit television (CCTV) equipment at each camera station as described in these special provisions and as shown on the plans:

1. One camera unit.
2. One pan/tilt unit.
3. One CCTV pole.
4. One camera junction box (JCB).
5. One camera control unit (CCU).
6. One video encoder unit (VEU).
7. One integrated services digital network terminal adapter (ISDN TA).
8. Connectors and fittings as required.
9. Cable and conductors as required.

CABLES AND CONNECTORS.-- The camera unit cable assembly box mounting connector shall be mounted on one side of the JCB and shall be prewired to the 20 position terminal block as shown on the plans. The video signal pins of the camera unit cable assembly box mounting connector shall be terminated to a Bayonet Nut Connector (BNC) jack connector via Type RG-59/U coaxial cable stub.
The television control cable (TVC) and television power conductors (TVP) shall be wired to the 20 position terminal block as shown on the plans. The television control power cable (TVCP) cable shall go through but does not terminate inside the JCB. A watertight strain-relief box connector shall installed at the JCB hole for the TVCP cable.

INSTALLATION OF CAMERA STATION.-- The work to be done at each camera station, as shown on the plans, as a minimum, shall consist of the following:

1. Attach pan/tilt unit to the mounting plate.
2. Attach camera unit to the pan/tilt unit.
3. Install JCB junction box.
4. Terminate TVC and TVP inside JCB junction box.
5. Terminate television video cable (TVL) inside JCB junction box with a BNC plug connector.
6. Install camera unit cable assembly wiring harness (Type JC).
7. Terminate TVCP with pan/tilt mating connector.
8. Connect TVCP to the pan/tilt unit.
9. Terminate TVL inside controller cabinet with BNC plug connector.
10. Terminate TVP, TVCP and TVC with CCU mating connectors C1, C2 and C3, respectively.

11. Adjust limit stops of the pan/tilt unit as directed by the Engineer.

12. Adjust camera unit to provide the optimum picture for the full range of daylight and night time conditions as directed by the Engineer.

CAMERA UNIT MOUNTING.-- The camera unit shall be secured to the pan/tilt unit using the stainless steel bolts provided with the camera unit. Before each bolt is fastened, a locking type coating shall be applied to the threads. The coating shall lock the bolt and nut in place, making it impossible to turn the bolt or nut without tools. This coating shall last through and be effective through at least ten insertions and withdrawals of the bolt or nut.

The work to be done between the camera mount and the controller cabinet or between the camera mount and the DECs (in HUB building), as shown on the plans, as a minimum, shall consist of the following:

1. Install conduits, conductors, cables and pull boxes as required up to the controller cabinet.
2. Install and terminate TVL, TVC, TVCP, and TVP as shown on the plans.

The work to be done at each CCTV controller cabinet or at each DECs (in HUB building), as shown on the plans, as a minimum, shall consist of the following:

1. Install CCU.
2. Connect TVC, TVCP, TVP and TVL to CCU via their respective connectors.
3. Install VEU.
4. Install ISDN TA.
5. Connect CCU to VEU.
6. Connect ISDN TA to VEU.
7. Install 8-position connecting block.
8. Connect ISDN TA to the 8-position connecting block.

The Contractor shall furnish all materials necessary to provide a complete and functional camera station in accordance with these special provisions. Miscellaneous equipment, and materials not mentioned but necessary to provide a complete and fully operational camera station shall be furnished by the Contractor as incidental to the work for which no additional compensation will be allowed therefor.

All items furnished under this contract shall be new and shall be the latest version.

The Contractor shall be responsible for demonstrating proper operation of the camera station using test software and diagnostics which shall be provided to the Engineer as incidental items at no additional cost. Testing procedures are described elsewhere in these special provisions.

CAMERA STATION TESTING.--Upon completion of work, each camera station shall be subjected to post-installation tests as outlined herein. All testing shall be performed by the district Electrical Systems Branch personnel, arranged by the Engineer and in the presence of the Contractor. The Contractor shall notify the Engineer in writing fifteen days prior to the scheduled testing. Upon receipt of the notification, the Engineer shall contact the Electrical Systems Branch at (510) 286-4770. The Contractor shall provide all necessary equipment required to access the CCTV equipment for testing.

The testing shall consist of five consecutive days of continuous satisfactory operation of each camera station. If any material and equipment furnished and installed by the Contractor in this project is found defective or otherwise unsuitable, or the workmanship does not conform with the accepted standards, the Contractor shall replace such defective material and equipment at no cost to the State.

Rejected material or equipment may be offered again by the Contractor for consideration provided all non-compliance has been corrected and pretested by the Contractor. After all defects have been corrected, the camera station shall be re-tested until five consecutive days of continuous satisfactory operation is obtained.

The post-installation tests shall consist of, but not be limited to, inspection and functional testing in accordance with these special provisions.

Inspection shall consist of, but not be limited to, verification of correct wiring terminations, correct cable interconnections, good workmanship and compliance with these special provisions.

Functional testing shall include, but not be limited to, the following:

a. Verification of all local mode CCTV operations using the CCU front panel controls.
b. Verify video signal output from CCU with a National Television Systems Committee (NTSC) monitor.
c. Verify the correct operation of the auto/manual iris and power zoom .
d. Verify the correct operation of the pan/tilt unit. The pan/tilt unit shall be functionally tested over 355 degrees in the horizontal plane and ±60 degrees in the vertical plane. The pan/tilt unit limit stops shall then be adjusted to optimize the camera viewing coverage of the freeway as directed by the Engineer.
e. Verify the correct operation of the preset positions.

10-3.27 CAMERA POLES

Camera poles shall conform to the provisions in Section 86-2.04, "Standards, Steel Pedestals and Posts," of the Standard Specifications.

10-3.28 SERVICE MANUAL REQUIREMENTS

The Contractor shall provide to the Engineer a minimum of two copies of service manuals for the camera unit, pan/tilt unit, camera control unit (CCU), video encoder unit (VEU), and integrated services digital network terminal adapter (ISDN TA) unit under this special provisions. Each manual shall contain the following sections and sub-sections.

1. General information section --
   a. A list of applicable subassemblies that comprise the specified equipment.
   b. Overall description of the equipment design features (including all enhance features if applicable), performance, and applications.
   c. Equipment specifications summary.
   d. Equipment installation instructions.

2. Theory of operations section --
   a. Theory of operation of the standard equipment, with unique or unusual circuitry described in detail.
   b. Theory of operation reflecting any modifications to the standard equipment.

3. Maintenance section --
   a. Recommended test equipment and fixtures, or minimum operational and performance requirements for appropriate test equipment.
   b. Trouble shooting information and charts.
   c. Removal and installation procedures for replacing assemblies and subassemblies, if not obvious or if improper sequencing of steps may result in component damage.

4. Replacement parts section --
   a. Each manual shall contain an equipment replacement parts list including electrical parts, mechanical parts and assemblies.
   b. All semiconductors shall be identified by the supplier's numbers and by JEDEC numbers if applicable.

5. Diagram section --
   a. Schematic diagrams(s) identifying all circuit components and showing normal test voltages and levels.
   b. An overall functional block diagram.
   c. Detailed interconnecting diagram(s) showing wiring between modules, circuit boards and major components.
   d. Pictorial circuit board layout diagram(s) showing both component placement and printed wiring detail.
   e. Diagram(s) showing location of circuit boards and other subassemblies.
   f. Exploded view diagram(s) of complex mechanical assemblies.

6. Physical requirements --
   a. All pages, including latest revisions, shall be securely fastened together between protective covers (loose-leaf ring binding is acceptable).
   b. No page shall be subject to fading from exposure to any normal source of ambient lighting (ozalid reproduced pages are not acceptable).

10-3.29 HUB INTERNALS

Furnishing of the uninterruptible power system, and the external maintenance bypass switch are covered under "Uninterruptible Power System" and "External Maintenance Bypass Switch" elsewhere in these special provisions. All other items required for the proper installation and operation of the hub internals shall be considered as part of the hub internals.

The work to be done in hub internals, as shown on the plans, shall consist of the following:

1. Install console and distribution equipment cabinets (DECs).
2. Install uninterruptible power system (UPS).
3. Install external maintenance bypass switch (EMBS).
4. Install UPS disconnect switch.
5. Install surge suppressor device.
6. Install Caltrans telephone closet (CTC).
7. Install power to DECs and console, consisting as a minimum, installing the following:

a. Wireways
b. Duplex outlet boxes
c. Power strips
d. UPS panel board
e. Main panel board
f. Conduits, pull ropes, and wiring
g. Nameplates
h. Main disconnect enclosed circuit breakers

CONSOLE AND DECs.--

General-- Console and DECs shall be fastened together to form a single unit as shown on the plans. All components shall be of extra heavy gauge steel construction. Frames shall be 14-gauge and 16-gauge steel. Front blank panels shall be 18-gauge. Sides, doors, and tops shall be 20-gauge. All components shall be easy to assemble with nut and bolt fasteners. All units shall be of sand textured finish. All units shall have panel mounting rails, fully adjustable, front and rear rails tapped on EIA-310C spacing. All units shall have adjustable stem levelers mounted on 12 gauge steel caster brackets in base. All units shall be provided with four 1 inch diameter knockout holes in the front and rear of the base. All units shall have back doors. For the purpose of these special provisions "cabinet" includes top, bottom base, and side assemblies and a "cabinet frame" does not include side assemblies.

Console-- The console shall consist of an incline cabinet frame, upright cabinet frames, upright cabinet, top upright cabinets, writing tops, Plexiglas panels and side assemblies.

a. Incline Cabinet Frame-- The incline cabinet frame shall have 268 mm EIA-310C rack space in upper 20 degrees slope section and 760 mm of EIA-310C rack space in the lower vertical section. The incline cabinet's overall dimensions shall be 610 mm (W) x 1300 mm (H) x 780 mm (D). A 75 mm knockout hole shall be provided on each side of the cabinet . The center of the holes shall be located 275 mm from the top and 75 mm from the rear of the incline section of the cabinet.

b. Upright Cabinet Frames-- Each upright cabinet frame overall dimensions shall be 750 mm (H) x 610 mm (W) with an overall depth of 780 mm. Each upright cabinet frame shall have 525 mm of panel space.

c. Upright Cabinet - The upright cabinet overall dimensions shall be 750 mm (H) x 610 mm (W). The upright cabinet shall have 525 mm of panel space and overall depth of 780 mm.

d. Top Upright Cabinet-- The top upright cabinet overall dimensions shall be 750 mm (H) x 610 mm (W). The top upright cabinet shall attach to the top of the incline cabinet frame.

e. Writing Top-- Each writing top shall be 15.63 mm thick and made from formica, having an antique white finish. Formed spacers shall be used to mount on the top of the uprights. Angle spacers shall be used to mount in the panel space. Each writing top shall have mounting rails of 780 mm in length. The writing tops at the ends of the console shall have side assemblies and the sides of the remaining shall be excluded.

f. Plexiglas Panel-- A single piece of 6.25 mm thick Plexiglas panel shall be installed on top of the console, covering the writing tops and upright cabinets.

g. Side Assemblies-- Incline side assemblies shall be mounted on the upper sides of the incline cabinet frame.

DECs-- The DECs shall consist of a cabinet, cabinet frames Plexiglas front doors and side assemblies. Each DEC shall have 20-gauge back doors as specified in these special provisions.

Cabinet-- Overall dimensions shall be 2000 mm (H) x 761 mm (W) with an overall depth of 780 mm. The side panels shall be louvered.

Cabinet Frames-- Overall dimensions shall be 1900 mm (H) x 610 (W) with an overall depth of 780 mm.

Side Assemblies-- Exposed side of the DECs shall have a modified side assembly as shown on the plans.

Plexiglas Front Door-- Each Plexiglas front door shall have a height of 1.8 meters with a rack vertical panel space of 1.75 meters. The door shall be hinged left, clear panel space when opened 90 degrees or more, and hold closed with magnetic tape. The door shall be easily removed or installed through use of spring pin hinges. The door shall fit cabinets mentioned in these special provisions. The doors shall be made up of the following materials:

Members shall be of aluminum extrusions.
Top support shall be of minimum 3 mm thick aluminum.
Corner connector shall be 11 gauge zinc coated.
Plexiglas shall be 3.125 mm thick and smoke gray.
Magnetic tape shall be plastiform and self-adhesive.
Hinge pins shall be stainless steel.
Hinge knuckles and hardware shall be steel.
Members, top and bottom supports, shall have caustic etch and clear anodize finish.

The Contractor shall submit for review and approval by the Engineer, shop drawings of the completed assembly of console and DECs as shown on the plans.

DUPLEX RECEPTACLE BOX.--Each duplex receptacle box shall be mountable with IMC, rigid metallic conduit, EMT or flexible metallic conduit. The duplex receptacle box shall be equipped with NEMA 5-30R, 30A, 3-wire duplex straight-blade receptacles. The appropriate box cover and all necessary hardware for mounting the box shall be included as part of the duplex receptacle box. The duplex receptacle box shall be installed as shown on the plans.

POWER STRIP.--The power strip shall be designed to fit an EIA-310C rack. The power strip shall meet or exceed the requirements of UL 1950, be UL listed, and shall have the following features:

1. EMI/RFI Filter--A dual protection EMI/RFI filter shall provide common mode, line to ground, and differential mode, line to line protection. The EMI/RFI filter shall meet or exceed the requirements of UL 1449.
2. Spike and Surge Suppression-- The maximum ratings shall be at least 150 VAC rms, 200 VDC, 80 Joules (at 10/100 µs pulse), and 6,500 A peak current (at 8/20 µs pulse). The spike and surge suppressor shall meet or exceed the requirements of ANSI/IEEE C62.41 (Category A & B).
3. Outlets-- Ten switched outlets shall be mounted on the rear, and two additional unswitched outlets shall be mounted on the front panel.
4. Indicator Lights-- An indicator light shall be mounted on the front panel that shall be turned on when the unit power switch is off, and be turned off when the unit power switch is on.

The power strip shall meet the following specifications:

AC Voltage Input	120 VAC
AC Voltage Output	120 VAC
Frequency Range	47 to 63 Hz
Rated Current	24 A
Full Load Capacity	2880 VA
Input Circuit Breaker	2-pole 20 A
Filter EMI/RFI	30 A
Operating Temperature	0° C. to 60° C.
Storage Temperature	-40° C. to 70° C.
Altitude-Maximum	3000 meters
Relative Humidity	95% maximum
Power Cable AWG	3 No. 10 AWG
Power Cable Plugs-NEMA	5-30P & L5-30P
Input Receptacle-NEMA	L5-30R
Outlets-NEMA	5-20R

One power cable shall be included with each power strip. Each power strip used for a DEC requires a 1.5 meters long power cable, and the power strips used for the console require a 6 meters long power cable.

Two power strips shall be installed in the console with the orientation as shown on the plans. One power strip shall be installed at the location shown on the plans and the other in the upright cabinet frame of the console.

MAIN DISCONNECT ENCLOSED CIRCUIT BREAKERS.--The circuit breakers shall be NEMA 1 enclosed circuit breakers. The enclosure shall be surface mounted. The breakers shall be front operable and shall be capable of being padlocked in the OFF position. The circuit breakers shall be listed with UL as approved for service entrance application.

10-3.30 UNINTERRUPTIBLE POWER SYSTEM

Installation of the uninterruptible power system (UPS) is covered under "Hub Internals" elsewhere in these special provisions.

GENERAL.--This special provision defines the requirements for an on-line, solid-state, uninterruptible power system (UPS) designed to supply continuous, regulated AC power to computer-type loads with reliable and precise AC power under normal and abnormal conditions including but not limited to the temporary loss of AC power.

The UPS shall always be on-line, conditioning and regulating incoming AC power. It shall provide a reliable source of uninterruptible power with no fluctuations in AC output power during complete or partial interruption of incoming line power. The AC output of the UPS shall not be affected by the quality of the AC input.

The UPS shall include as part of its standard package the following supporting equipment:

1. Input and output AC filters.
2. Electronic AC line conditioner.
3. AC input circuit breaker.
4. DC breaker.
5. Computerized bi-directional converter.
6. Shielded isolation transformer.
7. Full keypad for complete system control.
8. Audible alarms.
9. Alphanumeric display for monitoring controls.
10. UPS bypass authorization circuit.
11. External maintenance bypass switch as specified elsewhere in these special provisions.

ELECTRICAL REQUIREMENTS.--

1. Input to UPS:

a. Current protection: Molded case circuit breaker.
b. Voltage: Either 120/208 VAC, 3 phase, 4-wire plus ground or 208 VAC, 3 phase, 3-wire plus ground.
c. Minimum line voltage range: 102/176 VAC to 132/228 VAC (i.e. Nominal +10 percent, -15 percent).
d. Minimum voltage variation accepted without discharging the battery: 176 VAC to 228 VAC (phase to phase voltage).
e. Frequency: 60 Hz.
f. Frequency range: 57 Hz to 63 Hz.
g. Frequency variation accepted without discharging the battery: User definable(+0.5 to -3.0) Hz.
h. Power factor: Between 0.8 leading and 0.8 lagging at full load and nominal input voltage.
i. Current distortion: Maximum of 10 percent total harmonic distortion (THD) and 5 percent single harmonic distortion (SHD) at nominal input voltage and at full UPS load.
j. Input transient protection: ANSI C62.41 (IEEE 587, Category B)

2. Output from UPS: Output requirements apply during both normal and emergency operation unless otherwise specified.

a. Voltage: 120/208 VAC, 3 phase, 4 wire plus ground.
b. Frequency: 60 Hz.
c. Output waveform: Sine wave
d. Power rating: 20 kVA minimum at 0.8 lagging load power factor.
e. Voltage distortion:

i. Maximum of 3 percent for any single harmonic with a linear load.
ii. Maximum of 5 percent of root mean square (rms) total harmonic distortion (THD) with a non-linear load.

f. Voltage regulation:

i. Stay within 2 percent for a steady-state condition.
ii. Within ±5 percent for 100 percent step load change with a return to 2 percent in 3 cycles.
iii. The UPS maintains output voltage regulation during battery operation all the way to low battery shut off.

g. Voltage unbalance:

i. Within ±2 percent of nominal for balanced loads.
ii. Within ±3 percent of nominal for 50 percent unbalanced loads.
iii. Within ±5 percent of nominal for 100 percent unbalanced loads.

h. Voltage transient response:

i. Within ±6 percent from nominal for 100 percent step load.
ii. Within ±4 percent from nominal for 50 percent step load.
iii. Recovery to steady-state within 35 milliseconds after any out of tolerance variation.

i. Frequency slew rate: From 0.1 to 1 Hz/sec, synchronized to input (Selectable in 0.1 Hz increments).
j. Frequency stability: Within ±0.1 percent free-running.
k. Crest factor: 3 to 1 maximum.
l. Power factor: From 0.8 leading to 0.8 lagging of the output with a purely resistive load.
m. Electrical efficiency of the UPS:

i. 94 percent minimum with AC input power.
ii. 88 percent minimum with UPS operating solely on battery power due to input AC failure.

n. Overload ratings:

i. 125 percent of full load for at least 10 minutes.
ii. 300 percent of full load for at least 10 seconds.
iii. 700 percent of full load for at least 15 milliseconds.

o. Overload protection: DC breaker for inverter input.

3. Battery system: The batteries case and cover shall be made of flame retardant polypropylene. The batteries shall have a pressure relief valve constructed to prevent acid leakage and oxidation. The batteries shall be leak proof and spill proof, shall not require special ventilation, shall not cause gassing during normal operation, and shall never require watering.

a. Batteries shall meet applicable UL 1778 requirements, meet ANSI/UL flammability tests, and shall be suitable for use in computer rooms.
b. Sealed maintenance free batteries, fully wired and enclosed in an isolated compartment within the UPS cabinet.
c. Battery backup time: The ampere-hour rating of the battery shall be sufficient to supply the DC power to the UPS while operating at full rated load for a period of no less than 7 minutes at 25° C.
d. Maximum battery charge time: 10 times the discharge rate.
e. Battery design life shall be a minimum of 10 years.
f. Current Protection: Battery input circuit breaker.

4. Mechanical:
a. Storage temperature: -20° C. to 70° C.
b. Operating temperature: 0° C. to 40° C.
c. Relative humidity: 0 percent to 95 percent, non-condensing.
d. Weight: 1,270 kg, maximum (including batteries).
e. Audible noise: 57 dB as recorded under ANSI "A" Scale at 1 meter.

5. Quality assurance:
a. The manufacturer shall have a quality assurance program with checks on incoming parts, assemblies and final products.
b. The UPS shall be "burned-in" at full rated load without failure for a minimum of 8 hours.
c. A final test procedure for the product shall include a check performance specifications before and after the 24-hour "burn-in".
d. An on-site test procedure shall include a check of controls and indicators after installation of the equipment.
e. The UPS shall be designed, manufactured and tested in accordance with the applicable portions of the following standards, in addition to Section 86-1.02 of the Standard Specifications.

i. UL Standard 1778.
ii. IEEE.
iii. NEMA Standard PE-1.
iv. OSHA.
v. ANSI C.62.41 (IEEE 587, Category B) Standards for Surge Withstandability.
vi. FCC Rules and Regulations Part 15, Subpart J, Class A.

UPS DIAGNOSTICS AND CONTROLS.--

1. User interface: The user interface shall include an EIA-232 serial asynchronous communications port and a user interface panel. The user interface panel is located on the front of the UPS and shall include an emergency shutdown control, a message screen, a control keypad, and an audio alarm.

2. EIA-232 serial asynchronous communications port: A device connected to the EIA-232 serial port shall allow the UPS to report its status, data, and alarms to a host computer upon the request of the host computer. The following operations shall be included:

a. Receive alarm information.
b. Receive alarm(s).
c. Receive utility input data.
d. Receive UPS output data.
e. Receive battery status.
f. Send change UPS mode commands.
g. Receive UPS identification.

3. Emergency shutdown control: This control shall avoid accidental shutdown by at least tripping off the AC input and battery input circuit breakers.

4. Message screen: The message screen displays all the messages available to the operator. These messages include menus, data, alarms, warnings, operating instructions, and status information.

5. Keypad: The keypad is the control keyboard for the UPS. By depressing keys in specific sequences you can start-up the UPS, change the UPS mode, and call-up messages such as menus, alarms, warnings, instructions, data, and status messages.

6. The following conditions shall result in an audible alarm signal:

a. System bypass.
b. Output overload.
c. Low battery voltage.
d. High DC voltage.
e. Inverter failure.
f. Temperature extremes.
g. Charger failure.

7. The following indicators shall appear on the message screen when conditions are warranted:

a. System normal.
b. System on battery.
c. Impending low battery.
d. Output overload.
e. Temperature.
f. Load on bypass.
g. AC input failure.
h. System on manual bypass.

8. All the following system parameters shall be displayed on the message screen when selected by the user from the keypad:

a. Input (voltage, frequency, current, and power in kW and kVA).
b. Output (voltage, frequency, current, power in kW and kVA, and percentage of full load capacity).
c. Battery (voltage, current, ampere-hour rating, and backup time remaining).
d. Bi-directional converter (voltage, current, and frequency).
e. An alarm listing indicating time, date, and type of occurrence.
f. Time and date.
g. Telephone number for auto dial to service center in the event of a service problem.
h. User-friendly prompts for start-up and shutdown sequences.

9. UPS controls:

a. Current protection:

i. AC input circuit breaker.
ii. Battery input circuit breaker.
iii. DC breaker before inverter input.

b. Manual controls:

i. Alarm silence.
ii. Emergency shutdown.
iii. Full keypad for system control.
iv. External make-before-break manual bypass switch as specified in "Maintenance Bypass Switch" elsewhere in these special provisions.

c. UPS bypass authorization circuit: This circuit shall be connected to the maintenance bypass switch (MBS) lockout circuit and shall be enabled only when the UPS needs to be relocated for maintenance purposes or when a maintenance operation requires that no load be connected to the UPS. If the UPS bypass authorization circuit is disabled, then the MBS lockout circuit shall lockout the MBS. If the UPS bypass authorization circuit is enabled, then the MBS lockout circuit shall allow external bypass of the UPS.

INSTALLATION.--

1. Input line and output distribution shall be configured for a hardwired installation.
2. The UPS cabinet shall include bracket feet for seismic anchoring to its final resting place.
3. The equipment shall be started by a customer support representative from the equipment manufacturer.
4. The bending of the utility input conduit shall meet NEC requirements, and if access ducts are required, they shall be included with the UPS.

SCHEMATICS, DOCUMENTATION AND TRAINING.--

1. Detailed circuit schematics and system drawings shall be included with the UPS system covering wiring installation, troubleshooting of the UPS and battery unit. These schematics must be complementary to the schematics shown in the User's Handbook and shall be approved by the Engineer.
2. Training at 111 Grand Ave in Oakland shall be provided for the start-up procedure, operation and troubleshooting techniques. The Engineer may approve training at another location but all travel and per-diem related expenses shall be paid by the Contractor.

WARRANTY AND SERVICE.--

1. The manufacturer shall guarantee the UPS unit to be defect-free in materials and workmanship for 12 months from the date the UPS comes on-line.
2. The UPS manufacturer shall guarantee the UPS batteries for a minimum of 5 years (full replacement, transportation and labor) with an additional 5 years of pro-rated warranty by the battery manufacturer. The total warranty shall be 10 years or longer.
3. The manufacturer shall have an established network of service centers capable of servicing said equipment.
4. Service center and manufacturer's personnel shall be on call 24 hours a day, 365 days a year. Personnel shall be factory trained and certified in the maintenance and repair of specified equipment.

10-3.31 EXTERNAL MAINTENANCE BYPASS SWITCH

Installation of the external maintenance bypass switch (EMBS) is covered under "Hub Internals" elsewhere in these special provisions.

The EMBS shall provide simple and effective means for bypassing the uninterruptible power system (UPS) while maintaining continuity of power to the traffic operations system (TOS) equipment critical loads. The EMBS shall have a lockout circuit. Unauthorized bypass of the UPS shall be prevented by connecting the EMBS lockout circuit and the UPS bypass authorization circuit. If the UPS bypass authorization circuit is disabled, then the EMBS lockout circuit shall lockout the EMBS, else it shall allow bypass of the UPS. Pilot devices to show UPS "Normal" , UPS "Bypassed", and UPS "Lockout" enabled shall be provided with the EMBS. The EMBS shall have a make before break electrical operation and shall switch three phases and neutral. The EMBS shall have a manually initiated electrical operation. A solid ground bar with connections for common ground shall be provided.

The EMBS shall be fully rated and if it is not available from the manufacturer of the UPS, then it shall be compatible with the UPS that is being furnished. The EMBS shall meet UL 1008 requirements.

The Contractor shall install the EMBS as shown on the Plans and as recommended by the manufacturer. The Contractor shall not be exempted from furnishing an external EMBS even if the UPS has provisions for an internal MBS.

The EMBS shall meet the following specifications:

Frequency	60 Hz
Nominal Voltage	208/120 VAC
Nominal Voltage Range	156/91 VAC to 239/138 VAC
Operating Temperature	0° C. to 70° C.
Storage Temperature	-20° C. to 85° C.
Humidity	95% Non-condensing

10-3.32 UNINTERRUPTIBLE POWER SYSTEM DISCONNECT SWITCH

Installation of the uninterruptible power system (UPS) disconnect switch is covered under "Hub Internals" elsewhere in these special provisions.

The uninterruptible power system (UPS) disconnect switch shall be a non-fusible heavy duty safety switch enclosed in a NEMA 1 enclosure with electrical characteristics and ratings as specified on the plans. The switch shall have insulated shields to prevent accidental contact with line terminals and a quick make-quick break mechanism to prevent contact teasing. The door of the enclosure shall be hinged. The switch operating handle shall be front-operated, have a visible red vinyl sleeve, and be padlockable in the "OFF" position. The switch assembly plus the operating handle shall be an integral part of the enclosure base. The switch shall have dual door interlocks to prevent the door from opening when the operating handle is in the "ON" position, and to prevent the switch contacts from closing when the door is open. A door latch shall securely hold the door closed and permit padlocking. Engravings on the door and labels on the right of the enclosure shall indicate "OFF" when the operating handle is in the down position, and "ON" when the operating handle is in the up position. The switch shall be UL listed, comply with UL standard UL98, and meet NEMA specifications KS 1-1990. A factory installed metal nameplate, front cover mounted, shall contain a permanent record on catalog number, switch type, and maximum ratings.

10-3.33 SURGE SUPPRESSION DEVICE

Installation of the surge suppression device is covered under "Hub Internals" elsewhere in these special provisions.

The surge suppression device shall provide effective high energy surge diversion for application in ANSI/IEEE C62.41 Location Category B3. The device shall be tested per ANSI/IEEE C62.45 using ANSI/IEEE C62.41 Category B3 waveforms and amplitudes. The device and its individual units shall be UL listed under UL 1449 Standard for Transient Voltage Surge Suppressions (TVSS) and shall have the surge ratings permanently affixed to it.

The surge suppression device shall be designed, manufactured, tested, and installed in compliance with ANSI/IEEE C62.11, C62.41, and C62.45, NEMA, and Underwriters Laboratories (UL) 1449.

ELECTRICAL REQUIREMENTS.--The system operating voltage shall be 120/208 VAC, 3 phase, 4 wires plus ground. The surge suppression device shall be installed in parallel with the protected equipment as shown on the plans.

Operating Requirements.--

Operating temperature range shall be -40° C to 55° C.
Storage temperature range shall be -40° C to 85° C.
Operation shall be reliable in an environment with 0 percent to 95 percent non-condensing relative humidity.
The system shall be capable of operation up to an altitude of 13,000 feet above sea level.
The clamping level at 1 mA shall be 220 V.
The response time shall be less than 5 nanoseconds.
The power consumption shall be 1 W per phase.
The design life shall be greater than 15 years.
The maximum continuous operating voltage shall be no less than 115 percent of the nominal rated line voltage.
The power frequency range shall be from 47 Hz to 440 Hz.

Transient voltage surge suppression shall be provided for line to neutral, and neutral to ground. The total energy rating shall be 2110 Joules (10/1000 µs waveform). The peak suppression voltage shall be 400 V at 3 kA (8/20 µs waveform).

The maximum surge current capacity per phase of the specified device, based on the standard IEEE 8/20 µs waveform, shall be at least one event at 65 kA, 100 events at 12 kA, 1,000 events at 11 kA, and 10,000 events at 5 kA. The transient suppression capability shall be bi-directional and suppress both positive and negative impulses.

The device shall be capable of interrupting a 65 kA, short circuit current delivered from the AC power line. The interrupt capability must be confirmed and documented by a recognized independent testing laboratory.

The device shall be designed so as to minimize the internal surge path impedance. Direct point-to-point internal wiring is inherently inductive and shall not be acceptable. Connection to the power service shall be constructed as recommended by the manufacturer for best performance.

SYSTEM COMPONENTS.--

Replaceable Modules.--The surge suppression device shall provide redundant protection for each phase with one field replaceable plug-in protector module and two field replaceable fuses per phase. Each protector module shall consist of

multiple 40 mm metal oxide varistors (MOV). The status of each protector module shall be locally monitored with a red LED per MOV that shall illuminate if the phase protection is reduced.

Self-Diagnostics.--Red and green solid state LED indicators shall be provided on the front cover to indicate device status. An illuminated green LED shall indicate that power is present at the device on all phases, and an illuminated red LED shall indicate that one or more of the phases have reduced protection. Together front panel and internal LEDs shall provide phase reduced protection and power present indications.

Remote Alarm Capability.--Relay alarm contacts shall be provided for remote alarm monitoring capability of device status. Normally open and normally closed contacts shall be provided with current limiting and voltage protection. Relay operation shall be continuously energized so that power failure, reduced phase protection, or a break in the remote monitoring line will cause a fault alarm indication at the remote monitor.

Enclosure.--The enclosure shall meet NEMA 12 requirements, shall have a hinged front cover, and shall be made of 14 gauge steel, with corrosion resistant hardware.

INSTALLATION AND MAINTENANCE.--The surge suppression device shall be installed in accordance with the manufacturer's printed installation instructions to maintain the warranty, and as close as possible to the panelboard to which it is connected, preferably within 2 feet of the panelboard.

Detailed maintenance instructions shall be printed on the front panel to insure the safety of maintenance personnel.

10-3.34 CALTRANS TELEPHONE CLOSET (CTC)

Installation of the Caltrans telephone closet (CTC) is covered under "Hub Internals" elsewhere in these special provisions.

The CTC shall have a surface mount frame with continuously hinged doors. The enclosure shall house one of the telephone and data line surge protector systems, mounted on a 600 mm (W) x 600 mm (H) x 15.6 mm (D), marine plywood sub-panel having two coats of insulation varnish.

The CTC shall have an ANSI 61 gray enamel finish over phosphatized surface, and shall include a key-lock handle.

The CTC shall meet NEMA 1 requirements and shall be UL listed.

10-3.35 WIREWAYS

Installation of the wireways is covered under "Hub Internals" elsewhere in these special provisions.

The wireways shall meet NEMA 1 requirements and be UL listed.

The 102 mm and 153 mm wireways shall be fabricated from a 16 gauge steel and shall have an open side for lay-in of wiring with hinge cover. Knockouts shall be provided for conduits to be installed as shown on the plans. 102 mm wireways shall allow for concentric knockouts capable of handling conduits with a diameter of up to 63 mm. Six inch wireways shall allow for concentric knockouts capable of handling conduits with a diameter of up to 104 mm.

300 mm wireways shall be fabricated from 14 gauge steel, shall have an open side for lay-in of wiring with hinge cover, and shall allow for concentric knockouts capable of handling conduits with a diameter of 125 mm.

All wireways shall have ANSI 61 gray enamel finish inside and out over phosphatized surfaces.

Wireways shall be grounded according to the provisions of NEC Article 250 and installed according to the provisions of NEC Article 362.

10-3.36 TELEPHONE AND DATA LINES SURGE PROTECTOR SYSTEM

The telephone and data line surge protector system shall be a fail-safe device designed to protect any equipment using regular telephone lines (Private Automated Branch Exchange, PABX) or low voltage data lines (data transmission networks) against transients, spikes, and lightning related over-voltages.

The surge protector system shall have a modular configuration and shall protect a minimum of 25-pairs of conductors. The system may be composed of three bases and fifteen surge protector modules. Each base shall include mounting hardware for mounting on a backboard, a printed circuit board, wiring blocks and plug-in connectors and shall accept 10-pairs of conductors. The bases for the Integrated Services Digital Network (ISDN) and T1/E1 lines shall have three rows; a row of 110 Insulation Displacement Connectors (IDC) wiring blocks for connection of the incoming lines, a row of connectors in the middle for plugging in the surge protector modules, and a row of 110 IDC wiring blocks for connection of the outgoing lines. The 110 IDC wiring blocks and the plug-in connectors shall be mounted in a printed circuit board as part of the base. The base for the dial-up lines shall have 66 clip connecting blocks for the input and output and provisions for five surge protector modules.

One row of the 110 IDC wiring blocks shall be connected to the OPDDC and the other to the DDC. Connections between DDC and the OPDDC shall correspond exactly pair by pair. The Contractor shall connect DDC and OPDDC, and install surge protector modules as follows:

OPDDC or DDC Pairs	IDC Wiring Blocks Type	Surge Protector Module Application
1 thru 10	110 Connector	ISDN
11 thru 20	110 Connector	T1/E1

The dial-up lines shall be connected as follows:

TC	IDC Wiring Blocks Type	Surge Protector Module Application
Pair 1 thru Pair 6	66 Clip Connector	Dial-up

Each surge protector module shall protect a minimum of two pairs of conductors, and shall be color coded according to the nominal voltage of the application available from 6 V up to the telephone or data line voltage level. Each surge protector module shall arrest a surge within one nanosecond and shall be capable of dissipating up to 5,000 A. The surge protector modules shall be installed for the following applications as applicable and shall have the following characteristics:

Application	Phone & Dial-up Lines	4-20 mA Loop & Leased Lines	ISDN & T1/E1 Lines	100 Mbps
Nominal Voltage	170 V	48 V	48 V	6 V
Max. Output Voltage	250 V	62 V	62 V	10 V
DC Resistance in Series	10 Ω	10 Ω	4.7 Ω	0 Ω
Leakage Current	5 µA	2 µA	10 µA	2 µA
Line Capacitance	70 pF	200 pF	<30 pF	<5 pF
Insertion Loss			<0.3 dB at 2 MHz	<1 dB at 100 MHz
Response Time	<1 ns	<1 ns	<1 ns	<1 ns

Input--

Surge Arrestor Breakdown Voltage (100 V/Sec)	190 V	55 V	55 V	7 V
Extinguish Volt with 600 Ω in series	>85 V	>85 V	>85 V	>85 V

Rated Discharge Capacity-- (3 kV/5,000 A, 2 seconds between each time.)

Shock wave 8/20 µs (10 times)	5 kA	5 kA	5 kA	
Shock wave 10/1000 µs (200 times)	240 A	240 A	240 A	
AC Current (10 times)	5 A	5 A	5 A	
AC Current (1 time)				200 A

Destruction--

Short-circuit (5 A for 12 seconds)	<1Ω	<1Ω	<1Ω	<1Ω

The telephone and data line surge protector systems shall be installed on the upper half of the backboard inside the Caltrans telephone enclosure.

10-3.37 HUB DATA

Furnishing of the access node and master traffic control unit are covered under "Access Node" and "Master Traffic Controller Unit" elsewhere in these special provisions. All other items required for the proper installation and operation of the hub data shall be considered as part of the hub data.

GENERAL.--Hub data shall consist of the following components to function as described in these special provisions and as shown on the plans:

1. One -- distribution patch panel (DPP).
2. Two -- distribution line cords
3. One -- distribution data cable
4. One -- access node
5. One -- master traffic control unit (MTCU).
6. One -- operations workstation
7. One -- Switch Ethernet Hub
8. One -- Modem assembly
9. Three -- ethernet cable assemblies
10. Two -- V.35 interface cables
11. Two -- dual tone multi-frequency (DTMF) cables.
12. Three -- coaxial cable assemblies
13. Five -- serial cables

PATCH PANELS.--The serial and distribution patch panels shall be EIA-310C rack mountable and meet the following specifications:

1. Front Interconnection -- The patch cord side shall have 32 (2 rows of 16) 8-position 8-conductor modular jacks with a T568A wiring configuration.
2. Back Interconnection -- The cabling side shall have 32 eight-position, 110 IDC wiring blocks capable of accepting solid or stranded 22 AWG to 26 AWG wires and rated to 200 retermination cycles minimum.
3. Controls -- Sixteen patching switches shall be provided to eliminate the need for patching cords in normal use. The patching switches shall connect the upper and lower ports when switched to the "PATCH" position and disconnect the upper and lower ports when switched to the "OFF" position.
4. Cable Routing and Clamping -- Thirty-two anchors for clamping cables and thirty-two 4.8 mm wide cable ties shall be provided in the back. The anchor design shall ensure that cables or cable ties do not protrude from the 89 mm or 2U boundaries in order to avoid physical interference with adjacent equipment.
5. Cover -- A back cover for complete Electromagnetic Interference/Radio Frequency Interference (EMI/RFI) protection shall be provided.

Pin assignment for the patch panels shall be as follows:

Rear View- Ports 1-16:

IDC Block Pin No.	Conductor Color	Pair ID
8	Brown	R4
7	White-Brown	T4
6	White-Blue	T1
5	Blue	R1
4	Orange	R2
3	White-Orange	T2
2	Green	R3
1	White-Green	T3

Rear View- Ports 17-32:

IDC Block Pin No.	Conductor Color	Pair ID
1	White-Green	T3
2	Green	R3
3	White-Orange	T2
6	Orange	R2
4	Blue	R1
5	White-Blue	T1
7	White-Brown	T4
8	Brown	R4

Front View- Ports 1-32:

Port No.	Row	Connector Pin Assignments
1-16	Upper row	1-8 left to right
17-32	Lower row	1-8 left to right

Distribution Patch Panels.--The distribution patch panels shall have shielded modular jacks. The patch panel shall be compliant with EIA/TIA-568-A Category 5. An option for grounding cable shielding to common or individual ground connections shall be provided for each port. The connections for individual ground connections shall accept solid or stranded 14 AWG to 26 AWG wires and shall be spring loaded. The patching switches shall simultaneously switch the 8 wires and ground path of each port.

PATCH CORDS.--The serial and distribution patch cords shall be unshielded twisted pair (UTP) and shall consist of 24 AWG tinned-copper, stranded conductors insulated with high density polyethylene (PE) or PVC. The insulated conductors shall be tightly twisted into individual pairs and jacketed with PVC. Patch cords shall have an EIA T568A wiring configuration, and a nominal length of 1 meter unless otherwise specified on the plans, in these special provisions or by the Engineer.

Patch cords (i.e. UTP patch cable) shall be color coded as follows:

PAIR	COLOR CODE (T//R)	8-position Modular Plug T568A Pin No. (T//R)
1	White/Blue//Blue/White	5//4
2	White/Orange//Orange/White	3//6
3	White/Green//Green/White	1//2
4	White/Brown//Brown/White	7//8

Distribution Patch Cords.--The distribution patch cords shall consist of 4 pairs of conductors and shall include 8-position, 8-conductor modular plugs on both ends. The patch cords shall be EIA/TIA-568 Category 5 compliant and shall meet the following specifications:

DC Resistance (maximum)	.0886 Ω / m at 20° C. (nominal)
Mutual Capacitance	44.3 pF/ m (nominal)
Characteristics Impedance	100 Ω ±15 % from 1 MHz to 100 MHz

LINE CORDS.--Line cords shall be unshielded twisted pair (UTP) and shall consist of 24 AWG stranded copper conductors insulated with high density polyethylene (PE) or PVC. The insulated conductors shall be tightly twisted into individual pairs and jacketed with PVC. Line cords shall have a T568A wiring configuration, and a nominal length of 3.7 meters unless otherwise specified on the plans, in these special provisions or by the Engineer.

Line cords for Integrated Services Digital Network (ISDN) and T1/E1 lines shall be color coded as follows:

PAIR	COLOR CODE (T//R)	110 IDC Connector T568A Pin No. (T//R)	8-position Modular Plugs T568A Pin No. (T//R)
1	White/Blue // Blue/White	5//4	5//4
2	White/Orange // Orange/White	3//6	3//6
3	White/Green // Green/White	1//2	1//2
4	White/Brown // Brown/White	7//8	7//8

Distribution Line Cords.--The distribution line cords for ISDN and T1/E1 lines shall consist of 4-pairs of conductors and shall include a 110P connector on one end, and on the other end a 8-position, 8-conductors modular plug. The distribution line cords shall be EIA/TIA-568A Category 5 compliant. The distribution line cords for dial-up lines shall consist of 2-pair of conductors. One end of line cords shall be terminated with 110P in conformance with T568A wiring and the other end shall be terminated with a 4-position, 4-conductor plug in conformance with USOC wiring. Pairs one and two at one end shall match pairs at the other end. The distribution line cords shall meet the following specifications:

DC Resistance (maximum)	.0886 Ω / m at 20° C. (nominal)
Mutual Capacitance	44.3 pF/ m (nominal)
Characteristics Impedance	100 Ω ±15 % from 1 MHz to 100 MHz

ETHERNET CABLE ASSEMBLY.--Each ethernet cable assembly shall be an 50 Ω thinnet coaxial cable (IEEE 802.3 10 Base 2; NEC CL2). The cable shall be terminated with BNC plug, bayonet-lock connectors on each end. The cable assembly shall meet the following specifications.

Center Conductor	20 AWG stranded (19x32)
Jacket	PVC; O.D. 4.6 mm
Dielectric	cellular polyethylene; O.D. 2.6 mm
Shielding	foil 93% tinned copper braid
DC resistance D.C.R	.0190 Ω/ m
Velocity of Propagation	78 % nominal
Capacitance (nominal)	88.6 pF/ m
Attenuation	.0141 dB/ m at MHz
Connector	Nickel plated brass, polyethylene insulator, gold plated contact, tarnish and corrosion resistant.

HUB DATA INSTALLATION.--The Contractor shall provide a crew with a minimum of three years of experience installing telecommunications wiring systems and computer networks to perform any work on the hub data.

The work to be done in hub data installation , as shown on the plans, shall consist of the following:

Data distribution system.--

1. Install one DDC.
2. Install DPPs
3. Terminate DDC in the Caltrans telephone closet (CTC) and in the DPP.
4. Install ISDN TAs.
5. Furnish distribution line cords.
5. Furnish fractional channel service unit/data service unit (CSU/DSU).
6. Install and connect distribution line cords between ISDN TA and fractional CSU/DSU.
7. Terminate distribution line cords in the DPPs.
8. Furnish V.35 interface cables.
9. Install and connect V.35 interface cables between ISDN TA, video encoder units (VEUs), fractional CSU/DSU and access node.

The DDC shall be separated into ten pairs for ISDN, ten pairs for T1/E1 lines.

The two pairs of conductors for each T1/E1 line of the DDC shall be terminated on pairs one and three of each 110 IDC on the DPPs in conformance with T5678A wiring.

Local area network.--

1. Install MTCU, workstation, access node, and CCP.
2. Furnish ethernet cable assemblies.
3. Install and connect ethernet cable assemblies between MTCU, access node and operations workstation.

HUB DATA INSTALLATION TESTING.--The Contractor shall provide all of the test equipment necessary to perform all tests specified in these special provisions. All testing shall be performed in the presence of the Engineer and documented results shall be submitted to the Engineer for approval before the next phase testing is started.

As a minimum, the following equipment, or equal shall be used to test the hub data:

Telecommunications Techniques Corporation's Firebird 6000 and Wavetek's Lantek Pro XL.

Local area network.--Upon completion of work, each ethernet coaxial cable assembly shall be tested by the Contractor. Any cable found to have faults shall be replaced. The testing shall utilize a time domain reflectometer.

For the purpose of these special provisions, a fault in a coaxial cable assembly is defined as any of the following:

1. A return loss measurement indicating that there is a short in the cable.
2. A return loss measurement indicating a cut or open circuit in the cable.
3. A visual inspection which reveals exposure or damage to the cable shielding.

Data distribution systems.--All three phases of testing shall be performed on the data distribution system.

Phase one testing.--Each pair in the wiring system shall be tested for continuity, polarity, open, short, split, cross twist, and reversed circuits (pairs).

Phase two testing.--After the completion of phase one testing, the Contractor shall test every circuit for attenuation and near-end-crosstalk (NEXT). NEXT testing shall be inclusive of all terminations including DPPs, SPPs, patch cords, serial line cords, etc.

Final testing.--The Contractor shall perform bit error rate (BER) analysis, timing measurements, and other test associated with the physical layer (OSI layer 1) of each network link as directed by the Engineer. After the transmission impairments testing and the access node has been installed, the Contractor shall configure the customer premises equipment (CPE) to the network and perform network routing tests.

10-3.38 ACCESS NODE

Installation of the access node is covered under "Hub Data" elsewhere in these special provisions.

General.--Each access node shall be comprised of an access router/bridge modules, and a node processor module. Each access node shall establish end-to-end connections for integrated transport of local area network (LAN) and serial data over wide area links in a frame relay network. Each access node shall include multiple levels of network level priority to provide service guarantees for delay-sensitive traffic.

Software and configuration of each access node shall be locally accessible from a "workstation" and remotely accessible from any other access node linked to the same frame relay network.

The access nodes shall be used to form the nodes of a frame relay network by constructing a pure peer level data network of up to 10,000 physical nodes that can accommodate up to 90,000 physical data ports.

Connectivity and control features shall be as follows:

1. Data compression and encryption.
2. Automatic re-route and dial back-up mechanisms to circumvent link failures.
3. On-line reconfiguration.
4. LAN and serial data integration.
5. Ethernet, wide area network (WAN) and serial data support.

Remote management shall be capable of:

1. Remote download of system software.
2. Remote download/upload and storage of device configuration.

Access Router/Bridge Modules.--The access router/bridge modules shall be a microprocessor-based devices that performs bridging and routing functions amongst LANs. The modules shall support these functions either locally or via WAN connections. The access router/bridge modules shall be configurable to support all interfaces and protocols listed below and the configuration shall be stored in nonvolatile memory.

Complexity shall be reduced in the WAN by creating a logical data link layer path from each router to every other router bordering the WAN to eliminate network layer routing at intermediate nodes.

The access router and bridge modules shall meet the following requirements:

1. The LAN interface shall be Ethernet IEEE 802.3 with one 10BASE-2 port and one 10BASE-5.
2. The WAN interfaces shall be as follows:

   a. CCITT V.35 interface-- Provide two serial ports, made with female 34-pin "M-series" connectors. The interface shall meet the full T1 and fractional T1 link specifications and allow Nx64 kbps or Nx56 kbps data transmission up to 2.048 Mbps (T1/E1)

   b. The primary serial port shall be used to establish a backup connection for the WAN interface linked to the frame relay network. The primary interface shall automatically detect failing leased line bandwidth (i.e. frame relay service) and replaces it with dial-up bandwidth (i.e. ISDN BRI service). The dial-up bandwidth shall be automatically removed when the leased line bandwidth is restored.

   c. The secondary serial port shall be used to connect to an ISDN TA unit and dial-up to another access node automatically if the frame relay network physical or logical link fails.

   d. The access node shall automatically switch to the secondary serial port whenever the frame relay network link fails and automatically return to the primary serial port when the frame relay link becomes available.

3. The WAN protocols shall be as follows:

   a. Frame Relay Access/Switching-- 1024 DLCI/Port Frame Relay protocol network access. Compatible with full T1 and fractional T1 frame relay protocols.

   b. Point to Point Protocol-- The access node shall provide point to point protocol (PPP) over transmission control protocol/internet protocol (TCP/IP).

4. The network layer protocols shall be as follows:

   a. Internet Protocol-- The access node shall support internet protocol (IP) over transmission control protocol (TCP).

   b. Internet Control Messaging Protocol-- The access node shall support internet control messaging protocol (ICMP) over user datagram protocol (UDP).

5. The routing protocols shall be a routing information protocol (RIP). The access node shall provide RIP over TCP/IP.
6. Meet the IEEE bridging standard, IEEE 802.1d and support spanning tree or transparent bridging.

Node Processor Module.--The node processor module shall be a self-managing packet switch designed to operate as a node in advanced packet networks. The node processor module shall store software for data switching, packet assembly/disassembly, host interface, diagnostics, and network management in battery backed-up nonvolatile RAM. NiCAD batteries shall be used to retain information in program and configuration RAM for up to 30 days with no power applied.

After initialization, the access nodes shall exchange setup information and automatically generate routing tables for the entire network. New access nodes can be dynamically added or deleted at any time in any location without interruption to network operation. At connection, a new access node becomes an operating part of the network as soon as it broadcasts the necessary setup information to all other access nodes.

The node processor module shall meet the following:

1. Central Processor Unit --

a. Memory:

Boot	256 kB PROM
Data Buffer	2 MB RAM
Configuration	128 kB non-volatile RAM
Program	896 kB RAM

b. Performance:

Data throughput	96 kbps
Node delay	5-6 ms/packet Nominal

2. Serial Interfaces-- The access node shall be capable of handling five serial interface modules each having up to two serial ports. Serial ports shall support packet communications as specified in the CCITT Recommendation X.25, and serial communications as specified in EIA-232 and EIA-449.

a. CCITT V.28-- Provide four serial ports, DB-25 female connectors, EIA-232. Asynchronous data rates shall be up to 38.4 kbps. Synchronous data rates shall be up to 128 kbps.

3. Serial Data Protocols--SNA/SDLC/QLLC, HDLC, DDCMP, asynchronous.
4. Access Protocols--

a. Frame Relay Access/Switching-- Provide 1024 DLCI/Port Frame Relay protocol network access with a frame relay access device (FRAD) to the serial interfaces. The FRAD shall be compatible with full T1 and fractional T1 frame relay protocols. The serial interfaces data shall be integrated with the LAN interface data by encapsulating the data in a data link layer protocol such as Frame Relay or a network layer protocol such as TCP/IP.

5. Network Management--

a. Simple Network Management Protocol-- Allows management by any simple network management protocol (SNMP)-based manager acting as a SNMP agent.
b. Management Information Base II-- management information base (MIB) II allows configuration for management of multi-vendor products compliant with SNMP.
c. TELNET-- The TCP/IP (Transmission Control Protocol/Internet Protocol) application program that provides virtual terminal emulation. It allows a device on the TCP/IP network to emulate a dumb terminal in order to communicate with other devices on the network.

6. Security--

a. Filtering-- Access control filtering at data link and network layers.
b. Network Level Module-- Support US Department of Defense Network Level Module (NLM) for the secure traffic of IP datagram traffic.
c. Cryptographic Resynchronization-- Crypt-resync at serial interfaces to allow resynchronization with external encryption devices.
d. Encryption-- Proprietary data encryption for ASCII data shall be available on all data across the network or on a point-to-point or call-by-call basis.
e. Network Security-- Unauthorized intrusion into the network shall be prevented by requiring any access node to provide correct identifying information in an encrypted format before it can become part of the network. The access node shall be capable of logically dividing a network into multiple virtual sub-networks each with its own closed user access groups to protect different users and resources.

7. Compression-- ASCII data compression shall be available on a call-by-call basis.

Mechanical Requirements.--The access node shall consist of a single, EIA-310C rack mountable, physical enclosure.

Environmental Requirements.--

CHARACTERISTIC	REQUIREMENT
Ambient Operating Temperature	0° C. to 45° C.
Relative Humidity	10% to 90% (non-condensing)
Maximum Altitude	3050 meters
Ambient Storage Temperature	-20° C. to 40° C.

Power Requirements.--

CHARACTERISTIC	REQUIREMENT
Voltage	90 VAC to 265 VAC
Power Cord Type	3-14 AWG, 12 AMPS Maximum Current
Power Cord Connectors	NEMA 5-15P & CEE-22 Female Receptacle
Frequency	47 Hz to 63 Hz

10-3.39 CHANNEL SERVICE UNIT/DATA SERVICE UNIT

Installation of the channel service unit/data service unit (CSU/DSU) is covered under "Hub Data" elsewhere in these special provisions. The CSU/DSU shall be compatible with T1 and fractional T1 services operating at up to 1.544 Mbps and shall interface the access node to the telephone company (TELCO) frame relay services.

Network Port Interface.--The network port interface shall be a female 8-position RJ48C modular jack. The interface's line coding shall be selectable to either alternate mark inversion (AMI) or Binary-8-Zero suppression (B8ZS). The interface's frame format shall be selectable to either D4/Superframe (SF) or Extended Superframe (ESF). The network port interface must meet the specifications for T1 and fractional T1 services operating up to 1.544 Mbps. The network port interface shall be fully compatible with public frame relay wide area network (WAN) services.

Data Port Interface.--The data port interface shall be synchronous CCITT V.35 with a 34-pin "M-series", female connector. The interface data rates shall be N x 56 kbps or N x 64 kbps where N = 1 to 24.

Cable Assembly.--A cable assembly 3 meter long with a male 34-pin "M-series" connector on each end shall be furnished by the Contractor.

System Timing.--The clock sources shall be selectable to internal, frequency locked to data port, or frequency locked to network port.

Specifications and Regulation Compliance.--The CSU/DSU shall meet the requirements of AT&T Technical Publications TR62411, TR54016 and either TR54019A1 or TR54019A. It shall also meet the requirements of ANSI T1.403, UL 1459, FCC Part 68, and FCC Part 15 Subpart A.

Programming.--The CSU/DSU unit shall include a front panel with a display and a keypad for programming and configuring the unit.

Network Management.--The CSU/DSU shall include an integral SNMP agent which shall not require additional hardware to support a proxy. The SNMP agent shall be compatible with the internet standards RFC 1157 (SNMP), RFC 1213 (MIB-II) and RFC 1232 (DS1 MIB). The SNMP agent shall connect to the WAN through a terminal server over asynchronous EIA-232 serial links and shall communicate using serial link internet protocol (SLIP).

Diagnostics.--The CSU/DSU's built-in software shall include test pattern generators and detectors with error counters to perform self diagnostic tests and loopback tests via local, remote or CSU.

Power Requirements.--Power requirement shall be 115 VAC at 60 Hz (±1 percent).

Mechanical Requirements.--The CSU/DSU shall have a maximum weight of 55 kg.

Environmental Requirements.--Operating temperature shall be 0° C. to 40° C. and operating humidity shall be 0 to 90 percent (non-condensing).

10-3.40 SWITCH ETHERNET HUB

Installation of the switch ethernet hub is covered under "Hub Data" elsewhere in these special provisions.

Each switch ethernet hub shall be compliant with the standards: IEEE 802.3 10BASE-T and IEEE 802.3u 100BASE-T. The switch ethernet hub shall have 24 switched 10BASE-T Ethernet ports, built-in 2 dual-speed 10-/100-Mbps UTP ports and high-speed uplinks to accept 10-/100-Mbps UTP or single mode fiber optic interface (100BASE-FX). All items on each switch ethernet hub shall be CE approved. A rack-mount bracket shall be included for mounting the unit on a 48.3 mm EIA 310 rack cabinet.

Each switch ethernet hub shall meet the following requirements:

Backplane Bandwidth	1024 Mbps
MAC Addresses	8K (Hardware).
Forwarding Rate	Up to 650,000 pps
Filtering rate	Up to 650.000 pps
Full-duplex support	On both 10- and 100-Mbps
LED Indicators	LINK, RX, COL/FD
Power	120 VAC, 40W
Weight	4 kg max.

10-3.41 MODEM ASSEMBLY

Installation of the modem assembly is covered under "Hub Data" elsewhere in these special provisions.

DESCRIPTION.--Each Modem Assembly shall consist of eight Rack Mount Modem units, one Patch Panel, two Octopus Cables, sixteen Type A Patch Cords, sixteen Type B Patch Cords, one Trunk Cable, and thirty-two DB25/RJ11 connectors.

MODEM UNITS.--Each Rack Mount Modem Unit shall be a MultiTech model MT2834BA modem

PATCH PANEL.--Each Patch Panel shall be Black Box Model GC-JPM023 Modular Patch Panel (32 port RJ-11-6C, 48.3 mm x 89 mm),with USOC wiring configuration, or equal.

OCTOPUS CABLE.--Each Octopus Cable shall consist of 25 pairs of No. 24 solid copper conductors, twisted in pairs. Each cable shall be EIA/TIA-568-compliant with a PVC outer jacket. One end shall be terminated in a 50-pin telco-type plug connector, and the other end shall be terminated in 8 RJ-11-6C connectors, with USOC wiring configuration. Each telco-type connector shall have FCC Part 68f-compliant gold-plated contacts. Each cable shall be 2 M in length, minimum.

TYPE A PATCH CORDS.--Each Type A Patch Cord shall consist of 6 No. 28 conductors. Each cable shall have a gray, flat satin PVC outer jacket. Each end shall be terminated in an RJ-11, 6-wire connector, with straight-through pinning. Each cable shall be 1 M in length.

TYPE B PATCH CORDS.--Each Type B Patch Cord shall consist of 6 No. 28 conductors. Each cable shall have a gray, flat satin PVC outer jacket. Each end shall be terminated in an RJ-11, 6-wire connector, with straight-through pinning. Each cable shall be 2 M in length.

TRUNK CABLES.--Each Trunk Cable shall consist of 25 pairs of No. 24 solid copper conductors, twisted in pairs. Each cable shall be EIA/TIA-568-compliant with a PVC outer jacket. One end shall be terminated in a 50-pin telco-type plug connector, and the other end shall be terminated in a 50-pin telco-type socket connector, with pin-for-pin wiring configuration. Each telco-type connector shall have FCC Part 68f-compliant gold-plated contacts. Each cable shall be 30.5 M in length, minimum.

DB-25/RJ-11 CONNECTORS.--Each DB25/RJ11 Connector shall be Siemon Model DB25M-MJ6K Modular to D-sub Adapter Kit, DB-25 male to RJ11-6C, or equal. The connector hoods shall be blue.

10-3.42 HUB ANALOG VIDEO

Furnishing of the video encoder unit, video monitor, and analog video routing switcher unit described below are covered elsewhere in these special provisions.

GENERAL.--Hub analog video shall consist of following components, to function as described herein and as shown on the plans:

1. One video encoder unit.
2. One color CCTV monitor.
3. One analog video routing switcher unit (AVS).
4. Coaxial cable assemblies as required.

COAXIAL CABLE ASSEMBLY.--Each coaxial cable assembly shall be a standard RG 59/U, 75 Ω coaxial cable, terminated with BNC plug connectors on each end. The cable shall have stress relief PVC boots integrally molded to connector body and cable jacket. The connectors shall be bayonet-lock, two piece crimp type. The cable assembly shall meet the following specifications:

Center conductor	22 AWG solid copper.
Dielectric	solid polyethylene; O.D. = 3.7 mm. nominal.
Shield	Bare copper 95% covered
Jacket	polyvinyl chloride PVC; O.D. = 6.15 mm. nominal.
Attenuation	6 dB/30 at 1 MHz.
Velocity of propagation	66% nominal.
Nominal capacitance	21.0 pF/300 mm.
Connector	Nickel plated brass, polyethylene insulator, gold plated contact, tarnish and corrosion resistant.

The length of each coaxial cable assembly shall be as shown on the plans.

INSTALLATION.--The work to be done, as shown in the plans, in general, shall consist of the following:

1. Install AVS.
2. Install video encoder.
3. Install color CCTV monitor .
4. Install and connect coaxial cable assemblies from video encoder unit (VEU) to AVS.
5. Install and connect coaxial cable assemblies from video monitors to AVS.

ACCEPTANCE TEST.--Each coaxial cable assembly shall be tested by the Contractor. Any cable found to have faults shall be replaced. The testing shall utilize a Time Domain Reflectometer (TDR).

For the purpose of these special provisions, a fault in a coaxial cable assembly is defined as any of the following:

1. A return loss measurement indicating that there is a short in the cable.
2. A return loss measurement indicating a cut or open circuit in the cable.
3. A visual inspection which reveals exposure or damage to the cable shielding.

The Contractor shall verify that the AVS conforms to the specifications as specified elsewhere in these special provisions and is fully operational after installation. The Contractor shall provide an NTSC signal at the inputs of the AVS and switch it to any of the outputs into a NTSC video monitor. The switching shall be done both manually or and using a portable IBM or compatible computer. All tests shall be performed in the presence of the Engineer.

As a minimum, the following equipment shall be used to test the AVS:

1. One portable IBM or compatible computer.
2. One NTSC video monitor.
3. One NTSC generator.

The Contractor shall provide all of the test equipment necessary to perform all tests specified in on these special provisions.

10-3.43 COLOR CCTV MONITOR

Each CCTV monitor shall be color and fully compatible with TV standards; NTSC, PAL and SECAM. Each CCTV monitor shall meet or exceed EIA-170A. A rack mounting bracket shall be provided to mount two monitors side by side on a 475 mm EIA standard rack.

Each CCTV monitor shall meet the following requirements:

CRT type	225 mm High Resolution (HR) Trinitron Tube, minimum visible picture size 200 mm measured diagonally, 70 degree deflection, AG pitch 0.25 mm
Resolution	Composite video: 450 TV lines measured at center.
Frequency response	6.0 Mhz (-3.0dB)
Aperture Correction	-4.0dBs to +6.0dB (@3.0MHz)
Synchronization	AFC time constant 1.0 msec
Normal scan	6% overscan
Underscan	3% Underscan
Horizontal Linearity	Less than 7% (Typical)
Vertical Linearity	Less than 7% (Typical)
Convergence	Central: 0.5 mm (Typical) Corner: 0.5 mm (Typical)
Raster size stablility	Horizontal: 1.0% Vertical: 1.5%
High Voltage regulation	3.0% (Cut off to High light)
Color Temperature	6500K
Operating temperature	0 to 35°C
Storage temperature	-10 to 40°C
Humidity	0 to 90%
Power Requirement	AC: 120V, 60Hz
Power Consumption	40W (Typical)
Video Inputs	Line A/Line, Line B: Composite video, Loop-though BNC connector, 1Vp-p±6dB, Sync Negative, Automatic 75-ohm Termination
Component Video input	BNC Connector Red/R-Y: Non-composite: 0.7Vp-p±6dB, Positive, 75-ohms terminated Sync on Green/Green/Y: Composite: 1Vp-p±6dB, Positive, Non-composite: 0.7Vp-p±6dB, positive, 75-ohm terminated Blue/B-Y: Non-composite: 0.7Vp-p±6dB, positive, 75-ohms terminated
External sync	Loop-through BNC Connector 4.0Vp-p±6dB,negative, Automatic 75-ohm termination
Audio Input	Line A/Line: Loop-through BNC connector, -5dBs, high impedance Line B: Loop-through Phono connector, -5dBs, high impedance RGB/Y, R-Y, B-Y: Phono Connector, -5dBs, high impedance
Speaker output	0.5W, 8 ohms, monaural
Physical	Weight: 7.5 kg maximum Dimensions: 212.5 mm(W) x 212.5 mm(H) x 350 mm(D)

10-3.44 ANALOG VIDEO ROUTING SWITCHER

Installation of the analog video routing switcher (AVS) is covered under "Hub Analog Video" elsewhere in these special provisions.

GENERAL.--The AVS shall consist of a routing system, power supply, controller module with firmware, general purpose interface, one universal control panel with X-Y capability, and a rack-mount kit.

Each AVS shall be configured for 8 inputs by 16 outputs.

The routing system shall consist of a video/video (8 input, 16 output) matrix frame, one controller/change over package, a 12 video crosspoint module and two video output modules.

All active circuitry shall be mounted on plug-in printed circuit modules with each circuit module being fully solder masked and bearing complete silk screened component legends. All printed circuit module edge connectors shall be gold plated. Integrated circuits shall be mounted in sockets for ease of replacement. Module extenders shall provide full access to all board mounted components and shall provide all electrical connections required for functional board testing. For safety, module extenders shall not permit power supplies to be operated outside the frame.

All power supply voltages shall be individually regulated on each printed circuit module. The fuses shall be quickly and easily replaced (i.e., not soldered in place). Input voltages to each module shall be pre-regulated.

Each type of printed circuit module within the matrix shall be fully interchangeable with all other modules of the same type. Reprogramming shall not be required when modules are interchanged.

Video output modules shall incorporate provisions for plug-in installation of fixed cable equalizers.

Video input/output connectors shall be female BNC type connectors.

VIDEO PERFORMANCE.--All of the special provisions in this section shall be applicable to a system operating in a NTSC environment.

Video inputs shall be 75 Ω internally terminated. Input return loss shall be 40 dB or greater to 8 MHz.

The system shall provide DC restoration of video input signals in order to ensure a clean bounce-free switch. It shall be possible to intermix DC coupled and DC restored types in the same system, recognizing that when this is done DC offsets at the output may differ (a function of DC offsets which may be present at any input to a DC coupled module). Two 75 Ω source terminated outputs shall be provided for each bus. Return loss shall be 40 dB or greater to 8 MHz.

Transmission characteristics at system level shall be 1 V p-p nominal, 2 V p-p maximum. Gain scatter shall not exceed 0.1 dB (measured at 1 MHz) for any input to output path. Timing scatter shall not exceed ±1.0 degree for any input to one output.

Frequency response (linear distortion) shall be flat within +0.15 dB over the range of 100 kHz to 5.5 MHz. The response at 8 MHz shall drop off no more than 0.5 dB from the flat range. The 2T pulse and bar response shall be 0.25 percent or better for pulse shape, bar slope, and pulse-to-bar ratio. Field rate tilt (low frequency square wave response) shall not exceed 0.5 percent.

Differential gain (non-linear distortions) shall be no more than 0.1 percent measured at 1 V p-p output over the range of 10 to 90 percent APL. Differential phase shall be no more than 0.1 degree measured at 1 V p-p output over the range of 10 to 90 percent APL.

Crosstalk shall be less than -60 dB at 8 MHz. The measurement shall be made by terminating any single input in 75 Ω (the "friendly" input) and driving all other inputs with a 1 V p-p phase-coherent "hostile" signal. The hostile signal shall be selected on all busses except the one under test. The friendly signal shall then be selected on the test bus and the amplitude of the hostile signal artifacts observed.

System signal-to-noise ratio shall be greater than 75 dB measured as p-p signal versus rms noise over a bandwidth of 5.5 MHz. Switching transients shall not exceed 30 mV.

The blanking level of output video signals shall be 0.000 V ±50 mV except when selecting inputs served by DC coupled modules .

A sync input shall be provided to lock the system trigger pulse generator to plant sync. Switch timing shall be adjustable and shall be factory set to line 10 of the vertical interval. Sync stripper circuitry shall be incorporated to allow the substitution of color black or any 1.0 V p-p nominal video signal in lieu of composite sync. In the absence of a reference signal, the trigger pulse generator shall free run and provide random switch timing.

MATRIX CONTROL SYSTEM.--All matrix control functions, including interface to external control devices, shall be managed by a single microprocessor based printed circuit module. The system shall be designed to accept backup control module in a "hot standby" configuration. It shall be possible to program this backup module with absolute/transcode input tables and input exclusion tables different from those programmed to the primary controller if desired to accommodate changing operational scenarios. It shall be possible to copy the contents of these tables from one module to the other at any time without interrupting system operation.

The system shall incorporate a changeover printed circuit module to provide a "fail-safe" means for selection of either the main or the backup control module. The chargeover printed circuit module shall also meet the following requirements:

1. Include circuits which monitor the operation of the two control modules. Status LEDs and alarm relays shall provide an immediate indication of failure of either module.
2. Provide both manual and automatic operating modes. Manual selection of either control module shall be possible in either mode. In the automatic mode, failure of active control module shall cause the alternate module to be automatically selected if, the alternate module is present and functional.
3. The design shall ensure that the changeover module itself does not constitute a single point failure mechanics. Specifically, no control paths shall pass through the changeover module nor shall the presence of the changeover module be required for proper operation of the system.
4. The changeover module, as well as the primary and backup controller modules shall be accommodated within the main electronics frame of the system and shall be powered by the system power supply and any backup power supply in place for that frame.
5. The changeover printed circuit module shall incorporate a real-time system clock with battery backup. This clock shall be the time reference for reporting of system abnormalities, and actuation of timed salvo switching when appropriate.

User control shall interface to the system via a high speed bi-directional command bus. Physical connection shall be via a 75 Ω coaxial cable, terminated using female BNC connectors. In addition, the user control of the system shall also meet the following requirements:

1. A minimum of four command bus connections shall be provided, each with its own line driver and receiver. Failure of one of the command buses shall not affect the operation of the other three.

2. The command busses shall employ time division multiplex techniques to allow up to 240 distinct control devices to be connected to the system. The data rate shall be sufficiently high to assure that under normal circumstances, a control panel command is processed in 150 milliseconds or less. The number of control device addresses (polling addresses) shall be user-selectable from minimum 15 upward to maximum 240, ensuring fastest possible command bus speed.

3. Physical connection of a control device to the command bus shall be by means of a loop through. Control devices shall have only a single female BNC connector, making it obligatory to effect the connection with a "Tee" fitting. Thus, it shall not be necessary to break the control loop to remove or replace a control device.

4. All interfaces to the command bus shall incorporate hardware and software interlocks to prevent the accidental transmission of random or spurious data in the event of a control device failure.

5. The command bus shall be designed so that failure of a line driver on a single control device shall not affect the operation of other control devices on the bus.

6. Proper command bus operation shall be possible with as much as 1,000 m of coaxial cable.

The system shall provide a dedicated EIA-422 serial port for maintenance x-y control. This port shall be distinct from the command bus ports.

The system shall provide a dedicated EIA-232 port for a system installation and maintenance terminal. This port shall support a simple ASCII message protocol and shall be capable of operating at several standard baud rates in the range of 300 to 19200 baud.

The installation and maintenance terminal port shall provide access to the following functions:

Define matrix maximum input.
Maximum output.
Maximum level.
Write/edit input transcode table.
Write/edit remote control panel programming table (for button-per-source single destination panels and machine control options).
Set/read system real time clock.
Display system module map.
Display system error registers.
Display control panel serial data.
Display system memory.
Display system protect map.
Log (when used with external printer) protect transactions (or both type transactions).
Create, preview, and erase "Phantom Transcodes."
Capable of instituting multiple switching transactions in response to control panel commands.

Installation dependent data entered through the terminal shall be stored in electrically erasable read-only memory (EEPROM).

Matrix status shall be stored in battery protected random access memory (RAM). Each control module shall have sufficient memory to store the status of a 8 input by 16 output 4 level matrix. The battery supply shall have sufficient capacity to power the memory for minimum 10 days.

The matrix control system shall support, as a minimum, the following bus-specific commands, specified input with the input currently selected:

"take" specified input
"tally" bus status
"protect" bus
"chop"

The system shall include two registers for storage of true salvo (simultaneous ganged-bus) switching.

The command bus protocol shall be designed to prevent execution of an erroneous or improperly formatted command message.

External control devices (including control panels) shall append a checksum to all messages sent to the matrix control module. Upon receipt of a message, the active control module shall compute its own checksum and compare it to the value of the received checksum. Processing of the command shall take place if, and only if, the two checksums match. If they do not match, the control module shall transmit an error response on the command bus.

The matrix control module shall perform the following checks on all messages received:

Command op-code validity.
Proper message format.
Crosspoint presence.

Crosspoint protect status.
Checksum match.

Failure of any of these checks shall cause the control module to abort further processing of the message and to transmit a coded error response on the command bus. Maximum permissible length of logic inter-connect cables between matrix levels, for example video, shall be 150 m.

The system shall accept multiple control panel types. Among these shall be:

1. Single destination, capable of all-level (or semi-permanently designated single level or ranges of levels) switching, employing keypad entry.
2. Single destination, capable of "breakaway" switching, employing keypad entry.
3. Single destination, permitting single button source selection to a semi-permanently assigned level (or group of levels). Sources up to 32, assigned to this panel, shall be selected from the maintenance/ installation terminal and shall not be changeable by an operator from the panel itself. Different source assignments shall be possible for each panel of this type.
4. Multi-destination, permitting field programming of destinations driven by each of eight operational segments. These segments shall individually display the source currently selected to their associated destination. These panels shall also be capable of being programmed on-site to operate a single level, any combinations of levels, or all levels of system. It shall also be possible to program these panels for control of no levels, effectively locking the panel from unauthorized operation. Multi-destination panels shall incorporate ten storage registers for multi-level "breakaways" which may be recalled at any time for use to any destination represented on the panel. Programming the front of the panel, requiring the holding down of two switches plus actuation of a third to enter each programming mode. Such panels shall incorporate front-panel accessible diagnostics to test all indicators and readouts.
5. Multi-destination, as specified elsewhere, adapted to simultaneous use with a companion machine control system.
6. Alpha-numeric X-Y type capable of selecting, by number, a destination; reporting the source currently selected to that destination; and subsequently changing the source selection. This panel shall support "breakaway" switching involving any grouping of system levels.
7. Maintenance X-Y type panel shall operate via an EIA-422 port and be capable of two handed numeric destination/source selection while in the "normal" mode. Entry of a two digit security code shall cause the panel to enter a "maintenance" mode in which single-handed numeric switching is permitted. This mode also will permit such maintenance functions as sequential switching of sources to a single destination or of multiple sources sequentially to a single destination.

The system shall support multiple optional interface devices which permit bi-directional communication between the matrix and "dumb" terminals or computers equipped with a serial data port. Each interface device shall have physically separate and electrically dissimilar ports for maintenance/installation and operational purposes to prevent unauthorized access to configuration tables. Each interface device shall support a minimum of two operational ports, each of which shall be independently configurable for:

EIA-422 or EIA-232 operation.
BAUD rate.
Parity.
Number of stop bits.
Number of data bits per character.
Signaling protocol.
Exclusion of control over any destination or list of destinations (which need not be contiguous).

There shall be a machine control system which fully integrates with the routing system.

1. The machine control system shall utilize the same command bus as the primary system.
2. The machine control system shall support at least the following user selectable operational modes.

Free Access/Parallel Control (any control point able to control any machine at any time).
First Come First Served (any panel may assume control of any unused machine at any time).
First Come First Served with Priority (establishes hierarchy, allowing high priority panels to assume control from lower priority panels).
Machine Delegation (requires use of assignment panels at machine location which delegate control to one individual control point at any given time).

3. Control panels shall be available which integrate routing system and machine control functions on the same panel. Such integrated panels shall display in graphic form, current status of each machine selected to the panel. It shall

be possible to configure the system to permit or inhibit display of the status of machines which are selected to the panel but for which control has not been assigned.

4. Control panels shall be available which are user programmable for association with machines on a semi-permanent basis, reprogrammable from the system maintenance terminal.

5. Interfacing to machines shall be via relays (for commands directed toward machines) and opto-isolators (or other suitable isolation devices) to receive tally information from machines and return it to the system.

6. Machine interfaces shall be modular in construction with removable processor modules, each of which is independently programmable for association with up to four VTR type machines or one VTR type plus one three projector/one camera film chain.

7. Each interface frame shall accommodate at least two processor modules as described above (one shall serve as a hot-backup) in addition, a single independently removable power supply shall serve as primary power and a secondary power supply as hot-backup.

8. Provision shall be made in each interface frame to allow two command bus connections per interface module, permitting the use of up to four command buses to each frame for maximum protection against bus interruption.

ELECTRICAL.--Input voltage shall be 95 to 130 VAC, 47 to 63 Hz single phase. Power consumption per 12-rack unit video frame, when utilizing DC restored input modules, shall be 300 W.

MECHANICAL.--The AVS shall mount in 262.5 mm of EIA 310 rack space with a maximum depth of 19 inches.

ENVIRONMENTAL.--The AVS shall be fully operational over a temperature range of 0° C. to 50° C. and over a humidity range of 0 to 95 percent without condensation.

TRAINING.--The Contractor shall provide a complete training course on the operation and maintenance of the AVS. The training course shall cover installation and initial check-out procedures, routine maintenance practices and procedures, and troubleshooting techniques and operational procedures for the equipment specified herein. The course shall consist of a minimum of 8 hours of instruction and practical exercise. The training course shall be conducted by a qualified instructor and, if applicable, result in the certification of the technician to perform normal maintenance. The vendor shall supply up to 20 trainees with individual training and instruction documents as required for participation in the course. The documentation shall include coverage of all options and modifications made to the equipment. Each trainee shall be supplied with a copy of the operation and maintenance manuals.

WARRANTY.--The AVS shall be warranted by the manufacture for a period of two years after it has been installed and accepted by the Engineer.

10-3.45 EQUIPMENT SEISMIC ANCHORING

All equipment being installed in the traffic operations system (TOS) HUB building, shall be mounted and braced to withstand without damage, seismic acceleration forces in both horizontal and vertical directions of 0.33G, for installation in seismic zone 4, in accordance with the Uniform Building Code (UBC), Section 2312.

The Contractor shall submit for review and approval by the Engineer, design calculations and shop drawings of the anchoring methods intended for use. The calculations and drawings shall be performed and signed by a registered professional civil engineer.

The installation shall meet the requirements of the Essential Services Building Seismic Safety Act, Senate bill 239, 1986, Title 24.

10-3.46 REMOVING, REINSTALLING OR SALVAGING ELECTRICAL EQUIPMENT

Salvaged electrical materials shall be hauled to Caltrans Electrical Maintenance Station, 30 Rickard Street, San Francisco, CA 94134, (415) 330-6509, and stockpiled.

The Contractor shall provide equipment, as necessary, to safely unload and stockpile the material. A minimum of 2 working days' notice shall be given prior to delivery.

10-3.47 PAYMENT

Full compensation for hauling and stockpiling electrical materials shall be considered as included in the contract price paid for the item requiring the material to be salvaged, and no additional compensation will be allowed therefor.

Full compensation for irrigation service, traffic operations system, changeable message sign system and traffic signal shown at location 4 shall be considered as included in the contract lump sum price paid for lighting and sign illumination (location 4) and no separate payment will be made therefor.

The contract lump sum price paid for traffic operations system shall include full compensation for furnishing all labor, materials (except items covered by other bid items), tools, equipment, and incidentals, and for doing all the work involved in installing traffic operations system, complete in place, as shown on the plans, including camera poles and foundations, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Full compensation for lighting, flashing beacons and sign illumination (modify) shall be considered as included in the contract lump sum price paid for traffic operations system at said location and no separate payment will be made therefor.

The contract unit price paid for each of the following items shall include full compensation for furnishing all materials, tools, equipment, and incidentals, as shown on the plans, as specified in these special provisions, and as directed by the Engineer:

1. One telephone and data lines surge protector system
2. One switch ethernet HUB
3. One channel service unit / data service unit
4. One modem assembly
5. One camera unit
6. One pan / tilt unit
7. One camera control unit
8. One video encoder unit
9. One integrated service digital network terminal adapters
10. One access node
11. One analog video routing switcher
12. One uninterruptible power system
13. One color CCTV monitor

The contract lump sum price paid for hub internals shall include full compensation for furnishing all labor, materials (except items covered by other bid items), tools, equipment, and incidentals and for doing all the work involved in installing the hub internals, complete in place, including documentation, testing and training, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

The contract lump sum price paid for hub data shall include full compensation for furnishing all labor, materials (except items covered by other bid items), tools, equipment, and incidentals and for doing all the work involved in installing the hub data, complete in place, including documentation, testing and training, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

The contract lump sum price paid for hub analog video shall include full compensation for furnishing all labor, materials, tools, equipment and incidentals and for doing all the work involved in installing hub analog video, complete in place, including testing, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

The contract lump sum price paid for lighting (stage construction) shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in installing lighting (stage construction), complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

10-3.48 WEATHER STATION

The weather station shall consist of a precipitation sensor, a datalogger, telemetric devices, power supply, a tower and a base station.

The precipitation sensor shall consist of a 20-cm diameter aluminum funnel that collects water and directs it to a tipping mechanism. The tipping mechanism shall tip and dump when 0.25 mm of water is collected. Each tip of the bucket shall cause a magnet to pass over a reed switch and effect a momentary contact closure. The tip shall also cause a second bucket to come into position below the funnel. The reed switch shall be connected to the datalogger by an appropriate interface cable.

The tipping mechanism shall be a plastic catch bucket with sapphire bearings. The tipping mechanism and the funnel shall be housed in a metallic cylinder 43.1cm. high with a weight of 3.9 Kilograms, with a gage orifice diameter of 20 centimeters. The gage orifice shall be protected from falling debris by a screen. The cylindrical housing shall include a bubble level and shall be mounted on a horizontal pedestal. The accuracy of the precipitation sensor shall be within 0.5 mm for a rainfall intensity of up to 25 mm per hour, and within 2% of total incidental precipitation for greater intensities.

The datalogger shall have a single 37-pin CPC connector which provides 8 user-configurable single-ended analog inputs, 1 frequency input, 1 switch closure input and 1 SDI-12 interface. The datalogger shall also have 1 input/output port that interfaces with MS-DOS for programming and/or data retrieval. It shall have 128 kilobytes of onboard memory storage, and its temperature-compensated real time clock shall not drift by more than 15 seconds per month. A 37-pin male mating connector assembly and a DB9 cable assembly shall be provided.

The datalogger shall be equipped with a 33.6 kilobaud or higher telephone modem, which shall be full duplex and its baud rate selectable from the datalogger software. The modem shall be capable of being programmed to transmit or receive data by dialing out automatically on a specified schedule or in response to an incoming call. The modem shall be powered by a 12-volt power supply and shall typically require a current of 100 milliamperes. The modem shall also have a RJ-11 connector for connection to a standard telephone line.

A 7 ampere-hour rechargeable battery with a high-impact polystyrene case shall provide electrical power for the datalogger. The battery's nominal voltage shall be 12 volts and it shall weigh no more than 2.72 kilograms. The battery shall be kept charged by two photovoltaic solar panes each weighing no more than 0.45 kilogram and measuring no more than 150

mm by 355 mm. Each solar panel shall be capable of supplying 100 milliamperes at a design operating voltage of 14.5 volts, and the output voltage shall be clamped at 15 volts. The solar panels shall be electrically connected in parallel. All mounting hardware and connection cables for the solar panels shall be provided.

A NEMA-4 enclosure with an appropriate internal mounting plate shall house the datalogger equipment and the battery. The weather station equipment shall be mounted on a 2.74-meter self-guyed tower, which shall be made of aluminum and weigh no more than 6.8 kilograms. All mounting hardware for the various pieces of equipment shall be provided.

The tower shall be mounted on a relatively flat surface with orifice horizontal. The height of the orifice above the surface on which the tower is mounted shall be not more than 2 meters and not less than 1.2 meters. The distance between surrounding objects and the precipitation gage shall be no closer than 2 times their height above the gage orifice.

The tower shall be mounted at a location shown on the plans or as directed by the Engineer. The contractor shall provide a permanent telephone connection for the weather station, but Caltrans will procure the telephone service.

A complete set of manuals and software for the datalogger shall be provided to the Engineer. The software shall run on an MS-DOS or Windows platform and shall contain a library of pre-defined sensor and data processing algorithms. The software shall allow the user to create and manage custom programs that govern all aspects of the weather station's operation: reading the sensor(s), analyzing and converting the data, storage and telemetry. The software shall be capable of transferring the completed custom program to the non-volatile memory of the datalogger.

The base station shall consist of a complete computer system, including keyboard and 17-inch monitor, with a microprocessor running at 400 Megahertz or above. The computer operating system shall be compatible with Windows 98 or later, with at least 32 megabytes of random access memory, 6 gigabytes of hard disk storage, 3_-inch floppy disk drive, CDROM writer, 56 kilobaud hardware modem, and 10/100 megabit ethernet card with RJ-45 connection.

The base station shall be installed in the Caltrans Engineering Services Branch office at the Caltrans District Office, 111 Grand Avenue, Oakland, as directed by the Engineer.

The Contractor shall furnish and install weather station in accordance with this Special Provisions and the manufacturers' recommendations. The Contractor shall connect all individual pieces of weather station equipment so that the weather station shall be fully operational by September 1, 2000. The Contractor shall be responsible for the safety of the weather station equipment until end of contract.

The contract lump sum price paid for weather station shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, including verification of operational readiness, and for doing all the work involved in furnishing, installing, maintaining, and protecting from damage during duration of contract work, the weather station, complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

SECTION 11. QUALITY CONTROL / QUALITY ASSURANCE

SECTION 11-1. ASPHALT CONCRETE

11-1.01 GENERAL

Asphalt concrete for this project shall conform to the requirements of this Section 11-1, "Asphalt Concrete," and the section entitled "Asphalt Concrete" in Section 10-1, "General," elsewhere in these special provisions. Section 39, "Asphalt Concrete," of the Standard Specifications shall not apply for Type A and Type B asphalt concrete for this project.

SECTION 39

ASPHALT CONCRETE

39-1 GENERAL

39-1.01 Description

This work shall consist of furnishing and mixing aggregate and asphalt binder at a central mixing plant, spreading and compacting the mixture, and furnishing and placing pavement reinforcing fabric, all as specified in this specification and the section entitled "Asphalt Concrete" in Section 10-1, "General," elsewhere in these special provisions.

The Contractor shall be responsible for controlling the quality of the asphalt concrete product entering the work, including mix design, mixing, spreading, and compacting asphalt concrete and of the work performed, and for developing, implementing and maintaining a quality control program. The Contractor shall also be responsible for the inspection, sampling and testing required to control the quality of the asphalt concrete and the work performed, and for the inspection, sampling and testing required to provide the Engineer with the information and test data necessary for acceptance of the asphalt concrete, complete in place.

The inspection, sampling and testing required by the Contractor to control the quality of the workmanship and the asphalt concrete product shall conform to the requirements specified herein, and the Department's "Manual for Quality Control and Quality Assurance for Asphalt Concrete," dated April 1996.

Asphalt concrete is designated as Type A or Type B. The type of asphalt concrete will be shown on the plans or specified in "Asphalt Concrete" in Section 10-1, "General," elsewhere in these special provisions.

Asphalt concrete shall be produced in a batch mixing plant, a continuous pugmill mixing plant, or a drier-drum mixing plant. Proportioning shall be either by hot-feed control or cold-feed control.

39-2 MATERIALS

39-2.01 Mix Design

The Contractor shall submit to the Engineer a proposed mix design and material proposed for each asphalt concrete mixture to be used, at least two weeks prior to production of that asphalt concrete mixture. The proposed mix designs shall conform to the asphalt concrete mixture quality requirements specified in Section 39-2.03, "Aggregate," of this specification. Aggregate shall conform to the quality and gradation requirements specified in Section 39-2.03, "Aggregate," of this specification, for the asphalt concrete types and sizes specified in "Asphalt Concrete," in Section 10-1, "General," elsewhere in these special provisions.

The Contractor shall furnish test data in support of each proposed mix design. The test data furnished shall be for an asphalt concrete mixture that conforms to the proposed target values. In addition, the Contractor shall also furnish samples of the aggregate, asphalt binder and all additives proposed for use in each asphalt concrete mixture. The Contractor shall submit the following for each asphalt concrete mixture proposed for use under the contract:

A. Aggregate and mineral filler:

1. Target values for percent passing each sieve size for the aggregate blend. The proposed target values, for the specified type and aggregate size, shall conform to the aggregate gradation limits specified in Section 39-2.03, "Aggregate," of this specification;
2. Results of tests for aggregate quality requirements specified in Section 39-2.03, "Aggregate," of this specification;
3. Source of each aggregateto be used;
4. Percentage of each aggregate stockpile or hot bin to be used;
5. Gradation of each aggregate stockpile or hot bin to be used; and
6. Samples from each aggregate stockpile or hot bin to be used. These samples shall be representative of the material to be used and shall have been processed in a manner representative of that for the material to be used in the work.

    a. 60 kg of each coarse aggregate;
    b. 40 kg of each intermediate and fine aggregate; and
    c. 5 kg of each mineral filler.

B. Asphalt binder:

1. Target value for asphalt binder content for each proposed asphalt concrete mixture;
2. Four individual one-liter samples of the asphalt binder to be used in each proposed asphalt concrete mixture;
3. Results of the asphalt binder quality tests as specified in Section 92, "Asphalts," of the Standard Specifications; and
4. Material safety data sheets.

C. Antistrip additives, when applicable:

1. A 5-kg sample of dry additive or a one-liter sample of liquid antistrip additive, including name of product, manufacturer, manufacturer's numerical designation (if any) and proposed rate, location and method of addition; and
2. Material safety data sheets.

The Engineer will test the Contractor's proposed asphalt concrete mix design for verification using the proposed aggregate gradation and asphalt binder content target values, and the quality and asphalt concrete mixture requirements specified in Section 39-2.03, "Aggregate," of this specification. Asphalt concrete production for this project shall not begin until the Contractor has received written notification that the proposed mix design to be used has been verified by the Engineer.

Changes from one mix design to another shall not be made during the progress of the work, unless permitted in writing by the Engineer. The Contractor shall submit to the Engineer a proposed mix design for each new asphalt concrete mixture to be used at least two weeks prior to production of that mixture. Asphalt concrete mix designs not verified by the Engineer shall not be used. Changes in stockpile or hot bin proportions to conform to aggregate grading requirements will not be

considered changes in the mix design. Changes in asphalt binder content or aggregate grading target values will not be applied retroactively for acceptance or payment.

The Engineer will determine all asphalt concrete mix design evaluation costs incurred as a result of Contractor requested verification of additional asphalt concrete mix design proposals. The mix design evaluation costs, as determined by the Engineer, will be deducted from any moneys due or to become due the Contractor.

39-2.02 Asphalts

Asphalt binder to be mixed with aggregate shall be a steam-refined paving asphalt conforming to the provisions in Section 92, "Asphalts," of the Standard Specifications, and shall be of the grade designated in "Asphalt Concrete," in Section 10-1, "General," elsewhere in these special provisions, or as determined by the Engineer. The amount of asphalt binder to be mixed with the aggregate will be determined by the Contractor and verified by the Engineer, as specified in Section 39-2.01, "Mix Design," of this specification. In support of the material certification requirements specified in Section 92, "Asphalts," of the Standard Specifications, the Contractor shall obtain 2 individual one-liter samples of the asphalt binder for each day of asphalt concrete production. The sample containers shall be labeled with the date and time of sampling and shall be submitted to the Engineer on a weekly basis.

Liquid asphalt for prime coat shall conform to the provisions in Section 93, "Liquid Asphalts," of the Standard Specifications, and shall be the grade designated by the contract item or specified in "Asphalt Concrete," in Section 10-1, "General," elsewhere in these special provisions.

Asphalt emulsion for paint binder (tack coat) shall conform to the provisions in Section 94, "Asphaltic Emulsions," of the Standard Specifications, for the rapid-setting or slow-setting type and grade approved by the Engineer.

Paving asphalt to be used as a binder for pavement reinforcing fabric shall be a steam-refined paving asphalt conforming to the provisions in Section 92, "Asphalts," of the Standard Specifications, and shall be Grade AR-4000, unless otherwise ordered by the Engineer.

39-2.03 Aggregate

All aggregates shall be clean and free from decomposed or organic materials and other deleterious substances. Coarse aggregate is material retained on the 4.75-mm sieve, fine aggregate is material passing the 4.75-mm sieve, and supplemental fine aggregate is added fine material passing the 600-µm sieve, including, but not limited to, cement and stored fines from dust collectors.

The combined aggregate shall conform to the requirements of this section.

The target value for the percent passing each designated sieve size for the aggregate blend used in the proposed asphalt concrete mixture shall fall within the "Target Value Limits" of the following table:

AGGREGATE GRADATION
Type A and Type B Asphalt Concrete
Percentage Passing

19-mm Maximum, Coarse		19-mm Maximum, Medium	
Sieve Sizes	Target Value Limits	Sieve Sizes	Target Value Limits
25-mm	100	25-mm	100
19-mm	90-100	19-mm	90-100
9.5-mm	60-75	9.5-mm	65-80
4.75-mm	45-50	4.75-mm	49-54
2.36-mm	32-36	2.36-mm	36-40
600-µm	15-18	600-µm	18-21
75-µm	3-7	75-µm	3-8

12.5-mm Maximum, Coarse		12.5-mm Maximum, Medium	
Sieve Sizes	Target Value Limits	Sieve Sizes	Target Value Limits
19-mm	100	19-mm	100
12.5-mm	95-100	12.5-mm	95-100
9.5-mm	75-90	9.5-mm	80-95
4.75-mm	55-61	4.75-mm	59-66
2.36-mm	40-45	2.36-mm	43-49
600-µm	20-25	600-µm	22-27
75-µm	3-7	75-µm	3-8

During asphalt concrete production, aggregate gradation shall be within the limits specified in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification. Conformance with these grading requirements will be determined by California Test 202, modified by California Test 105 when there is a difference in specific gravity of 0.2 or more between the coarse and fine portions of the aggregate or between the blends of the different aggregates.

The combined aggregate shall conform to the following quality requirements prior to the addition of the asphalt binder:

Aggregate Quality Requirements

Quality	California Test	Asphalt Concrete Type A	Asphalt Concrete Type B
Percent of Crushed Particles	205		
Coarse Aggregate (Min.)		90%	25%
Fine Aggregate (Passing 4.75-mm, Retained on 2.36-mm) (Min.)		70%	20%
Los Angeles Rattler	211		
Loss at 100 Rev. (Max.)		12%	
Loss at 500 Rev. (Max.)		45%	50%
Sand Equivalent (Min.)	217	47	42
K_cFactor (Max.)	303	1.7	1.7
K_fFactor (Max.)	303	1.7	1.7

The asphalt concrete mixture, composed of the proposed aggregate blend and the proposed asphalt binder content as determined by California Test 367, shall conform to the following requirements:

Asphalt Concrete Mixture Requirements

Design Parameters	California Test	Asphalt Concrete Type A	Asphalt Concrete Type B
Hveem Stabilometer Value (Min.)	366	37	35
Percent air voids	367	3-5	3-5
Swell Max. (Millimeters)	305	0.76	0.76

39-2.04 Pavement Reinforcing Fabric

Pavement reinforcing fabric shall conform to the provisions in Section 88, "Engineering Fabrics," of the Standard Specifications.

39-3 CONTRACTOR QUALITY CONTROL

39-3.01 General

The Contractor shall establish, provide and maintain a quality control system which will provide assurance to the Engineer that all materials and completed construction, submitted for acceptance, conform to the contract requirements specified herein. The Contractor shall also be responsible for the quality of all component materials contained within the asphalt concrete product, complete in place, procured from subcontractors or vendors.

At least 14 days prior to the start of production of asphalt concrete, the Contractor shall submit to the Engineer for approval a written Quality Control Plan which shall be used to ensure the quality of the product and the work. The production of asphalt concrete shall not begin until the Quality Control Plan is approved by the Engineer.

39-3.02 Quality Control Plan

The Contractor shall provide a Quality Control Plan which shall describe the organization and procedures which the Contractor shall use to administer the quality control system including the procedures used to control the production process, to determine when changes to the production process are needed, and the procedures proposed to be used to implement the required changes. The Quality Control Plan shall meet the minimum standards set forth in the Department's "Manual for Quality Control and Quality Assurance for Asphalt Concrete," dated April 1996.

Approval of the Quality Control Plan will be based on the inclusion of all of the required information. Approval of the Quality Control Plan does not imply any warranty by the Engineer that adherence to the plan will result in production of asphalt concrete that complies with these specifications. It shall remain the responsibility of the Contractor to demonstrate such compliance. The Contractor may propose in writing a supplement to the Quality Control Plan as work progresses and must propose a supplement whenever there are changes in production or placement of asphalt concrete or to quality control procedures or personnel. Asphalt concrete production and placement shall not resume or continue until the revisions to the Quality Control Plan or quality control personnel have been approved in writing by the Engineer.

The Quality Control Plan shall include the name and qualifications of a Quality Control Manager. The Quality Control Manager shall be responsible for the administration of the Quality Control Plan, including compliance with the plan and any plan modifications. The Quality Control Manager shall be directly responsible to the Contractor and shall have the authority to make decisions where quality of the work or product are concerned. All sampling, inspection and test reports shall be reviewed and signed by the Quality Control Manager prior to submittal to the Engineer.

The Quality Control Plan shall include the name and qualifications of an independent testing laboratory mutually agreed to by the Contractor and the Engineer to serve as the Third Party Laboratory in any dispute resolution. Attention is directed to Section 39-4.05, "Dispute Resolution," of this specification.

39-3.03 Quality Control Inspection, Sampling and Testing

The Contractor shall perform quality control sampling and testing, provide inspection, and exercise management control to ensure that asphalt concrete production and placement conforms to the requirements specified herein.

The Contractor shall provide the required sampling, testing and inspection during all phases of the asphalt concrete work. Sampling, testing and inspection shall be performed at a rate sufficient to ensure that the asphalt concrete product conforms to the requirements specified herein. Sampling, testing, and inspection to be used by the Engineer for acceptance and determination of payment shall be performed at the minimum frequency specified in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification, and as outlined in the approved Quality Control Plan. The Contractor shall provide quality control inspection on the project at all times asphalt concrete paving operations are in progress.

Sampling locations for quality control tests, as specified herein, shall be determined by the Contractor using a random sampling plan approved by the Engineer. The Contractor shall establish a statistically based procedure of random sampling.

The Contractor shall obtain and split into representative portions samples in conformance with California Test 125. One representative split portion of each sample shall be reserved for possible retest during dispute resolution, according to the requirements designated in Section 39-4.05, "Dispute Resolution," of this specification.

The Contractor shall provide a testing laboratory with adequate equipment and personnel for the performance of the quality control tests. Laboratory facilities shall be clean and all sampling and testing equipment shall be maintained in proper working condition. The Engineer shall be given unrestricted access to the laboratory for inspection and to witness the Contractor's quality control activities during working hours.

Testing laboratories and inspection, sampling and testing personnel shall conform to the minimum requirements as set forth in the Department's "Manual for Quality Control and Quality Assurance for Asphalt Concrete," dated April 1996.

39-3.04 Control Charts and Records

The Contractor shall record all sampling, testing and inspection data on forms approved by the Engineer. The Contractor shall maintain complete testing and inspection records and post all test data in the laboratory.

Upon written request by the Contractor, the Engineer will provide the test data of testing done by the State.

39-3.04A Control Charts

The Contractor shall develop and maintain linear control charts. The control charts shall identify the project, test number, test parameter, applicable upper and lower specification limits, and test data. The control charts shall be used as part of the quality control system to document variability of the asphalt concrete production process, identify construction and equipment problems, and identify potential pay factor adjustments.

When test data for any quality characteristic deviates beyond the specification limits specified in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification, the Contractor shall take the necessary corrective action to bring the production within the specification limits, and shall document the corrective action taken in the records of inspection and testing as designated in Section 39-3.04B, "Records of Inspection and Testing," of this specification. When 3 consecutive sets of test data for any quality characteristic deviate beyond the specification limits designated in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification, the Contractor shall cease production of asphalt concrete, and shall propose corrective measures to the Engineer. Production of asphalt concrete may continue when the corrective measures have been approved by the Engineer and implemented by the Contractor.

Control charts shall be kept current and shall be posted in a location accessible to the Engineer. Control charts shall be updated each day of asphalt concrete production, and up-to-date copies shall be posted prior to the beginning of the next day's production of asphalt concrete.

39-3.04B Records of Inspection and Testing

For each day of asphalt concrete production, the Contractor shall prepare an "Asphalt Concrete Construction Daily Record of Inspection", on a form approved by the Engineer. The inspection record shall include the following certification signed by the Quality Control Manager:

> "It is hereby certified that the information contained in this record is accurate, and that all work documented herein complies with the requirements of the contract. Any exceptions to this certification are documented as a part of this record."

For each day of asphalt concrete production,, the Contractor shall prepare an "Asphalt Concrete Testing Record" on a form approved by the Engineer. The testing record shall include the following certification signed by the Quality Control Manager:

> "It is hereby certified that the information contained in this record is accurate, and that all tests and calculations documented herein comply with the requirements of the contract and the standards set forth in the testing procedures. Any exceptions to this certification are documented as a part of this record."

The Contractor shall submit sampling, testing and inspection records and certifications to the Engineer within 24 hours or by noon of the next day's asphalt concrete production, whichever period is agreed to by the Engineer at the beginning of the asphalt concrete production. If the record is incomplete or in error, a copy of the record will be returned to the Contractor with the deficiencies noted by the Engineer. The Contractor shall correct the deficiencies and return the updated record to the Engineer by the start of the following working day. When errors or omissions in the sampling, inspection or testing records repeatedly occur, the Contractor shall correct the procedures by which the records are produced.

If control charts, sampling, testing and inspection records and certifications are not posted or provided as required within the time specified herein, the Engineer may require work to be suspended until the missing control charts, sampling, testing and inspection records, and certifications have been provided.

39-4 ENGINEER QUALITY ASSURANCE

39-4.01 General

The Engineer will verify conformance to contract specifications by inspection of the Contractor's procedures, evaluation of the Contractor's quality control records, and independent sampling and testing of the materials. The Engineer will obtain random samples and perform tests to verify the test data of the quality control testing performed by the Contractor.

In addition to the minimum sampling and testing requirements specified in this specification, the Contractor shall, when directed by the Engineer, obtain representative samples of any asphalt concrete mixture or material component that appears defective or inconsistent. These samples will be obtained and split into representative portions in accordance with California Test 125. The Contractor shall provide the Engineer with one representative split portion of each sample taken and shall reserve one representative split portion of each sample for possible retest during dispute resolution, according to the requirements designated in Section 39-4.05, "Dispute Resolution," of this specification. The material need not be sampled if the Contractor elects to remove and replace the material, at the Contractor's expense, or if the Contractor uses a method of correcting the situation which has been approved by the Engineer. Test data from these additional material samples shall not be used as a basis for a calculated pay factor.

39-4.02 Engineer Sampling for Verification

The Engineer will obtain random samples of aggregate, asphalt binder and asphalt concrete mixture, and test for in-place density independent of the Contractor's quality control testing. These samples may be obtained at any time during asphalt concrete production and placement operations, and will be obtained and split into representative portions in accordance with California Test 125. One of the representative split portions will be provided to the Contractor, one of the representative split portions will be tested by the Engineer and used to verify quality control test data furnished by the Contractor that has not yet been verified, and two representative split portions will be reserved by the Engineer for third party testing in accordance with the requirements of Section 39-4.05, "Dispute Resolution," of this specification.

The Engineer will permit the Contractor to witness all verification sampling. However, the Engineer will not be required to notify the Contractor of anticipated sampling schedules or locations. The Engineer will not delay sampling for the Contractor to witness the sampling.

39-4.03 Engineer Testing for Verification

Test data from the samples taken by the Engineer will be used to verify the Contractor's quality control test data.

The Engineer will sample and test for all material quality characteristics specified for statistical acceptance of the work. The Engineer's verification tests will be at a frequency of not less than 10 percent of the minimum quality control sampling and testing frequency required of the Contractor, and will be in accordance with Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification. The Engineer's verification tests will be performed using the same test methods used by the Contractor.

A standard statistical test, the *t*-test for sample means, as specified in Section 39-4.04, "Statistical Verification Tests," of this specification, will be used by the Engineer to verify the Contractor's quality control sampling and testing for acceptance of the material. All quality control test data reported by the Contractor since the last completed verification by the Engineer, for each indexed quality characteristic, will be used in the comparison. If the *t*-test does not indicate that the difference between the Contractor's test data and the corresponding Engineer's verification test data is significant ($t \leq t_{crit}$), the Contractor's test data will be deemed verified and used by the Engineer to accept the material. If the *t*-test indicates that the difference between the Contractor's test data and the corresponding Engineer's verification test data is significant ($t > t_{crit}$), the Contractor's test data will be deemed unverified.

When the Contractor's test data are not verified by the Engineer, the Contractor will be notified in writing of the difference, and the Engineer and Contractor will cooperate to attempt to determine the source of the discrepancy. In addition, the Engineer will continue to sample asphalt concrete production, and will compare the cumulative verification test data to the cumulative unverified test data reported by the Contractor for the indexed quality characteristic in question.

If, after 5 consecutive verification tests by the Engineer, the Contractor's quality control test data is not verified ($t > t_{crit}$), acceptance and payment determination for the indexed quality characteristic in question on the asphalt concrete represented by the unverified test data will be made on the basis of the Engineer's verification test data only. The unverified test data will not be considered for acceptance purposes by the Engineer, nor will the test data be included in any subsequent *t*-test verification by the Engineer, pending the findings of the dispute resolution process as designated in Section 39-4.05, "Dispute Resolution," of this specification. In addition, the Contractor's sampling and testing program shall be deemed unacceptable and shall be disqualified from further sampling and testing. Before proceeding with asphalt concrete production, the Contractor shall propose in writing remedial measures which will be taken to provide an acceptable sampling and testing program. Asphalt concrete production shall not resume until the Contractor has received written notification that the revised sampling and testing program has been approved by the Engineer.

The Contractor shall not use any representative split portion of the samples taken by the Engineer for verification tests for determination of quality control test data.

Test data from the reserved representative split portions of verification samples will be used in the dispute resolution process as designated in Section 39-4.05, "Dispute Resolution," of this specification.

39-4.04 Statistical Verification Tests

The Engineer shall determine the acceptability of the Contractor's quality control test data for material acceptance purposes using the *t*-test for sample means.

The Contractor's quality control test data will be considered verified at a level of significance, _ = 0.01.

The *t*-value of the group of test data to be verified (t) is computed as follows:

$$t = \frac{\left| \overline{X}_c - \overline{X}_v \right|}{S_p \sqrt{\frac{1}{n_c} + \frac{1}{n_v}}} \qquad \text{and} \qquad S_p^2 = \frac{S_c^2 (n_c - 1) + S_v^2 (n_v - 1)}{n_c + n_v - 2}$$

where: n_c = Number of Contractor's quality control tests (min. 2 required)

n_v = Number of Verification tests (min. 1 required)

$\overline{X}_c$ = Mean of the Contractor's quality control tests

$\overline{X}_v$ = Mean of the Verification tests

S_p = Pooled standard deviation

(When $n_v = 1$, $S_p = S_c$)

S_c = Standard deviation of the Contractor's quality control tests

S_v = Standard deviation of the Verification tests (when $n_v > 1$)

(Use the standard deviation of the Contractor's quality control tests when $n_v = 1$)

Compute t using the equation above and compare to the critical *t*-value, t_{crit} , from the following table:

Critical t-value for Verification Testing

degrees of freedom (nc+nv-2)	tcrit for a = 0.01	degrees of freedom (nc+nv-2)	tcrit for a = 0.01
1	63.657	18	2.878
2	9.925	19	2.861
3	5.841	20	2.845
4	4.604	21	2.831
5	4.032	22	2.819
6	3.707	23	2.807
7	3.499	24	2.797
8	3.355	25	2.787
9	3.250	26	2.779
10	3.169	27	2.771
11	3.106	28	2.763
12	3.055	29	2.756
13	3.012	30	2.750
14	2.977	40	2.704
15	2.947	60	2.660
16	2.921	120	2.617
17	2.898	∞	2.576

When the *t*-value of the test data from the Engineer's verification tests and the Contractor's quality control tests is compared to t_{crit} from the previous table, if t is less than or equal to t_{crit} ($t \leq t_{crit}$), the difference between the Contractor's quality control test data and the corresponding Engineer's verification test data is not significant, and the Contractor's test data are verified. When t is greater than t_{crit} ($t > t_{crit}$), the difference between the Contractor's quality control test data and the corresponding Engineer's verification test data is significant, and the Contractor's test data are not verified.

39-4.05 Dispute Resolution

The Contractor and Engineer will work in partnership to avoid potential conflicts and to resolve any differences that may arise from unverified test data. As soon as an unsuccessful verification attempt is reported by the Engineer, both parties will review their sampling and testing procedures and share their findings. If an error in the Contractor's testing is detected during this review, the Contractor will either recalculate, if appropriate, or retest using the reserved representative split portions of quality control samples. This new test data shall be resubmitted to the Engineer for verification purposes. If an error in the Engineer's testing is detected, the Engineer will recalculate, if appropriate, or retest using a reserved representative split portion of the verification samples. Using the new test data, the Engineer will repeat the verification calculation of the Contractor's resubmitted test data using the statistical *t*-test as designated in Section 39-4.04, "Statistical Verification Tests," of this specification.

If the initial review does not reveal the source of the discrepancy, the Contractor may test the split verification samples and submit this test data to the Engineer for verification according to the requirements designated in Section 39-4.04, "Statistical Verification Tests," of this specification.

If the Contractor's quality control test data remain unverified after 5 consecutive verification samples have been obtained and tested, the Engineer will use the statistical *t*-test as designated in Section 39-4.03, "Statistical Verification Tests," of this specification, to verify the Contractor's test data on the 5 representative split portions of the verification samples. If the Contractor's test data for the 5 representative split portions of the verification samples are verified by the Engineer, then for purposes of acceptance and payment determination, the Contractor's unverified quality control test data will be replaced by the paired averages of the Engineer's and Contractor's test data for the 5 verification samples. If the Contractor's test data for the 5 representative split portions of the verification samples are not verified, the asphalt concrete represented by the unverified quality control tests will be accepted and paid for solely on the basis of the Engineer's verification test data. In either case, the Contractor's sampling and testing program will remain disqualified.

If neither the Contractor's quality control test data nor the test data of the representative split portions of the verification samples are verified by the Engineer, the Contractor may retain the services of the Third Party Laboratory designated in the Contractor's approved Quality Control Plan to resolve the difference. The Third Party Laboratory will perform the test method in question using the reserved representative split portions of the 5 verification samples. This test data will be submitted to the Engineer for verification . The Engineer will use the statistical *t*-test designated in Section 39-4.04, "Statistical Verification Tests," of this specification, to compare the Third Party Laboratory test data to the Engineer's verification test data. Both the Contractor and Engineer may witness the Third Party Laboratory testing.

If the Third Party Laboratory test data verifies the Engineer's verification test data, the asphalt concrete represented by the unverified quality control test data will be accepted and paid for using the paired averages of the Third Party Laboratory test data and the Engineer's verification test data. All costs related to the Third Party Laboratory testing shall be responsibility of the Contractor, and no additional compensation will be allowed. The Contractor's sampling and testing program shall remain disqualified.

If the Third Party Laboratory test data does not verify the Engineer's verification test data, the Engineer will use the statistical *t*-test to compare the Third Party Laboratory test data to the Contractor's unverified quality control test data. If the Contractor's quality control test data are verified by the Third Party Laboratory test data, acceptance and payment determination by the Engineer will be based on the Contractor's quality control test data. All costs of the Third Party Laboratory testing will be the Engineer's responsibility. The Contractor's quality control sampling and testing program shall be considered qualified, and the Engineer's verification sampling and testing program will be modified as necessary.

If the Third Party Laboratory test data fails to verify either the Engineer's verification test data or the Contractor's quality control test data, acceptance and payment determination will be based on the Third Party Laboratory test data. All costs for the Third Party Laboratory testing shall be split equally by the Engineer and the Contractor. The Contractor's sampling and testing program shall remain disqualified. The Engineer's verification sampling and testing program will be modified as necessary.

When the dispute is over relative compaction, the Third Party Laboratory will obtain test maximum densities using the reserved representative split portions of the verification samples. The Third Party Laboratory will re-calibrate the Engineer's nuclear density gage with cores obtained from the most recent 200 m of complete in place asphalt concrete surfacing not yet opened to public traffic. If no 200-m section of asphalt concrete surfacing not yet opened to public traffic is available, the Contractor shall construct a 200-m test strip, to the thickness to be placed, at a location on the project approved by the Engineer. The Third Party Laboratory will use the new calibration to re-calculate the nuclear density gage readings for determination of the Engineer's verification test data and will use the new calibration to determine relative compaction. If the re-calculated relative compaction test data verifies the Engineer's verification test data, subsequent testing by the Engineer will use the re-calibrated nuclear density gage. If the re-calculated relative compaction test data verifies the Engineer's verification test data, all costs related to the Third Party Laboratory testing shall be the responsibility of the Contractor, and no additional compensation will be allowed. The Contractor's sampling and testing program shall remain disqualified. If the re-calculated relative compaction test data do not verify the Engineer's verification test data, the Engineer may choose to re-calibrate the Engineer's nuclear density gage or may use the Third Party Laboratory calibration and all costs for the re-calibration shall be the responsibility of the Engineer. The Contractor's sampling and testing program shall remain disqualified.

If the Contractor's sampling and testing program is disqualified, the Contractor shall submit a plan for improving the Contractor's sampling and testing program which satisfies the requirements of the Quality Control Plan, as designated in Section 39-3, "Contractor Quality Control," of this specification. The Contractor shall not continue to use the disqualified sampling and testing program for quality control sampling and testing to be considered for acceptance and payment determination during the dispute resolution process as specified herein.

Should the Third Party Laboratory test data obtained during the process of dispute resolution, as specified herein, verify the Contractor's quality control test data and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay caused by the dispute resolution process, the delay will be considered a right of way delay as specified in Section 8-1.09, "Right of Way Delays," of the Standard Specifications.

39-5 STORING, PROPORTIONING AND MIXING MATERIALS

39-5.01 Storage

The Contractor shall store aggregate for asphalt concrete so that separately sized aggregates will not be intermingled, and shall store asphalt binder so that different grades of asphalt will not be intermingled. Any aggregate which has been intermingled with another size of aggregate shall be removed by the Contractor and replaced with aggregate of specified grading. "Hot-feed control" and "cold-feed control," indicates the location of measuring devices or controls.

When the Contractor adds supplemental fine aggregate, each such supplemental fine aggregate used shall be stored separately and kept thoroughly dry.

The measurement and storage requirements of this Section 39-5, shall not apply to the dust collected in skimmers and expansion chambers (knock-out boxes) or to the dust collected in centrifugal (cyclone) collectors. Dust from these collectors may be returned to the aggregate without being measured or stored separately, provided the dust is returned uniformly at a point in advance of the sampling device in batch-mixing plants or is returned at or before mixing in continuous mixing plants.

Aggregate and asphalt binder shall be stored in conformance with the following requirements:

39-5.01A Aggregate Cold Storage

The Contractor shall feed the material from storage with a mechanical feeder. Before being fed to the drier, aggregate shall be separated into 3 or more sizes and stored separately.

39-5.0lB Aggregate Hot Storage

The Contractor shall store aggregate for asphalt concrete to be mixed in batch mixing plants, after being dried, in accordance with the following requirements:

Aggregates for asphalt concrete shall be separated into 3 or more sizes.

After the aggregate is separated, each size shall be stored in a separate bin and shall be recombined in conformance with the provisions specified in Section 39-5.03, "Proportioning for Batch Mixing," of this specification, to conform to the gradings specified in Section 39-2, "Materials," of this specification. Storage bins shall be provided with chutes to prevent overflow into adjacent bins.

39-5.01C Asphalt Binder Storage

Asphalt to be used as a binder for asphalt concrete shall be stored in tanks accurately calibrated in uniform intervals of 375- to 400-L intervals and maintained to this accuracy. The tanks shall be made accessible for measuring the volume of asphalt at any time.

The Contractor shall provide a suitable sampling device in asphalt feed lines connecting plant storage tanks to the asphalt weighing system or spray bar. The sampling device shall consist of a valve with a nominal diameter between 10 and 20 mm, constructed in such a manner that a one-liter sample may be withdrawn slowly at any time during plant operations. The Contractor shall maintain the valve in good condition and, if the valve fails to function properly, replace the valve. The sampling device shall be readily accessible and in an area free of dangerous obstructions and shall be between 600 and 750 mm above the platform. A drainage receptacle shall be provided by the Contractor for flushing the device prior to sampling.

The Contractor shall maintain the discharge end of the asphalt binder circulating pipe below the surface of the asphalt binder in the storage tank to prevent discharging hot asphalt binder into open air.

The Contractor shall install a temperature sensing device in the asphalt feed line. The device shall measure the temperature of the asphalt and shall be accurate to 5°C increments. An automatic, continuous recording device shall be provided and used to maintain accurate records of the asphalt temperature during production.

39-5.02 Drying

Aggregate shall be fed directly to a drier-drum mixer or to a drier at a uniform rate.

Drying shall continue for a sufficient period of time and at a sufficiently high temperature so that, at the time of spreading, the moisture content of the completed asphalt concrete mixture shall not exceed 1.0 percent and the minimum and maximum asphalt concrete mixture temperatures are not exceeded. Moisture content will be determined by California Test 310 or 370.

The drier or drier-drum mixer shall be provided with a device which senses the temperature of the material leaving the drier or the drier-drum mixer. The temperature-sensing device shall be accurate to the nearest 5°C. The device shall be located so that changes of 5°C in temperature of the material will be indicated within one minute. An automatic continuous recording device shall be provided and used to maintain accurate records of the temperatures during production.

The burner used for heating the aggregate shall achieve complete combustion of the fuel.

39-5.03 Proportioning for Batch Mixing

When the Contractor elects to use batch mixing equipment, each aggregate storage bin shall be equipped with a suitable, safe sampling device which will provide a sample, representative of actual production, of the aggregate discharged into the weigh hopper or volumetric proportioning bin. When such samples are taken from a location above ground level, a means shall be provided for lowering the aggregate samples to the ground.

Fine material collected in all dust control systems, other than centrifugal collectors or knock-out boxes, is considered to be supplemental fine aggregate. When supplemental fine aggregate is used, it shall be proportioned by mass as provided in the subsection, "Mass Proportioning," of Section 39-5.03A, "Manual Proportioning," of this specification. A suitable, safe sampling device shall be installed in each feed line or surge tank preceding the weigh hopper.

Aggregate and asphalt shall be proportioned by mass or by volume as follows:

39-5.03A Manual Proportioning

An automatic plant shall not be operated manually unless the automatic circuitry is disconnected to the extent that it cannot be activated by the mere operation of a switch, circuit breaker, or some other similar routine procedure.

When manual proportioning is used in the production of asphalt concrete, proportioning shall conform to the following:

1. Mass Proportioning.—The zero tolerance for aggregate scales shall be 0.5-percent of the total batch mass of the aggregate. The zero tolerance for separate scales for weighing supplemental fine aggregate or asphalt binder shall be 0.05-percent of the total batch mass of the aggregate.
   The indicated mass of material drawn from storage for any draft of material shall not vary from the preselected scale setting by more than the following percentages of the total batch mass of the aggregate:

   a. Aggregate shall be within one percent, except that when supplemental fine aggregate is used and is weighed cumulatively with the aggregate, the draft of aggregate drawn immediately before the supplemental fine aggregate shall be within 0.5-percent.
   b. Supplemental fine aggregate shall be within 0.5-percent.
   c. Asphalt binder shall be within 0.1-percent.

   The asphalt binder shall be measured by a tank scale.

2. Volumetric Proportioning.—Each size of aggregate, except supplemental fine aggregate, shall be proportioned in a separate bin that is adjustable in size. Each bin shall have a gate or other device designed so that the bin shall be completely filled and struck off in measuring the volume of aggregate to be used in the mix. Means shall be provided for calibrating the mass of material in each measuring bin at any time. The plant shall be operated so that the material in each aggregate bin is within 2 percent of the mass pre-selected for the type of mixture being produced.
   Asphalt binder shall be proportioned by a meter or an adjustable calibrated tank. When meters are used, the asphalt lines leading to the asphalt meters shall be full-circulating or shall be regulated so that during plant stoppages, the temperature of the asphalt does not change more than 10°C from the temperature maintained while the plant is in full operation. Asphalt binder shall be proportioned to within 2 percent of the mass preselected for the asphalt concrete mixture being produced.

39-5.03B Automatic Proportioning

When automatic batch mixing is required in "Asphalt Concrete," in Section 10-1, "General," elsewhere in these special provisions, or when the Contractor elects to use an automatic batching system, the proportioning devices shall be automatic to the extent that the only manual operation required for proportioning all materials for one batch shall be a single operation of a switch or starter.

When automatic proportioning is used in the production of asphalt concrete, proportioning shall conform to the following:

1. Mass Proportioning.—Automatic proportioning devices shall be of a type in which materials discharged from the several bins are controlled by gates or by mechanical conveyors. The batching devices shall be so interlocked that no new batch may be started until all weigh hoppers are empty, the scales are at zero, and the discharge gates are closed. The means of withdrawal from the bins and of discharge from the weigh box shall be interlocked so that not more than one bin can discharge onto any given scale at one time, and that the weigh box cannot be tripped until the required quantity from each of the bins has been deposited therein. In addition, automatic proportioning devices shall be interlocked so that the weighing cycle will be interrupted whenever the amount of material drawn from any storage varies from the preselected amount by more than the tolerances specified in Section 39-5.03A, "Manual Proportioning," of this specification. Whenever the weighing cycle is interrupted, that specific batch shall not be used in the work unless it can be manually adjusted to meet the specified tolerances based on the total mass of the batch. When partial batches are batched automatically, the interlock tolerances, except the zero tolerance, shall apply to the total mass of aggregate in the partial batch.
   Automatic proportioning devices shall be operated so that all mass increments required for a batch are preset on the controls at the same time. Controls shall be designed so that these settings may be changed without delay, and the order of discharge from the several bins can be changed.
   Automatic proportioning controls shall be equipped with means for inspection of the interlock tolerance settings, and instructions for doing so shall be immediately available at the point of operation.
   The Contractor shall provide the necessary means to check the mass of various proportioned amounts on a separate scale located at the plant.

2. Volumetric Proportioning.—Asphalt binder shall be proportioned by an adjustable calibrated tank.
   Automatic volumetric proportioning devices shall be of a type which will not allow the bins to discharge into the mixer unless the mixer is empty and the mixer discharge gate is closed and will not operate unless the aggregate bins and asphalt binder tank are full. The automatic proportioning device shall operate in such a manner that the material in each aggregate bin and the asphalt binder tank is within 2 percent of the preselected mass.
   The Contractor shall provide the necessary means to check the mass of various proportioned amounts on a separate scale located at the plant.

39-5.03C Proportioning for Continuous Mixing

Asphalt binder shall be introduced into the mixer through a meter conforming to the requirements of Section 9-1.01, "Measurement of Quantities," of the Standard Specifications. The asphalt meter shall automatically compensate for changes in asphalt temperature, unless the meter is of the mass flow, coriolis effect, type. The system shall be capable of varying the rate of delivery of binder proportionate with the delivery of aggregate. During any day's run, the temperature of asphalt binder shall not vary more than 30°C. The meter and lines shall be heated and insulated. The binder storage shall be equipped with a device for automatic plant cut-off when the level of binder is lowered sufficiently to expose the pump suction line.

When supplemental fine aggregate is used, it shall be proportioned by mass by a method that uniformly feeds the material within 2 percent of the required amount. Supplemental fine aggregate shall be discharged from the proportioning device directly into the mixer.

The supplemental fine aggregate proportioning system shall function with a degree of accuracy such that, when operated at between 30 percent and 100 percent of maximum operating capacity, the average difference between the indicated mass of material delivered and the actual mass delivered shall not exceed one percent of the actual mass for three, 15-minute runs. For any of the 3, individual 15-minute runs, the indicated mass of material delivered shall not vary from the actual mass delivered by more than 2 percent of the actual mass.

The fine material collected in all dust control systems may be returned to the aggregate production stream without proportioning if returned at a rate commensurate with overall plant production, and if returned at or before the mixer. Any return rate of less than 100 percent of the collection rate shall be metered as specified above for supplemental fine aggregate.

The asphalt feeder, each of the aggregate feeders, the supplemental fine aggregate feeder, if used, and the combined aggregate feeder, shall be equipped with devices by which the rate of feed can be determined while the plant is in full operation.

The combined aggregate shall be weighed using a belt scale. The belt scale shall be of such accuracy that, when the plant is operating between 30 percent and 100 percent of belt capacity, the average difference between the indicated mass of material delivered and the actual mass delivered shall not exceed one percent of the actual mass for three, 3-minute runs. For any of the 3 individual 3-minute runs, the indicated mass of material delivered shall not vary from the actual mass delivered by more than 2 percent of the actual mass.

The actual mass of material delivered for proportioning device calibrations shall be determined by a vehicle scale conforming to the requirements of Section 9-1.01, "Measurement of Quantities," of the Standard Specifications. The vehicle scale shall be located at the plant and shall be sealed within 24 hours of checking the plant's proportioning devices. The plant shall be equipped so that this accuracy check can be made prior to the first production operation for a project and at any other time as directed by the Engineer.

The belt scale for the combined aggregate, the proportioning devices for supplemental fine aggregate, if used, and the asphalt proportioning meter shall be interlocked so that the rates of feed of the aggregates and asphalt will be adjusted automatically (at all production rates and production rate changes) to maintain the asphalt ratio (kilograms of asphalt per 100 kg of dry aggregate including supplemental fine aggregate, if used) designated in the verified mix design provided by the Contractor in accordance with the requirements of Section 39-2.01, "Mix Design," of this specification. The plant shall not be operated unless this automatic system is functioning and in good working condition.

Asphalt meters and aggregate belt scales used for proportioning aggregates and asphalt shall be equipped with rate-of-flow indicators to show the rates of delivery of asphalt and aggregate. Meters and scales shall be equipped with resettable totalizers so that the total amounts of asphalt and aggregate introduced into the asphalt concrete mixture can be determined. Rate-of-flow indicators and totalizers for like materials shall be accurate within one percent when compared directly. The asphalt cement totalizer shall not register when the asphalt metering system is not delivering material to the mixer.

The bin or bins containing the fine aggregate and supplemental fine aggregate, if used, shall be equipped with vibrating units or other equipment which will prevent any hang-up of material while the plant is operating. Each belt feeder shall be equipped with a device to monitor the depth of aggregate between the troughing rollers. The device for monitoring depth of aggregate shall automatically shut down the plant whenever the depth of aggregate is less than 70 percent of the target depth. To avoid erroneous shut down by normal fluctuations, a delay between sensing less than 70 percent flow and shutdown of the plant will be permitted, as determined by the Engineer, at the time of the initial California Test 109. A second device shall be located either in the stream of aggregate beyond the belt or where it will monitor movement of the belt by detecting revolutions of the tail pulley on the belt feeder. The device for monitoring no-flow or belt movement, as the case may be, shall stop the plant automatically and immediately when there is no flow. The plant shall not be operated unless both low-flow and no-flow monitoring devices are in good working condition and functioning properly.

For continuous pugmill mixing plants an aggregate sampling device which will provide a 25- to 40-kg sample of the combined aggregate while the plant is in full operation shall be provided in advance of the point where the aggregate enters the mixer.

For drier-drum mixing plants an aggregate sampling device which will provide a 25- to 40-kg sample of the combined aggregate while the plant is in full operation shall be provided in advance of the point where the aggregate enters the drier-drum mixer.

When the samples are taken from a location above ground level, the Contractor shall provide a means for safely lowering the aggregate samples to the ground.

When supplemental fine aggregate is used, a suitable, safe sampling device shall be installed in each feed line or surge tank preceding the proportioning device for the supplemental fine aggregate.

39-5.04 Mixing

Aggregate, supplemental fine aggregate, and asphalt binder shall be mixed in a batch mixer, continuous mixing pugmill mixer, or continuous mixing drier-drum. The charge in a batch mixer, or the rate of feed to a continuous mixer, shall not exceed that which will permit complete mixing of all of the material. Dead areas in the mixer, in which the material does not move or is not sufficiently agitated, shall be corrected by a reduction in the volume of material or by other adjustments.

Asphalt binder shall be at a temperature of not less than 120°C nor more than 190°C when added to the aggregate.

The temperature of the aggregate before adding the binder shall be not more than 135°C.

Mixing shall conform to the following requirements:

39-5.04A Batch Mixing

When asphalt concrete is produced by batch mixing, the mixer shall be equipped with a sufficient number of paddles of a type and arrangement so as to produce a properly mixed batch.

The binder shall be introduced uniformly into the mixer along the center of the mixer parallel to the mixer shafts, or by pressure spraying. When a pan is used, it shall be equipped with movable vanes in order that the flow of binder may be directed across the width of the pan, as desired. The vanes shall be equipped with a means for quick adjustment, and a positive lock to prevent shifting.

The mixer platform shall be of ample size to provide safe and convenient access to the mixer and other equipment. The mixer housing and weighbox housing shall be equipped with gates of ample size to permit ready sampling of the discharge of aggregate from each of the plant bins and from each feed line or surge tank of supplemental fine aggregate, if used. The Contractor shall provide a sampling device capable of delivering a representative sample of sufficient size to permit the required tests.

The mixer shall be equipped with a timing device which will indicate by a definite audible or visual signal the expiration of the mixing period. The device shall measure the time of mixing within 2 seconds.

The time of mixing a batch shall begin on the charging stroke of the weighhopper dumping mechanism and shall end when discharge is started. Mixing shall continue until a homogeneous asphalt concrete mixture of uniformly distributed and properly coated aggregates of unchanging appearance is produced. The time of mixing shall be not less than 30 seconds.

When automatic proportioning or automatic batch mixing is required in "Asphalt Concrete," in Section 10-1, "General," elsewhere in these special provisions, or when the Contractor elects to use an automatic batching system, an interval timer shall control the time of mixing. The interval timer shall be interlocked so that the mixer cannot be discharged until all of the materials have been mixed for the full amount of time specified.

39-5.04B Continuous Mixing

Continuous mixing plants shall utilize pugmill or drier-drum mixers.

When asphalt concrete is produced by pugmill mixing, the mixer shall be equipped with paddles of a type and arrangement to provide sufficient mixing action and movement to the asphalt concrete mixture to produce properly mixed asphalt concrete. The combined aggregate shall be fed directly from the drier to the mixer at a uniform and controlled rate.

Mixing shall continue until a homogeneous asphalt concrete mixture of thoroughly and uniformly coated aggregates of unchanging appearance is produced at the discharge point from the mixer.

The temperature of the completed asphalt concrete mixture shall not exceed 165°C upon discharge from the mixer.

The mixer shall discharge into a storage silo with a capacity of not less than that specified in Section 39-5.05, "Asphalt Concrete Storage," of this specification. The Contractor shall provide a means of diverting the flow of asphalt concrete away from the silo to prevent incompletely mixed portions of the asphalt concrete mixture from entering the silo.

39-5.05 Asphalt Concrete Storage

When asphalt concrete is stored, it shall be stored only in silos. Asphalt concrete shall not be stockpiled. The minimum quantity of asphalt concrete in any one silo during mixing shall be 18 tonnes except for the period immediately following a shutdown of the plant of 2 hours or more. A means shall be provided to indicate that storage in each silo is being maintained as required.

Storage silos shall be equipped with a surge-batcher sized to hold a minimum of 1800 kg of material. A surge-batcher consists of equipment placed at the top of the storage silo which catches the continuous delivery of the completed asphalt concrete mix and changes it to individual batch delivery to prevent the segregation of product ingredients as the completed

asphalt concrete mix is placed into storage. The surge-batcher shall be center loading and shall be thermally insulated or heated to prevent material buildup. Rotary chutes shall not be used as surge-batchers.

The surge-batcher shall be independent and distinct from conveyors or chutes used to collect or direct the completed asphalt concrete mixture being discharged into storage silos and shall be the last device to handle the material before it enters the silo. Multiple storage silos shall be served by an individual surge-batcher for each silo. Material handling shall be free of oblique movement between the highest elevation (conveyor outfall) and subsequent placement in the silo. Discharge gates on surge-batchers shall be automatic in operation and shall discharge only after a minimum of 1800 kg of material has been collected and shall close before the last collected material leaves the device. Discharge gate design shall prevent the deflection of material during the opening and closing operation.

Asphalt concrete stored in excess of 18 hours shall not be used in the work. Asphalt concrete mixture containing hardened lumps shall not be used. Any storage facility which contained the material with the hardened lumps shall not be used for further storage until the cause of the lumps is corrected.

39-5.06 Asphalt Concrete Plants

Any plant, including commercial plants, that produce asphalt concrete that is subject to these specifications shall conform to the provisions in Section 7-1.01F, "Air Pollution Control," of the Standard Specifications, and shall be equipped with a wet-tube dust washer or equal and other devices which will reduce the dust emission to the degree that adjacent property is not damaged. The washer and other equipment shall function efficiently at all times when the plant is in operation.

During production, petroleum products such as diesel fuel and kerosene shall not be used as a release agent on belts, conveyors, hoppers or hauling equipment.

Plants shall be equipped with an inspection dock constructed so that a quality control technician or inspector standing on the dock can inspect the completed asphalt concrete mixture and take samples, as necessary, from the hauling vehicle before the vehicle leaves the plant site. This inspection dock shall allow the hauling vehicle to pull alongside and shall meet all applicable safety requirements of the California Division of Occupational Safety and Health. Haul vehicle drivers shall be instructed to stop at the dock whenever a quality control technician or inspector is on the dock and to remain there until directed to leave by that individual.

39-6 SUBGRADE, PRIME COAT, PAINT BINDER (TACK COAT), AND PAVEMENT REINFORCING FABRIC

39-6.01 Subgrade

Immediately prior to applying prime coat or paint binder (tack coat), or immediately prior to placing the asphalt concrete when a prime coat or paint binder (tack coat) is not required, the subgrade to receive asphalt concrete shall conform to the compaction requirement and elevation tolerances specified for the material involved and shall be free of loose or extraneous material. If the asphalt concrete is to be placed on an existing base or pavement which was not constructed as part of the contract, the surface shall be cleaned by sweeping, flushing or other means to remove all loose particles of paving, dirt and all other extraneous material immediately before applying the prime coat or paint binder (tack coat).

39-6.02 Prime Coat and Paint Binder (Tack Coat)

A prime coat of liquid asphalt shall be applied to the areas to be surfaced when there is a contract item for the work or when the work is required in "Asphalt Concrete," in Section 10-1, "General," elsewhere in these special provisions.

Prime coat shall be applied only to those areas designated by the Engineer.

Prime coat shall be applied at the approximate total rate of 1.15 L per square meter of surface covered. The exact rate and number of applications will be determined by the Engineer.

Prime coat shall be applied at a temperature conforming to the range of temperatures provided in Section 93-1.03, "Mixing and Applying," of the Standard Specifications, for distributor application of the grade of liquid asphalt being used.

A paint binder (tack coat) of asphaltic emulsion shall be furnished and applied in accordance with the provisions in Section 94, "Asphaltic Emulsions," of the Standard Specifications, and shall be applied to all vertical surfaces of existing pavement, curbs, gutters, and construction joints in the surfacing against which additional material is to be placed, to a pavement to be surfaced, and to other surfaces designated in "Asphalt Concrete," in Section 10-1, "General," elsewhere in these special provisions.

Paint binder (tack coat) shall be applied in one application at a rate of from 0.10- to 0.45-L per square meter of surface covered. The exact rate of application will be determined by the Engineer.

At the Contractor's option, paving asphalt may be used for paint binder (tack coat) instead of asphaltic emulsion. If paving asphalt is used, the grade to be used and the rate of application will be determined by the Engineer. The paving asphalt shall be applied at a temperature of not less than 140°C, nor more than 175°C.

Prime coat or paint binder (tack coat) shall be applied only so far in advance of placing the surfacing as may be permitted by the Engineer. When asphaltic emulsion is used as a paint binder (tack coat), the asphalt concrete shall not be placed until the asphaltic emulsion has cured.

Immediately in advance of placing asphalt concrete, additional prime coat or paint binder (tack coat) shall be applied as directed by the Engineer to areas where the prime coat or paint binder (tack coat) has been damaged, and loose or extraneous material shall be removed, and no additional compensation will be allowed therefor.

39-6.03 Pavement Reinforcing Fabric

Pavement reinforcing fabric shall be placed on existing pavement to be surfaced or between layers of asphalt concrete when such work is shown on the plans, or specified in "Asphalt Concrete," in Section 10-1, elsewhere in these special provisions, or ordered by the Engineer.

Before placing the pavement reinforcing fabric, a binder of paving asphalt shall be applied to the surface to receive the pavement reinforcing fabric at an approximate rate of 1.15 L per square meter of surface covered. The exact rate will be determined by the Engineer. The binder shall be applied to a width equal to the width of the fabric mat plus 75 mm on each side.

Before applying binder, large cracks, spalls and depressions in existing pavement shall be repaired as directed by the Engineer, and the repair work will be paid for as extra work as provided in Section 4-1.03D, "Extra Work," of the Standard Specifications.

The fabric shall be aligned and placed with no wrinkles that lap. The test for lapping shall be made by gathering together the fabric in a wrinkle. If the height of the doubled portion of extra fabric is 15 mm or more, the fabric shall be cut to remove the wrinkle, then lapped in the direction of paving. Lap in excess of 50 mm shall be removed. Pavement reinforcing fabric shall not be placed in areas of conform tapers where the thickness of the overlying asphalt concrete is 30 mm or less.

If manual laydown methods are used, the fabric shall be unrolled, aligned, and placed in increments of approximately 9 m.

Adjacent borders of the fabric shall be lapped 50 to 100 mm. The preceding roll shall be lapped 50 to 100 mm over the following roll in the direction of paving at ends of rolls or at any break. At fabric overlays, both the binder and the fabric shall overlap previously placed fabric by the same amount.

Seating of the fabric with rolling equipment after placing will be permitted. Turning of the paving machine and other vehicles shall be gradual and kept to a minimum to avoid damage to the fabric.

A small quantity of asphalt concrete, to be determined by the Engineer, may be spread over the fabric immediately in advance of placing asphalt concrete surfacing in order to prevent fabric from being damaged by construction equipment.

Public traffic shall not be allowed on the bare reinforcing fabric, except that public cross traffic may be allowed to cross the fabric under traffic control after the Contractor has placed a small quantity of asphalt concrete over the fabric.

Care shall be taken to avoid tracking binder material onto the pavement reinforcing fabric or distorting the fabric during seating of the fabric with rolling equipment. If necessary to protect the pavement reinforcing fabric, exposed binder material may be covered lightly with sand.

39-7 SPREADING AND COMPACTING EQUIPMENT

39-7.01 Spreading Equipment

Asphalt pavers shall be self-propelled mechanical spreading and finishing equipment, provided with a screed or strike-off assembly capable of distributing the material to not less than the full width of a traffic lane. Screed action shall include any cutting, crowding, or other practical action which is effective on the asphalt concrete mixture without tearing, shoving, or gouging, and which produces a surface texture of uniform appearance. The screed shall be adjustable to the required section and thickness. The screed shall be provided with a suitable full width compacting device. Pavers that leave ridges, indentations or other marks in the surface shall not be used unless the ridges, indentations, or marks are eliminated by rolling or prevented by adjustment in the operation.

The asphalt paver shall operate independently of the vehicle being unloaded or shall be capable of propelling the vehicle being unloaded in a satisfactory manner. The load of the haul vehicle shall be limited to that which will insure satisfactory spreading. While being unloaded the haul vehicle shall be in contact with the machine at all times, and the brakes on the haul vehicle shall not be depended upon to maintain contact between the vehicle and the machine.

No portion of the mass of hauling or loading equipment, other than the connection, shall be supported by the asphalt paver, and no vibrations or other motions of the loader, which could have a detrimental effect on the riding quality of the completed pavement, shall be transmitted to the paver.

When asphalt concrete is placed directly upon asphalt treated permeable base, the asphalt concrete shall be placed with a paver equipped with tracks unless the layer being placed is 45 mm or less in compacted thickness.

39-7.02 Compacting Equipment

The Contractor shall furnish a sufficient number of rollers to obtain the specified compaction and surface finish required by these specifications. One roller each shall be provided for breakdown, intermediate, and finish rolling. The Contractor shall size the rollers to achieve the required results.

All rollers shall be equipped with pads and water systems which prevent sticking of asphalt concrete mixtures to the pneumatic or steel-tired wheels. A parting agent which will not damage the asphalt concrete mixture, as approved by the Engineer, may be used to aid in preventing the sticking of the asphalt concrete mixture to the wheels.

39-8 SPREADING AND COMPACTING

39-8.01 General Requirements

Asphalt concrete shall be handled, spread and compacted in a manner which is in conformance with this specification.

Asphalt concrete shall be placed in such a manner so that cracking, shoving and displacement will be avoided.

Type A and Type B asphalt concrete shall be placed only when the atmospheric temperature is above 10°C.

Asphalt concrete shall not be placed when the underlying layer or surface is frozen or when weather conditions will prevent proper handling, finishing, or compaction of the mixture.

Asphalt concrete shall be spread and compacted in the number of layers of the thicknesses indicated in the following table:

All thicknesses shown are in millimeters							
Total Thickness Shown on the Plans*	Number of Layers	Top Layer Thickness		Next Lower Layer Thickness		All Other Lower Layers Thickness	
		Min.	Max.	Min.	Max.	Min.	Max.
60 or 75	one	—	—	—	—	—	—
105 through 140	2	45	60	60	75	—	—
150 or more	**	45	60	45	75	60	120

Notes:

* When pavement reinforcing fabric is shown to be placed between layers of asphalt concrete, the thickness of asphalt concrete above the pavement reinforcing fabric shall be considered to be the "Total Thickness Shown on the Plans" for the purpose of spreading and compacting the asphalt concrete above the pavement reinforcing fabric.

**At least 3 layers if total thickness is 150 mm or more and less than 270 mm. At least 4 layers if total thickness is 270 mm or more.

A layer shall not be placed over a layer which exceeds 75 mm in compacted thickness until the temperature of the layer being covered is less than 70°C at mid depth.

Asphalt concrete to be placed on shoulders, and on other areas off the traveled way having a width of 150 mm or more, shall be spread in the same manner as above.

The completed mixture shall be deposited on the roadbed at a uniform quantity per linear meter, as necessary to provide the required compacted thickness without resorting to spotting, picking-up or otherwise shifting the mixture.

Segregation shall be avoided, and the surfacing shall be free from pockets of coarse or fine material. Asphalt concrete containing hardened lumps shall not be used.

Longitudinal joints in the top layer shall correspond with the edges of proposed traffic lanes. Longitudinal joints in all other layers shall be offset not less than 150 mm alternately each side of the edges of traffic lanes.

Unless otherwise provided herein or permitted by the Engineer, the top layer of asphalt concrete for shoulders, tapers, transitions, road connections, private drives, curve widenings, chain control lanes, turnouts, left turn pockets, and other such areas, shall not be spread before the top layer of asphalt concrete for the adjoining through lane has been spread and compacted. At locations where the number of lanes is changed, the top layer for the through lanes shall be paved first. When existing pavement is to be surfaced and the specified thickness of asphalt concrete to be spread and compacted on the existing pavement is 75 mm or less, shoulders or other adjoining areas may be spread simultaneously with the through lane provided the completed surfacing conforms to the requirement of this specification. Tracks or wheels of spreading equipment shall not be operated on the top layer of asphalt concrete in any area until final compaction has been completed.

At location shown on the plans, specified in "Asphalt Concrete," in Section 10-1, "General," elsewhere in these special provisions, or as directed by the Engineer, the asphalt concrete shall be tapered or feathered to conform to existing surfacing or to other highway and non-highway facilities.

At locations where the asphalt concrete is to be placed over areas inaccessible to spreading and rolling equipment, the asphalt concrete shall be spread by any means practicable so as to obtain the specified results and shall be compacted thoroughly to the required lines, grades and cross sections by means of pneumatic tampers, or by other methods that will produce the same degree of compaction as pneumatic tampers.

39-8.02 Test Strip Start Up Procedures

On the first day of asphalt concrete production, of each asphalt concrete mixture, the Contractor shall produce an initial quantity of asphalt concrete mixture sufficient to construct a test strip of asphalt concrete surfacing to the thickness representative of the operations for each asphalt concrete mixture. The test strip shall contain a minimum of 100 tonnes, and a maximum of 500 tonnes, of complete in place asphalt concrete. The amount of asphalt concrete to be initially produced for the construction of the test strip shall be proposed to the Engineer by the Contractor, and shall be approved by the Engineer. The Contractor shall construct the test strip on the project at a location approved by the Engineer. The purpose of the test strip is to establish a rolling pattern which will produce the specified asphalt concrete density, to develop a correlation between cores taken from the test strip and the Contractor's and Engineer's nuclear density gage readings taken at the core

locations on the test strip, and to verify the Contractor's mix design and asphalt concrete mixture quality characteristics which shall be produced for the contract.

The Contractor shall construct the test strip using asphalt concrete mix production, lay-down and compaction procedures and equipment intended for the entire project. The Contractor shall stop production of asphalt concrete after construction of the test strip until the specified quality characteristics of the asphalt concrete mixture and the density values of the test strip have been tested, verified and accepted by the Engineer according to the requirements designated in Section 39-4.03, "Engineer Testing for Verification," of this specification.

Attention is directed to longitudinal and transverse construction joint requirements specified in "Asphalt Concrete" in Section 10-1, "General," elsewhere in these special provisions.

The following sampling and testing requirements shall apply to the test strip:

The Contractor shall obtain 3 representative asphalt concrete mixture samples from the test strip and shall evaluate the material for conformance to the asphalt concrete mixture requirements specified in Section 39-2.03, "Aggregate," of this specification. The Engineer will also obtain 3 representative asphalt concrete mixture samples from the test strip at the same location for purposes of verification of the Contractor's test data. The Contractor's test data will be considered verified if the asphalt concrete mixture design parameters conform to the requirements specified herein for minimum and maximum values and the design parameter of asphalt concrete mixture percent air voids is within ± 1.0 percent of the percent air voids designated in the Contractor's verified mix design submitted in accordance with the requirements of Section 39-2.01, "Mix Design," of this specification. The asphalt concrete mixture is acceptable if the verified test data for the design parameters from the 3 asphalt concrete mixture samples are within the specified limits.

The Contractor shall obtain 5 representative samples and shall evaluate the material quality characteristics for conformance to the requirements specified in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification. If the test data from one or more samples are outside the specified limits, but the average of the test data from all samples is within the specification limits, the Engineer may tentatively accept the test strip with the Contractor's assurance that adjustments to the process will be made to correct the indicated quality characteristic deficiencies in the asphalt concrete mixture.

The Contractor shall obtain nuclear density gage readings for density and relative compaction determinations, and obtain 2 core samples each at a minimum of 10 locations selected at random within the test strip. The Engineer will obtain nuclear density gage readings at the same locations within the test strip as the Contractor. The Contractor shall perform the nuclear density gage tests according to California Test 375. The test data of each of the 2 cores shall be averaged to obtain one set of test data per location. The Contractor shall furnish the Engineer with the Contractor's calibration values for correlation of the nuclear density gage readings to the core densities. The test strip density will be acceptable if all core test data yield a pay factor of 0.90 or greater when determined in accordance with Section 39-10.02B, "Statistical Evaluation," of this specification.

The Contractor shall repeat the test strip process until the material properties and mix design parameters produced conform to the requirements specified herein and the test strip is accepted by the Engineer. Test strips that are accepted by the Engineer may remain in place and payment for the test strips will be determined by the procedure specified in Section 39-10.02B, "Statistical Evaluation and Determination of Pay Factor," of this specification. Test data used to accept the test strips will not be included with the test data used for acceptance of the work according to the requirements of Section 39-10, "Acceptance," of this specification. Upon receipt of written approval from the Engineer that the test strip has been accepted, full production of asphalt concrete may commence. The Contractor may request that unacceptable test strips be left in place.

The Contractor shall use the test strip start-up procedures specified herein when resuming production of asphalt concrete after a termination of asphalt concrete production due to unsatisfactory material quality characteristics.

39-8.03 Spreading

All layers, except as otherwise specified, shall be spread with an asphalt paver, unless otherwise approved by the Engineer. Asphalt pavers shall be operated in such a manner as to insure continuous and uniform movement of the paver.

In advance of spreading asphalt concrete over an existing base, surfacing, or bridge deck, if there is a contract item for asphalt concrete (leveling), or if ordered by the Engineer, asphalt concrete shall be spread by any mechanical means that will produce a uniform smoothness and texture to level irregularities, and to provide a smooth base in order that subsequent layers will be of uniform thickness. Section 39-10.02, "Statistical Evaluation and Determination of Pay Factor," of this specification, shall not apply to asphalt concrete (leveling).

When directed by the Engineer, paint binder (tack coat) shall be applied to any layer in advance of spreading the next layer.

Before placing the top layer adjacent to cold transverse construction joints, the joints shall be trimmed to a vertical face and to a neat line. Transverse joints shall be tested with a 3.6 m ± 0.06-m straightedge and shall be cut back as required to conform to the requirements specified in Section 39-8.04, "Compacting," of this specification, for surface smoothness. Connections to existing surfacing shall be feathered to conform to the requirements for smoothness. Longitudinal joints shall be trimmed to a vertical face and to a neat line if the edges of the previously laid surfacing are, in the opinion of the Engineer, in such condition that the quality of the completed joint will be affected.

39-8.04 Compacting

Compacting equipment shall conform to the provisions of Section 39-7.03, "Compacting Equipment," of this specification.

Rolling shall commence at the lower edge and shall progress toward the highest portion, except that when compacting layers which exceed 75 mm in compacted thickness, and if approved by the Engineer, rolling shall commence at the center and shall progress outwards.

The Contractor shall monitor density during the compaction process with nuclear density gages calibrated to the control strip core density test data. Asphalt concrete shall be compacted to a relative compaction of not less than 96 percent and shall be finished to the lines, grades, and cross sections shown on the plans. In-place density of asphalt concrete will be determined prior to opening the pavement to public traffic.

Relative compaction shall be determined by California Test 375. Laboratory specimens shall be compacted in conformance with California Test 304. Test locations will be established for asphalt concrete areas to be tested, as specified in California Test 375.

Upon completion of rolling operations, if ordered be the Engineer, the asphalt concrete shall be cooled by applying water. Applying water shall conform to the provision in Section 17, "Watering," of the Standard Specifications.

The completed surfacing shall be thoroughly compacted, smooth, and free from ruts, humps, depressions, or irregularities. Any ridges, indentations or other objectionable marks left in the surface of the asphalt concrete by blading or other equipment shall be eliminated by rolling or other suitable means. The use of any equipment that leaves ridges, indentations, or other objectionable marks in the asphalt concrete shall be discontinued, and acceptable equipment shall be furnished by the Contractor.

When a straightedge 3.6 m ± 0.06-m long is laid on the finished surface and parallel with the center line, the surface shall not vary more than 3 mm from the lower edge of the straightedge. The transverse slope of the finished surface shall be uniform to a degree such that no depressions greater than 6 mm are present when tested with a straightedge 3.6 m ± 0.06-m long in a direction transverse to the center line and extending from edge to edge of a 3.6-m traffic lane.

Pavement within 15 m of a structure or approach slab shall conform to the smoothness tolerances specified in Section 51-1.17, "Finishing Bridge Decks," of the Standard Specifications.

39-9 (BLANK)

39-10 ACCEPTANCE OF WORK

39-10.01 General

The Engineer will select the procedure used to determine the quantities of asphalts concrete for acceptance and payment determination in conformance with the requirements specified herein.

The Contractor's quality control test data which has been verified by the Engineer will form the basis for acceptance of the work. The quality requirements on which acceptance will be based are specified in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification.

Work determined by the Engineer to conform to the requirements specified herein will be paid for at the contract price per tonne for asphalt concrete and may be subject to compensation adjustment in accordance with Section 39-10.02C, "Pay Factor Determination and Compensation Adjustment," of this specification.

Work that does not conform to the specified requirements may be rejected by the Engineer at any time and shall be removed and replaced by the Contractor, at the Contractor's expense.

If a lot is concluded with fewer than 5 samples, the work will be accepted or rejected based on the quality requirements specified in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification. Section 39-10.02, "Statistical Evaluation and Pay Factor Determination," of this specification, shall not apply to the lot. The Engineer may reject any batch, load, or portion of roadway that appears to not be in compliance with these specifications.

Any quantity of material that is determined to be defective may be rejected by the Engineer based on visual inspection or noncompliance with the specifications herein.

Rejected material shall not be incorporated into the roadway unless authorized in writing by the Engineer. The Contractor may request that work rejected by the Engineer on a visual basis be tested for conformance to the specifications.

If the Contractor elects to have material tested which was visually rejected by the Engineer, a minimum of 5 random samples of the material shall be obtained and split into representative portions and tested for compliance with the material quality requirements specified herein. Sampling of the material shall be witnessed by the Engineer. The Contractor shall provide the Engineer with one representative split portion of each sample obtained for verification testing purposes according to the requirements of Section 39-4.04 "Statistical Verification Tests," of this specification. If the Engineer cannot verify the Contractor's test data, no payment will be made and the material shall be removed at the Contractor's expense. In addition, the cost of the Engineer's verification testing will be deducted from any moneys due or to become due the Contractor. If the Engineer verifies the Contractor's test data, and the test data indicates that the material is in compliance with the material quality requirements specified herein, the cost of the Engineer's verification testing will be borne by the State. The test data obtained from testing this rejected material will be excluded from the payment determination of the lot .

39-10.02 Statistical Evaluation and Determination of Pay Factor

Statistical evaluation of the work shall be used to verify the Contractor's quality control test data to determine compliance with the specified requirements.

39-10.02A General

The quality characteristics to be evaluated, test methods, and specification limits are specified in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification. Asphalt content, aggregate gradation (600-µm and 75-µm sieves), and relative compaction are considered for purposes of this specification to be critical quality characteristics.

A lot is a discrete quantity of work to which the statistical acceptance procedure is applied. For this contract, a lot represents the total quantity of asphalt concrete placed. More than one lot will occur if changes in the target values, material sources, or mix design are requested by the Contractor in writing and made in accordance with the requirements of this specification, or if production of asphalt concrete is terminated due to unsatisfactory material quality characteristics.

The frequency of sampling is specified in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification. Five samples is the minimum number of samples required to perform a statistical *t*-test evaluation. The maximum obtainable pay factor with 5 samples is 1.01. A minimum of 8 samples is required to obtain a 1.05 pay factor. If the sampling frequencies and quantity of work would otherwise result in fewer than 8 samples, the Contractor may submit a written request to increase the sampling frequency to provide for a minimum of 8 samples. The Contractor shall provide the Engineer with the request to increase the sampling frequency at least 48 hours before the beginning of asphalt concrete production.

The point of sampling is indicated in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification. The location of sampling shall be determined by a random method approved by the Engineer. The Engineer will obtain random samples for verification testing independent of the Contractor.

The work in the lot will be accepted and a final pay factor determined when all sampling, inspection and test data are completed and have been submitted, evaluated and approved by the Engineer. Contractor quality control test data shall be verified by the Engineer using the *t*-test as designated in Section 39-4.04, "Statistical Verification Tests," of this specification, before the data will be accepted by the Engineer.

If the current composite pay factor of a lot is less than 1.00, the work represented by the lot will be accepted by the Engineer, provided the lowest single pay factor is not within the reject portion of Table 39-2, "Pay Factors," of this specification.

If the current composite pay factor of a lot is less than 1.00, and the lowest single pay factor is within the reject portion of Table 39-2, "Pay Factors," of this specification, the lot will be rejected. The Contractor shall remove all rejected material from the work, at the Contractor's expense.

If the current composite pay factor of a lot is less than 0.90, the Contractor shall terminate asphalt concrete production and the Engineer will terminate the lot. Production of asphalt concrete may resume after the Contractor takes necessary actions to improve the quality of the asphalt concrete product, and the proposed actions are approved in writing by the Engineer.

If any pay factor for a critical quality characteristic designated in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification, is less than 0.90 for the lot, or is within the rejection range for the last five tests, the Contractor shall terminate asphalt concrete production. Asphalt concrete production may resume after the Contractor takes necessary actions to improve the quality of the asphalt concrete product and the proposed actions are approved in writing by the Engineer. A new lot will be established when production resumes.

When approved in writing by the Engineer, the Contractor may voluntarily remove defective material and replace it with new material to avoid or minimize a pay factor of less than 1.00. New material will be sampled, tested, and evaluated for acceptance according to the requirements of this specification.

39-10.02B Statistical Evaluation

The Variability-Unknown/Standard Deviation Method will be used to determine the estimated percentage of the lot that is outside specification limits. The number of significant figures used in the calculations will in accordance with the requirements of AASHTO Designation R-11, Absolute Method.

The estimated percentage of work that is outside of the specification limits for each quality characteristic will be determined as follows:

(1) Calculate the arithmetic mean ($\overline{X}$) of the test values;

$$\overline{X} = \frac{\Sigma x}{n}$$

where: _ = summation of
x = individual test value
n = total number of test values

(2) Calculate the standard deviation (s);

$$s = \sqrt{\frac{n\sum(x^2)-(\sum x)^2}{n(n-1)}}$$

where: _(x ^{2}) = summation of the squares of individual test values
(_x)^2 = summation of the individual test values squared

(3) Calculate the upper quality index (Q_u);

$$Q_u = \frac{USL - \overline{X}}{s}$$

where: USL = upper specification limit
s = standard deviation
$\overline{X}$ = arithmetic mean

(Note: The USL is equal to the contract specification limit or the target value plus the allowable deviation.)

(4) Calculate the lower quality index (Q_L);

$$Q_L = \frac{\overline{X} - LSL}{s}$$

where: LSL = lower specification limit
s = standard deviation
$\overline{X}$ = arithmetic mean

(Note: The LSL is equal to the contract specification limit or the target value minus the allowable deviation.)

(5) From Table 39-1, "Estimated Percent of Work Outside Specification Limits," of this specification, determine P_U;

where: P_U = the estimated percentage of work outside the USL.
(P_U corresponds to a given Q_U; P_U = 0, when USL is not specified.)

(6) From Table 39-1, "Estimated Percent of Work Outside Specification Limits," of this specification, determine P_L;

where: P_L = the estimated percentage of work outside the LSL.
(P_L corresponds to a given Q_L; P_L = 0, when LSL is not specified.)

(7) Calculate the total estimated percentage of work outside the USL and LSL, Percent Defective;

Percent Defective = $P_U + P_L$

(8) Repeat Steps 1 through 7 for each quality characteristic listed for acceptance.

39-10.02C Pay Factor Determination and Compensation Adjustment

The pay factor and compensation adjustment for a lot will be determined as follows:

1. From Table 39-2, "Pay Factors," of this specification, determine the pay factor for each quality characteristic, (PF_{QC}) using the total number of test data values and the total estimated percentage outside the specification limits ($P_U + P_L$) from Step 7 in Section 39-10.02B, "Statistical Evaluation," of this specification.

2. The pay factor for the lot is a composite of single pay factors determined for each quality characteristic designated in Table 39-3, "Minimum Quality Control Required for Acceptance," of this specification. The following formula is used:

$$PF_C = \sum_{i=1}^{8} w_i \, PF_{QC_i}$$

where: PF_C = the composite pay factor for the lot,
PF_{QC} = the pay factor for the individual quality characteristic,
w = the weighting factor listed in Table 39-3, and
i = the quality characteristic index number.

3. Payment to the Contractor for the lot of asphalt concrete will be subject to a compensation adjustment. The Compensation Adjustment Factor (CAF) will be determined as follows:

$$CAF = PF_C - 1$$

The amount of the compensation adjustment will be calculated as the product of: (1) the Compensation Adjustment Factor, (2) the total tonnes represented in the lot, and (3) the contract unit price per tonne for the contract item of asphalt involved. If the compensation adjustment is a negative value, the compensation adjustment will be deducted from any moneys due, or that may become due, the Contractor under the contract. If the compensation adjustment is a positive value, it will be added to any moneys due, or that may become due, the Contractor under the contract.

Table 39-1.—Estimated Percent of Work Outside Specification Limits

Estimated Percent Outside Specification Limits (P_u and/or P_L)	Upper Quality Index Q_u or Lower Quality Index Q_L						
	n=5	n=6	n=7	n=8	n=9	n=10 to n=11	n=12 to n=14
0	1.72	1.88	1.99	2.07	2.13	2.20	2.28
1	1.64	1.75	1.82	1.88	1.91	1.96	2.01
2	1.58	1.66	1.72	1.75	1.78	1.81	1.84
3	1.52	1.59	1.63	1.66	1.68	1.71	1.73
4	1.47	1.52	1.56	1.58	1.60	1.62	1.64
5	1.42	1.47	1.49	1.51	1.52	1.54	1.55
6	1.38	1.41	1.43	1.45	1.46	1.47	1.48
7	1.33	1.36	1.38	1.39	1.40	1.41	1.41
8	1.29	1.31	1.33	1.33	1.34	1.35	1.35
9	1.25	1.27	1.28	1.28	1.29	1.29	1.30
10	1.21	1.23	1.23	1.24	1.24	1.24	1.25
11	1.18	1.18	1.19	1.19	1.19	1.19	1.20
12	1.14	1.14	1.15	1.15	1.15	1.15	1.15
13	1.10	1.10	1.10	1.10	1.10	1.10	1.11
14	1.07	1.07	1.07	1.06	1.06	1.06	1.06
15	1.03	1.03	1.03	1.03	1.02	1.02	1.02
16	1.00	0.99	0.99	0.99	0.99	0.98	0.98
17	0.97	0.96	0.95	0.95	0.95	0.95	0.94
18	0.93	0.92	0.92	0.92	0.91	0.91	0.91
19	0.90	0.89	0.88	0.88	0.88	0.87	0.87
20	0.87	0.86	0.85	0.85	0.84	0.84	0.84
21	0.84	0.82	0.82	0.81	0.81	0.81	0.80
22	0.81	0.79	0.79	0.78	0.78	0.77	0.77
23	0.77	0.76	0.75	0.75	0.74	0.74	0.74
24	0.74	0.73	0.72	0.72	0.71	0.71	0.70
25	0.71	0.70	0.69	0.69	0.68	0.68	0.67
26	0.68	0.67	0.67	0.65	0.65	0.65	0.64
27	0.65	0.64	0.63	0.62	0.62	0.62	0.61
28	0.62	0.61	0.60	0.59	0.59	0.59	0.58
29	0.59	0.58	0.57	0.57	0.56	0.56	0.55
30	0.56	0.55	0.54	0.54	0.53	0.53	0.52
31	0.53	0.52	0.51	0.51	0.50	0.50	0.50
32	0.50	0.49	0.48	0.48	0.48	0.47	0.47
33	0.47	0.48	0.45	0.45	0.45	0.44	0.44
34	0.45	0.43	0.43	0.42	0.42	0.42	0.41
35	0.42	0.40	0.40	0.39	0.39	0.39	0.38
36	0.39	0.38	0.37	0.37	0.36	0.36	0.36
37	0.36	0.35	0.34	0.34	0.34	0.33	0.33
38	0.33	0.32	0.32	0.31	0.31	0.31	0.30
39	0.30	0.30	0.29	0.28	0.28	0.28	0.28
40	0.28	0.25	0.25	0.25	0.25	0.25	0.25
41	0.25	0.23	0.23	0.23	0.23	0.23	0.23
42	0.23	0.20	0.20	0.20	0.20	0.20	0.20
43	0.18	0.18	0.18	0.18	0.18	0.18	0.18
44	0.16	0.15	0.15	0.15	0.15	0.15	0.15
45	0.13	0.13	0.13	0.13	0.13	0.13	0.13
46	0.10	0.10	0.10	0.10	0.10	0.10	0.10
47	0.08	0.08	0.08	0.08	0.08	0.08	0.08
48	0.05	0.05	0.05	0.05	0.05	0.05	0.05
49	0.03	0.03	0.03	0.03	0.03	0.03	0.03
50	0.00	0.00	0.00	0.00	0.00	0.00	0.00

(Table continued next page)

Table 39-1 (continued).—Estimated Percent of Work Outside Specification Limits

Estimated Percent Outside Specification Limits (Pu and/or PL)	Upper Quality Index Qu or Lower Quality Index QL					
	n=15 to n=17	n=18 to n=22	n=23 to n=29	n=30 to n=42	n=43 to n=66	n=67 to ∞
0	2.34	2.39	2.44	2.48	2.51	2.56
1	2.04	2.07	2.09	2.12	2.14	2.16
2	1.87	1.89	1.91	1.93	1.94	1.95
3	1.75	1.76	1.78	1.79	1.80	1.81
4	1.65	1.66	1.67	1.68	1.69	1.70
5	1.56	1.57	1.58	1.59	1.59	1.60
6	1.49	1.50	1.50	1.51	1.51	1.52
7	1.42	1.43	1.43	1.44	1.44	1.44
8	1.36	1.36	1.37	1.37	1.37	1.38
9	1.30	1.30	1.31	1.31	1.31	1.31
10	1.25	1.25	1.25	1.25	1.26	1.26
11	1.20	1.20	1.20	1.20	1.20	1.20
12	1.15	1.15	1.15	1.15	1.15	1.15
13	1.11	1.11	1.11	1.11	1.11	1.11
14	1.06	1.06	1.06	1.06	1.06	1.06
15	1.02	1.02	1.02	1.02	1.02	1.02
16	0.98	0.98	0.98	0.98	0.98	0.98
17	0.94	0.94	0.94	0.94	0.94	0.94
18	0.91	0.90	0.90	0.90	0.90	0.90
19	0.87	0.87	0.87	0.87	0.87	0.87
20	0.83	0.83	0.83	0.83	0.83	0.83
21	0.80	0.80	0.80	0.80	0.80	0.79
22	0.77	0.76	0.76	0.76	0.76	0.76
23	0.73	0.73	0.73	0.73	0.73	0.73
24	0.70	0.70	0.70	0.70	0.70	0.70
25	0.67	0.67	0.67	0.67	0.67	0.66
26	0.64	0.64	0.64	0.64	0.64	0.63
27	0.61	0.61	0.61	0.61	0.61	0.60
28	0.58	0.58	0.58	0.58	0.58	0.57
29	0.55	0.55	0.55	0.55	0.55	0.54
30	0.52	0.52	0.52	0.52	0.52	0.52
31	0.49	0.49	0.49	0.49	0.49	0.49
32	0.47	0.46	0.46	0.46	0.46	0.46
33	0.44	0.44	0.43	0.43	0.43	0.43
34	0.41	0.41	0.41	0.41	0.41	0.40
35	0.38	0.38	0.38	0.38	0.38	0.38
36	0.36	0.36	0.36	0.36	0.36	0.36
37	0.33	0.33	0.33	0.33	0.33	0.32
38	0.30	0.30	0.30	0.30	0.30	0.30
39	0.28	0.28	0.28	0.28	0.28	0.28
40	0.25	0.25	0.25	0.25	0.25	0.25
41	0.23	0.23	0.23	0.23	0.23	0.23
42	0.20	0.20	0.20	0.20	0.20	0.20
43	0.18	0.18	0.18	0.18	0.18	0.18
44	0.15	0.15	0.15	0.15	0.15	0.15
45	0.13	0.13	0.13	0.13	0.13	0.13
46	0.10	0.10	0.10	0.10	0.10	0.10
47	0.08	0.08	0.08	0.08	0.08	0.08
48	0.05	0.05	0.05	0.05	0.05	0.05
49	0.03	0.03	0.03	0.03	0.03	0.03
50	0.00	0.00	0.00	0.00	0.00	0.00

Notes:1. If the value of Q_U or Q_L does not correspond to a value in the table, use the next lower value.

2. If Q_U or Q_L are negative values, P_U or P_L is equal to 100 minus the table value for P_U or P_L

Table 39-2.—Pay Factors

PAY FACTOR	Sample Size												
	n=5	n=6	n=7	n=8	n=9	n=10 to n=11	n=12 to n=14	n=15 to n=17	n=18 to n=22	n=23 to n=29	n=30 to n=42	n=43 to n=66	n=67 to ∞
	Maximum Allowable Percent of Work Outside Specification Limits for A Given Pay Factor ($P_u + P_L$)												
1.05				0	0	0	0	0	0	0	0	0	0
1.04			0	1	3	5	4	4	4	3	3	3	3
1.03		0	2	4	6	8	7	7	6	5	5	4	4
1.02		1	3	6	9	11	10	9	8	7	7	6	6
1.01	0	2	5	8	11	13	12	11	10	9	8	8	7
1.00	22	20	18	17	16	15	14	13	12	11	10	9	8
0.99	24	22	20	19	18	17	16	15	14	13	11	10	9
0.98	26	24	22	21	20	19	18	16	15	14	13	12	10
0.97	28	26	24	23	22	21	19	18	17	16	14	13	12
0.96	30	28	26	25	24	22	21	19	18	17	16	14	13
0.95	32	29	28	26	25	24	22	21	20	18	17	16	14
0.94	33	31	29	28	27	25	24	22	21	20	18	17	15
0.93	35	33	31	29	28	27	25	24	22	21	20	18	16
0.92	37	34	32	31	30	28	27	25	24	22	21	19	18
0.91	38	36	34	32	31	30	28	26	25	24	22	21	19
0.90	39	37	35	34	33	31	29	28	26	25	23	22	20
0.89	41	38	37	35	34	32	31	29	28	26	25	23	21
0.88	42	40	38	36	35	34	32	30	29	27	26	24	22
0.87	43	41	39	38	37	35	33	32	30	29	27	25	23
0.86	45	42	41	39	38	36	34	33	31	30	28	26	24
0.85	46	44	42	40	39	38	36	34	33	31	29	28	25
0.84	47	45	43	42	40	39	37	35	34	32	30	29	27
0.83	49	46	44	43	42	40	38	36	35	33	31	30	28
0.82	50	47	46	44	43	41	39	38	36	34	33	31	29
0.81	51	49	47	45	44	42	41	39	37	36	34	32	30
0.80	52	50	48	46	45	44	42	40	38	37	35	33	31
0.79	54	51	49	48	46	45	43	41	39	38	36	34	32
0.78	55	52	50	49	48	46	44	42	41	39	37	35	33
0.77	56	54	52	50	49	47	45	43	42	40	38	36	34
0.76	57	55	53	51	50	48	46	44	43	41	39	37	35
0.75	58	56	54	52	51	49	47	46	44	42	40	38	36
	60	57	55	53	52	51	48	47	45	43	41	40	37
	61	58	56	55	53	52	50	48	46	44	43	41	38
Reject	62	59	57	56	54	53	51	49	47	45	44	42	39
	63	61	58	57	55	54	52	50	48	47	45	43	40
	64	62	60	58	57	55	53	51	49	48	46	44	41
Reject Values Greater Than Those Shown Above													

Notes:

1. To obtain a pay factor when the estimated percent outside specification limits from Table 39-1 does not correspond to a value in the table, use the next larger value.
2. The maximum obtainable pay factor is 1.05 (with a minimum of 8 test values).

Table 39-3.—Minimum Quality Control Required for Acceptance

Index (i)	*Quality Characteristic*	*Specification Limits*	*Weighting Factor (w) for Pay*	*Test Method*	*Minimum Sampling and Testing Frequency*	*Point of Sampling*
1	Asphalt Content **	TV ± 0.5%	0.30	Extraction or calibrated nuclear asphalt content gage California Test 310, 379 (Or) Ignition Oven (Test Method under development)	One sample per 450 tonnes or portion thereof In all cases not less than one sample per day	Mat behind paver
	Gradation			Washed sieve analysis, California Test 202	One sample per 450 tonnes or portion thereof In all cases not less than one sample per day	Batch plant - from hot bins Drum Plant - from cold feed
2	19-mm or 12.5mm*.	TV ± 5%	0.01			
3	9.5-mm	TV ± 6%	0.01			
4	4.75-mm	TV ± 7%	0.05			
5	2.36-mm	TV ± 5%	0.05			
6	600µm**	TV ± 4%	0.08			
7	75µm**	TV ± 2%	0.10			
8	Relative Compaction **	96%	0.40	California Test 375	Per Test Method. Test Lot 450 tonnes	Finished mat after final rolling
	Test Maximum Density			California Test 375	Per Test Method.	Mat behind the paver
	Mix Moisture Content	<1%		California Test 310 or 370	One sample per 450 tonnes or portion thereof In all cases not less than one sample per day	Mat behind the Paver
	Asphalt and Mix Temperature	120°C to 175°C (Asphalt) ≤135°C (Mix)			Continuous using an automated recording device	Plant

Notes:

1. TV = Target Value from Contractor's proposed mix design
2 Production quantities which are less than the minimum specified in the Table shall be tested per the requirements of the Table.
3. * Depending on aggregate gradation specified.
   **Quality characteristics 1, 6, 7 and 8 are defined as critical quality characteristics in the verification testing process.

39-11 MEASUREMENT AND PAYMENT

39-11.01 Measurement

Asphalt concrete will be measured by mass. The quantity to be paid for will be the combined mass of the mixture for the various types of asphalt concrete, as designated in the Engineer's Estimate.

The mass of the materials will be determined as provided in Section 9-1.01, "Measurement of Quantities," of the Standard Specifications.

Quantities of paving asphalt, liquid asphalt and asphaltic emulsion to be paid for as contract items of work will be determined in accordance with the methods provided in Sections 92, "Asphalts," 93, "Liquid Asphalts," or 94, "Asphaltic Emulsions," of the Standard Specifications, as the case may be.

When recorded batch masses are printed automatically, these masses may be used for determining pay quantities providing the following requirements are complied with:

A. Total aggregate and supplemental fine aggregate mass per batch shall be printed. When supplemental fine aggregate is weighed cumulatively with the aggregate, the total batch mass of aggregate shall include the supplemental fine aggregate.
B. The total bitumen mass per batch shall be printed.
C. Zero-tolerance mass shall be printed prior to weighing the first batch and after weighing the last batch of each truckload.
D. The time, date, mix number, load number and truck identification shall be correlated with the load slip.
E. A copy of the recorded batch masses shall be certified by a licensed weighmaster and submitted to the Engineer.

Pavement reinforcing fabric will be measured and paid for by the square meter for the actual pavement area covered.

39-11.02 Payment

Asphalt concrete placed in the work, unless otherwise specified, will be paid for at the contract price per tonne for asphalt concrete of the types designated in the Engineer's Estimate.

Compensation adjustment for asphalt concrete will be as specified in Section 39-10.02C, "Pay Factor Determination and Compensation Adjustment," of this specification.

When there is a contract item for asphalt concrete (leveling), quantities of asphalt concrete placed for leveling will be paid for at the contract price per tonne for asphalt concrete (leveling). When there is no contract item for asphalt concrete (leveling), and leveling is ordered by the Engineer, asphalt concrete so used will be paid for as extra work as provided in Section 4-1.03D, "Extra Work," of the Standard Specifications.

Full compensation for the Contractor's Quality Control Plan, including furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in developing, implementing, modifying and fulfilling the requirements of the Quality Control Plan, as specified in this specification, shall be considered as included in the contract price paid per tonne for asphalt concrete of the types designated in the Engineer's Estimate and no additional compensation will be allowed therefor.

Full compensation for Contractor sampling, testing, inspection, testing facilities, and preparation and submission of data, all as specified in these specifications, shall be considered as included in the contract price paid per tonne for asphalt concrete of the types designated in the Engineer's Estimate and no additional compensation will be allowed therefor.

Quantities of pavement reinforcing fabric placed and paving asphalt applied as a binder for the pavement reinforcing fabric will be paid for at the contract price per square meter for pavement reinforcing fabric and per tonne for paving asphalt (binder-pavement reinforcing fabric). Full compensation for furnishing and spreading sand to cover exposed binder material, if necessary, shall be considered as included in the contract price paid per tonne for paving asphalt (binder-pavement reinforcing fabric) and no separate payment will be made therefor.

Small quantities of asphalt concrete placed on pavement reinforcing fabric to prevent the fabric from being displaced by construction equipment or to allow traffic to cross over the fabric, shall be considered as part of the layer of asphalt concrete to be placed over the fabric and will be measured and paid for by the tonne as asphalt concrete.

When there is a contract item for liquid asphalt (prime coat), the quantity of prime coat will be paid for at the contract price per tonne for the designated grade of liquid asphalt (prime coat). When there is no contract item for liquid asphalt (prime coat) and the special provisions require the application of prime coat, full compensation for furnishing and applying prime coat shall be considered as included in the contract price paid per tonne for the asphalt concrete, and no separate payment will be made therefor.

When there is a contract item for asphaltic emulsion (paint binder), the quantity of asphaltic emulsion or paving asphalt used as paint binder (tack coat) will be paid for at the contract price per tonne for asphaltic emulsion (paint binder). When there is no contract item for asphaltic emulsion (paint binder), full compensation for furnishing and applying paint binder (tack coat) shall be considered as included in the contract price paid per tonne for the asphalt concrete, and no separate payment will be made therefor.

Fog seal coat will be paid for as provided in Section 37-1, "Seal Coats," of the Standard Specifications.

No adjustment of compensation will be made for any increase or decrease in the quantities of paint binder (tack coat) or fog seal coat required, regardless of the reason for such increase or decrease. The provisions in Section 4-1.03B, "Increased or Decreased Quantities," of the Standard Specifications, shall not apply to the items of paint binder or fog seal coat.

The above contract prices and payments shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in constructing asphalt concrete complete in place, as shown on the plans and as specified in this specification and in Section 10-1, "General," elsewhere in these special provisions, and as directed by the Engineer.

SECTION 12. BUILDING WORK

SECTION 12-1. GENERAL REQUIREMENTS

12-1.01 SCOPE

Building work described herein and as shown on the plans shall conform to the requirements of these special provisions and Sections 1 through 9 of the Standard Specifications. Sections 10 through 95 of the Standard Specifications shall not apply to the work in this Section 12 except when specific reference is made thereto.

The building work to be done consists, in general, of constructing a toll booth lane extension including space frame canopy extension, new toll booth, barrier railing and crash cushion; and pre-engineered concrete enclosure for communication equipment; including mechanical and electrical and such other items or details, not mentioned above, that are required by the plans, Standard Specifications, or these special provisions shall be performed, placed, constructed or installed.

12-1.02 ABBREVIATIONS

Section 1-1.02, "Abbreviations," of the Standard Specifications is amended by adding the following:

AAMA	American Architectural Manufacturers' Association
ACI	American Concrete Institute
AGA	American Gas Association
AITC	American Institute of Timber Construction
AMCA	Air Movement and Control Association
APA	American Plywood Association
ARI	American Refrigeration Institute
ASHRAE	American Society of Heating, Refrigeration and Air Conditioning Engineers
CS	Commercial Standards (US Department of Commerce)
ESO	Electrical Safety Orders
FGMA	Flat Glass Marketing Association
FM	Factory Mutual
FS	Federal Specification
ICBO	International Conference of Building Officials
NAAMM	National Association of Architectural Metal Manufacturers
NBFU	National Board Fire Underwriters
NEC	National Electrical Code
NFPA	National Fire Protection Association
PEI	Porcelain Enamel Institute
PS	Product Standard (US Department of Commerce)
RIS	Redwood Inspection Service
SCPI	Structural Clay Products Institute
SMACNA	Sheet Metal and Air Conditioning Contractors' National Association
SSPC	Steel Structures Paint Council
TCA	Tile Council of America
TPI	Truss Plate Institute
UBC	Uniform Building Code
UL	Underwriters Laboratories, Inc.
WCLIB	West Coast Lumber Inspection Bureau (stamped WCLB)
WCLB	Grade stamp for WCLIB
WIC	Woodwork Institute of California
WWPA	Western Wood Products' Association

When reference is made to the Uniform Building Code (UBC) on the plans or in the special provisions, it shall be the 1994 Uniform Building Code as amended by the 1995 Title 24 California Building Standards Code.

12-1.03 GUARANTEE

The Contractor hereby unconditionally guarantees that the building work will be done in accordance with the requirements of the contract, and further guarantees the building work of the contract to be and remain free of defects in workmanship and materials for a period of one year from the date of acceptance of the contract, unless a longer guarantee period is required elsewhere in these special provisions. The Contractor hereby agrees to repair or replace any and all building work, together with any other adjacent work which may be displaced in so doing, that may prove to be not in accordance with the requirements of the contract or that may be defective in its workmanship or material within the guarantee

period specified, without any expense whatsoever to the Department, ordinary wear and tear and unusual abuse or neglect excepted.

The performance bond for the contract, or a portion thereof, in the sum equal to one-half the contract price of the building work, shall remain in full force and effect during the guarantee period.

The Contractor further agrees, that within 10 calendar days after being notified in writing by the Department of any building work not in accordance with the requirements of the contract or any defects in the building work, he shall commence and prosecute with due diligence all work necessary to fulfill the terms of this guarantee, and shall complete the work within a reasonable period of time, and, in the event he fails to comply, he does hereby authorize the Department to proceed to have such work done at the Contractor's expense and he shall honor and pay the cost and charges therefor upon demand. The Department shall be entitled to all costs and expenses, including reasonable attorney's fees, necessarily incurred upon the Contractor's refusal to honor and pay the above costs and charges.

12-1.04 SUBMITTALS

Working drawings, material lists, descriptive data, samples and other submittals specified in these special provisions shall be submitted for approval in accordance with the provisions in Section 5-1.02, "Plans and Working Drawings," of the Standard Specifications and these special provisions.

Unless otherwise permitted in writing by the Engineer, all submittals required by these special provisions shall be submitted within 35 days after the contract has been approved.

Attention is directed to the provisions in Section 5-1.01, "Authority of Engineer," of the Standard Specifications. The Engineer may request submittals for materials or products where submittals have not been specified in these special provisions, or may request that additional information be included in specified submittals, as necessary to determine the quality or acceptability of such materials or products.

Attention is directed to Section 6-1.05, "Trade Names and Alternatives," of the Standard Specifications. The second indented paragraph of the first paragraph of said Section 6-1.05 is amended to read:

> Whenever the specifications permit the substitution of a similar or equivalent material or article, no test or action relating to the approval of such substituted material will be made until the request for substitution is made in writing by the Contractor accompanied by complete data as to the equality of the material or article proposed. Such request shall be made within 35 days after the date the contract has been approved and in ample time to permit approval without delaying the work, but need not be made in less than 35 days after award of the contract.

Work requiring the submittal of working drawings, material lists, descriptive data, samples, or other submittals shall not begin prior to approval of said submittal by the Engineer. Fifteen working days shall be allowed for approval or return for correction of each submittal or resubmittal. Should the Engineer fail to complete his review within the time specified and if, in the opinion of the Engineer, the Contractor's controlling operation is delayed or interfered with by reason of the delay in review, an extension of time commensurate with the delay in completion of the work thus caused will be granted as provided in Section 8-1.07, "Liquidated Damages," of the Standard Specifications.

Submittals shall be delivered to the locations indicated in these special provisions. If a specific location is not indicated, the submittal shall be delivered to the Office of Structure Design, Documents Unit, Fourth Floor, Mail Station 9, 1801 30th Street, Sacramento, California 95816, telephone (916) 227-8252, or the submittals shall be mailed to the Office of Structure Design, Documents Unit, Mail Station 9, P. O. Box 942874, Sacramento, California 94274-0001.

Each submission of drawings, material lists and descriptive data shall consist of at least 5 copies. Two copies will be returned to the Contractor either approved for use or returned for correction and resubmittal.

Each separate item submitted shall bear a descriptive title, the name of the project, district, county, and contract number. Plans and detailed drawings shall be not larger than 559 mm x 914 mm.

The material list shall be complete as to name of manufacturer, catalog number, size, capacity, finish, all pertinent ratings, and identification symbols used on the plans and in the special provisions for each unit.

Parts lists and service instructions packaged with or accompanying the equipment installed in the work shall be delivered to the Engineer at the jobsite. Required operating and maintenance instructions shall be submitted in triplicate.

Manufacturer's warranties for products installed in the work shall be delivered to the Engineer at the jobsite.

Unapproved samples and samples not incorporated in the work shall be removed from State property, when directed by the Engineer.

12-1.05 PROGRESS SCHEDULE

A progress schedule shall be submitted in duplicate for the building work in accordance with the requirements in Section 8-1.04, "Progress Schedule," of the Standard Specifications.

12-1.06 SCHEDULE OF VALUES

The Contractor shall prepare and submit to the Engineer 2 copies of a Schedule of Values covering each lump sum item for building work. The Schedule of Values, showing the value of each kind of work, shall be acceptable to the Engineer before any partial payment estimate is prepared.

The sum of the items listed in the Schedule of Values shall equal the contract lump sum price for building work. Overhead and profit shall not be listed. Bond premium, temporary construction facilities, plant, and other such items will not be paid for under the various building work items and shall be included in the mobilization bid item for the entire project.

12-1.07 INSPECTION

All items covered or all stages of work that are not to remain observable must be inspected and approved before progress of work conceals portions to be inspected. The Contractor shall notify the Engineer not less than 72 hours in advance of when such inspection is needed.

12-1.08 OBSTRUCTIONS

Attention is directed to Sections 7-1.11, "Preservation of Property," 7-1.12, "Responsibility for Damage," 7-1.16, "Contractor's Responsibility for the Work and Materials," and 8-1.10, "Utility and Non-Highway Facilities," of the Standard Specifications.

The Contractor shall notify the Engineer and the appropriate regional notification center for operators of subsurface installations at least 5 working days prior to performing any excavation or other work close to any underground pipeline, conduit, duct, wire or other structure. Regional notification centers include but are not limited to the following:

Underground Service Alert
Northern California (USA)
Telephone: 1(800)642-2444

Underground Service Alert
Southern California (USA)
Telephone: 1(800)422-4133

South Shore Utility
Coordinating Council (DIGS)
Telephone: 1(800)541-3447

Western Utilities
Underground Alert, Inc.
Telephone: 1(800)424-3447

12-1.09 TEMPORARY UTILITIES

The Contractor may obtain electrical power and water from existing State electrical power and water outlets within the contract limits free of charge for contract operations where such utilities exist, provided that such utility services are in service and are not required by the State for other purposes and subject to the provisions in the section "Cooperation" of these special provisions.

The Contractor shall make his own arrangements to obtain any additional electrical power and water or other utilities required for his operations and shall make and maintain the necessary service connections at his own expense.

When existing utility systems are being modified, periods of shutdown will be determined by the Engineer.

The Contractor shall provide adequate temporary lighting to perform the work and allow the Engineer to inspect the project as each portion is completed.

The Contractor shall provide and pay for telephone service he may require. State telephone facilities shall not be used.

12-1.10 SANITARY FACILITIES

State sanitary facilities will not be available for use by the Contractor's employees. Tools shall not be cleaned nor shall cleaning liquids be disposed of in State sanitary facilities or sewers.

12-1.11 MEASUREMENT AND PAYMENT

The contract lump sum price paid for building work shall include full compensation for furnishing all labor, materials, tools, equipment, and incidentals, and for doing all the work involved in constructing the building work, complete in place, as shown on the plans, as specified in the Standard Specifications and these special provisions, and as directed by the Engineer.

Full compensation for any incidental materials and labor, not shown on the plans or specified, which are necessary to complete the buildings and appurtenances shall be considered as included in the contract lump sum price paid for building work and no additional compensation will be allowed therefor.

12-1.12 FIELD ENGINEERING

This section specifies administrative and procedural requirements for field engineering services to be performed by the Contractor.

Lines and grades.--Attention is directed to Section 5-1.07 “Lines and Grades,” of the Standard Specifications.

Such stakes or marks will be set by the Engineer as he determines to be necessary to establish the lines and grades required for the completion of the work shown on the plans and as specified in these special provisions. In general, these will consist of the primary vertical and horizontal control points.

Stakes and marks set by the Engineer shall be carefully preserved by the Contractor. In case such stakes and marks are destroyed or damaged they will be replaced at the Engineer's earliest convenience. The Contractor will be charged for the cost of necessary replacement or restoration of such stakes and marks which in the judgment of the Engineer were carelessly or willfully destroyed or damaged by the Contractor's operations. This charge will be deducted from any moneys due or to become due the Contractor.

All other stakes or marks required to establish the lines and grades required for the completion of the work shall be the responsibility of the Contractor.

Existing utilities and equipment.--The existence and location of underground and other utilities and construction indicated as existing are not guaranteed. Before beginning sitework, the Contractor shall investigate and verify the existence and location of underground utilities and other construction.

Prior to construction, the Contractor shall verify the location and invert elevation at points of connection of sanitary and septic sewers, storm sewer, and water or fire service piping.

Surveys for layout and performance.--The Contractor shall perform all surveys for layout and performance, reduce field notes, and make all necessary calculations and drawings necessary to carry out the work.

The Contractor shall locate and layout site improvements, and other work requiring field engineering services, including pavements, stakes for grading, fill and topsoil placement, utility slopes and invert elevations by instrumentation and similar appropriate means.

Batter boards shall be located and laid out for structures, building foundations, column grids and locations, floor levels and, control lines and levels required for mechanical and electrical work.

Survey accuracy and tolerances.--The tolerances generally applicable in setting survey stakes for foundations, slabs, and underground work shall not exceed the following:

Survey Stakes or Markers	Tolerance
Rough grading or excavation	30 mm
Trimming or preparation of subgrade for roadways	15 mm
Roadway surfacing, steel or concrete pipe	6 mm
Structures or building construction	3 mm

Such tolerance shall not supersede stricter tolerances required by the plans or special provisions, and shall not otherwise relieve the Contractor of responsibility for measurements in compliance therein.

12-1.13 SUBSTITUTION OF NON-METRIC MATERIALS AND PRODUCTS

Only materials and products conforming to the requirements of the specifications shall be incorporated in the work. When metric materials and products are not available, and when approved by the Engineer, and at no cost to the State, materials and products in the inch-pound (imperial) system which are of equal quality and of the required properties and characteristics for the purpose intended, may be substituted for the equivalent metric materials and products, subject to the following requirements:

Materials and products shown on the plans or in the special provisions as being equivalent may be substituted for the metric materials and products specified or detailed on the plans.

Before other non-metric materials and products will be considered for use the Contractor shall furnish, at the Contractor's expense, evidence satisfactory to the Engineer that the materials and products proposed for use are equal to or better than the materials and products specified or detailed on the plans. The burden of proof as to the quality and suitability of substitutions shall be upon the Contractor and the Contractor shall furnish all information necessary as required to the Engineer. The Engineer will be the sole judge as to the quality and suitability of the substituted materials and products and the Engineer's decision shall be final.

When the Contractor elects to substitute non-metric materials and products, including materials and products shown on the plans or in the special provisions as being equivalent, a list of substitutions to be made shall be submitted for approval.

The following substitutions of materials and products will be allowed:

SUBSTITUTION TABLE FOR SIZES OF HIGH STRENGTH STEEL FASTENERS, ASTM Designation: A 325M	
METRIC SIZE SHOWN ON THE PLANS mm x thread pitch	IMPERIAL SIZE TO BE SUBSTITUTED inch
M16 x 2	5/8
M20 x 2.5	3/4
M22 x 2.5	7/8
M24 x 3	1
M27 x 3	1-1/8
M30 x 3.5	1-1/4
M36 x 4	1-1/2

SUBSTITUTION TABLE FOR REINFORCEMENT	
METRIC BAR DESIGNATION NUMBER AS SHOWN ON THE PLANS	IMPERIAL BAR DESIGNATION NUMBER TO BE SUBSTITUTED
10	3
13	4
16	5
20	6
22	7
25	8
29	9
32	10
36	11
43	14
57	18

SUBSTITUTION TABLE FOR WELDED PLAIN WIRE REINFORCEMENT, ASTM DESIGNATION: A 185	
	US CUSTOMARY UNITS SIZE TO BE SUBSTITUTED $inch^2$ x 100
MW9	W1.4
MW10	W1.6
MW13	W2.0
MW15	W2.3
MW19	W2.9
MW20	W3.1
MW22	W3.5
MW25	W3.9, except W3.5 in piles only
MW26	W4.0
MW30	W4.7
MW32	W5.0
MW35	W5.4
MW40	W6.2
MW45	W6.5
MW50	W7.8
MW55	W8.5, except W8.0 in piles only
MW60	W9.3
MW70	W10.9, except W11.0 in piles only
MW80	W12.4
MW90	W14.0
MW100	W15.5

CONVERSION TABLE FOR SIZES OF: (1) STEEL FASTENERS FOR GENERAL APPLICATIONS, ASTM Designation: A 307 or AASHTO Designation: M 314, Grade 36 or 55, and (2) HIGH STRENGTH STEEL FASTENERS, ASTM Designation: A 325 or A 449	
DIAMETER	
METRIC SIZE SHOWN ON THE PLANS mm	EQUIVALENT IMPERIAL SIZE inch
6, or 6.35	1/4
8 or 7.94	5/16
10, or 9.52	3/8
11, or 11.11	7/16
13 or 12.70	1/2
14, or 14.29	9/16
16, or 15.88	5/8
19,or 19.05	3/4
22, or 22.22	7/8
24, 25, or 25.40	1
29, or 28.58	1-1/8
32, or 31.75	1-1/4
35, or 34.93	1-3/8
38 or 38.10	1-1/2
44, or 44.45	1-3/4
51, or 50.80	2
57, or 57.15	2-1/4
64, or 63.50	2-1/2
70 or 69.85	2-3/4
76, or 76.20	3
83, or 82.55	3-1/4
89 or 88.90	3-1/2
95, or 95.25	3-3/4
102, or 101.60	4

CONVERSION TABLE FOR NOMINAL THICKNESS OF SHEET METAL			
UNCOATED HOT AND COLD ROLLED SHEETS		HOT-DIPPED ZINC COATED (GALVANIZED) SHEETS	
METRIC THICK-NESS SHOWN ON THE PLANS mm	EQUIVA-LENT US STAND-ARD GAGE inch	METRIC THICK-NESS SHOWN ON THE PLANS mm	EQUIVA-LENT GALVAN-IZED SHEET GAGE inch
7.94	0.3125		
6.07	0.2391		
5.69	0.2242		
5.31	0.2092		
4.94	0.1943		
4.55	0.1793		
4.18	0.1644	4.270	0.1681
3.80	0.1495	3.891	0.1532
3.42	0.1345	3.510	0.1382
3.04	0.1196	3.132	0.1233
2.66	0.1046	2.753	0.1084
2.28	0.0897	2.372	0.0934
1.90	0.0747	1.994	0.0785
1.71	0.0673	1.803	0.0710
1.52	0.0598	1.613	0.0635
1.37	0.0538	1.461	0.0575
1.21	0.0478	1.311	0.0516
1.06	0.0418	1.158	0.0456
0.91	0.0359	1.006 or 1.016	0.0396
0.84	0.0329	0.930	0.0366
0.76	0.0299	0.853	0.0336
0.68	0.0269	0.777	0.0306
0.61	0.0239	0.701	0.0276
0.53	0.0209	0.627	0.0247
0.45	0.0179	0.551	0.0217
0.42	0.0164	0.513	0.0202
0.38	0.0149	0.475	0.0187

CONVERSION TABLE FOR WIRE		
METRIC THICKNESS SHOWN ON THE PLANS mm	EQUIVALENT USA STEEL WIRE THICKNESS inch	GAGE NO.
6.20	0.244	3
5.72	0.225	4
5.26	0.207	5
4.88	0.192	6
4.50	0.177	7
4.11	0.162	8
3.76	0.148	9
3.43	0.135	10
3.05	0.120	11
2.69	0.106	12
2.34	0.092	13
2.03	0.080	14
1.83	0.072	15
1.57	0.062	16
1.37	0.054	17
1.22	0.048	18
1.04	0.041	19
0.89	0.035	20

CONVERSION TABLE FOR LUMBER	
METRIC NOMINAL SURFACE DRY SIZE mm	EQUIVALENT NOMINAL SURFACE DRY U S SIZE inch
51	2
102	4
152	6
203	8
254	10
305	12

CONVERSION TABLE FOR PLYWOOD	
METRIC mm	ENGLISH inch
6.4	1/4
7.9	5/16
9.5	3/8
11.1	7/16
11.9	15/32
12.7	1/2
15.1	19/32
15.9	5/8
18.3	23/32
19.1	3/4
22.2	7/8
25.4	1
28.6	1 1/8

CONVERSION TABLE FOR INSULATION R-VALUE	
METRIC (m^2K/W)	ENGLISH (HR FT^2 F/BTU)
0.5	3
0.7	4
1.4	8
1.9	11
2.3	13
2.5	14
3.3	19
5.3	30

CONVERSION TABLE FOR VAPOR TRANSMISSION RATING	
METRIC (Perm-m)	ENGLISH (perm-inch)
0.29	0.02

CONVERSION TABLE FOR LOW PRESSURE	
METRIC (Pa)	ENGLISH (Inches of Water Column)
30	0.125
60	0.25
120	0.50
145	0.60
175	0.70
185	0.75
200	0.80
250	1.00
310	1.25

CONVERSION TABLE FOR PRESSURE	
METRIC (kPa)	ENGLISH (psi)
10	1.5
210	30
280	40
350	50
690	100
860	125
1040	150
1100	160
1210	175
1380	200
1730	250
2070	300
2170	315
2410	350
2590	375
2760	400
4830	700
5170	750
5520	800
13800	2000
17200	2500
20700	3000
27600	4000
34500	5000
137900	20000

CONVERSION TABLE FOR MIL THICKNESS	
METRIC (mm)	ENGLISH (inch/1000)
0.10	4
0.10	5
0.50	20
0.75	30
1.00	40

CONVERSION TABLE FOR HVAC DUCTING.	
METRIC (mm)	ENGLISH (inch)
100	4
125	5
150	6
175	7
200	8
225	9
250	10
300	12
360	14
410	16
460	18
510	20
560	22
610	24
660	26
710	28
760	30

CONVERSION TABLE FOR MECHANICAL PIPING		
METRIC (GSP, PVC, BSP, DUCTILE IRON)	METRIC (mm)	ENGLISH (inch)
NPS 1/2	15	1/2
NPS 3/4	20	3/4
NPS 1	25	1
NPS 1 1/4	32	1 1/4
NPS 1 1/2	40	1 1/2
NPS 2	50	2
NPS 2 1/2	65	2 1/2
NPS 3	75	3
NPS 4	100	4
NPS 6	150	6

CONVERSION TABLE FOR LUBRICATION PIPING TUBING WALL THICKNESS	
METRIC (mm)	ENGLISH (inch)
2.1	0.083
0.9	0.035

CONVERSION TABLE FOR HOSE/TUBING SIZES O. D.	
METRIC (mm)	ENGLISH (inch)
6	1/4
10	3/8
13	1/2
16	5/8
19	3/4
22	7/8
25	1

CONVERSION TABLE FOR DRUM SIZES			
METRIC		ENGLISH	
L	kg	gallons	pounds
205	180	55	400
60	55	16	120
	16	5	35

CONVERSION TABLE FOR POWER	
METRIC (kW)	ENGLISH (HP)
0.037	1/20
0.075	1/10
0.18	1/4
0.25	1/3
0.37	1/2
0.55	3/4
0.75	1
1.1	1 1/2
1.5	2
2.2	3
3.7	5
5.5	7 1/2
7.5	10
11	15
15	20
18.5	25
22	30
30	40
37	50
45	60
55	75
75	100
90	120
110	150

CONVERSION TABLE FOR IMPELLER BALANCE		
SYNCHRONOUS RPM	METRIC (g mm/kg)	ENGLISH (ounce-inch/pound)
720	94	0.059
900	73	0.046
1200	54	0.034
1800	41	0.026
3600	17	0.011

CONVERSION TABLE FOR ELECTRICAL CONDUIT	
METRIC SIZE SHOWN ON THE PLANS mm	EQUIVALENT IMPERIAL SIZE inch
16	1/2
21	3/4
27	1
35	1 1/4
41	1 1/2
53	2
103	4

SECTION 12-2. SITEWORK

12-2.01 REMOVING PORTIONS OF EXISTING FACILITIES

PART 1.- GENERAL

Scope.--This work shall consist of removing portions of the existing facilities, including removal of existing work to gain access to or for new work, in accordance with the details shown on the plans and these special provisions.

PART 2.- PRODUCTS (Not applicable)

PART 3.- EXECUTION

PREPARATION.--

General.--The limits of removal shall be located and identified. Items to be removed and the interface of items to be removed and items to remain intact shall be identified and marked.

Prior to removing concrete, a saw cut approximately 25 mm deep shall be made along the limits of removal on all faces which will be visible in the completed work.

REMOVAL.--

General.--Removal shall be to the limits shown on the plans. Removal shall be done carefully to minimize damage to the portions to remain. Remaining portions that are damaged by the Contractor's operation shall be restored to original condition at the Contractor's expense.

Existing apparatuses, devices, or accessories which would be functionally impaired by new construction or remodeling shall be moved, brought out to new surfaces, or provided with new access covers, as necessary to restore apparatuses, devices, or accessories to their original usefulness.

Piping and conduits to be abandoned shall be capped or plugged.

Surfaces that are exposed to view at the limits of removal work shall be patched, bumps shall be removed and depressions filled, and the surface shall be finished to match the existing surrounding surfaces. Depressions in concrete less than 25 mm deep shall be deepened to 25 mm minimum depth before filling with cement mortar.

Anchor bolts and reinforcement shall be removed at least 25 mm below the surrounding surfaces, and the resulting hole shall be patched with cement mortar.

Existing reinforcement that is to be incorporated into the new work shall be protected from damage and thoroughly cleaned before being embedded in new concrete.

DISPOSAL.--

General.--Materials that are to be removed, shall become the property of the Contractor and shall be disposed of outside the highway right of way in accordance with the requirements in Section 7-1.13, "Disposal of Material Outside of the Highway Right of Way," of the Standard Specifications.

12-2.02 EARTHWORK FOR BUILDING WORK

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of performing earthwork for building work in accordance with the details shown on the plans and these special provisions.

Earthwork for building work shall consist of excavation for footings, foundations, walls, slabs; filling under slabs; backfilling; trenching and backfilling for pipes and conduits; backfilling holes resulting from removal of existing facilities; and any other earthwork, not mentioned, but necessary to complete the building work.

Attention is directed to the requirements of “Field Engineering” in Section 12-1, “General Requirements,” of these special provisions.

QUALITY ASSURANCE.--

Samples.--Samples of sand, pea gravel, or crushed stone, weighing not less than 11 kg, shall be submitted to the Engineer at the jobsite for approval.

SITE CONDITIONS.--

Existing underground piping and conduit.--The location of existing underground piping and conduit is based on the best records available. Before beginning work, the Contractor shall accurately locate the piping and conduit involved in the work. If the location of the existing piping or conduit deviates from the location shown on the plans by more than 1.5 meters, or, if no elevations are indicated and the piping or conduit is more than 0.9 meter below grade, the cost of the additional excavation, backfill, piping or conduit, and removal and replacement of concrete, if any, will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications.

Existing surfaced areas.--Existing surfaced areas that are removed, broken or damaged by the Contractor's operations shall be restored to their original condition except as otherwise shown on the plans or specified herein. Restoration materials shall be equal to or better than the original materials.

PART 2.- PRODUCTS

BACKFILL MATERIALS.--

Structure backfill.--

Structure and trench backfill shall be free of organic and other deleterious material and shall be suitable for the required compaction. Gravel without sand matrix shall not be used except as free draining granular material beneath slabs and footings.

Sand.--

Sand shall be clean, washed sand, free from clay or organic material graded such that 100 percent passes the 6 mm sieve, 90 percent to 100 percent passes the 4.75 mm sieve and not more than 5 percent passes the 75 µm sieve size.

PART 3.- EXECUTION

PREPARATION.--

Sawcutting.--Prior to excavation or trenching, existing surfacing shall be removed to saw cut lines, or to existing wood dividers or expansion joints, if any. The saw cut shall be to a neat line and have a depth not less than 25 mm. Surfacing shall be replaced to match the thickness, grades and finish of the adjacent surrounding surfaces.

EXCAVATION.--

Footing excavation.--The footing depths shown on the plans shall be changed to suit field conditions when directed by the Engineer. Solid rock at or near required depths shall not be disturbed. Unsuitable material shall be excavated down to firm bearing as directed by the Engineer. Work and materials required because of excavation in excess of the depths shown on the plans, when such excavation has been ordered by the Engineer, will be paid for as extra work as provided in Section 4-1.03D of the Standard Specifications.

Limits of the excavation shall allow for adequate working space for installing materials and as required for safety of personnel. Such working space excavation shall be replaced in kind and compacted at the Contractor's expense.

Overdepth excavation for footings shall be backfilled with concrete or such other material recommended by the Contractor and approved by the Engineer. Relative compaction shall be not less than 95 percent.

Excavation for pipes and conduits.--Pipes or conduits in the same trench shall have a minimum clear distance between pipes or conduits of 150 mm. Pipes or conduits shall have not less than 0.75 meter of cover from top of pipes or conduits to finished grade unless otherwise shown on the plans or specified.

Trenching shall be of sufficient depth to permit placing a minimum depth of 100 mm of compacted sand under all pipes and conduits.

Dewatering.--Excavations shall be kept clear of standing water. Water shall be removed by pumping if necessary. Water removed from excavation shall be carried away from the building site and disposed of in a manner that will not harm State or adjacent property.

BACKFILLING.--

General.--Backfill shall be placed and compacted in horizontal layers, not more than 150 mm thick prior to compaction, and to the lines and grades shown on the plans or to original ground.

Structure backfill.--After structures are in place and forms are removed, wood and other debris shall be removed from excavations before placing backfill.

Backfilling pipes and conduits.--Backfill placed under pipe and conduits shall be compacted sand, 100 mm minimum depth. Backfill material placed to a level 150 mm above tops of pipes and conduits shall be sand or fine earth and particles shall not exceed 13 mm in greatest dimension. For wrapped, coated, or plastic pipe or conduits, sand shall be used for backfill. Backfill material placed higher than 150 mm above tops of pipes or conduits shall consist of material free of stones or lumps exceeding 100 mm in greatest dimension except:

(a) The top 300 mm of backfill under roads, walks or paving shall consist of aggregate base material.

Unless otherwise shown on the plans, pipe under roads, with less than 0.75 m of cover over the top of pipe, shall be backfilled with concrete to a level 100 mm above the top of pipe. Concrete for backfill shall be commercial quality concrete containing not less than 350 kg/m^3 of cement.

COMPACTION.--

General.--Relative compaction shall be determined in accordance with California Test 216 or 231.

Unless otherwise noted below, all backfill shall be compacted to a relative compaction of 90 percent.

Compact original ground.--Original ground within 400 mm of the top of concrete slabs and asphalt concrete surfacing shall be compacted to a relative compaction of not less than 95 percent.

Structure backfill.--Structure backfill shall be compacted to a relative compaction of not less than 95 percent.

Trench backfill.--Trench backfill placed beneath slabs or paved areas shall be compacted to a relative compaction of not less than 95 percent.

DISPOSAL.--

Surplus material.--Surplus material from the excavation shall be removed and disposed of outside the right-of-way in accordance with Section 7-1.13 of the Standard Specifications.

FIELD QUALITY CONTROL.--

Inspection.--When the excavation is substantially completed to grade, the Contractor shall notify the Engineer. No concrete shall be placed until the foundation has been approved by the Engineer.

Testing.--The State will conduct compaction tests during the backfilling and compacting operations.

12-2.03 PILING (TOLL BOOTH)

Piling (Toll Booth) shall conform to the requirements specified under Section 10, "Piling (Toll Booth)", of these special provisions.

12-2.04 AGGREGATE BASE

PART 1.-GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing, spreading and compacting aggregate base in accordance with the details shown on the plans and these special provisions.

PART 2.-PRODUCTS

Aggregate base.--

Aggregate base shall be commercial quality aggregates consisting of broken stone; crushed gravel; natural, clean, rough-surfaced gravel and sand; or a combination thereof.

Aggregate base shall conform to the following grading as determined by California Test 202:

Sieve or Screen Size	Percentage Passing
25 mm	100
19 mm	90 - 100
4.75 mm	35 - 60
6 µm	10 - 30
75 µm	2 - 9

Aggregate base shall also conform to the following quality requirements:

Tests	California Test No.	Test Requirements
Durability Index	229	35 Min.
Resistance (R-Value)	301	78 Min.
Sand Equivalent	217	22 Min.

PART 3.-EXECUTION

SPREADING AND COMPACTING.--

Spreading.--Aggregate base shall be placed and compacted to the lines and grades shown on the plans.

Spreading and compacting shall be performed by methods that will produce a uniform base, free from pockets of coarse or fine material.

Compaction.--Relative compaction of each layer of compacted base material shall be not less than 95 percent, as determined by California Test 216 or 231.

12-2.05 FREE DRAINING GRANULAR MATERIAL

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and placing free draining granular material beneath slabs in accordance with the details shown on the plans and these special provisions.

PART 2.- PRODUCTS

Free draining granular material.--

Free draining granular material shall be clean, hard, durable, free-draining rock. The material gradation shall be such that all passes the 25 mm screen, and not more than 5 percent passes the 4.75 mm sieve as determined by California Test 202. Granular material shall be free from organic material, clay balls or other deleterious substances.

PART 3.- EXECUTION.--

SPREADING AND CONSOLIDATING.--

General.--Free draining granular material shall be placed, spread and consolidated by tamping or vibrating.

12-2.06 CORE CONCRETE

GENERAL.--This work shall consist of coring holes through existing concrete surfaces in accordance with the details shown on the plans and these special provisions.

EXECUTION.--

Holes shall be cored by methods that will not shatter or damage the concrete adjacent to the holes.

The diameter of the cored holes shall be as shown on the plans.

Water for the core drilling operations shall be from the domestic water supply.and shall not contain more than 1,000 parts per million of chlorides as Cl, nor more than 1,300 parts per million of sulfates as SO_4, nor shall it contain any impurities in a sufficient amount to cause discoloration or etching of the surface.

Water from the core drilling operations shall not be permitted to flow into sewers or other drainage facilities.

12-2.07 VEHICLE CRASH CUSHION

GENERAL.--This work shall consist of furnishing and installing vehicle crash cushions in accordance with the details shown on the plans and these special provisions.

Vehicle crash cushions installed at the toll booth islands shall be Quadquard LMC System, as made by Energy Absorption Systems, Inc. or equal, and shall include all the items listed for "Vehicle Crash Cushion" shown on the plans.

Manufacturer	Distributer
Energy Absorption Systems, Inc. One East Wacker Drive Chicago, IL 60601-2076 Telephone (312) 467-6750	Energy Absorption Systems, Inc. P.O. Box 3333-#291 Encinitas, CA 92024 Telephone (619) 438-7887 FAX (619) 438-7848 Energy Absorption Systems, Inc. Customer Service Department One East Wacker Drive Chicago, IL 60601-2076 Telephone (800) 255-3240 FAX (312) 467-0201

The price quoted by the manufacturer for the above Model No.Quadguard LMC, FOB Pell City, Alabama is $27,446, not including sales tax.

The above price will be firm for all orders placed on or before August 31, 2000, provided delivery is accepted within 90 days after the order is placed.

Pipe sleeves in the backup wall and anchor bolts, nuts and washers for the front anchorage plate, shall conform to the provisions in Section 75-1.03, “Miscellaneous Bridge Metal,” of the Standard Specifications, except that high-strength bolts shall be galvanized and conform to ASTM Designation: A 325.

The Contractor shall provide the Engineer one copy of the manufacturer’s plans and parts lists for each model installed.

The Contractor shall provide the Engineer with the manufacturer's Certificate of Compliance in accordance with the provisions of Section 6-1.07, "Certificate of Compliance," of the Standard Specifications. Said certificate shall certify that the crash cushions comply with the contract plans and specifications, conform to the prequalified design and material requirements, and were manufactured in accordance with the approved quality control program.

SECTION 12-3. CONCRETE AND REINFORCEMENT

12-3.01 CAST-IN-PLACE CONCRETE

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of constructing cast-in-place concrete facilities in accordance with the details shown on the plans and these special provisions.

SUBMITTALS.--

Product data.--Manufacturer's descriptive data for admixtures, expansion joint material, vapor barrier, hardener, and sealer shall be submitted for approval.
Descriptive data shall be delivered to the Engineer at the jobsite.

QUALITY ASSURANCE.--

Certificates of Compliance.--Certificates of Compliance shall be furnished for cement, reinforcement, admixtures, freeze-thaw aggregates and epoxy products in accordance with the requirements specified in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications.

PART 2.- PRODUCTS

CONCRETE MIXES.--

Concrete (structural work).--
Commercial quality concrete shall be proportioned to provide a workable mix suitable for the intended use; shall have not less than 350 kg/m^3 of cement; 0 to 50 mm penetration, inclusive, as determined by California Test 533.

Concrete (minor work).--
Commercial quality concrete for concrete stoops shall be proportioned to provide a workable mix suitable for the intended use; shall have not less than 300 kg/m^3 of cement; 0 to 50 mm penetration, inclusive, as determined by California Test 533.

CONCRETE MATERIALS.--

Cement.--
Cement shall conform to ASTM Designation: C 150, Types II, or III portland cement; or Type IP (MS) Modified cement. Type IP (MS) Modified shall conform to ASTM Designation: C 595 and shall be comprised of an intimate mixture of Type II Modified cement and not more than 20 percent of a pozzolanic material.

Aggregates.--
Aggregates shall be free from deleterious coatings, clay balls and other extraneous materials.

Admixtures.--
Admixtures used in portland cement concrete shall be included on the Department's current list of approved admixtures, and shall conform to ASTM Designation: C 494, Types A, B, D, F or G for chemical admixtures; ASTM Designation: C 260 for air-entraining admixtures; and ASTM Designation: C 618 for mineral admixtures, except loss on ignition shall not exceed 4 percent. Properties of admixtures shall be uniform in each lot.

FORM MATERIALS.--

Forms for exposed finish concrete.--

Forms for exposed surfaces shall be plywood, metal or other panel type materials. Plywood shall be not less than 16 mm thick and without scars, dents, and delaminations. Forms shall be furnished in largest practical pieces to minimize number of joints.

Plywood shall conform to the requirements of U. S. Product Standard PS-1 for Exterior B-B (Concrete Form) Class I.

Forms for edges of slabs shall be nominal 50 mm solid stock lumber, plywood, or metal forms.

Forms for unexposed finish concrete.--

Forms for unexposed finish concrete surfaces shall be plywood, lumber, metal or other acceptable material.

Form ties.--

Form ties shall be factory fabricated, removable or snapoff metal ties for use as necessary to prevent spreading of forms during concrete placement.

Form oil.--

Form oil shall be commercial quality form oil which will permit the ready release of the forms and will not discolor the concrete.

REINFORCING MATERIALS.--

Bar reinforcement.--

Bar reinforcement shall be epoxy coated reinforcement as specified under "Epoxy Coated Reinforcement," elsewhere in these special provisions.

RELATED MATERIALS.--

Anchor bolts, nuts, and washers.--

Nonheaded anchor bolts shall conform to ASTM Designation: A 36M, with a minimum hook length of 6.2 diameters.

Headed anchor bolts shall conform to ASTM Designation: A 307.

Nuts shall conform to ASTM Designation: A 563M, Grade A.

Washers for anchor bolts shall be commercial quality.

Expansion joint material.--

Expansion joint material shall be commercial quality asphalt impregnated pressed fiber sheets, 13 mm minimum thickness.

Vapor barrier.--

Vapor barrier shall be commercial quality polyethylene sheets not less than 0.15 mm thick.

Bond breaker.--

Bond breaker shall be Type I asphalt saturated organic felt or such other material approved by the Engineer.

Mortar.--

Mortar shall consist of one part cement to 2 parts clean sand and only enough water to permit placing and packing.

Curing compound.--

Curing compound shall be a non-pigmented curing compound with fugitive dye conforming to the requirements of ASTM Designation: C 309, Type 1-D, Class A.

ADMIXTURES.--

General.--Admixtures shall be used when specified or ordered by the Engineer and may be used at the Contractor's option to conserve cement or to facilitate any construction operation.

Calcium chloride shall not be used in any concrete containing reinforcement or other embedded items.

Admixtures shall be combined with concrete materials by methods that produce uniform properties throughout the concrete.

If more than one admixture is used, said admixtures shall be compatible with each other so that the desirable effects of all admixtures will be realized.

Mineral admixtures may be used to replace up to 15 percent of Type II portland cement provided the weight of mineral admixture used is not less than the weight of cement replaced. Mineral admixtures shall not be used to replace Type IP (MS) Modified or Type III cements. Chemical admixtures may be used to reduce up to 5 percent of the portland cement except that the cement content shall not be less than 300 kg/m^3. When both chemical and mineral admixtures are used with Type II cement, the weight of cement replaced by mineral admixture may be considered as cement in determining the resulting cement content.

Mineral admixtures will be required in the manufacture of concrete containing aggregates that are determined to be "deleterious" or "potentially deleterious" when tested in accordance with ASTM Designation: C 289. The use of mineral admixture in such concrete shall conform to the requirements in this section except that the use of set retarding admixtures will not be permitted.

When the use of a chemical admixture is specified or is ordered by the Engineer, the admixture shall be used at the rate specified or ordered. If no rate is specified or ordered, or if the Contractor uses a chemical admixture for his own convenience, the admixture shall be used at the dosage normally recommended by the admixture manufacturer.

When air-entrainment is specified or is ordered by the Engineer, the air-entraining admixture shall be used in amounts to produce concrete having the specified or ordered air content as determined by California Test 504. If the Contractor uses air-entrainment for his own convenience, the average air content shall not exceed 4 percent and no single test shall exceed 5 1/2 percent.

Chemical admixtures and air-entraining admixtures shall be dispensed in liquid form. Dispensers shall have sufficient capacity to measure at one time the total quantity required for each batch. If more than one liquid admixture is used in the concrete, a separate measuring unit shall be provided for each liquid admixture and dispensing shall be such that the admixtures are not mixed at high concentrations. When air-entraining admixtures are used with other liquid admixtures, the air-entraining admixtures shall be the first to be incorporated into the mix. Unless liquid admixtures are added to premeasured water for the batch, they shall be discharged to flow into the stream of water so that the admixtures are well dispersed throughout the batch.

MIXING AND TRANSPORTING CONCRETE.--

General.--When a truck mixer or agitator is used for transporting concrete to the delivery point, discharge shall be complete within 1 1/2 hours, or before 250 revolutions of the drum or blades, whichever comes first, after the introduction of cement to the aggregates.

Truck mixers or agitator shall be equipped with electrically or mechanically actuated revolution counters by which the number of revolutions of the drum or blades may readily be verified. The counters shall be of the continuous-registering type, which accurately register the number of revolutions and shall be mounted on the truck so that the Engineer may safely and conveniently inspect them from alongside the truck. Under conditions contributing to quick stiffening of the concrete, or when the temperature of the concrete is 30°C or above, a time less than 1 1/2 hours may be required.

When non-agitating hauling equipment is used for transporting concrete to the delivery point, discharge shall be complete within one hour after the introduction of cement to the aggregates. Under conditions contributing to quick stiffening of the concrete, or when the temperature of the concrete is 30°C, or above, the time between the introduction of cement to the aggregates and discharge shall not exceed 45 minutes.

Each load of concrete for the work shall be accompanied by a trip ticket, a copy of which shall be delivered to the Engineer at the jobsite. The trip ticket shall show volume of concrete, weight of cement and aggregates, quantity of each admixture, quantity of water including water added at the jobsite, time of day the concrete is batched, and revolution counter readings on transit mix trucks at the times the truck is charged and unloaded.

PART 3.- EXECUTION

PREPARATION.--

Existing concrete construction.--Where fresh concrete joins existing or previously placed concrete or masonry, the contact surfaces of the existing or previously placed material shall be roughened, cleaned, flushed with water and allowed to dry to a surface dry condition immediately prior to placing the fresh concrete. The roughened surface shall be no smoother than a wood trowelled surface. Cleaning of the contact surfaces shall remove laitance, curing compounds, debris, dirt and such other substances or materials which would prevent bonding of the fresh concrete.

Abrasive blast methods shall be used to clean horizontal construction joints to the extent that clean aggregate is exposed.

Exposed reinforcing steel located at the contact surfaces which is to be encased in the fresh concrete shall be cleaned to remove any substance or material that would prevent bonding of the fresh concrete.

Forms.--Forms shall be mortar tight, true to the dimensions, lines, and grades shown on the plans, securely fastened and supported, and of adequate rigidity to prevent distortion during placing of concrete.

Forms for exposed surfaces shall be constructed with triangular fillets not less than 19 mm x 19 mm attached so as to prevent mortar runs and to produce smooth straight chamfers at all sharp edges of the concrete.

Form fasteners shall be removable without chipping, spalling, heating or otherwise damaging the concrete surface. Form ties shall be removed to a depth of at least 25 mm below the surface of the concrete.

The inside surfaces of forms shall be cleaned of all dirt, mortar and foreign material. Forms shall be thoroughly coated with form oil prior to use.

Soffit forms and supports shall not be released or removed until at least 10 days after placing concrete. Other forms shall not be stripped until at least 40 hours after placing concrete.

Anchorages and embedded items shall be placed and rigidly secured at their planned locations prior to placing concrete.

Reglets or embedded flashing shall be installed on concrete forms before the concrete is placed.

Redwood dividers shall have 4 mm x 89 mm galvanized nails partially driven into both vertical faces at 450 mm on centers.

Vapor barrier.--Vapor barrier shall be lapped 150 mm and securely taped at splices. Vapor barrier shall be protected with a 75 mm layer of clean uncompacted sand cover.

Unless otherwise shown on the plans, vapor barrier shall be placed under portions of the floor slab scheduled to receive finish flooring.

PLACING CONCRETE.--

General.--Concrete shall be placed and consolidated by means of internal vibrators to form dense, homogeneous concrete free of voids and rock pockets.

Forms and subgrade shall be thoroughly moistened with water immediately before placing concrete.

Concrete shall be placed as nearly as possible to its final location and the use of vibrators for extensive shifting of the concrete will not be permitted.

Concrete shall be deposited and consolidated in a continuous operation within limits of construction joints, until the placing of the panel or section is completed.

When concrete is to be placed in large areas requiring more than two pours, concrete shall be placed in alternate long strips between construction joints and the final slab infilled.

Vibrators used to consolidate concrete containing epoxy-coated bar reinforcement shall have a resilient covering to prevent damage to such reinforcement.

FINISHING CONCRETE SURFACES.--

Finishing unformed surfaces.--Slabs shall be placed full thickness to finish elevation and leveled to screeds by use of long straightedges. The screeds shall be set to grade at approximately 1.8 meter centers. After leveling, screeds shall be removed and the surface shall be floated with wooden floats.

Type A control joint strips shall be inserted into the floated concrete so that the bottom of the top flange is flush with the finish elevation. Strips shall be standard manufactured lengths and shall be placed on an approximate straight line. The top flange of the strips shall be removed after the concrete has set and cured.

The floated surface shall be trowelled with steel trowels. Troweling shall form a dense, smooth and true finish. Walkways, pedestrian ramps, stairs and outdoor slabs for pedestrian traffic shall be given a non-slip broom finish unless a different finish is called for on the plans or in these special provisions.

The application of cement dust coat will not be permitted.

Steel trowel finish and broom finish will not be required for slabs to receive exposed aggregate finish nor for slabs to be covered with ceramic tile.

Concrete floor surfaces to receive ceramic tile shall be floated to grade and then, before final set of the concrete, the floated surfaces shall be roughened with stiff bristled brushes or rakes.

Finished surfaces of floor slabs shall not deviate more than 3 mm from the lower edge of a 3-meter long straight edge.

Finishing formed surfaces.--Formed concrete surfaces shall be finished by filling holes or depressions in the surface, repairing all rock pockets, and removing fins. All surfaces of formed concrete exposed to view shall have stains and discolorations removed, unsightly bulges removed, and all areas which do not exhibit the required smooth, even surface of uniform texture and appearance shall be sanded with power sanders or other approved abrasive means until smooth, even surfaces of uniform texture and appearance are obtained.

Cement mortar, patching and finishing materials used to finish exposed surfaces of concrete shall closely match the color of surrounding surfaces.

CURING CONCRETE.--

General.--Freshly placed concrete shall be protected from premature drying and excessive cold or hot temperatures.

Initial curing of floor slabs shall start as soon as free water has disappeared from the concrete surface. The concrete shall be kept continuously moist for not less than 7 days after the concrete has been placed.

Cotton mats, rugs, carpets, or sand blankets may be used as a curing medium to retain the moisture during the curing period. Curing materials that will stain or discolor concrete shall not be used on surfaces exposed to view.

Prior to placing the curing medium, the entire surface of the concrete shall be kept damp by applying water with a nozzle that so atomizes the flow that a mist and not a spray is formed, until the surface of the concrete is covered with the curing medium. At the expiration of the curing period, the concrete surfaces shall be cleared of all curing mediums.

Concrete surfaces, other than floor slabs, shall be kept moist for a period of at least 5 days by leaving the forms in place or by covering the exposed surfaces using moist rugs, cotton mats or other curing materials approved by the Engineer.

Concrete curbs, sidewalks, collars, and gutter depressions shall be cured with a curing compound.

PROTECTING CONCRETE.--

General.--Concrete shall not be placed on frozen or frost covered surfaces.

Concrete shall be protected from damage due to rain, freezing or inclement weather, and shall be maintained at a temperature of not less than 4°C for 72 hours. When required by the Engineer, the Contractor shall provide a written outline of his proposed methods of protecting concrete.

Vehicles, equipment, or concentrated loads weighing more than 140 kg individually and material stockpiles weighing more than 240 kg/m^2 will not be permitted on the concrete within 10 calendar days after placing.

12-3.02 EPOXY-COATED REINFORCEMENT

GENERAL.--

Epoxy-coated reinforcement shall conform to the provisions in Section 52, "Reinforcement," of the Standard Specifications and these special provisions, except that Sections 52-1.10 "Measurement" and 52-1.11_"Payment" shall not apply.

Section 52-1.02B, “Epoxy-coated Bar Reinforcement,” of the Standard Specifications is replaced with the following:

52-1.02B Epoxy-coated Prefabricated Reinforcement.—Bar reinforcement to be epoxy-coated shall conform to the ASTM Designation and grade required or permitted by Section 52-1.02A, “Bar Reinforcement,” for the location or type of structure involved. The coated bar reinforcement shall conform to the provisions of ASTM Designation: A 934/A 934M except as provided herein.

Wire reinforcement to be epoxy-coated shall conform to the ASTM Designation and grade required or permitted by Section 52-1.02D, “Reinforcing Wire and Plain Bars,” for the location or type of structure involved. The coated wire reinforcement shall conform to the provisions for Class A, Type 2 coating of ASTM Designation: A 884/A 884M except as provided herein.

Appendices X1 and X2, “Guidelines For Job-Site Practices,” of ASTM Designation: A 884/A 884M and A 934/A 934M, respectively, shall apply except as provided herein. The term “shall” shall replace the term “should” in these appendices. Section X1.2 of Appendix X1 and Section X2.2 of Appendix X2 shall not apply.

All coatings shall be purple or gray in color.

Except for field welding of butt splices, all welding of reinforcement shall be complete prior to epoxy coating the reinforcement.

Bending of epoxy-coated reinforcement after the coating has been applied will not be allowed.

When any portion of a reinforcing bar or wire requires epoxy coating, the entire bar or wire shall be coated; except, when the bar or wire is spliced outside of the limits of epoxy coating shown on the plans, epoxy coating will not be required on the portion of bar or wire beyond the splice.

Within areas where epoxy-coated reinforcement is required, tie wire and bar chairs or other metallic devices used to secure or support the reinforcement shall be plastic-coated or epoxy-coated to prevent corrosion of the devices or damage to the coated reinforcement.

Prior to coating, the Contractor shall furnish to the Transportation Laboratory a representative 110 g sample from each batch of epoxy coating material to be used. Each sample shall be packaged in an airtight container identified with the manufacturer’s name and batch number.

Two 700-mm long samples of coated bar or wire reinforcement from each size and from each load shipped to the jobsite shall be furnished to the Engineer for testing. These samples shall be representative of the material furnished. These samples, as well as any additional random samples taken by the Engineer, may be tested for specification compliance. Additional sampling, and all tests performed by the Engineer, may be performed at any location deemed

appropriate by the Engineer. Failure of any sample to meet the requirements of the specifications will be cause for rejection.

If any bar tested for coating thickness or for adhesion of coating fails to meet the requirements for coated bars in Section 9 of ASTM Designation: A 934/A 934M, 2 retests on random samples taken from bars represented by the failed test will be conducted for each failed test. If the results of both retests meet the specified requirements, the coated bars represented by the samples may be certified as meeting the test requirements.

If any wire reinforcement tested for coating thickness or for flexibility fails to meet the requirements for coated wire in Section 8 of ASTM Designation: A 884/A 884M, 2 retests on random samples taken from wire represented by the failed test will be conducted for each failed test. If the results of both retests meet the specified requirements, the coated wire represented by the samples may be certified as meeting the test requirements.

Epoxy-coated reinforcement shall be covered with an opaque polyethylene sheeting or other suitable protective material to protect the reinforcement from exposure to sunlight, salt spray and weather. For stacked bundles, the protective covering shall be draped around the perimeter of the stack. The covering shall be adequately secured; however, it should allow for air circulation around the reinforcement to prevent condensation under the covering. Epoxy-coated reinforcement shall not be stored within 300 meters of ocean or tidal water for more than 2 months.

All visible damage to coatings caused by shipping, handling or installation shall be repaired as required for repairing coating damaged prior to shipment as specified in ASTM Designation: A 934/A 934M for bar reinforcement or ASTM Designation: A 884/A 884M for wire reinforcement. When the extent of coating damage prior to repair exceeds 2 percent of the bar or wire surface area in any 300-mm length, repair of the bar or wire will not be allowed and the coated bar or wire will be rejected.

The patching material and process shall be suitable for field application. The patching material shall be prequalified as required for the coating material and shall be either identified on the container as a material compatible with the reinforcement coating, or shall be accompanied by a Certificate of Compliance certifying that the material is compatible with the reinforcement coating. Damaged areas shall be patched in accordance with the patching material manufacturer's recommendations.

Except for lap splices, all splices for epoxy-coated reinforcement shall be coated with a corrosion protection covering that is on the Department's list of approved products. The covering shall be installed in accordance with the manufacturer's recommendations and as directed by the Engineer. The list is available from the Office of Materials Engineering and Testing Services, 5900 Folsom Boulevard, Sacramento, CA 95819, telephone (916) 227-7000.

The third paragraph of Section 52-1.04, "Inspection," of the Standard Specifications is amended to read:

A Certificate of Compliance conforming to the provisions in Section 6-1.07, "Certificates of Compliance," shall be furnished for each shipment of epoxy-coated bar or wire reinforcement certifying that the coated bars or wire conform to the requirements of ASTM Designation: A 934/A 934M for bars or A 884/A 884M for wire and Section 52-1.02B, "Epoxy-coated Bar Reinforcement." Said Certificate of Compliance shall include all the certifications specified in ASTM Designation: A 934/A 934M for bars or ASTM Designation: A 884/A 884M for wire and a statement that the coating material has been prequalified by acceptance testing performed by the Valley Forge Laboratories, Inc., Devon, Pennsylvania.

12-3.03 NEW WALL OPENINGS

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of constructing new wall openings in existing concrete walls in accordance with the details shown on the plans and these special provisions.

PART 2.- PRODUCTS

Epoxy resin adhesive.--

Epoxy resin adhesive shall conform to State of California Specification No. 8040-21M-08.

Concrete.--

Concrete shall be as specified for minor work under "Cast-In-Place Concrete" in Section 12-3, "Concrete and Reinforcement," of these special provisions.

PART 3.- EXECUTION

CONSTRUCTION.--Metal frame shall be installed in place before placing concrete in collars around new openings in existing walls. The interior of the frames shall be filled with concrete concurrently with filling the collars.

Sills at wall openings shall be restored to the plane of adjoining floor surfaces with concrete placed over an epoxy resin adhesive bond coat. The concrete shall be deposited on the bond coat before the bond coat begins to set. Concrete at sills shall conform to the requirements for concrete in collars except that the penetration shall not exceed 13 mm, as determined by California Test 533, and a water-reducing admixture shall be added at the dosage recommended by the manufacturer of the admixture.

12-3.04 DRILL AND BOND DOWELS

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of drilling holes in existing concrete and installing and bonding bar reinforcing steel dowels into such drilled holes in existing concrete in accordance with the details shown on the plans and these special provisions.

PART 2.- PRODUCTS

Bonding material.--
The bonding material shall be magnesium phosphate concrete, either single component (water activated) or dual component (with a prepackaged liquid activator).

Dowels.--
Dowels shall be bar reinforcing steel, as specified under ""Epoxy Coated Reinforcement" in Section 12-3, "Epoxy Coated Reinforcement," of these special provisions.

PART 3.- EXECUTION

INSTALLATION.--The holes shall be drilled by methods that will not shatter or damage the concrete adjacent to the holes. The diameter of drilled holes shall be 13 mm larger than the nominal diameter of the dowels unless otherwise shown on the plans.

Immediately prior to placing the dowels, the holes shall be cleaned of dust and other deleterious materials, and the holes shall be dry.

Sufficient bonding material shall be placed in the hole so that no voids remain after the dowels are inserted.

Dowels which fail to bond or are damaged before new concrete is placed shall be removed and replaced.

Magnesium phosphate concrete shall be formulated for minimum initial set time of 15 minutes and minimum final set time of 25 minutes at 21°C. The materials, prior to use, shall be stored in a cool, dry environment.

Mix water used with water activated material shall be free from oil and impurities and contain not more than 2000 parts per million as Cl nor more than 1500 parts per million of sulfate as SO_4.

The quantity of water for single component type or liquid activator for dual component type to be blended with the dry component, shall be within the limits recommended by the manufacturer and shall be the least amount required to produce a pourable batter.

Magnesium phosphate concrete shall not be mixed in containers or worked with tools containing zinc, cadmium, aluminum, or copper metals.

The surface of any dowel coated with zinc or cadmium shall be coated with a colored lacquer before installation of the dowel. The lacquer shall be allowed to dry thoroughly before embedment of said dowels.

SECTION 12-4. BLANK

SECTION 12-5. METALS

12-5.01 STRUCTURAL STEEL FOR BUILDINGS

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of fabricating, assembling, furnishing and erecting structural steel in accordance with the details shown on the plans and these special provisions.

Structural steel includes canopy roof and fascia framing, and toll booth framing; consisting of ,steel tubing, steel plates and angles, stainless steel tubing, and stainless steel plates

Source quality control.--Materials and fabrication procedures are subject to inspection and tests in mill, shop and field, conducted by the Engineer or a qualified inspection agency. The Contractor or fabricator shall provide access to the Engineer or testing agency to places where the structural steel work is being fabricated or produced so that the required inspection and testing can be accomplished. Such inspections and tests will not relieve the Contractor of responsibility for providing materials and fabrication procedures in compliance with specified requirements. The testing agency may inspect the structural steel at the plant before shipment; however, the Engineer reserves the right, at any time before final acceptance to reject the material that does not conform to the contract requirements.

REFERENCES.--

General.--Structural steel shall be fabricated, assembled and erected in accordance with American Institute of Steel Construction (AISC), "Specifications for the Design, Fabrication and Erection of Structural Steel for Buildings."

Welding shall be in accordance with American Welding Society (AWS) D1.1, "Structural Welding Code - Steel."

Dimensional details and workmanship for welded joints in tubular and pipe connections shall conform to Part C, Structural Details; Part D, Special Provision for Welding Tubular Joints; and Part E, Workmanship, in Section 10 of AWS D1.1.

SUBMITTALS.--

Product data.--Product data for items to be incorporated into the work, including structural steel, high strength bolts, nuts and washers and alternative connectors, shall be submitted for approval.

Working drawings.--Working drawings and calculations shall be submitted for approval.

Working drawings shall show any changes proposed in the work, details of connections and joints exposed to the weather, details for connections not dimensioned on the plans, the sequence of shop and field assembly and erection, welding sequences and procedures. If required, the location of butt welded splices on a layout drawing of the entire structure, and the location and details of any temporary supports that are to be used.

Calculations and working drawings for falsework to be used for the erection of structural steel shall be submitted for approval. The falsework shall be designed and constructed to provide the necessary rigidity and to support loads which will be applied. Working drawings and design calculations shall be stamped and signed by an engineer who is registered as a Civil or Structural Engineer in the State of California. The expiration date of the registration shall be shown.

QUALITY ASSURANCE.--

Qualifications for welding.--A certified copy of qualification test record for welders shall be submitted to the Engineer at the jobsite. Descriptive data for equipment for field welding structural steel, including type and electric power requirements, shall be submitted for approval.

Certificates of Compliance.--Certificate of Compliance shall be furnished for structural steel products in accordance with the requirements specified in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications. Certificate of Compliance shall include mill test certificates for each heat number used in the work.

DELIVERY, HANDLING AND STORAGE.--

Structural materials shall be loaded, transported, unloaded and stored so that it is kept clean and undamaged. Material shall be stored above ground on platforms, skids, or other supports. Covers and protection shall be provided to protect the materials from corrosion.

Anchorages and anchor bolts, which are to be embedded in concrete or masonry, shall be delivered in ample time to not delay the work.

PART 2.- PRODUCTS

MATERIALS.--

Steel bars, plates and shapes.--

Steel bars, plates and shapes shall conform to ASTM Designation: A 36/A 36M or A 572/A 572M, Grade 50 [345].

Pipe.--

Pipe shall conform to ASTM Designation: A 53, standard weight, unless otherwise shown on the plans.

Steel tubing.--
Steel tubing shall conform to ASTM Designation: A 500, Grade B, or A 501.

Stainless steel angles and plates.--
Stainless steel angles and plates shall be No. 304 stainless steel, No. 4 satin finish.

Stud connectors.--
Stud connectors shall conform to ASTM Designation: A 108, Grades 1018 through 1020, cold drawn, either semi- or fully killed.

Anchor bolts, nuts and washers.--
Nonheaded anchor bolts shall conform to ASTM Designation: A 36/A 36M, with a minimum hook length of 6.2 diameters.

Headed anchor bolts shall conform to ASTM Designation: A 307.

Nuts shall conform to ASTM Designation: A 563M, Grade A.

Washers for anchor bolts shall be commercial quality.

Machine bolts, nuts and washers.--
Machine bolts and nuts shall conform to ASTM Designation: A 307.

Washers for machine bolts shall be commercial quality.

High strength (HS) bolts, nuts and washers.--
High strength (HS) bolts, nuts and washers shall conform to ASTM Designation: A 325M.

Direct tension indicators.--
Direct tension indicators shall conform to ASTM Designation: F 959.

Tension control fasteners.--
Tension control bolts shall have a splined end extending beyond the threaded portion of the bolt and which shears off when the specified bolt tension is attained.

Inorganic zinc primer.--
Inorganic zinc primer shall be a waterborne inorganic zinc primer conforming to the requirements of AASHTO Designation: M 300-92 I, Type II. Inorganic zinc primer shall be listed on the qualified products list which may be obtained from the Transportation Laboratory, (916) 227-7000.

Mortar.--
Mortar shall consist of one part cement, measured by volume, to 2 parts clean sand and only enough water to permit placing and packing.

FABRICATION.--

Shop fabrication and assembly.--Workmanship and finish shall be equal to the best general practice in modern shops.

Cuts shall not deviate more than 2 mm from the intended line. Roughness, notches or gouges shall be removed.

Bearing stiffeners at points of loading shall be square with the web and shall have at least 75 percent of the stiffener in contact with the flanges.

Finished members shall be true to line, shall have square corners and smooth bends and shall be free from twists, kinks, warps, dents and open joints.

Exposed edges and ends of metal shall be dressed smooth, with no sharp edges and with corners slightly rounded.

Stud connectors.--Steel surfaces shall be prepared as recommended by the manufacturer of the stud connectors. Stud connectors shall be welded to the flanges of beams or girders as shown on the plans. Automatic end welding of headed stud connectors shall be in accordance with the manufacturer's instructions.

Connections.--Abutting surfaces at connections shall be clean.

Cutting and welding at the jobsite will not be allowed except as shown on the approved drawings or specifically approved by the Engineer.

Finished holes for bolts shall be cylindrical and perpendicular to the plane of the connection. Subpunched and subdrilled holes shall be 6 mm smaller in diameter than the diameter specified for the finished hole.

Bolted Connections.--Bolts for connecting steel to steel shall be machine bolts conforming to ASTM Designation: A 307 or high-strength bolts conforming to ASTM Designation: A 325M as shown on the plans.

High-strength structural steel bolts, or equivalent fasteners, other bolts attached to structural steel, nuts, and washers shall be galvanized by mechanically deposited coating.

Holes for other work.--Holes for securing other work to structural steel and passage of other work through steel framing members shall be as shown on the approved drawings.

Threaded nuts or specialty items for securing other work to steel members shall be as shown on the approved drawings.

Holes shall be cut, drilled or punched perpendicular to metal surfaces. Holes shall not be flame cut or enlarged by burning. Holes are to be drilled in bearing plates.

SHOP PAINTING.--

General.--Structural steel members shall be finish painted.

Surface preparation.--Surfaces of structural steel to be receive inorganic zinc primer shall be blast cleaned in accordance with Steel Structures Painting Council, SSPC-SP 10, "Near-White Blast Cleaning."

Bolted connections.--Contact surfaces of high strength bolted connections and ungalvanized anchor bolt assemblies shall be blast cleaned and coated with waterborne inorganic zinc primer before assembly. The total thickness of primer on each surface shall be between 0.025 mm to 0.076 mm and may be applied in one application.

Painting.--Immediately after surface preparation, surfaces of structural steel shall receive an undercoat of waterborne inorganic zinc primer. Color shall essentially match Federal Standard 595B, No. 36373.

The manufacturer's published mixing and application instructions for inorganic zinc primer shall be followed.

PART 3.- EXECUTION

ERECTION AND ASSEMBLY.--

Field splices.--Field splices shall be made only at the locations shown on approved working drawings.

The parts shall be accurately assembled in their final position as shown on the plans and in true alignment with related and adjoining work before final fastening.

All parts shall be supported adequately and at locations to provide a vibration free, rigid, and secure installation.

Bolted connections.--All high strength bolted connections shall be made with high strength bolts installed with direct tension indicator washers or tension control fasteners.

When used, one mechanically galvanized direct tension washer shall be installed with each high strength bolt. Bolts shall be tightened until a direct tension indicator washer gap is 0.13 mm or less. A zero gap will not be cause for rejection.

During installation of tension control bolts, the torque required to turn the nut on the tension control bolt shall be counterbalanced by the torsion shear resistance of the splined end of the bolt.

The bolt head type and head location shall be consistent within a joint.

Nuts shall be on side of member least exposed to view.

Setting bases and bearing plates.--Concrete and masonry surfaces shall be cleaned and roughened to improve bond.

Bottom of base and bearing plates shall be clean.

Base plates and bearing plates for structural members shall be set on wedges or other adjusting devices.

Anchor bolts shall be wrench tightened after supported members have been positioned and plumbed.

Mortar shall be solidly packed between bearing surfaces and base or bearing plates to ensure that no voids remain. Exposed surfaces shall be finished and allowed to cure.

FIELD PAINTING.--

Touch-up painting.--After erection, the Contractor shall clean field welds, bolted connections, and abraded areas of shop paint and apply the same materials as applied for shop painting.

Surfaces that are scheduled to receive finish coats shall be painted with an additional prime coat and finish coats in accordance with the requirements specified for shop primed steel under "Painting" in Section 12-9.

QUALITY CONTROL.--

Testing and inspection.--Ultrasonic examination shall be performed by the Contractor on at least 50 percent of all full penetration butt-welded splices in accordance with the requirements of AWS D1.1 and these special provisions.

Welding procedures and methods shall be subject to inspection for conformance with AWS D1.1.

Butt welds shall be tested in accordance with AWS D1.1, Chapter 6, Part C, Ultrasonic Testing of Groove Welds.

Examination, reporting and disposition of tests shall be in accordance with the provisions of 6.12, AWS D1.1.

In addition to ultrasonic examinations by the Contractor, welds may be subject to inspection or non-destructive testing by the Engineer.

When additional inspection or non-destructive testing is required by the Engineer, the Contractor shall provide sufficient access facilities in the shop and at the jobsite to permit the Engineer or his agent to perform such inspection and testing.

The Contractor shall correct all deficiencies in the structural steel work which inspections and laboratory test reports have indicated to be not in compliance with these special provisions. Additional tests shall be performed by the Contractor at his expense to reconfirm any non-compliance of original work, and to show compliance of the corrected work.

12-5.02 SPACE FRAME CANOPY

Part 1 - GENERAL

Scope:

This work shall consist of fabricating, assembling, furnishing and erecting a steel space frame canopy in accordance with the details shown on the plans and these special provisions.

Associated work for construction of the canopy is as indicated below:

1. Material supply of anchor bolts to be placed in concrete supports

2. Spaceframe

3. Associated tubular purlin secondary steel to support, metal fascia system, roof deck roofing system.

DESCRIPTION OF WORK:

A) The extent of the steel spaceframe and associated work as defined above is shown on the drawings.
B) The work includes the following:

1) Engineering design of spaceframe, purlins including submittals of structural calculations.
2) Fabrication, packaging and delivery to job site.
3) Erection by an installer approved by space frame manufacturer. or under the direction of a technical advisor of the space frame manufacturer.

DESIGN REQUIREMENTS:

A) Design steel spaceframes for the spans and overhangs shown on the drawings and to the following loading requirements and codes:

1) Live Load: shown on the plans
2) Superimposed Dead Load:shown on the plans
3) Wind Load: 113 kilometers per hour
4) Temperature variation: -1 to 43 degrees (C)
5) Seismic loading: as per requirements for seismic zone 4 .
6) All loading shall be applied through the spaceframe nodes and not via bending of the tubular members.
7) Other miscellaneous loads shall be provided to spaceframe supplier prior to design.

SPACEFRAME FINISH

1) Galvanizing:

a) All spaceframe tubes shall be hot dip galvanized on both exterior and interior surfaces to a minimum zinc film thickness of 55 microns as per ASTM A123-84. All threaded or tapped parts shall be electro-galvanzied according to ASTM B633-85.
b) Welding of tube ends to be done before galvanizing or by special manufacturing technique which does not damage finish. Application of zinc rich primer over weld as corrosion protection in lieu of the above is not acceptable.

2) Color Coating:

a) Tubes, connectors, purlins and bases shall be finished with a thermoset polyester powder applied by electrostatic spray process, minimum thickness two mils, oven dried and cured. Nodes to be coated with a wet applied polyurethane finish in color to match.

b) Color for all structural spaceframe components shall match the original color of the existing spaceframe structure.

FABRICATION:

A) Provide all structural members, member connectors, fasteners and accessories required for a complete installation of the spaceframe as indicated in approved shop drawings. Design, engineering and fabrication of spaceframe is to be by one source. Outsourcing, subcontracting or joint ventures to achieve a single source are not acceptable.
B) Code each part for easy identification. Cross reference coding to shop and erection drawings and to shipping lists.

QUALITY ASSURANCE:

A) Spaceframe Fabricator's Qualifications:

1) The Steel Spaceframe Fabricator shall have not less than 5 years continuous experience utilizing the same name in manufacturing forged solid spherical node spaceframe structures for projects of equivalent size and complexity,

B) Quality Control of Steel Spaceframe Components:

1) Quality control of nodes, tubes and bolts shall be carried out at the factory, with test loading.
2) In all cases, the loads applied in the tests must attain the following minimum value: 2 x the value of the permissible member load for all parts in tension; 1.7 x the value of the permissible member load for all parts in compression.
3) Submit certificate of compliance with the above factory test procedure.

REFERENCE STANDARDS:

A) American Society of Testing Materials (ASTM).

1) ASTM A123-84 (Table 1) for hot dip galvanizing cylindrical tubes both interior and exterior, 55 microns.

SUBMITTALS:

A) Structural Calculations:

Prior to fabrication of spaceframes, submit design calculations prepared in accordance with current design rules of the AISC or DIN and applicable codes. Include analysis for all pertinent load cases live, dead, wind, thermal, seismic, etc…

1. Supply reactions at supports for review by project engineer and maximum frame deflections.
2. Supply calculations for support and other details as necessary. Members shall be sized by the spaceframe manufacturer.
3. Existing test reports will not be acceptable in lieu of calculations.

B) Shop Drawings:

Submit complete shop drawings including tube layouts and details. Show dimensioned layout of steel spaceframes in relation to adjacent work such as walls, columns, beams and slabs, etc.

1) Include details of all supports and data to show provisions for vertical and horizontal expansion/contraction of spaceframes as necessary.
2) Identify all materials and all attachments devices and accessories.

C) Erection Drawings:

After approval of shop drawings, provide a detailed set of field erection drawings.

Identify each part by size and number. Describe method for erecting spaceframe including sequence of installation.

D) Samples:

1) Fabricator shall submit one sample full size “node” with connecting member having the specified finish.
2) Submit one sample of all cladding elements which fall under this section.
3) Submit product data showing compliance with galvanizing of components and other quality and fabrication requirements.

PRODUCT DELIVERY, STORAGE AND HANDLING:

Factory finished components shall be stored, handled and shipped in a manner that will provide unscratched and undamaged units delivered to the site. Time delivery of material to the site to insure uninterrupted progress of work.

PART 2 - PRODUCTS

Steel space frame

A) Manufacturer: The construction documents are based on the KK round ball and tube system as manufactured by MERO Structures, Inc.
B) System Description: The system shall be formed from steel and consist of cylindrical tubes and solid forged spherical nodes.

Steel Tubes

Tube material shall conform to ASTM A500 Grade B or equivalent minimum.
Assembly to consist of a hollow tube with forged cone welded on each end. Tube ends to remain open. Substitution of sealed tubes not acceptable.

Steel Nodes

To consist of one piece hot forged solid steel spheres conforming to A1S1 C1045 material - normalized. They shall be drilled and tapped as necessary. Extra holes other than those specifically shown in approved drawings not permitted.
Joints using gusset plates, two piece assemblies or friction fit shall not be used.

Bolts

Shall conform to ASTM A307, A325 or A490 or equivalent.

Purlins

To consist of tubular steel elements with proper bearing surface dimension to receive cladding Spacing as required to provide appropriate support for cladding. Finish to match primary structure.

Space frame source.--

The space frame shall be a KK round ball and tube system, manufactured by MERO STRUCTURES, or equal.

Arrangements have been made to insure that any successful bidder can obtain the space frame, from the following source:

Manufacturer

Mero Structures, Inc.
N112 W18810 Mequon Road
Germantown, WI 53022
(800) 323-0072
Terry Peterson

The lump sum price quoted by the distributor for the space frame FOB San Mateo/Haward Br. Toll Plaza, California is $139,620, including sales tax.

The above price will be firm for all orders placed on or before July 30, 2000, provided delivery is accepted within 90 days.

The price includes the following Terms and Conditions

MATERIALS

1. Spaceframe material, described in these specifications.
2. Color selected from MERO'S pallete of available colors to match existing.
3. California Registered Civil Engineer's Stamp of the design.
4. ICBO approved Quality Control Program.
5. Freight to the site at the San Mateo-Hayward Bridge in California.
6. Shop drawings.
7. Tapped holes (20 mm diameter) in nodes to accept purlin system by others.

INSTALLATION

1. Installation
2. One site trip for installation.
3. Crane and lifting equipment as required.
4. MERO crew will have their own safety equipment as required.
5. MERO will use union labor.
6. Standard working hours for assembly, Erection of assembly sections into place will be done at night between 11 pm and 4 am Monday thru Thursday night. MERO will not supply any traffic control..
7. Assembly location will be within 300 meters of the toll plaza.
8. Return trips and cleaning are not included in the above price.

ADMINISTRATION

1. Sales tax at rate of 8.25%.
2. MERO's standard one year warranty.
3. MERO's standard insurance limits.
4. MERO's indemnification against its own actions or the actions of those under MERO's control.

SCHEDULE

Submittal of shop drawings after receipt of order	8-10 weeks
Fabrication from receipt of approved drawings	10-12 weeks
Installation (assuming good weather)	3-4 weeks

EXCLUSIONS

Items not specifically mentioned above are excluded.

PART 3 - EXECUTION:

Erector shall examine the spaceframe support locations and the areas and conditions under which the spaceframe work is to be installed. Notify the Contractor of any known conditions detrimental to the proper and timely completion of the work. Do not proceed with installation until unsatisfactory conditions have been corrected in a manner acceptable to the Erector. (However, ground assembly may proceed).

Erect spaceframe and accessory items in strict accordance with the manufacturer’s approved shop and erection drawings and installation procedures.

Members shall not be positioned by the use of force.

Members shall be installed such that condensation drain holes are pointing downwards.

Provide temporary bracing and support as required to ensure stability during erection process.

Touch up paint work with matching air dry paint as necessary.

Contractor shall provide protection measures for completed spaceframes and accessories to prevent damage or deterioration from subsequent work.

12-5.03 METAL ROOF DECKING

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing metal roof decking in accordance with the details shown on the plans and these special provisions.

Metal roof decking includes ribbed sheet steel decking units, bent plates, sealants, accessories, fasteners and such other components, not mentioned, but required for a rigid, secure and complete installation.

REFERENCES.--

General.--The design, fabrication and erection of metal roof decking shall conform to the applicable requirements of the American Iron and Steel Institute (AISC) publication, "Specifications for the Design of Light Gauge Cold Formed Steel Structural Members," and the applicable Steel Deck Institute Design Manual and these special provisions.

Welding shall be in accordance with American Welding Society (AWS) D1.3, "Structural Welding Code - Sheet Steel."

SUBMITTALS.--

Product data.--Manufacturer's descriptive data for each type of decking and accessories shall be submitted for approval.

Working drawings.--Working drawings showing complete erection layouts, details, dimensions, deck section properties shall be submitted for approval. Drawings shall show types and gages, fastening methods, including the location, type and sequence of connections, sump pans, cut openings, surface finishes and temporary supports or bracing.

The metal deck supplier shall submit a fastening schedule and calculations stamped by an engineer who is registered as a Civil or Structural Engineer in the State of California showing that the metal roof panels, clips, and fasteners conform to the span and design loads shown on the plans and the wind uplift requirements of the Uniform Building Code as amended by Title 24, Part 2, California Code of Regulations.

QUALITY ASSURANCE.--

Qualification of field welding.--Welding processes and welding operators shall be qualified in accordance with "Welder Qualification," procedures in American Welding Society (AWS) D1.1, "Structural Welding Code - Steel."

Welding decking in place is subject to inspection and testing. Defective work shall be removed and replaced with acceptable work.

Certificates of Compliance.--Certificates of Compliance shall be furnished for the metal decking in accordance with the requirements specified in Section 6-1.07, "Certificate of Compliance," of the Standard Specifications.

DELIVERY, HANDLING AND STORAGE.--

General.--Metal roof deck units and accessories shall be transported, stored and erected in a manner that will prevent corrosion, distortion or other damage.

Deck units shall be stored off the ground with one end elevated to provide drainage.

PART 2.- PRODUCTS

MANUFACTURERS.--Acceptable manufacturers shall be H. H. Robertson Co.; Verco Manufacturing Co.; ASC Pacific Co.; or equal.

MATERIALS.--

Deck units.--

Deck units, closures and plates shall be fabricated from galvanized sheet steel conforming to ASTM Designation: A 446M. Minimum yield strength shall be not less than 228 MPa.

Galvanizing shall conform to the requirements of ASTM Designation: A 525M, coating class G60.

Miscellaneous steel shapes.--

Miscellaneous steel shapes shall conform to ASTM Designation: A 36M.

Sealant, anchor clips, vent clips, flashing, saddle plates, flexible closure strips and other accessories.--

Sealant, anchor clips, vent clips, flashing, saddle plates, flexible closure strips and other accessories shall be as recommended by the decking manufacturer.

Paint.--

Metal decking shall be finish painted as specified for galvanized metal surfaces under "Painting" in Section 12-9, Finishes, in these special provisions. FABRICATION.--

General.--Deck units shall be formed to span 3 or more supports, with flush, telescoped or nested 50 mm laps at ends and interlocking or nested side laps unless otherwise shown on the plans.

Deck units shall conform to the configurations, metal thickness, depth and width and section properties shown on the plans.

End bearing shall be not less than 38 mm.

Metal closure strips.--Metal closure strips for opening between deck units and other construction shall be fabricated from the same gage and material as the adjacent deck units. Strips shall be formed to provide tight-fitting closures at end of cells or flutes and sides of decking.

PART 3.- EXECUTION

INSTALLATION.--

General.--Deck units and accessories shall be installed in accordance with the manufacturer's recommendations and approved drawings and these special provisions.

Units shall be placed on supporting steel framework, adjusted in place and properly aligned before being permanently fastened. Ends of units shall have positive bearing over structural supports.

Cutting and fitting shall present a neat and true appearance with exposed burrs removed. Openings through the decking shall be cut square and shall be reinforced as recommended by the decking manufacturer.

The metal roof deck shall not be used as a working platform before deck units are fastened in place. Supplies, equipment or other loads shall not be stored on the deck. Mechanical equipment or other loads shall not be hung from metal roof decking.

Fastening roof deck units.--Roof deck units shall be fastened to supporting steel members by not less than 12 mm effective diameter fusion welds or elongated welds of equal strength, spaced not more than 305 mm at every support.

Welding.--Welding shall conform to AWS requirements and procedures for manual shielded metal arc welding, appearance and quality of welds, and methods used in correcting welding work.

Welding washers shall be used where recommended by the manufacturer.

Fastening side laps.--Side laps of adjacent deck units shall be mechanically fastened between supports at intervals not exceeding 915 mm, using self-tapping machine screws, welds or crimps (button punching) and as required to meet minimum diaphragm requirements.

Field painting:--Immediately following erection, field welds, bolted connections and abraded areas shall be cleaned with a wire brush.

Galvanized surfaces shall be touched-up with galvanizing repair paint recommended by the manufacturer.

Touchup paint primer shall be applied by brush or spray of the same thickness and material as that used for field paint.

12-5.04 BUILDING MISCELLANEOUS METAL

PART 1.- GENERAL

Scope.--This work shall consist of fabricating, furnishing and installing building miscellaneous metal in accordance with the details shown on the plans and these special provisions.

Building miscellaneous metal shall consist of pipe, checker pattern steel plate, expansion anchors, handrailing, headed anchors, including all anchors, fastenings, hardware, accessories and other supplementary parts necessary to complete the work.

REFERENCES.--

Codes and standards.--Welding of steel shall be in accordance with American Welding Society (AWS) D 1.1, "Structural Welding Code-Steel" and D 1.3, “Structural Welding Code-Sheet Steel.”

SUBMITTALS.--

Product data.--Submit manufacturer's specifications, anchor details and installation instructions for products used in miscellaneous metal fabrications.

Working drawings.--Working drawings of fabricated items shall be submitted for approval.

QUALITY ASSURANCE.--

Shop assembly.--Preassemble items in shop to the greatest extent possible to minimize field splicing and assembly. Disassemble units only as necessary for shipping and handling limitations. Clearly mark all units for reassembly and installation.

Inspection and tests.--Materials and fabrication procedures shall be subject to inspection and tests by the Engineer, in mill, shop and field. Such tests will not relieve the Contractor of responsibility of providing materials and fabrication procedures in compliance with specified requirements.

PART 2.- PRODUCTS

MATERIALS.--

Steel bars, plates and hot-rolled shapes.--

Steel bars, plates and hot-rolled shapes shall conform to ASTM Designation: A 36/A 36M.

Checkered floor plates.--

Checkered floor plates shall be 3.4 mm steel with standard raised pattern.

Pipe.--

Pipe shall be commercial quality standard steel pipe.

Stainless steel tube.--

Stainless steel tube shall conform to ASTM Designation: A269, UPS Designation: S30400, Type TP304, No. 4 finish.

Bolts, studs, threaded rods, nuts and washers.--

Bolts, studs, threaded rods, and nuts for general application shall conform to ASTM Designation: A 307.

Washers shall be commercial quality.

Fittings.--

Brackets, bolt, threaded studs, nuts, washers, and other fittings for railings and handrailings shall be commercial quality pipe and fittings.

Expansion anchors.--

Expansion anchors shall be ICBO approved for the purpose intended, integral stud type anchor or internally threaded type with independent stud, hex nut and washer.

Powder driven anchors.--

Powder driven anchors shall be plated, spring steel alloy drive pin or threaded stud type anchors for use in concrete or steel. Spring steel shall conform to ASTM Designation: A 227M, Class 1. The diameter, length and type of shank and the number and type of washer shall be as recommended by the manufacturer for the types and thickness of material being anchored or fastened.

Mortar.--

Mortar shall consist of one part cement, measured by volume, to 2 parts clean sand and only enough water to permit placing and packing.

FABRICATION.--

Workmanship and finish.--Workmanship and finish shall be equal to the best general practice in modern shops.

Miscellaneous metal shall be clean and free from loose mill scale, flake rust and rust pitting, and shall be well formed and finished to shape and size with sharp lines and angles. Bends from shearing or punching shall be straightened.

The thickness of metal and details of assembly and support shall give ample strength and stiffness.

Built-up parts shall be true to line and without sharp bends, twists and kinks. Exposed ends and edges of metal shall be milled or ground smooth, with corners slightly rounded.

Joints exposed to the weather shall be made up to exclude water.

Galvanizing.--Items indicated on the plans to be galvanized shall be hot-dip galvanized after fabrication. The weight of galvanized coating shall be at least 460 grams per square meter of surface area, except drainage grates shall have at least 610 grams per square meter of surface area.

Painting.--Building miscellaneous metal items not galvanized shall be cleaned and prime painted prior to erection in accordance with the requirements specified for steel and other ferrous metals under "Painting" in Section 12-9, "Finishes," of these special provisions.

Loose bearing and leveling plates.--Loose bearing and leveling plates shall be furnished for steel items bearing on concrete construction, made flat, free from warps or twists, and of required thickness and bearing area. Plates shall be drilled to receive anchor bolts. Galvanize after fabrication.

PART 3.- EXECUTION

GENERAL.--

Anchorages.--Anchorage devices and fasteners shall be provided for securing miscellaneous metal in-place construction; including threaded fasteners for concrete and masonry inserts, toggle bolts, through bolts, lag bolts, wood screws and other connectors.

Cutting, drilling and fitting shall be performed as required for installation of miscellaneous metal fabrications.--
Work is to set accurately in location, alignment and elevation, plumb, level, true and free of rack, measured from established lines and levels.

Loose leveling and bearing plates.--Plates shall be set on wedges or other adjustable devices. Anchor bolts shall be wrench tightened after the plates have been positioned and plumbed. Mortar shall be packed solidly between bearing surfaces and plates to ensure that no voids remain.

Powder driven anchors.--Powder driven anchors shall be installed with low velocity powder actuated equipment in accordance with the manufacturer's instructions and State and Federal OSHA regulations.

DAMAGED SURFACES.--

General.--Galvanized surfaces that are abraded or damaged at any time after the application of the zinc coating shall be repaired by thoroughly wire brushing the damaged areas and removing all loose and cracked coating, after which the clean areas shall be painted with 2 applications of unthinned zinc-rich primer (organic vehicle type). Aerosol cans shall not be used.

12-5.05 MISCELLANEOUS STAINLESS STEEL SHEET METAL

PART 1.- GENERAL
Scope.-- This work shall consist of fabricating, furnishing and installing miscellaneous stainless steel sheet metal in accordance with the details shown on the plans and these special provisions.
Miscellaneous stainless steel sheet metal shall consist of:

Stainless steel gutters, flashing and cover plates

Working Drawings.--Working drawings for all fabricated items shall be submitted for approval.

PART 2.- PRODUCTS

MATERIALS.--

General.--Sheet metal materials shall be flat, smooth and free from surface blemishes where exposed to view in the finished units. Materials which exhibit pittingor discolorations shall not be used.
Stainless steel plate, sheet and strip.--

Stainless steel plate, sheet and strip shall conform to the requirements of ASTM Designation: A240/A 240M, Type 304. Stainless steel material shall have a No. 4 satin finish.

Fasteners.--

Fasteners shall be stainless steel.

FABRICATION.--

General.--Fabrication of sheet metal items shall conform to the requirements indicated for quality, thickness and finish as well as dimensions and details.

Stainless steel sheet metal shall be accurately dieformed and assembled in jigs.units shall be square and rigid. Flat or plane surfaces shall be free of heat of other types of distortions.

All free edges shall be hemmed.

Rounded corners shall be true and straight and shall be formed on a constant radius.

All welded seams shall be continuous welds.

Fabrication and installation shall conform to the recommendation in SMACNA publications.

Sheet metal items shall be formed in the maximum lengths and keep joints to a minimum.

PART 3.- EXECUTION

Installation.--Sheet metal items shall be set plumb, level and in alignment with adjacent work.

Une concealed fasteners and anchorages where possible.

Joints shall be fitted tight with exposed conncetions accurately fitted together.

12-5.06 METAL PANEL FASCIA

PART 1.- GENERAL

SUMMARY.--

Scope.-- This work shall consist of designing, fabricating, furnishing and installing metal panels on the fascia and soffit of the toll plaza canopy as shown on the plans and these special provisions.

At the option of the Contractor, the panels for the metal panel system may be either aluminum faced composite panels, aluminim faced panels with corrugated interior core, or panels of single thickness sheet aluminum. The panel option selected shall be complete with all the required accessories, fasteners and sealants.

Working Drawings.--Working drawings, engineering calculations, five metal panel samples, five panel anchorage system samples,catalog cuts,descriptive data, and installation instructions for sealing system products shall be submitted for approval.

Working drawings shall show the shape, size, thickness, and method of attachment for each component used in the work; the layout and spacing of fasteners; details of connections and closures; and details for expansion joints and weathertight joints.

Design calculations for the fastening system with the substrate shown on the plans shall be submitted to verify compliance with the design requirements.

Working drawings and design calculations shall be stamped and signed by an engineer who is registered as a Civil or Structural Engineer in the State of California. The expiration date of the registration shall be shown. The Engineer's signature shall be original.

The engineering calculations for the design of the metal panel system shall include applied loads, load combinations, panel stresses and deflections, fastener loads and stresses and provisions for accomodating the canopy deflections.

Design Requirements.--Composite metal panels shall conform to the wind design requirements for uplift in Chapter 16 of the Uniform Building Code for the wind speed and exposure shown on the plans. The metal panel syustem shall accomodate all canopy deflections without overstress or distortion.

DELIVERY, HANDLING AND STORAGE.--

Delivery and handling.--Panels shall be protected against damage and discoloration.

Storage.--Panels shall be stored above ground, with one end elevated for drainage and protected against standing water and condensation between adjacent surfaces.

PART 2.- PRODUCTS

MATERIALS.--

SHEET MATERIAL.--

Panels.--

Panels shall match the existing color and shall be one of the following:

1. Composite metal panel shall consist of two exterior sheets of 0.5 mm thick 3003 aluminum alloy and a core of extruded thermoplastic. The exterior sheets and core shall be bonded together with no glues or adhesives. The panels shall be integrated into a 41 mm maximum depth extruded aluminum edge grip system. The edge grip system shall be of 6063-T5 aluminum alloy.

2. Metal panel with corrugated core, 6 mm thick and consist of 1 mm, 3003H-14 aluminum alloy, exterior skin; aluminum alloy, interior skin. The exterior sheet, core and interior sheet shall be laminated by means of a press, heat and epoxy film adhesive method. The panels shall be fastened to extruded perimeter trim pieces by mechanical and adhesive system.

3. Single thickness metal panel, minimum 5 mm thick and shall be solid plate 3003-H134 aluminum alloy. The single thickness panels shall be integrated into a 41 mm maximum depth extruded aluminum support system. The support system shall be 6063-T5 aluminum alloy.

The maximum allowable fabriction tolerances for the panels shall be as follows:

Panel Bow	Maximum of 0.2 percent of length or width panel dimension
Width	3 mm
Length	3 mm
Camber	0.8 mm
End Squareness	0.25 percent of panel width

All panels surfaces shall be free of seams, warp, and buckling. The panel lines, breaks and angles shall be sharp and true.

The metal panel system shall have an internal drainage system to catch condensation and any other accumulations of moisture and direct it to discharge weeps.

Attachment clips, fasteners, furring, shims, brackets and miscellaneous hardware shall be stainless steel and shall be as recommended by the metal panel manufacturer. All fasteners shall be concealed.

Sealant.--

Sealant shall be one-part, low modulus, weatherporoofing silicone, Dow Corning Corporation, 795 Silicone Building Sealant; General Electric Company, Silpruf Sealant; or equal and shall be as recommended by the metal panel manufacturer.

Backer rod.--

Backer rod shall be round closed cell polyurethane that is compaticble with the sealant used and shall be sized such that it must be compressed between 25 and 75 percent of its compressed diameter when insterted in the joint.

EXECUTION

The alignment and dimensions of the steel members of the canopy shall be verified prior to commencing installation of the metal panel system.

The metal panel system shall be installed rigidly and securely in accordance with the manufacturer’s recommendations.

Extreme care shall be exercised in handling, storing, moving and installation the metal panels to avoid twisting, racking, scratching, denting, chipping, staining or any other type of damage or distortion. Panels damaged in handling or installing shall be replaced by the Contractor at his expense.

Panels shall be accurately positioned to maintain the straight joint lines between panels as shown on the plans. joints shall be kept uniform throughout the installation and shall be of sufficient width to accommodate the deflections of the canopy frame and the expansion and contraction of the panels but shall not exceed 12 mm in width.

Adjoining panels shall be aligned and matched to one another at the joints. The surfaces at the edges of each panel shall be flush with the surfaces at the edges of the adjacent panels.

Neoprene isolation material shall be provided to separate adjoining incompatible metals. Nylon washers shall be used to separate canopy members, fasteners, clips, and brackets of ferrous metal (except stainless steel) from aluminum.

Openings through the panels for penetrations shall be cut square to present a neat and true appearance with exposed burrs, removed. Openings shall be reinforced and sealed as recommended by the panel manufacturer.

Other surfaces necessary to use sealants, shall be clean and shall be free of dust, dirt oil, moisture and other deleterious matter which might adversely affect the adhesion of the sealant. Primer shall be applied in accordance with the sealant manufacturer's instructions. Application of backing materials and sealant shall be in accordance with the panel manufacture's working drawings and the sealant manufacturer's instructions, all as specified under Submittals," in these special provisions.

Joints to be sealed shall be inspected for construction defects which would adversely affect the execution of the work. Unsatisfactory conditions shall be corrected before the joint sealant is applied.

The installation metal panel system shall be weathertight.

The metal panel system shall be thoroughly cleaned after installation to remove excess sealant, tapes, dirt, grease or other unsightly materials. Cleaning shall be in accordance with the panel and sealant manufacturer's instructions.

12-5.07 STAIR NOSING.--

GENERAL.--This work shall consist of furnishing and installing stair nosing in accordance with the details shown on the plans and these special provisions.

SUBMITTALS.--Manufacturer's descriptive data and a color sample 25 mm long shall be submitted for approval.

PRODUCTS.--

Stair nosing.--

Stair nosing shall meet the OSHA requirements for nonslip safety on stairs.

Stair nosing shall be approximately 76 mm wide and 9 mm in thickness. The anchoring system shall stabilize the nosing, prevent rocking and loosening and shall permanently lock the nosing in place.

Stair nosing shall be commercial, first grade quality combination of cast nickel and cast bronze with cross hatched abrasive surface. Nonslip granules shall range from size No. 16 to No. 24 and shall be uniformly distributed at a minimum rate of 613 grams per meter, and a minimum of 1.6 mm in thickness over the wearing surface while the nosing is in the molten stage.

EXECUTION

INSTALLATION.--Stair nosing shall be protected during shipping and installation with factory applied protective material.

Stair nosing shall be one piece, full length of the stair width, and shall be installed rigidly in accordance with the manufacturer's instructions.

SECTION 12-6. BLANK

SECTION 12-7. THERMAL AND MOISTURE PROTECTION

12-7.01 INSULATION (GENERAL)

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing insulation in accordance with the details shown on the plans and these special provisions.

Insulation materials shall be as specified in these special provisions, and shall be compatible with existing or new materials incorporated in the building.

SUBMITTALS.--

Product data.--A list of materials, manufacturer's descriptive data, location schedule, and time schedule shall be submitted for approval.

The list of materials to be used shall include the trade name, manufacturer's name, smoke developed and flame spread classification, resistance rating and thickness for the insulation materials and accessories.

Schedules.--A location schedule and time schedule shall be submitted for approval.

The location schedule shall show where each material is to be installed.

The Contractor shall provide the Engineer at the jobsite with an accurate time schedule of the areas of the building to be insulated each day. The time schedule shall be submitted 3 working days in advance of the work.

Samples --Samples of insulation material shall be submitted to the Engineer at the jobsite.

QUALITY ASSURANCE.--

Codes and standards.--All insulating materials shall be certified to comply with the California Quality Standards for Insulating Materials and shall be listed in the Department of Consumer Affairs publication "Consumer Guide and Directory of Certified Insulation Material."

DELIVERY, STORAGE AND HANDLING.--

General.--Insulating materials shall be delivered to the jobsite and stored in a safe dry location with labels intact and legible.

Insulating materials shall be protected from physical damage and from becoming wet or soiled.

In the event of damage, materials shall be repaired or replaced as necessary to comply with these specifications.

PART 2.- PRODUCTS (Not applicable.)

PART 3.- EXECUTION (Not applicable.)

12-7.01A RIGID INSULATION

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing rigid insulation in accordance with the details shown on the plans and these special provisions.

Rigid insulation shall include rigid insulation, wood nailers (furring and blocking), fasteners and such other materials, not mentioned, which are required for the complete installation of the rigid insulation system.

QUALITY ASSURANCE.--

Codes and standards.--Rigid foam insulation shall have a flame-spread rating not to exceed 75 and a smoke density not to exceed 450 when tested in accordance with UBC Standard No. 8-1.

PART 2.- PRODUCTS

Rigid insulation.--

Rigid insulation shall be rigid rectangular boards of polyisocyanurate foam with aluminum foil facing on both sides, water vapor transmission not more than 0.50 g • mm/24 hr • m^2 • mmHg and an aged thermal resistance of R 2.1 K•m^2/W.

Wood nailers.--

Wood nailers shall be Douglas fir, Hem-fir or equivalent western softwood. Nailers in contact with concrete shall be pressure treated. Wood preservatives shall be waterborne type.

Insulation tape.--

Insulation tape shall be as recommended by the insulation manufacturer.

Fasteners.--

Fasteners shall be concrete nails; Bostich, Pneumatic Nail System; Buildex, Tampcon Fasteners; or equal.

EXECUTION.--

Installation of rigid insulation.--The preparation of the wall surfaces and the installation of insulation shall conform to the manufacturer's recommendations and these special provisions.

Rigid insulation placed behind gypsum board shall be tight fitting between 38 mm x 89 mm wood nailers laid flat and spaced 0.6 meters on center. Wood nailers shall also be placed at edges.

All joints between insulation boards and between insulation boards and wood nailers shall be taped.

Insulation panels with broken or crushed corners or edges shall be trimmed free of such defects or shall be discarded. Replacement boards less than 300 mm wide shall not be used.

Damaged insulation in the completed work shall be removed and replaced. Insulation that has been wet or is wet shall be considered damaged.

12-7.02 SHEET METAL FLASHING

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of fabricating, furnishing and installing sheet metal flashing in accordance with the details shown on the plans and these special provisions.

Sheet metal shall include metal flashings, sheet metal work for toll booth, toll booth cabinet work, counterflashings, straps, and roof jacks.

QUALITY ASSURANCE.--

Codes and standards.--Sheet metal work shall in accordance with the requirements in the latest edition of the Sheet Metal and Air Conditioning Contractors National Association "Standard Practice in Architectural Sheet Metal Work."

PART 2.- PRODUCTS

MATERIALS.--

Galvanized sheet steel.--

Galvanized sheet steel shall conform to ASTM Designation: A 361, not less than 0.71 mm (24-gage), unless otherwise shown on the plans. Surfaces to be painted shall not have factory coatings on galvanizing that cannot be removed by paint thinner.

Sheet metal for toll booth roofing.--

Sheet metal for toll booth roofing shall be ASTM Designation: A 366 or A 569, zinc coated, 12-gage minimum thickness unless otherwise shown on the plans. Surfaces to be painted shall not have factory coating on galvanizing that cannot be removed by paint thinner.

Sheet stainless steel.--

Sheet stainless steel shall be Type 302 or 304 with No. 4 finish on all exposed surfaces and conform to ASTM Designation: A 167. Sheet stainless steel shall be 14-gage minimum thickness for toll booth cabinet work, 24-gage minimum thickness for flashing, cap flashings and straps.

Fasteners for sheet stainless steel shall be 18-8 stainless steel.

Solder.--

Solder shall conform to ASTM Designation: B 32, Alloy Grade Sn50.

Soldering flux.--

Soldering flux shall be acid type, conforming to Federal Specification: O-F-506C, Type I, Form A.

Sealant.--

Sealant for exposed locations shall be a silicone sealant conforming to ASTM Designation: C 920.

Primer.--

Primer shall be as recommended by the sealant manufacturer.

Coal tar paint.--

Coal tar paint shall be coal-tar epoxy coating conforming to U.S. Corps of Engineers Specification: C-200 or Steel Structures Painting Council Paint Specification: SSPC-16-68T.

FABRICATION.--

General.--Sheet metal shall be assembled to Sheet Metal and Air Conditioning Contractors National Association Standards.

Sheet metal shall be formed to the sizes, shapes and dimensions shown on the plans or as specified herein with angles and lines straight, sharp and in true alignment. The number of joints shall be kept to a minimum.

Angle bends and folds for interlocking the metal shall be made with full regard for expansion and contraction to avoid buckling or fullness in the metal after it is installed.

Joints in sheet metal work shall be closed watertight unless slip joints are specifically required. Watertight joints shall be mechanically interlocked and then thoroughly soldered for metals other than aluminum. Watertight joints in aluminum or between aluminum and other metals shall be sealed with acrylic sealant.

Stainless steel sheet metal shall be accurately die-formed and assembled in jigs. Units shall be square and rigid. Flat or plane surfaces shall be free of heat or other types of distortion. Rounded corners shall be true and straight and shall be formed to a constant radius.

Stainless steel cabinets shall be of welded one-piece construction. Dividers and splash boards shall be welded integral with the countertops.

Stainless steel frames shall have mitered and welded corners. Exposed welds shall be ground smooth. Exposed butt welds shall be ground flush. All corners shall be rounded to 3 mm radius. All seams shall be sealed welded and ground smooth unless otherwise shown on the plans.

Connectors shall be concealed.

Sheet metal joints to be soldered shall be cleaned with steel wool or other means, pre-tinned and soldered watertight.

All joints shall be wiped clean of flux after soldering. Acid flux shall be neutralized by washing the joints with sodium bicarbonate.

Flashings shall have a 45 degree drip return at bottom edges. Unless otherwise shown on the plans, counterflashing shall extend not less than 100 mm over roofing or other materials protected by the counterflashing and shall be arranged so that roofing or materials can be repaired without damage to the counterflashing.

PART 3.- EXECUTION

PREPARATION.--Surfaces to receive sheet metal shall be clean, smooth and free from defects. Galvanized or stainless steel sheet metal that is marred or incorrectly bent will be considered damaged and shall be replaced with undamaged units.

PROTECTION.--Aluminum surfaces to be in contact with concrete, mortar, or dissimilar metals shall be given a heavy coat of coal tar paint.

INSTALLATION.--

Roof penetration flashings.--All pipes, ducts, vents and flues passing through roofs shall be made waterproof with flashings of storm collars or counterflashings.

Roof penetration flashings shall be fabricated from galvanized sheet steel, not less than 0.71 mm (24-gage). Size and shape shall be as shown on the plans.

12-7.03 SEALANTS AND CAULKING

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and applying sealants and caulking which are required for this project, but not specified elsewhere, in accordance with the details shown on the plans and these special provisions.

QUALITY ASSURANCE.--

Certificates of Compliance.--Certificates of compliance shall be furnished for the sealants and caulking in accordance with the requirements specified in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications.

SUBMITTALS.--

Product data.--Manufacturer's descriptive data and installation instructions for all sealants shall be submitted for approval.

Samples.--Color samples of all sealants shall be submitted for approval. Unless otherwise shown on the plans, colors will be selected by the Engineer from the manufacturer's standard colors.

PART 2.- PRODUCTS

MATERIALS.--

General.--All sealants, primers and accessories shall be non-staining to adjacent exposed surfaces. Products having similar applications and usage shall be of the same type and same manufacturer. Gun consistency compound shall be used unless otherwise required by the job conditions.

Acrylic sealant.--
Acrylic sealant shall be one compound, solvent release acrylic sealant.

Butyl sealant.--
Butyl sealant shall be one component, skinning type.

Silicone sealant.--
Silicone sealant shall be one component, low modulus building sealant. Sealant shall be tack-free in one hour, shall not sag or flow, shall be ozone resistant and capable of 100 percent extension without failure.

Joint sealant.--
Joint sealant shall be a two-part, non sag polysulfide base, synthetic rubber sealant formulated from liquid polysulfide polymer.

Backer rod.--
Backer rod shall be round, open or closed cell polyurethane. Backer rod shall be sized such that it must be compressed between 25 and 75 percent of its uncompressed diameter during installation in the joint.

Neoprene.--
Neoprene shall conform to the requirements of ASTM Designation: C 542.

PART 3.- EXECUTION

APPLICATION.--

General.--Unless otherwise shown on the plans, sealants shall be applied in accordance with the manufacturer's instructions.

Silicone sealants shall not be used in locations where painting is required.

Butyl sealants shall not be used in exterior applications, and acrylic sealants shall not be used in interior applications.

Sealants shall be applied in a continuous operation for the full length of the joint. Immediately following the application of the sealant, the sealant shall be tooled smooth using a tool similar to that used to produce concave masonry joints. Following tooling, the sealant shall remain undisturbed for not less than 48 hours.

12-7.04 TORCH APPLIED ROOFING

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing a torch applied roof covering system in accordance with the details shown on the plans and these special provisions.

Torch applied roofing system shall have a granular surfacing, and shall include all materials for constructing the roofing system complete and in place.

SUBMITTALS.--

Product data.--Manufacturer's descriptive data and installation instructions for torch applied roofing system shall be submitted for approval.

QUALITY ASSURANCE.--

Codes and standards.--Roofing materials shall conform to the rules for control of volatile organic emissions adopted by the local air pollution control district having jurisdiction in the area.

Field samples.--Materials shall be delivered to the jobsite in labeled containers or wrappings sufficiently ahead of their intended use to allow sampling and testing by the Engineer.

DELIVERY AND HANDLING.--

Storage.--Stored or stockpiled roof roofing shall be set on end and shall be protected from the elements. Roofing rolls compressed into oval cross section shall not be used

PART 2.- PRODUCTS

Torch applied roofing and flashing.--

Torch applied roofing shall be Class A or B fire retardant plastic bituminous roofing membrane. Plasticized bituminous compound, consisting of distilled asphalt mixed with polyproylene and petrochemical woven around a nonwoven polyester core.

Flashing rolls shall be properly sized for the application intended.

Top surface shall be embedded with slate flakes or mineral granules as follows:

4 mm smooth---	For torch applied roofing on curved surfaces.
4.5mm granular----	For torch applied roofing on flat surfaces.

Primer.--

Primer for torch applied roofing shall be a water-based asphalt primer for application for metal surfaces.

PART 3.- EXECUTION

PREPARATION.--

General.--Surfaces to be covered shall be smooth, hard, dry and shall be free from high spots, depressions, frost or defects from frost, dust loosened material or other debris.

Openings, holes or crevices shall be filled or covered before any roofing materials are applied.

INSTALLATION.--

Application.--Torch applied roofing shall be installed to provide a watertight, non-leaking roof covering in accordance with the manufacturer's recommendations.

Torch applied roofing work and work of other trades shall be coordinated and scheduled to minimize interference and to eliminate traffic on the completed roof.

Torch applied roofing and roofing materials shall not be applied in wet weather or when ambient temperature is below 50° F. All drainage fixtures shall be set at the proper level to permit the free flow of water.

Primer shall be applied in a thin, continuous, uniform coating without skips or dry spots.

Flashing.--Flashings shall be installed as the roofing progresses to protect new facilities from damage resulting from rain, wind and storms. Any damage resulting from rain, wind or storms shall be corrected at the Contractors expense.

Torch applied flashings shall be placed after roofing is placed unless otherwise shown on the plans.

Torch applied flashings shall be one layer of flashing applied to metal substrate free of voids, fish mouths and wrinkles. No splices will be permitted.

CLEANUP.--

General.--Upon completion of the work, the Contractor shall clean all exposed surfaces that are subject to spillage, over run and marking by torch applied roofing. Clean up methods and materials shall be as recommended by the manufacturer.

Cleanup methods shall not damage, discolor or otherwise affect the exposed surfaces.

SECTION 12-8. DOORS AND WINDOWS

12-8.01 HINGED AND SLIDING DOORS

GENERAL.--This work shall consist of furnishing and installing hinged doors and frames in accordance with the details shown on the plans and these special provisions.

SUBMITTALS.--Manufacturer's descriptive data, installation instructions for fire rated assemblies and a door schedule shall be submitted for approval. The door schedule shall include a description of the type, location and size of each door and frame.

PRODUCTS.--

Metal door.--

Metal door shall be flush, seamless steel door factory prepared and reinforced to receive hardware and having cold rolled stretcher leveled sheet steel face sheets not less than 1.2 mm thick (18-gage). Face sheets shall be bonded with thermosetting adhesive to rigid board honeycomb or precured foam core; or face sheets shall be welded to all parts of an assembled grid of cold formed pressed metal stiffeners and framing members located around edges, ends, openings and at all locations necessary to prevent buckling of face sheets. Seams shall be tack welded, filled and ground smooth. Bottom edge and internal stiffeners of grid type core shall have moisture vents. Welds on exposed surfaces shall be ground smooth. Louvered or glazed openings shall be provided where shown on the plans.

Where fire rated doors are required, doors shall be listed and labeled for the fire rating shown on the plans.

Active leaf of double door shall have a full height astragal of 3 mm flat bar or folded sheet strip, not less than 1.5 mm thick (16-gage), welded on the outside of the active leaf.

Door shall be cleaned and treated by the bonderized process or approved phosphatizing process and then given one factory application of metal protective rust inhibitive primer. Primer shall not contain lead type pigments.

Door assemblies (toll booth).--

Door assemblies (toll booth) shall conform to the requirements of metal doors in these special provisions.

Glazing for doors and toll booth.--

Glazing for doors and toll booth shall be safety glass as specified under "Glazing" in Section 12-8, "Doors and Windows," of these special provisions. Glazing shall be not less than 5 mm thick.

Pressed metal frame.--

Pressed metal frame shall be not less than 1.5 mm thick (16-gage) sheet steel with integral stop, mitered corners, face welded and ground smooth corners. Frames shall be reinforced for all hardware and shall be cleaned and treated by the bonderized process or an approved phosphatizing process and then given one factory application of metal protective rust inhibitive primer. Primer shall not contain lead type pigments.

Sealants.--

Sealants shall be ultraviolet and ozone resistant, gun grade polysulfide or polyurethane, multicomponent, Federal Specification: TT-S-227.

EXECUTION.--

INSTALLATION.--Doors and frames shall be installed rigidly, securely, plumb and true and in such a manner that the doors operate freely without rubbing or binding. Clearance between frame and door shall be not more than 3 mm. The exterior frame shall be sealed weathertight.

Pressed metal frames shall be secured with clips and anchors as shown on the plans.

PAINTING.--Except for the primer application specified herein, doors and frames shall be cleaned, prepared and painted in accordance with the requirements specified under "Painting" in Section 12-9, "Finishes," of these special provisions.

12-8.02 FINISH HARDWARE

GENERAL.--This work shall consist of furnishing and installing hardware items for doors in accordance with the details shown on the plans and these special provisions.

Hardware assemblies shall comply with the disabled accessibility requirements indicated on the plans and specified in these special provisions.

SUBMITTALS.--Manufacturer's technical information and catalog cuts for each item of door hardware and a door hardware schedule shall be submitted for approval prior to installation.

Manufacturer's catalog cuts shall include catalog numbers, material, grade, type, size, function, design, quality and finish of hardware.

The door hardware schedule shall indicate the location and size of door opening, the door and frame material, and the size, style, finish and quantity of the hardware components required.

FINISHES.--Hardware shall be provided with standard US 32D metal plated finish or aluminum sprayed finish where indicated.

KEYING INSTRUCTIONS.--New locks shall be compatible with the master key system of the existing facility and shall be keyed to the existing lock system in use.

Locks and cylinders shall be provided with six pin "O" cylinders and blank keys. Cylinders and blank keys shall be delivered to the Engineer for combinating of cylinders and cutting of keys.

The Contractor shall provide cylinders for use during construction. Construction cylinders shall remain in place until permanent cylinders are installed. Construction cylinders shall remain the property of the Contractor.

Key bows shall be stamped "State of California" and "Do Not Duplicate."

PRODUCTS.--

GENERAL.--Door hardware equal in material, grade, type, size, function, design, quality and manufacture to that specified herein may be submitted for approval.

Butt hinges.--

Butt hinges shall be steel, 1 1/2-pair per door unless otherwise specified or shown on the plans. Nonremovable pins shall be provided at outswing exterior doors. Hinge size shall be 114 mm x 114 mm unless otherwise noted.

Heavy weight hinges shall be:

Hager	BB 1168
McKinney	T4B.37869
Stanley	BB 168
or equal.	

Mortise locksets.--

Mortise locksets shall be steel case with 32 mm x 203 mm face plate and 70 mm backset. Door and frame preparation for mortise locksets, latchset and privacy sets shall conform to ANSI A115.1.

Lever operated lockset shall be:

Best	35H 6FW 15H
Falcon	LM521 DG
Schlage	L9453P x 06
or equal.	

Door closers.--

Parallel arms for closers shall be installed at outswing exterior doors. Closers shall have sprayed finish to match other hardware on door.

Door closers shall be:

LCN	4040
Norton	85001
Dorma	7800

or equal.

Floor mounted stops.--

Floor mounted stops shall be dome type. The height of the stop shall be determined by the clearance required when a threshold is used or not used.

Stops for openings with thresholds shall be:

Builders Brass	8063
Quality	431
Trimco	1213

or equal.

Automatic door bottom.--

Automatic door bottom shall be heavy duty, full mortise.

Bottom shall be:

Pemko	434 AR
Zero	360

or equal.

Thresholds and rain drips.--

Thresholds and rain drips shall conform to the sizes and configurations shown on plans. Thresholds at door openings with accessibility requirements shall not exceed 13 mm in height.

Threshold and rain drip manufacturers shall be Pemko, Reese, Zero, or equal.

Threshold bedding sealant.--

Threshold bedding sealant shall conform to Federal Specification: SS-C-153.

Weatherstrip.--

Weatherstrip shall conform to the sizes and shapes shown on plans. Assemblies shall be UL listed and shall be provided where shown on the plans or as specified in these special provisions.

Weatherstrip manufacturers shall be Pemko, Reese, Zero, or equal.

DOOR HARDWARE FOR TOLL BOOTH.--

Butt hinges.--

Butt hinges shall be stainless steel, Hager BB 1191, McKinney TB 2314TB, Stanley FBB 191, or equal. Outswing exterior doors shall have non removable pin.

Latch sets.--

Mortise type with lever handles on each side, Schlage K6010X20X715, Corbin 9510X799L or equal.

Lockset.--

Mortise type with lever handles each side, wrought escutcheons and deadbolt with 25 mm throw, cylinder on outside and thumb turn on inside. Lockset shall be Schlage K6453X20X715, Corbin 9549X799L, or equal.

Closers.--

Closers shall be LCN 4014, Norton 7704, or Corbin 110 or equal, sprayed finish with corrosion infiltrating prime coat. Closers shall be rated by the manufacturer for location and width of door to be used, except parallel arm closers shall be rated for width at least 100 mm greater than actual door width. Set at door operating force of 3.8 kg for exterior doors. Arm shall have 90 degrees dead stop.

Pivot set.--
Pivot set shall be top and bottom pivots with sealed bearing, surface applied.

Door holder.--
Door holder shall be door mounted, lever type with a 25 mm by 50 mm base and 4 mounting screws and a 102 mm rubber tipped leg.

Floor mounted door guide.--
Floor mounted door guide shall be stainless steel angle 38 mm by 38 mm by 50 mm long with countersunk machine screws and expansion shields.

Kickplate shall be 254 mm high by 1.5 mm thick by width 50 mm less than door width with beveled edges.

Flush bolts.--
Flush bolts shall be 305 mm extension bolt and 171 mm by 25 mm face. Flush bolts shall be BBW 5021-2. Quality 1358, Corbin 2846, or equal. Aluminum body is not acceptable.

Floor mounted door stop.--
Floor mounted door stop shall be stainless steel dome shaped approximately 50 mm diameter base with machine screw and expansion shield. Height of stop shall be as required to suit the conditions.

Slide bolt.--
Slide bolt shall be heavy duty stainless guide with 9.5 mm by 305 mm sliding bolt equipped with operating knob and stainless steel strike.

Overhead track and accessories.--
Overhead track and assemblies shall be one assembly per sliding door. Tracks shall be stainless steel, 2 hangers per sliding door. Hangers shall have bearing, rated for 68 kg minimum capacity. Wheel pendants shall be drop forged and equipped with hardened machined steel thrust rollers. All components shall be corrosion resistant to marine environment.

Wind screen hinge.--
Wind screen hinge shall be continuous 50 mm stainless steel.

Gate hinge.--
Gate hinge shall be continuous 64 mm stainless steel hinge.

Astragal.--
Astragal shall be full length stainless steel, No 304.

Gate latch.--
Gate latch shall be 44 mm throw with keeper. Handle shall lock onto position perpendicular to floor.

Wind screen latch.--

Wind screen latch shall be stainless steel plate and 18 mm screw standard duty dull zinc finish spring.

Flush lockset.--
Flush lockset shall be heavy duty stainless steel, project only 2 mm from door stile to avoid catching or scratching other panels. Slide button operator on side with deep finger pulls set flush into door. Compatible with folding door panels. Hookbolt shall be laminated stainless steel.

Threshold.--
Threshold shall be extruded aluminum, approximately 152 mm wide with configuration as shown on the plans. Threshold shall satisfy disabled accessible requirements.

Weatherstripping, pile type.--
Weatherstripping, pile type, shall be replaceable, continuous wool pile with metal backing at door sides and top.

Weatherstripping, sweep type.--

Weatherstripping, sweep type, shall be door sweep type of 3 mm ultraviolet resistant neoprene strip in continuous extruded aluminum holder.

Weatherstripping.--

Weatherstripping shall be 25 mm wide by 9 mm thick continuous neoprene.

Weatherstripping, brush type.--

Weatherstripping, brush type shall be polypriopylene brush fiber bonded in a polypropylene binder contained in extruded aluminum.

Weatherstripping tape.--

Weatherstripping tape shall be sponge, epdm neoprene, closed cells, adhesive back, 3 mm thickl by 9 mm wide.

TOLL BOOTH CABINET HARDWARE.--

Drawer and swing door flush pulls.--

Drawer and swing door flush pulls shall be stainless steel, approximate dimensions, 51 mm x 76 mm and 16 mm depth.

Hinges.--

Hinges shall be continuous 51 mm wide stainless steel hinge.

EXECUTION.--

DOORS AND FRAMES.--Doors and frames shall be set square and plumb and be properly prepared before the installation of hardware.

INSTALLATION.--Hardware items shall be accurately fitted, securely applied, and adjusted and lubricated in accordance with the manufacturer's instructions. Installation shall provide proper operation without bind or excessive play.

Hinges shall be installed at equal spacing with the center of the end hinges not more than 244 mm from the top and bottom of the door. Locksets shall be 1024 mm from the finished floor.

Thresholds shall be set in a continuous bed of sealant material.

Door controls shall be set so that the effort required to operate doors with closers shall not exceed 3.9 kg maximum for exterior doors.

Door stops located on concrete surfaces shall be fastened rigidly and securely in place with expansion anchoring devices.

Hardware, except hinges, shall be removed from surfaces to be painted before painting.

Upon completion of installation and adjustment, the Contractor shall deliver to the Engineer all dogging keys, closer valve keys, lock spanner wrenches, and other factory furnished installation aids, instructions and maintenance guides.

DOOR HARDWARE GROUPS AND SCHEDULE.--Hardware groups specified herein shall correspond to those shown on the plans:

TUNNEL DOOR HARDWARE

GROUP 1

1 1/2-pair butt hinges
1 each mortise lockset
1 each door closer
1 each weatherstrip
1 each mortise type automatic door bottom
1 each rain drip
1 each threshold with saddle
1 each floor mounted stop
1 each saddle rest

HUB BUILDING HARDWARE

GROUP 1

1 1/2-pair butt hinges
1 each mortise lockset
1 each door closer
1 each weatherstrip
1 each mortise type automatic door bottom
1 each rain drip
1 each threshold with saddle
1 each floor mounted stop
1 each saddle rest

GROUP 2

3-pair butt hinges
1 each lever operated mortise lockset on active leaf
1 each door closer on active leaf
1 each automatic door bottom
1 each rain drip
1 each threshold
1 each surface mounted manual door bolt top strike and bottom on inactive leaf
1 each weatherstrip
TOLL BOOTH HARDWARE.--

GROUP 1 (Entrance)

1 1/2-pair butt hinges
1 each lockset
1 each door close
1 each kickplate
1 each door holder
1 each weatherstrip
1 each threshold
1 each floor mounted stop

GROUP 2 (Side)

1 each overhead track and accessories
1 each floor mounted stop
1 each lockset
1 each weatherstripping

12-8.03 GLAZING

GENERAL.---This work shall consist of furnishing and installing glazing in accordance with the details shown on the plans and these special provisions.

Glazing shall consist of glass for windows, doors and other glazed openings.

All glass shall conform to ASTM Designation: C 1036 and the classifications specified herein and shall be clear glass except as noted.

Safety glass shall be furnished and installed at all locations designated in Consumer Product Safety Commission's Safety Standard For Architectural Glazing Materials 16 CFR 1201.

SUBMITTALS.--A detailed list of glazing materials including glass, sheet, sealants, tapes, setting blocks, shims, compression seals, and glazing channels shall be submitted for approval. The list shall include a schedule of the materials to be used at each location.

LABELS.--Each individual pane of heat strengthened or fully tempered glass shall bear an identification label in accordance with ASTM Designation: C 1048.

PRODUCTS.--

Glazing for door (toll booths)

Glazing for door (toll booth) shall be safety laminated

Safety glass.--
Safety glass shall conform to Consumer Product Safety Commission Safety Standard For Architectural Glazing Materials: 16 CFR 1201, and ANSI Standard Z97.1 and shall be one of the following:

Laminated glass.--
Laminated glass shall be safety glass, 6 mm minimum thickness, fabricated from 2 pieces of Type I, Class 1, Quality q4 or better glass fused to plastic interlayers.

Tinted glass shall be bronze; all the same tint.

Seals, caulks, putties, setting blocks, shims, tapes, compression seals, felt, spacers, and channels.--
Seals, caulks, putties, setting blocks, shims, tapes, compression seals, felt, spacers, and channels shall be top grade, commercial quality, as recommended by the glass or sheet manufacturer and shall conform to the requirements in the publications of the Flat Glass Marketing Association.

EXECUTION.--

INSTALLATION.--Glazing shall conform to the general conditions and applicable details in the publications of the Flat Glass Marketing Association.

Cut edges of tinted glass shall conform to the recommendations of the glass manufacturer. The glazier shall inspect each edge of tinted glass. Panes with edges that do not conform to the manufacturer's standards for tinted glass edges for sunny elevations shall not be used.

Panes shall be bedded fully and evenly, set straight and square within panels in such a manner that the pane is entirely free of any contact with metal edges and surfaces.

For all panes on the exterior of the building, the glazing on both sides of window panes shall provide a watertight seal and watershed. Seals shall extend not more than 2 mm beyond the holding members. A void shall be left between the vertical edges of the panes and the glazing channel. Weep systems shall be provided to drain condensation to the outside.

Panes in assemblies using extruded gasket glazing shall be set in accordance with the assembly manufacturer's instructions using gaskets and stops supplied by the manufacturer.

Laminated glass shall be set on setting blocks.

Whenever welding or burning of metal is in progress within 4.6 m of glazing materials, a protective cover shall be provided over exposed surfaces.

REPLACEMENT AND CLEANING.--All broken or cracked glass and glass with scratches which reduce the strength shall be replaced before completion of the project.

Panes shall be kept clean of cement and plaster products, cleansers, sealants, tapes and all other foreign material that may cause discoloration, etching, staining, or surface blemishes to the materials.

Excess sealant left on the surface of the glass or surrounding materials shall be removed during the work life of the sealant.

Solvents and cleaning compounds shall be chemically compatible with materials, coatings and glazing compounds to remain. Cleaners shall not have abrasives that scratch or mar the surfaces.

All panes shall be cleaned just before the final inspection. All stains and defects shall be removed. Paint, dirt, stains, labels (except etched labels), and surplus glazing compound shall be removed without scratching or marring the surface of the panes or metal work.

SECTION 12-9. FINISHES

12-9.01 GYPSUM BOARD

GENERAL.--This work shall consist of furnishing, installing and finishing gypsum wallboard in accordance with the details shown on the plans and these special provisions.

PRODUCTS.--

Water-resistant gypsum backing board.--
Water-resistant gypsum board shall conform to ASTM Designation: C 630.

Corner beads, metal trim and control joints.--

Corner beads, metal trim and control joints when shown on the plans, shall be galvanized steel of standard manufacture.

Fasteners.--

Fasteners shall be gypsum wallboard steel drill screws conforming to ASTM Designation: C 1002.

EXECUTION.--

DELIVERY AND STORAGE.--Materials shall be delivered in original packages, containers or bundles bearing brand name, applicable standard of manufacture, and name of manufacturer or supplier and shall be kept dry and fully protected from weather and direct sunlight exposure. Gypsum wallboard shall be stacked flat with adequate support to prevent sagging or damage to edges, ends and surfaces.

INSTALLATION.--Wallboard panels to be installed on ceilings shall be installed with the long dimension of the panels perpendicular to the framing members. Wallboard panels to be installed on walls may be installed with the long dimension of the panels either parallel or perpendicular to the framing members. The direction of placing the panels shall be the same on any one wall or partition assembly.

Edges of wallboard panels shall be butted loosely together. All cut edges and ends shall be smoothed as needed for neat fitting joints.

All edges and ends of gypsum wallboard panels shall coincide with the framing members, except those edges and ends which are perpendicular to the framing members. End joints on ceiling and on the opposite sides of a partition assembly shall be staggered.

Except where closer spacings are shown on the plans, the spacing of fasteners shall not exceed the following:

Screws	300 mm

Adhesives shall not be used for securing wallboard to framing.

Fasteners shall be located at least 10 mm from wallboard panel edges and ends. Screws shall penetrate into wood framing at least 20 mm. All metal fasteners shall be driven slightly below surface level without breaking the paper or fracturing the core.

Gypsum wallboard used as backing boards for rigid sheet wall covering shall be water resistant. Joints in backing board shall not be taped or filled and dimples at the fastener heads shall not be patched. Edges of cuts and holes in backing board shall be sealed with a primer or sealer that is compatible with the wall coveringadhesive to be used.

12-9.02 RUBBER BASE

GENERAL.--This work shall consist of furnishing and installing rubber base in accordance with the details shown on the plans and these special provisions.

SUBMITTALS.--Manufacturer's descriptive data, installation instructions, color palette, and samples of resilient base shall be submitted for approval. Samples shall be not less than 50 mm in length.

PRODUCTS.--

Resilient base.--

Resilient base shall be manufacturer's best grade, rubber with premolded internal and external corner pieces. The height and color shall be as shown on the plans.

Adhesive.--

Adhesive shall be as recommended by base manufacturer.

EXECUTION.--

Rubber bases shall be firmly and totally attached to walls with adhesive and shall be accurately scribed to trim, molding and cabinets. All joints shall be tight fitting. Bases between premolded corners or other termini may be installed continuous or installed using 1 m minimum standard manufactured lengths. Filler pieces shall be not less than 0.5 m.

12-9.03 PAINTING

GENERAL.--This work shall consist of preparing surfaces to receive coatings, and furnishing and applying coatings, in accordance with the schedules and details shown on the plans, and these special provisions.

The coatings specified in this section are in addition to any factory finishes, shop priming, or surface treatment specified elsewhere in these special provisions.

SUBMITTALS.--Manufacturer's descriptive data, a materials list, and color samples shall be submitted for approval.

Product descriptive data shall include product description, manufacturer's recommendations for product mixing, thinning, tinting, handling, site environmental requirements, product application and drying time.

Materials list shall include manufacturer's name, trade name, and product numbers for each type coating to be applied.

Color samples shall be manufacturer's color cards, approximately 50 mm x 75 mm, for each color of coating shown on the plans. Color samples for stains shall be submitted on wood of the same species, color, and texture as the wood to receive the stain.

REGULATORY REQUIREMENTS.--Coatings and applications shall conform to the rules for control of volatile organic compound emissions adopted by the air quality control district in the air basin in which the coatings are applied.

SITE ENVIRONMENTAL REQUIREMENTS.--Coatings shall not be applied when the air temperature is below 10°C (20°C for varnishes) or when the relative humidity exceeds 75 percent.

The surface to be coated shall be maintained at a minimum temperature of 7°C for a period of 24 hours prior to, and 48 hours after the application of the coating. Heating facilities shall be provided when necessary.

Continuous ventilation shall be provided during application of the coatings.

A minimum lighting level of 865 lux, measured 1 m from the surface to be coated, shall be provided while surfaces are being prepared for coatings and during coating applications.

DELIVERY, STORAGE, AND HANDLING.--Products shall be delivered to the site in sealed, labeled containers and stored in a well ventilated area at an ambient air temperature of not less than 7°C. Container labeling shall include manufacturer's name, type of coating, trade name, color designation, drying time, and instructions for tinting, mixing, and thinning.

MAINTENANCE STOCK.--Upon completion of coating work, a full 3.8 liter container of each type and color of finish coat and stain used shall be delivered to the location at the project site designated by the Engineer. Containers shall be tightly sealed and labeled with color, texture, and room locations where used, in addition to the manufacturer's standard product label.

PRODUCTS.--

GENERAL.--The products shall be the best quality grade coatings of the specified types as regularly manufactured by nationally recognized paint and varnish manufacturers that have not less than 10 years experience in manufacturing paints and varnishes. Products that do not bear the manufacturer's identification as the best quality grade product shall not be used. Products for each coating system shall be by a single manufacturer and shall not contain lead type pigments.

Thinners, shellac, fillers, patching compounds, coloring tint, and other products required to achieve the specified finish shall be the manufacturer's best quality and shall be used as recommended.

EXECUTION.--

INSPECTION.--Surfaces to be coated at the jobsite shall be approved by the Engineer prior to the application of coatings. The Contractor shall notify the Engineer at least 3 working days prior to the application of coatings.

SURFACE PREPARATION.--Surfaces scheduled to be coated shall be prepared in accordance with the following, except that the surfaces not specified herein shall be prepared as recommended by the coating manufacturer.

GENERAL.--Hardware, cover plates, light fixture trim, and similar items shall be removed prior to preparing surfaces for coating. Following the application of the finish coating, the removed items shall be reinstalled in their original locations.

WOOD.--Oil and grease shall be removed by solvent wash. Mildew shall be removed by mildew wash. Surfaces to be coated shall be cleaned of all dirt, excess material, or filler by hand cleaning. Smooth surfaced wood shall be sanded lightly.

A sealer composed of equal parts of shellac and alcohol shall be spot applied to knots, sap, pitch, tar, creosote, and other bleeding substances.

After the application of the prime coat, all nail holes, cracks, open joints, dents, scars, and surface irregularities shall be filled, hand cleaned, and spot primed to provide smooth surfaces for the application of finish coats.

Irregularities in wood surfaces to receive a transparent stain finish shall be filled and hand cleaned after the first coat of stain has been applied. The color of the filler shall match the color of the stained wood.

Irregularities in wood surfaces to receive a clear finish shall be filled and hand cleaned before the application of coatings. The color of the filler shall match the color of the coated wood.

GALVANIZED METAL.--Oils, grease, and fabrication lubricants shall be removed by solvent wash. Surfaces shall be cleaned of remaining surface treatments by hand cleaning. New surfaces shall be roughened by hand cleaning or light abrasive blasting.

Abraded or corroded areas shall be hand cleaned and spot coated with one coat of vinyl wash pretreatment. Abraded or corroded areas on new surfaces not scheduled to be painted shall be cleaned by solvent wash, hand cleaned, and given 2 spot applications of zinc rich paint.

STEEL AND OTHER FERROUS METALS.--Oils, grease, and fabrication lubricants shall be removed by solvent wash. Dirt, water soluble chemicals, and similar surface contamination shall be removed by detergent wash or steam cleaning. Mill scale and rust shall be removed by hand cleaning or abrasive blasting.

ALUMINUM AND OTHER NON-FERROUS METALS.--Oils, grease, and fabrication lubricants shall be removed by solvent wash. Dirt, water soluble chemicals, and similar surface contamination shall be removed by detergent wash.

GYPSUM BOARD.--Holes, cracks, and other surface imperfections shall be filled with joint compound or suitable filler prior to application of coatings. Taped joints and filled areas shall be hand sanded to remove excess joint compound and filler.

PREVIOUSLY COATED AND SHOP PRIMED SURFACES.--Dirt, oil, grease, or other surface contaminants shall be removed by water blasting, steam cleaning, or TSP wash. Minor surface imperfections shall be filled as required for new work. Mildew shall be removed by mildew wash. Chalking paint shall be removed by hand cleaning. The surfaces of existing hard or glossy coatings shall be abraded to dull the finish by hand cleaning or light abrasive blasting. Abrasive blasting shall not be used on wood or non-ferrous metal surfaces.

Chipped, peeling, blistered, or loose coatings shall be removed by hand cleaning, water blasting, or abrasive blasting. Bare areas shall be pretreated and primed as required for new work.

DEFINITIONS.--

DETERGENT WASH.--Removal of dirt and water soluble chemicals by scrubbing with a solution of detergent and water, and removal of all solution and residues with clean water.

HAND CLEANING.--Removal of dirt, loose rust, mill scale, excess base material, filler, aluminum oxide, chalking paint, peeling paint, or paint which is not firmly bonded to the surfaces by using hand or powered wire brushes, hand scraping tools, power grinders, or sandpaper and removal of all loose particles and dust prior to coating.

MILDEW WASH.--Removal of mildew by scrubbing with a solution of detergent, hypochlorite-type household bleach, and warm water, and removal of all solution and residues with clean water.

ABRASIVE BLASTING.--Removal of oil, grease, form release agents, paint, dirt, rust, mill scale, efflorescence, weak concrete, or laitance, by the use of airborne abrasives, and removal of loose particles, dust, and abrasives by blasting with clean air.

Abrasives shall be limited to clean dry sand, mineral grit, steel grit, or steel shot, and shall be graded to produce satisfactory results. Unwashed beach sand containing salt or silt shall not be used.

Abrasive blasting shall conform to the requirements of SSPC-SP6-85, Commercial Blast Cleaning, as defined in the Steel Structures Painting Council Manual.

Light abrasive blasting shall conform to the requirements of SSPC-SP7-85, Brush-Off Blast Cleaning, as defined in the Steel Structures Painting Council Manual.

SOLVENT WASH.--Removal of oil, grease, wax, dirt, or other foreign matter by using solvents, such as mineral spirits or xylol, or other approved cleaning compounds.

STEAM CLEANING.--Removal of oil, grease, dirt, rust, scale, or other foreign matter by using steam generated by commercial steam cleaning equipment, from a solution of water and steam cleaning compounds, and removal of all residues and cleaning compounds with clean water.

TSP WASH.--Removal of oil, grease, dirt, paint gloss, and other foreign matter by scrubbing with a solution of trisodium phosphate and warm water, and removal of all solution and residues with clean water.

WATER BLASTING.--High pressure, low volume water stream for removing dirt, light scale, chalking or peeling paint. Water blasting equipment shall produce not less than a 13 800 MPa minimum output pressure when used. Heated water shall not exceed 66°C. If a detergent solution is used, it shall be biodegradable and shall be removed from all surfaces with clean water.

PROTECTION.--The Contractor shall provide protective devices, such as tarps, screens or covers, as necessary to prevent damage to the work and to other property or persons from all cleaning and painting operations.

Paint or paint stains on surfaces not designated to be painted shall be removed by the Contractor at his expense and the original surface restored to the satisfaction of the Engineer.

APPLICATION.--

GENERAL.--Coatings shall be applied in accordance with the printed instructions and at the application rates recommended by the manufacturer to achieve the dry film thickness specified in these special provisions.

Mixing, thinning and tinting shall conform to the manufacturer's printed instructions. Thinning will be allowed only when recommended by the manufacturer.

Coatings shall be applied only when surfaces are dry and properly prepared.

Cleaning and painting shall be scheduled so that dust and other contaminants from the cleaning process will not fall on wet, newly coated surfaces.

Materials required to be coated shall have coatings applied to all exposed surfaces, including the tops and bottoms of wood and metal doors, the insides of cabinets, and other surfaces not normally visible from eye level.

APPLICATION SURFACE FINISH.--Each coat shall be applied to a uniform finish. Finished surfaces shall be free of surface deviations and imperfections such as skips, cloudiness, spotting, holidays, laps, brush marks, runs, sags, curtains, ropiness, improper cutting in, overspray, drips, ridges, waves, and variations in color and texture.

Each application of a multiple application finish system shall closely resemble the final color coat, except each application shall provide enough contrast in shade to distinguish the separate applications.

WORK REQUIRED BETWEEN APPLICATIONS.--Each application of material shall be cured in accordance with the coating manufacturer's recommendations before applying the succeeding coating. Enamels and clear finishes shall be lightly sanded, dusted, and wiped clean between applications.

Stain blocking primer shall be spot applied whenever stains bleed through the previous application of a coating.

TIMING OF APPLICATIONS.--The first application of the specified coating system shall be applied prior to any deterioration of the newly prepared surface. Metal surfaces shall be prepared and prime coated the same day that cleaning of bare metal is performed. Additional prime coats shall be applied as soon as drying time of the preceding coat permits.

Metal surfaces shall be prime coated within 12 hours of application of vinyl wash pretreatment.

Shellac sealer shall be allowed to dry at least 12 hours before applying the next coat.

Drying time between applications of water borne coatings shall be at least 12 hours.

APPLICATION METHODS.--Coatings shall be applied by brush, roller or spray. Rollers shall be of a type which do not leave a stippled texture in the paint film. Extension handles for rollers shall not be greater than 2 m in length.

If spray methods are used, surface deviations and imperfections such as, overspray, thickness deviations, lap marks, and orange peel shall be considered as evidence that the work is unsatisfactory and the Contractor shall apply the remainder of the coating by brush or roller, as approved by the Engineer.

DRY FILM THICKNESS.--

Vinyl wash pretreatment	0.007 mm to 0.13 mm, maximum.
Bituminous paint	0.1 mm, minimum.
Epoxy polyamide primer	0.1 mm, minimum.
Aliphatic polyurethane enamel	0.05 mm, minimum.
Other primers, undercoats, sealers, and coatings	As recommended by the manufacturer.

BACKPRIMING.--The first application of the specified coating system shall be applied to all wood surfaces (face, back, edges, and ends) of wood materials that are not factory coated, immediately upon delivery to the project site, except surfaces of interior finish woodwork that adjoin concrete or masonry shall be coated with one application of alkyd exterior wood primer before installation.

When clear or stain type coatings are required on millwork, trim, or paneling, varnish, reduced 25 percent by mineral spirits, shall be used for coating the back faces.

All primed metal surfaces in contact with concrete or concrete block exterior walls shall be coated with a bituminous paint on those surfaces in contact with the wall.

PATCHES IN PREVIOUSLY COATED SURFACES.--Where patches are made on surfaces of previously coated walls or ceilings, the entire surface to corners on every side of the patch shall be coated with a minimum of one application of the finish coat.

FINISHING MECHANICAL AND ELECTRICAL COMPONENTS.--Shop primed mechanical and electrical components shall be finish coated in accordance with the coating system entitled, "Shop Primed Steel." Louvers, grilles, covers, and access panels on mechanical and electrical components shall be removed and coated separately.

Interior surfaces of air ducts which are visible through grilles or louvers shall be coated with one application of flat black enamel, to limit of the sight line.

Exposed conduit, piping, and other mechanical and electrical components visible in public areas shall be painted.

Both sides and all surfaces, including edges and back of wood mounting panels for electrical and telephone equipment shall be finish coated before installing equipment.

CLEANING.--Upon completion of all operations, the coated surfaces shall be thoroughly cleaned of dust, dirt, grease, or other unsightly materials or substances.

Surfaces marred or damaged as a result of the Contractor's operations shall be repaired, at his expense, to match the condition of the surfaces prior to the beginning of the Contractor's operations.

COATING SYSTEMS.--The surfaces to be coated shall be as shown on the plans and as specified elsewhere in these special provisions. When a coating system is not shown or specified for a surface to be finish coated, the coating system to be used shall be as specified for the substrate material. The number of applications specified for each coating system listed herein is a minimum. Additional coats shall be applied if necessary to obtain a uniform color, texture, appearance, or required dry film thickness.

SYSTEM 1- ALUMINUM AND OTHER NON-FERROUS METALS.--

1 pretreat coat:	vinyl wash pretreatment
1 prime coat:	aluminum primer
2 finish coats:	acrylic, exterior enamel, semi-gloss

SYSTEM 2- GALVANIZED METAL.--

1 pretreat coat:	vinyl wash pretreatment
1 prime coat:	galvanized metal primer
2 finish coats:	acrylic, exterior enamel, semi-gloss

SYSTEM 3- GYPSUM BOARD.--

1 prime coat:	PVA wall sealer
2 finish coats:	acrylic, interior enamel, semi-gloss

SYSTEM 4- PREVIOUSLY COATED EXTERIOR SURFACES.--

1 prime coat :	alkyd exterior enamel undercoat
2 finish coats:	acrylic, exterior enamel, semi-gloss

SYSTEM 5- PREVIOUSLY COATED INTERIOR SURFACES.--

1 prime coat:	alkyd interior wood primer
2 finish coats:	acrylic, interior enamel, semi-gloss

SYSTEM 6- SHOP PRIMED STEEL.--

1 prime coat :	red oxide ferrous metal primer
2 finish coats:	alkyd, exterior enamel, semi-gloss

SYSTEM 7- STEEL AND OTHER FERROUS METALS.--

2 prime coats:	red oxide ferrous metal primer
2 finish coats:	alkyd, exterior enamel, semi-gloss

SYSTEM 8- WOOD, CLEAR FINISH.--

1 prime coat:	polyurethane varnish, satin, reduced 25 percent by mineral spirits
2 finish coats:	polyurethane varnish, satin

SYSTEM 9- WOOD, PAINTED.--

1 prime coat:	alkyd, exterior wood primer
2 finish coats:	acrylic, exterior enamel, semi-gloss

SYSTEM 10- WOOD, STAINED.--

2 finish coats:	semi-transparent, oil stain

SYSTEM 11- WOOD, TRANSPARENT STAIN FINISH.--

2 pretreat coats:	transparent stain
1 prime coat:	polyurethane varnish, satin, reduced 25 percent by mineral spirits
2 finish coats:	polyurethane, varnish, satin

COLOR SCHEDULE.--Colors shall be as shown on the plans.

12-9.04 FIBERGLASS REINFORCED PLASTIC PANELS

GENERAL.--This work shall consist of furnishing and installing fiberglass reinforced plastic (FRP) panels and trim molding in accordance with details shown on the plans and these special provisions.

SUBMITTALS.--Manufacturer's descriptive data, installation instructions, and finish options shall be submitted for approval.

Product descriptive data shall show the manufacturer's name and shall indicate conformance to these special provisions.

Installation instructions shall show the FRP panel manufacturer's recommended method of installation.

Finish options shall show the manufacturer's standard color palette for FRP panels and trim molding. Color shall be as shown on the plans. .

PRODUCTS.--

FRP panel.--

FRP panel shall be Class I flame-spread, minimum nominal thickness of 2 mm; Marlite, Class A/I FRP; Kemlite, Fire-X Glasbord; or equal.

Trim molding.--

Trim molding shall be manufacturer's standard vinyl molding with nailing flanges and a 9 mm deep channel of sufficient width to receive panels and sealant.

Adhesive and sealant.--

Adhesive and sealant shall be as recommended by the FRP panel manufacturer.

EXECUTION.--

INSTALLATION.--The FRP panels and trim molding shall be installed in accordance with the manufacturer's installation instructions.

Trim molding shall be nailed through the flange into solid wood backing. All nails shall be concealed by FRP panels in the completed installation. Trim shall be one continuous piece along each wall unless the wall length exceeds the manufacturer's standard trim length. If more than one piece is used on one wall, the pieces shall be approximately equal length, with no piece less than 1 m in length. All FRP panel edges shall be covered by a trim molding.

Panels shall be one continuous piece along each wall unless the wall length exceeds the manufacturer's standard panel length. If more than one panel piece is used on one wall, the pieces shall be approximately equal length, with no piece less than one meter in length.

CLEAN-UP.--Adjacent surfaces shall be protected from adhesive or sealant. Excess adhesive and sealant shall be removed as the installation progresses using a solvent or cleaning agent recommended by the FRP panel manufacturer.

12-9.05 ACOUSTIC CEILING FOR TOLL BOOTH

GENERAL.--This work consists of furnishing and installing suspended acoustic ceilings in accordance with the details shown on the plans and these special provisions.

Acoustic ceiling shall consist of acoustical insulation placed between furring strips.

The suspension system shall be designed to support the ceiling, lighting fixtures and other items supported by the suspended ceiling. Hangers shall be spaced at not more than 1220 mm on centers and shall be saddle tied or wrapped around main runners.

SUBMITTALS.--Manufacturer's descriptive data, installation instructions and 2 samples of the acoustic tile shall be submitted for approval.

PRODUCTS.--

Perforated panel.--

Perforated panel shall be 24 gage metal, 16 mm groove, 38 mm interval, 9 mm overall depth, galvanized steel panel conforming to ASTM Designation: A 525, G60, electrostatically painted before drying in infrared oven.

Lay-in panels.--

Lay-in panels shall be fiberglass reinforced polyester panels, 2 mm thick, chemically resistant, with a Class III/C flame spread.

Furring strips.--

Furring strips shall be stainless steel tee with approximately 25 mm wide exposed flanges and positive interlock between cross tie webs; wall molding shall be an angle with 19 mm exposed face.

Acoustic tile.--

Acoustic tile shall be 305 mm x 305 mm x 15 mm minimum thickness, square edges, nondirectional natural fissured texture, factory applied, washable, off-white vinyl latex finish. Tile shall conform to Federal Specification: SS-S-118B, Class A. Noise Reduction Coefficient (NRC) range shall be between 0.60 and 0.75.

EXECUTION.--

INSTALLATION.--The ceiling shall be leveled to within 3 mm in 3.6 m.

One extra unopened carton of acoustical panels shall be delivered to the Engineer.

SECTION 12-10. SPECIALTIES

12-10.01 LOUVERS

GENERAL.--This work consists of furnishing and installing louvers in accordance with the details shown on the plans and these special provisions.

Fire dampers shall conform to the requirements specified under "Ventilating and Air Conditioning Equipment and Systems" in Section 12-15, "Mechanical," of these special provisions.

SUBMITTALS.--Manufacturer's descriptive data and installation instructions shall be submitted for approval.

PRODUCTS.--

Louvers.--

Louvers shall be factory fabricated units of extruded aluminum alloy not less than 2 mm thick (12-gage) or galvanized steel sheet not less than 1.63 mm thick (16-gage) with standard "Z" type blades, and removable bronze 16 x 16 mesh insect screens mounted on the inside of the units.

Louvers shall have integral caulking strips and retaining beads.

EXECUTION.--

INSTALLATION.--Louvers shall be installed in accordance with the manufacturer's instructions. The completed louver installation shall be weather tight.

PAINTING.--Louvers shall be cleaned, prepared and painted in accordance with the requirements specified under "Painting" in Section 12-9, "Finishes," of these special provisions.

12-10.02 FIRE EXTINGUISHERS AND CABINETS

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing fire extinguishers with cabinets or mounting brackets in accordance with the details shown on the plans and these special provisions.

REFERENCES.--

General.--Fire Extinguishers shall conform to the requirements in National Fire Protection Association (NFPA) 10, “Portable Fire Extinguishers.”

SUBMITTALS.--

Product data.--Manufacturer's descriptive data and installation instructions shall be submitted for approval.

QUALITY ASSURANCE.--

Codes and standards.--Fire extinguishers shall be Underwriters Laboratories or Factory Mutual Laboratories approved for the type, rating and classification of extinguisher specified.

PART 2.- PRODUCTS

MANUFACTURER’S.--

Acceptable manufacturers,--Subject to contract compliance, manufacturers shall be J. L. Industries; Larsen’s Manufacturing; Potter-Roemer; or equal.

COMPONENTS.--

Fire extinguisher.--
Fire extinguisher shall be fully charged, multi-purpose dry chemical type, with charge indicator, hose and nozzle, and attached service record tag. Fire extinguisher shall be of the capacity and type rating shown on the plans.

Fire extinguisher cabinet.--
Fire extinguisher cabinet shall be factory fabricated, constructed of steel with a clear plastic panel in a steel door frame, and shall have a baked enamel finish. Color to be selected by the Engineer from the manufacturer’s standard colors.

Fire extinguisher cabinet shall be fully recessed as shown on the plans.

PART 3.- EXECUTION

INSTALLATION.--

General.--Fire extinguishers shall be installed in locations and at mounting heights indicated, or if not indicated, at heights to comply with applicable regulations of governing agencies.
Fire extinguisher mounting brackets and cabinets shall be attached to structure, square and plumb, in accordance with the manufacturer's recommendations.

IDENTIFICATION.--

Cabinet-mounted.--Extinguishers in cabinets shall be identified with letter spelling “FIRE EXTINGUISHER” applied to the cabinet door. Letter size, styles, and color shall be selected by the Engineer from manufacturer’s standard arrangements.

SERVICING.--

General.--Fire extinguishers shall be service, charged, and tagged not more than 5 days prior to contract acceptance.

12-10.03 TOLL BOOTH FLOOR MAT

GENERAL.--This work shall consist of furnishing and installing floor mats in accordance with the details shown on the plans and these special provisions.

SUBMITTALS.--Samples of floor mat and installation instructions shall be submitted for approval.

PRODUCTS.--

Floor mat.--

Floor mats shall be rubber and shall be the manufacturer's best grade. The thickness of the rubber shall be not less than 12 mm. The rubber shall be grease and oil proof, and chemical and acid resistant. Surfaces shall be slip resistant. Floor mats shall be Abort, Catalog No. 1122; Airstrip, Series 900, Cactus Type 2501 N, Golden State Mat Manufacturing Co., No. 800 "Cushion Mat"; or equal.

EXECUTION.--

Floor mats shall be installed at the locations shown on the plans.

12-10.04 BARRIER GATES

GENERAL.--This work shall consist of fabricating and installing barrier gates, with white and red reflective tape striping, for toll booth islands in accordance with the details shown on the plans and these special provisions.

Motor operated barrier gate shall be a complete electrical powered motor operated barrier gate.

SUBMITTALS.--Submittals for barrier gates, including working drawings and installation details, shall conform to the requirements specified under "Submittals," in Section 12-1, "General Requirements," in the special provisions.

PRODUCTS.--

Barrier gate.--

Barrier gate arm shall be constructed of fiberglass, aluminum or other lightweight material.

Motor.--

The motor shall be 120-volt, single-phase, 60 hertz, 1/2 HP.

Reflective tape shall be red and white in color and shall conform to Federal Highway Administration Specification FP-79, Section 718.01 (c).

Transmission.--

The transmission shall be a self-contained unit, totally enclosed in an oil and dust-tight steel housing. The design shall provide for uniform acceleration and deceleration of the gate arm travel, with maximum velocity at the midpoint of the travel.

Gate operator switch.--

Gate operator switch shall be as specified under 12-16, 'Electrical," in these special provisions.

Bars, bolts, plates and shapes.--

Bars, bolts, plates, and shapes shall be as specified in Section 12-5, "Building Miscellaneous Metal," in these specifications.

EXECUTION.--

INSTALLATION.--

The gate shall be electrically controlled, to raise or lower or stop, from a remote location as shown on the plans. The total time for the gate to raise or lower shall not be less than 4 seconds nor more than 6 seconds. The motor, reversing started and au other ewquioment required for arm operation shall be housed in a completely enclosed weatherproof enclosure. the

enclosure shall be designated to prevent unauthorized entry by the use of a padlock or built-in device. Two sets of keys and the padlock or locking device shall be furnished.

Overload heaters sensing totol motor current shall provided motor overload protection. A thermostatically controlled auxiliary space heater shall be installed inside the motor operation gate enclosure to prevent condensation on the motor. The motor shall be equipped with a solenoid and spring- controlled brake which shall assure positive quick acting stoping of gate arm.

Manual operation of the gate shall be provided in the event of a power failure or internal malfunction.

All gears shall run in an oil bath.

A handcrank shall be provided, with sufficient gear reduction to permit manually raising or lowering the gate arm without the use of excessive force.

The reversing starter shall be adequate rating for the motor supplied, and shall be located within the gate mechanism housing. A disconnect switch shall be provided within the gate mechanism housing to disconnect all power from the equipment. Limit switches shall be provided at the extreme ends of arm travel to automatically stop the gate arm, and in the down position to disconnect the obstruction control circuit. Limit switches shall prevent the gate arm from being reversed in direction before completing any cycle. Limit switch shall be rated not less than 10 amperes at 120 volts.

An obstruction switch shall be provided on the gate arm to automatically change direction in the event of an obstruction on its bottom edge.

Construction.--Weight of counterbalance shall be adjusted so that the gate will hold steady in wither open or closed position

Gate arm shall be painted using two undercoats of wood primer and two finish coats of sign post enamel and striped with 6 inch wide alternate red and white diagonal stripes of reflective tape.

The operating spring shall have proper strength and tension for correct operation of the gate.

Miscellaneous parts shall be of standard manufacture suitable for the purpose intended.

Metal parts shall be cleaned by wire brushing, mineral spirits, or other approved means and painted as specified for painting steel.

SECTION 12-11. AND 12-12. (BLANK)

SECTION 12-13. SPECIAL CONSTRUCTION

12-13.01 PRE-ENGINEERED CONCRETE ENCLOSURE

GENERAL.--This work shall consist of designing, fabricating, furnishing and erecting a pre-engineered concrete enclosure in accordance with the details shown on the plans and these special provisions.

Pre-engineered concrete enclosure shall provide a totally integrated weatherproof unit and shall include structural steel framing and decking, concrete floor, wall and roof panels, high strength bolts, fasteners, sealants, louvers, flashings, and such other parts, elements or components of the frame and outside walls and roof, not mentioned, which are required for the complete construction of a rigid, waterproof building.

The building dimensions shown on the plans are minimal and may be increased to accommodate manufacturer's standards. No additional compensation will be allowed for any changes required by such increased dimensions.

DESIGN CRITERIA.--The building and the building design shall be in accordance with the applicable requirements in Uniform Building Code, including the modification to loads or stresses indicated therein.

The building shall be designed to support the weight of the building components, including mechanical equipment located on the roof, and the live load, wind load, seismic load, and other loads shown in the design notes on the plans.

The enclosure shall be engineered to withstand any stresses caused during transportation and erection. Door, walls, and roof panels shall be reinforced to provide a rigid module. Deflection under specified loadings shall not exceed 6 mm per 3 m span.

Weathertight features of the design shall include closures and continuous seals at panel ends and sides, flashing, and sealing.

SUBMITTALS.--Complete working drawings, erection instructions and drawings, design calculations, shall be submitted for approval. Submittals shall be approved prior to the start of fabrication.

Working drawings, erection drawings and design calculations shall be stamped and signed by an engineer who is registered as a Civil or Structural Engineer in the State of California. The expiration date of the registration shall be shown.

Working drawings shall show the size, thickness, shape, configuration, type, grade, class and description if any, of all materials used in the building. Joint details, connection and anchoring details and details of all temporary and permanent supports shall be shown.

Calculations for the design of the building shall include a list of applied loads and load combinations with the resulting forces and stresses.

If the design calculations consist of computerized or tabulated calculations, the values pertaining to the building design for this project shall be identified, described or indexed in such a manner that a design check can be performed.

Proposed electrical equipment layout with conduit riser diagrams, required penetrations for conduits, and penetration sealing method shall be shown on the working drawings.

CERTIFICATES OF COMPLIANCE.--Certificates of Compliance shall be furnished for the pre-engineered concrete enclosure in accordance with the requirements specified in Section 6-1.07, "Certificates of Compliance," of the Standard Specifications.

DELIVERY, STORAGE AND HANDLING.--The pre-engineered concrete enclosure shall be transported and handled in such a manner as to prevent damage. Building components shall be stored off the ground.

PRODUCTS.--

Wall, floor, and roof panels.--

Wall, floor, and roof panels shall be concrete having a minimum compressive strength of 35 KPa 5000 psi in 28 days. Wall panels shall have a vertical 19 mm architectural fin. Reinforcing steel shall conform to ASTM Designation: A 615/A 615M, Grade 420.

Louvers.--

Louvers shall be as specified in Section 12-10, "Specialties," and Section 12-10, "Specialties," of these special provisions.

Hinged doors and frames and finish hardware.--

Hinged doors and frames and finish hardware shall be as specified in Section 12-8, "Doors and Windows," of these special provisions.

EXECUTION.--

FABRICATION.--Building frame components shall have all the bolt holes necessary for erecting, assembling and fastening made at the factory.

Bolt holes shall be either punched full size, drilled full size, subpunched and reamed, or subdrilled and reamed. The finished holes shall be cylindrical, perpendicular to the plane of the connection and shall be not more than 2 mm larger than the nominal diameter of the bolt. Mispunched or misdrilled holes shall not be corrected by welding unless approved by the Engineer.

FRAMING ERECTION.--Framing shall be erected plumb and true in accordance with the requirements of the AISC specifications and shall be secured rigidly in place in accordance with the details shown on the approved working drawings.

Temporary bracing shall be installed during erection to hold the framing plumb and true and in a safe position until sufficient permanent bracing and construction is in place to provide full stability. All permanent bracing shall be secured in place before any sustained permanent loads are applied to the framing system. Bracing shall be positioned to clear electrical work and openings for accessories.

Openings for accessories shall be plumb and level, of the correct dimensions, located approximately where shown on the plans and reinforced to support the loads of the accessories.

Cutting, welding or altering of the framing members at the site shall not be allowed without written approval of the Engineer.

ROOF, FLOOR, AND WALL PANEL INSTALLATION.--Roof, floor, and wall panels shall be installed in accordance with the details shown on the approved working drawings.

Panels shall be adjusted to final position and brought to bear on the structural supports prior to fastening. Side laps shall be as recommended by the building manufacturer and shall be located over structural supports.

Closures shall be installed and sealant shall be applied as recommended by the manufacturer to prevent weather penetration.

Vents and louvers shall be installed with the roof and wall panels and shall be weathertight.

The completed installation shall be without defacements, bends, sags, dimples, undulations, or other deformations; shall be free of vibration, rattles and noise due to wind or thermal movement; and shall be weathertight.

SEALING JOINTS.--Joints shall be sealed as shown on the plans. Sealant shall be applied in accordance with manufacturer's recommendations. Applications shall be a continuous operation for the length of the joint. Following the application of the sealant, the joint shall be tooled using a tool similar to that used to produce concave masonry joints. The joint shall remain undisturbed after tooling for not less than 48 hours.

SEALING CONDUIT PENETRATIONS.--Penetrations for conduits shall be permanently grout filled and sealed watertight after installation of the conduits.

SECTION 12-14. BLANK

SECTION 12-15. MECHANICAL

12-15.01 MECHANICAL WORK

GENERAL.--

Scope.--This work shall consist of performing mechanical work in accordance with the details shown on the plans and these special provisions.

Mechanical work shall include furnishing all labor, materials, equipment and services required for providing heating, ventilating, air conditioning, vacuum and Carbon Monoxide (CO) monitoring systems. .

Earthwork, foundations, sheet metal, painting, electrical, and such other work incidental and necessary to the proper installation and operation of the mechanical work shall be in accordance with the requirements specified for similar type work elsewhere in these special provisions.

System layouts are generally diagrammatic and location of equipment is approximate. Exact routing of pipes, ducts, etc., and location of equipment is to be governed by structural conditions and obstructions. Equipment requiring maintenance and inspection is to be readily accessible.

Penetrations shall be flashed and sealed watertight in accordance with the requirements specified under "Sheet Metal Flashing" in Section 12-7, "Thermal and Moisture Protection," of these special provisions.

SUBMITTALS.--

Product data.--A list of materials and equipment to be installed, manufacturer's descriptive data, and such other data as may be requested by the Engineer shall be submitted for approval.

Manufacturer's descriptive data shall include complete description, performance data and installation instructions for the materials and equipment specified herein. Control and wiring diagrams, and component layout shall be included where applicable.

Manufacturer's descriptive data shall be submitted for the following:

air conditioner
registers
slot diffuser
exhaust fan
heaters
insulation
insulation covers
damper
thermostat
timer
oval insulated duct

CLOSEOUT SUBMITTALS.--

Operation and maintenance manuals.--Prior to the completion of the contract, 3 identified copies of the operation and maintenance instructions with parts lists for the equipment specified herein shall be delivered to the Engineer at the jobsite. The instructions and parts lists shall be indexed and bound in a manual form and shall be complete and adequate for the equipment installed. Inadequate or incomplete material shall be returned. The Contractor shall resubmit adequate and complete manuals at no expense to the State.

Operation and maintenance manuals shall be submitted for the following equipment:

air conditioner
exhaust fan
heaters

QUALITY ASSURANCE.--

Codes and standards.--Mechanical work, including equipment, materials and installation, shall conform to the California Building Standards Code, Title 24, and to the California Code of Regulations, Title 8, Chapter 4, Division of Industrial Safety (DIS).

WARRANTY.--

Warranties and guarantees.--Manufacturer's warranties and guarantees for materials or equipment used in the work shall be delivered to the Engineer at the jobsite prior to acceptance of the contract.

12-15.02 PIPE, FITTINGS AND VALVES

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing pipes and fittings in accordance with the details shown on the plans and these special provisions. Pipe and fittings shall include such plumbing and piping accessories and appurtenances, not mentioned, that are required for the proper installation and operation of the plumbing and piping systems.

The pipe and fitting classes and material descriptions shall be as specified herein. No change in class or description shall be permitted without written permission from the Engineer.

QUALITY ASSURANCE.--

Codes and standards.--Pipe and fittings shall be installed in accordance with the requirements in the latest edition of the Uniform Plumbing Code, the manufacturer's recommendations and the requirements specified herein.

PART 2.- PRODUCTS

MATERIALS.--

PIPE AND FITTINGS --

Class Description

P3.--

Polyvinyl chloride (PVC) standard weight pipe and fittings, Schedule 40, conforming to ASTM Designation: D 1785. Pipe shall meet or exceed requirements of National Sanitation Foundation Standard No. 14. Pipe shall have bell ends conforming to ASTM Designation: D 2672. For pipe sizes NPS 3 and smaller, plain end pipe with solvent welded fittings conforming to ASTM Designation: D 2241, may be used.

Rigid non-metallic conduit shall be Schedule 40, high impact, nonconducting, self-extinguishing polyvinyl chloride (PVC) rigid non-metallic conduit for direct underground burial.

Couplings shall be PVC, socket type or thread on one end and socket type on the other end as required for the particular application.

Terminal adapters for adapting PVC conduit to boxes, threaded fittings or metallic conduit system shall be PVC adapters with threads on one end and socket type on the other.

P5.--

Polyvinyl tubing shall be the size and type to match the existing Carbon Monoxide (CO) system tubing.

PART 3.- EXECUTION

INSTALLATION.--

INSTALLATION OF PIPES AND FITTINGS.--

Pipe and fittings.--Pipe and fittings shall be installed in accordance with the following designated uses:

Designated Use	Pipe and Fitting Class
Vacuum system	P3
CO system conduit	P4
CO system sample tube	P5

Installing piping.--Piping shall be installed generally level, free of traps and bends, and arranged to conform to the building requirements.

Piping shall be installed parallel to walls. All obstructions shall be cleared, headroom preserved and openings and passageways kept clear whether shown or not. Piping shall not interfere with other work.

Where pipes pass through exterior walls, a clear space around pipe shall be provided. Space shall be caulked water tight with silicone caulk.

Pipe sleeves.--The Contractor shall provide sleeves, inserts and openings necessary for the installation of pipe and fittings. Damage to surrounding surfaces shall be patched to match existing.

PVC pipe sleeves shall be provided where each pipe passes through concrete floors, footings, walls or ceilings. Inside diameter of sleeves shall be at least 20 mm larger than outside diameter of pipe. Sleeves shall be installed to provide at least 10 mm space all around pipe the full depth of concrete. Space between pipes and pipe sleeves shall be caulked watertight.

Cutting pipe.--All pipe shall be cut straight and true and the ends shall be reamed to the full inside diameter of the pipe after cutting.

Damaged pipe.--Pipe that is cracked, bent or otherwise damaged shall be removed from the work.

Cleaning and closing pipe.--The interior of all pipe shall be cleaned before installation. All openings shall be capped or plugged as soon as the pipe is installed to prevent the entrance of any materials. The caps or plugs shall remain in place until their removal is necessary for completion of the installation.

Securing pipe.--Pipe in the buildings shall be held in place by iron hangers, supports, pipe rests, anchors, sway braces, guides or other special hangers. Material for hangers and supports shall be compatible with the piping or neoprene isolators shall be used. Allowances shall be made for expansion and contraction. Horizontal and vertical piping shall be securely supported and braced to prevent swaying, sagging or flexing of joints.

Hangers and supports.--Hangers and supports shall be selected to withstand all conditions of loading to which the piping and associated equipment may be subjected and within the manufacturer's load ratings. Hangers and supports shall be spaced and distributed so as to avoid load concentrations and to minimize the loading effect on the building structure.

Hangers and supports shall be sized to fit the outside diameter of pipe or pipe insulation. Hangers shall be removable from around pipe and shall have provisions for vertical adjustment after erection. Turnbuckles may be used.

Materials for holding pipe in place shall be compatible with piping material.

Hanger rods shall be provided with locknuts at all threaded connections. Hanger rods shall be sized as follows:

NPS Designator	Minimum Hanger Rod Diameter
1/2 to 2	10 mm
2 1/2 to 3 1/2	13 mm
4 to 5	16 mm
6	19 mm

FIELD QUALITY CONTROL.--

Testing.--The Contractor shall test piping at completion of roughing in, before backfilling, and at other times as directed by the Engineer.

The system shall be tested as a single unit, or in sections as approved by the Engineer. The Contractor shall furnish necessary materials, test pumps, instruments and labor and notify the Engineer at least 3 working days in advance of testing. After testing, the Contractor shall repair all leaks and retest to determine that leaks have been stopped. Surplus water shall be disposed of after testing as directed by the Engineer.

The Contractor shall take precautions to prevent joints from drawing while pipes and appurtenances are being tested. The Contractor shall repair damage to pipes and appurtenances or to other structures resulting from or caused by tests.

General tests.--All piping shall be tested after assembly. Systems shall show no loss in pressure or visible leaks.

The Contractor shall test systems according to the following schedule for a period of not less than 4 hours:

Test Schedule		
Piping System	Test Pressure	Test Media
Vacuum	150 kPa	Air
CO Test	70 kPa	Air

12-15.03 MECHANICAL INSULATION

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing mechanical insulation in accordance with the details shown on the plans and these special provisions.

Duct insulation shall be installed on all rigid ductwork installed in concealed non-conditioned spaces including all duct to the toll booth.

Duct liner shall be installed in all rectangular ductwork installed in exposed non-conditioned spaces and in exterior locations. Plenum liner shall be installed in all plenums in non-conditioned spaces or in walls facing a non-conditioned space.

QUALITY ASSURANCE.--

Codes and standards.--Mechanical insulation shall conform to California State Energy Commission regulations and, where applicable, shall meet American Society of Testing and Materials (ASTM) standards.

All materials shall bear the label of the Underwriters Laboratory (UL) or other approved testing laboratory indicating that the materials proposed for use conform to the required fire hazard ratings.

PART 2.- PRODUCTS

MATERIAL.--

General.--

Duct insulation and wrapping material, including adhesives and jackets, located within buildings and tunnels shall be certified to have a composite flame spread of not more than 25 and smoke development rating of not more than 50 when tested in accordance with ASTM Designation: E 84.

External duct insulation.--

External duct insulation shall be 38 mm thick or as shown on the plans, 0.5 kg density glass-fiber blanket type. Material and coatings shall be fire resistive and shall be approved by the State Fire Marshal. External duct insulation shall be Fiberglas, Type PF-336; Ultralite, No. 100; Pittsburgh Plate Glass, Superfine; Johns-Manville, Microlite; Silvercote, Silvercel; or equal.

Plenum and duct liner.--

Plenum and duct liner shall be 25 mm minimum thickness. Material and coatings shall be fire resistive and shall be approved by the State Fire Marshal. Liner shall be Gustin-Bacon, Ultra-Liner duct insulation; Owens-Corning Fiberglas, Type CE; Gustin-Bacon, coated insulation Board No. 90-A; Owens-Corning Fiberglas 0.7 kg density coated flexible duct liner; Johns-Manville, MicroBar, or 0.7 kg density coated Microlite; Pittsburgh Plate Glass, Superfine 0.7 kg density coated interior duct insulation; or equal.

Adhesive.--

Adhesive shall be non-flammable type: Benjamin Foster Company, No. 85-20 Spark Safe; Goodloe E. Moore Company, Tuff Bond No. 6; Permacel, No. PA-3l0; 3M, No. 38 Insulation Adhesive; Swift's, No. 7228 brush type or No. 7336 spray type; Chicago Mastic, 17-461; or equal.

PART 3.- EXECUTION

INSTALLATION.--

General.--Insulation materials shall be neatly installed with smooth and even surfaces, jackets drawn tight and smoothly cemented down.

Insulation material shall not be installed until all surfaces to be covered are tested for leaks, cleaned and dried, and foreign materials, such as rust, have been removed.

Duct insulation.--Ragged edges shall be repaired or taped. Coverings shall be neatly finished at joints and edges. Each joint shall have a 50 mm minimum lap.

Where transitions are made between externally covered ducts and lined ducts, the lined duct shall be overlapped 200 mm with external covering.

Insulation shall be flush with but not cover control devices, damper controls or access doors.

Before insulation is wrapped around concealed ducts, an adhesive shall be spot applied at a maximum of 100 mm centers on each side of the ducts to prevent sagging of the insulation. Insulation shall be wrapped entirely around the ducts and shall be wired securely in place with No. 16 copper clad wire, metal bands at least 10 mm wide or plastic ties. Supports shall be spaced a maximum of 300 mm on centers. Metal bands shall be installed with the use of a banding machine. Seams in the insulation shall be taped.

The finished insulation covering shall be even and level and shall not contain humps.

Plenum and duct liner.--Plenums and exposed ducts shall be lined with plenum and duct liner. Plenums and ducts shall be sized to provide the clear inside dimensions shown on plans after the liner is installed .

The insulation shall be applied with coated side exposed to air stream to prevent surface erosion.

The lining shall be fastened in place with adhesive and with studs with washers spaced a maximum of 500 mm on center each way.

Applying adhesive.--The adhesive shall be liberally applied over entire interior surfaces of ducts or plenums.

12-15.04 HEATING, VENTILATING AND AIR CONDITIONING EQUIPMENT AND SYSTEMS

PART 1.- GENERAL

Scope.--This work shall consist of furnishing, installing and testing heating, ventilating and air conditioning (HVAC) equipment and systems in accordance with the details shown on the plans and these special provisions.

The performance rating and electric service of the HVAC equipment shall be as shown on the plans.

Temperature controls.--Thermostats, relays, time switches, and other sensor type control devices required for this work shall be furnished and installed by the supplier of the heating, ventilating and air conditioning equipment. All temperature control wiring shall be furnished and installed in accordance with the requirements specified in Section 12-16, "Electrical," of these special provisions.

Codes and standards.--Equipment and systems shall conform to California State Energy Commission Regulations and, where applicable, shall be American Refrigeration Institute (ARI), Sheet Metal and Air Conditioning Contractors National Association, Inc. (SMACNA), and Air Movement and Control Association (AMCA) approved for performance ratings and application shown on the plans.

Any appliance for which there is a California standard established in the Appliance Efficiency Standards may be installed only if the manufacturer has certified to the Commission, as specified in those regulations, that the appliance complies with the applicable standards for that appliance. Space conditioning equipment may be installed only if the manufacturer has certified that the equipment meets or exceeds all applicable efficiency requirements listed in the Energy Efficiency Standards.

PART 2.- PRODUCTS

HEATING AND COOLING UNITS.--

Air conditioner unit (packaged).--

Air conditioner unit, (packaged),shall be standard, commercial quality, single package, curb pump mounted unit with weatherproof acoustically lined cabinet. The cabinet shall have convenient access panels and a baked-on enamel finish.

Compressor shall be hermetically sealed unit, vibration isolated, with quick-start components, short cycling protection, pressure relief valve, high and low pressure switches, liquid-line filter-dryer and crankcase heater.

The fan and fan motor shall provide the specified air flow, with wet coil, against the external static pressure as noted on the plans.

Motors shall have integral thermal overload protection.

Electric wall heater.--

Electric wall heater shall be forced air, recessed type or flush mounted as shown on the plans, and shall be equipped with a grille and integral thermostat.

FANS AND VENTILATORS.--

Fan (wall mounted).--

Fan shall be wall mounted, propeller type, AMCA certified, direct drive, metal blades, metal housing, venturi orifice, and safety grills. Fan shall be constructed in accordance with the requirements of the latest of AMCA Publication 99, "Standards Handbook."

Fan motor and fan assembly shall be isolated from base with rubber vibration isolators. Fan motor shall be single phase, continuous duty with integral thermal overload protection.

Wall fan shall be supplied with wall mount collar, motor side guard, backdraft damper and weatherhood. Weatherhood shall be galvanized steel with bird screen. All parts shall be supplied by the fan manufacturer.

HVAC CONTROLS.--

Thermostat.--

Thermostat shall be low voltage type, single set point range internally adjustable from 4°C to 30°C, and provided with a blank cover.

Time switch.--

Time switch shall be one-hour, spring-wound, "OFF" type time switch without a "HOLD" feature. Time switch shall be Intermatic, Type F60M; Tork, A500 Series; or equal.

AUXILIARY HVAC COMPONENTS.--

Unless specified herein, all components shall be sized and have the characteristics as shown on the plans.

Rigid ductwork.--

Rigid ductwork shall be galvanized or stainless steel sheet metal as shown on the plans conforming to the latest edition of the SMACNA "Low Velocity Duct Construction Standards." Galvanized steel shall be cleaned by washing with mineral spirit solvent sufficient to remove any oil, grease or other materials foreign to the galvanized coating.

Double wall insulated duct shall be used when shown on the plans.

Duct supports.--

Duct supports shall be hot-dip galvanized steel.

Supply register.--

Wall supply register shall be double-deflecting adjustable type, with vertical face bars and horizontal rear louvers, steel with oven baked-on enamel bone white finish or extruded aluminum, flanged frame with sponge or felt gasket; Hart and Cooley T62; Air Mate 240-HO or equal.

Return register.--

Wall return register shall be single deflecting type, with horizontal adjustable louvers, steel with oven baked-on enamel bone white finish or extruded aluminum, flanged frame with sponge or felt gasket; Hart and Cooley, T70; Air Mate, 200-HO; or equal.

Slot diffuser.--

Slot diffuser shall be extruded aluminum construction with volume and throw pattern adjusters. The unit shall install flush with the plenum top and use concealed fasteners. The diffuser shall have mitered ends as shown on the plans.

Motor operated damper.--

Motor operated damper shall be opposed metal blade type, motor operable including motor linkage.

Balance damper.--

Balance damper shall be butterfly type, 1.52 mm (16-gage) minimum galvanized steel blade, end bearings with steel shaft and locking and indicator operator. Balance damper shall be Ventlock, Young, Anemostat, or equal.

Air filter (for HVAC units).--

Air filter shall be permanent metal viscous impingement type, constructed of aluminum or galvanized steel, 50 mm minimum thickness and be approved for Class 2 use. Filter shall have a minimum efficiency rating of 50 percent as determined when tested in accordance with ASHRAE Test Standard 52. Filter shall be mounted in 1.52 mm (16-gage) galvanized steel holding frames. Two cans of recharging adhesive shall be provided with the filter and shall be nearly odorless, have a high flash point, rapid wetting characteristics, dye tracer and be water soluble. Filter shall be Airspan, Type AF, Eco-Air Products, Inc., Type HIA; Snyder General, Type AAF; or approved equal.

Refrigerant and condensate drain piping.--

Refrigerant and condensate drain piping shall be rigid, Type L copper tubing with brazed solder fittings. The suction line shall be insulated, with vapor barrier and shall be weatherproofed for exterior installation. Factory sealed tubing shall not be used.

PART 3.- EXECUTION

INSTALLATION.--

Heaters.--Heaters shall be installed in such a manner as to insure adequate clearance.
Wall heater shall be installed so that the bottom of the heating elements are at least 457 mm above the floor.

Condensate drains.--Air conditioning units shall be provided with condensate drain trap and piping.

Mounting heights.--Thermostats and time switches shall be installed as shown on the plans.

Air outlets.--Volume dampers shall be furnished and installed for all diffusers. Blocking shall be provided on all sides of air outlets between ceiling or wall joists. Collars shall be supplied for all outlets and shall be taped and sealed in place.

Access door.--Access doors shall be provided in rigid ducts and plenums for access to volume dampers, fire dampers and control devices located within such ductwork; and shall be provided at such other locations as shown on the plans.

Ducts and vents.--Ductwork within the building shall be installed to clear lighting fixtures, doors, windows and other obstructions. Ductwork shall preserve head room and shall keep openings and passageways clear whether shown on plans or not.

Ductwork shall be installed and braced according to the latest edition of the SMACNA "HVAC Duct Construction Standards."

Slopes in sides at transitions shall be approximately one to five. The ductwork system shall not contain abrupt changes or offsets of any kind unless otherwise shown on the plans.

Where ducts pass through walls, floors or ceilings, galvanized sheet metal or steel angle collars shall be installed around the ducts.

Duct sections shall be connected by beaded sleeve-type couplings using joint sealer as recommended by the duct manufacturer. Duct sections shall be mechanically fastened with pop rivets or sheet metal screws and sealed with mastic or insulated, reinforced silver tape.

Flexible connections shall be provided at both inlet and outlet of fan coil and ventilating units.

Sheet metal plenums shall be adequately braced and supported from the floor or structure with structural steel angles to prevent sagging, flexing and vibration.

All standing seams and transverse joints of supply, return and exhaust ducts and seams around plenums, fan and coil housings shall be sealed with sealant and taped.

FIELD QUALITY CONTROL.--

Pre-test requirements.--Before starting or operating systems, equipment shall be cleaned and checked for proper installation, lubrication and servicing.

In each system, at least one air path, from fan to final outlet, shall have all balance dampers open. The final air quantities shall be achieved by adjusting the volume dampers or the fan RPM.

Final adjustments and balancing of the systems shall be performed in such a manner that the systems will operate as specified and as shown on the plans.

The Contractor shall replace or revise any equipment, systems or work found deficient during tests.

All automatic operating devices which are pertinent to the adjustment of the aforementioned air systems shall be set and adjusted to deliver the required quantities of air and at temperatures specified by the Engineer. All control work shall be done in collaboration with the control manufacturer's representative.

Project completion tests.--The Engineer shall be notified at least 3 working days in advance of starting project completion tests.

The project completion tests shall consist of the following:

1. Air Systems.--All air systems shall be tested and balanced to the conditions set forth on the plans and in these special provisions. This work shall be performed by an Associated Air Balance Council (AABC) or National Environmental Balancing Bureau (NEBB) certified contractor. The air systems include, but are not necessarily limited to, the following:

    a. Supply air systems
    b. Return air systems
    c. Exhaust air systems

2. Operational Data.--The tests shall include operation of the heating, cooling, and ventilating systems for not less than two 8-hour days, each system shall operate at not less than 90 percent of their full specified capacities.

    The required data shall be accurately measured. The data shall be measured during one operational cycle in the presence of the Engineer and shall be submitted for approval.
    The following data shall be measured and tabulated:

    a. Ambient temperatures and conditions, °C
    b. Supply and return air quantities, m^3/sec, each room
    c. Thermostat set point, °C
    d. Air temperatures at room center, °C
    e. Fan motor amperages and voltages
    f. System static pressures, Pa

SECTION 12-16. ELECTRICAL

12-16.01 ELECTRICAL WORK

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of performing electrical work in accordance with the details shown on the plans, these special provisions, the Standard Plans and Section 86, "Signals, Lighting and Electrical Systems," of the Standard Specifications.

Electrical work shall include furnishing all labor, materials, equipment and services required to construct and install the complete electrical system shown on the plans and the work of installing electrical connections for the thermostats, motors, and controls specified elsewhere in these special provisions.

System layouts are generally diagrammatic and location of equipment is approximate. Exact routing of conduits and other facilities and location of equipment is to be governed by structural conditions and other obstructions, and shall be coordinated with the work of other trades. Equipment requiring maintenance and inspection shall be located where it is readily accessible for the performance of such maintenance and inspection.

Existing Electrical Facilities.—The location of the existing electrical facilities shown on the plans are approximate only. The exact location of equipment and the extent of the wiring shall be determined by the Contractor. All circuits to existing toll booths shall remain energized. No power outage will be permitted unless authorized by the Engineer. The Contractor shall obtain a safety circuit clearance daily from the Engineer before any work begins on an existing circuit. The appropriate electrical circuits will be disconnected by State personnel only. “Men at Work” signs shall be posted at the disconnect by the Contractor.

Related work.--Earthwork, foundations, sheet metal, painting, mechanical and such other work incidental to and necessary for the proper installation and operation of the electrical work shall be done in accordance with the requirements specified for similar work elsewhere in these special provisions.

CLOSEOUT SUBMITTALS.--

Operation and maintenance manuals.--Prior to the completion of the contract, 3 identified copies of the operation and maintenance instructions with parts lists for the equipment specified herein shall be delivered to the Engineer at the jobsite. The instructions and parts lists shall be in a bound manual form and shall be complete and adequate for the equipment installed. Inadequate or incomplete material will be returned. The Contractor shall resubmit adequate and complete manuals at no expense to the State.

Manuals shall be submitted for the following equipment:

Fire Alarm Control Panel
Security Monitor Panel

QUALITY ASSURANCE.--

Codes and standards.--All work performed and materials installed shall be in accordance with the National Electrical Code; the California Building Standards Code, Title 24, Part 3, "California Electrical Code," and the California Code of Regulations, Title 8, Chapter 4, "Electrical Safety Orders," and all local ordinances.

Warranties and guarantees.--Manufacturer's warranties and guarantees for materials or equipment used in the work shall be delivered to the Engineer at the jobsite prior to acceptance of the contract.

TESTING.--

After the electrical system installation work has been completed, the electrical system shall be tested in the presence of the Engineer to demonstrate that the electrical system functions properly. The Contractor shall make necessary repairs, replacements, adjustments and retests at his expense.

12-16.02 BASIC MATERIALS AND METHODS

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing conduits, conductors, fittings, and wiring devices in accordance with the details shown on the plans and these special provisions.

Conduits, conductors, fittings, and wiring devices shall include those accessories and appurtenances, not mentioned, that are required for the proper installation and operation of the electrical system.

Related work.--Roof penetrations shall be flashed and sealed watertight conforming to the requirements specified under "Sheet Metal Flashing" in Section 12-7, "Thermal and Moisture Protection," of these special provisions.

Where conduits pass through fire rated wall, floor or ceiling assemblies, the penetrations shall be protected in accordance with the requirements specified under "Through-Penetration Firestopping" in Section 12-7, "Thermal and Moisture Protection," of these special provisions.

SUBMITTALS.--

Product data.--A list of materials and equipment to be installed, manufacturer's descriptive data, and such other data as may be requested by the Engineer shall be submitted for approval.

Manufacturer's descriptive data shall include complete description, performance data and installation instructions for the materials and equipment specified herein. Control and wiring diagrams, rough-in dimensions for recessed junction and pull boxes, and component layout shall be included where applicable. All control and power conductors on the shop drawings shall be identified with wire numbers.

PART 2.- PRODUCTS

CONDUITS AND FITTINGS.--

Rigid steel conduit and fittings.--

Rigid steel conduit shall be threaded, full weight rigid steel, hot-dip galvanized inside and outside with steel or malleable iron fittings. Fittings shall be threaded unless otherwise specified or shown on the plans.

Insulated grounding bushings shall be threaded malleable cast iron body with plastic insulated throat and steel, lay-in ground lug with compression screw.

Insulated metallic bushings shall be threaded malleable cast iron body with plastic insulated throat.

Flexible metallic conduit and fittings.--

Flexible metallic conduit shall be fabricated in continuous lengths from galvanized steel strip, spirally wound and formed to provide an interlocking design.

Fittings shall be electroplated screw-in type with malleable cast iron body and threaded male hub with insulated throat.

Liquid tight flexible metallic conduit and fittings.--

Liquid tight flexible metallic conduit shall be fabricated in continuous length from galvanized sheet steel, spirally wound and formed to provide an interlocking design with an extruded polyvinyl chloride cover.

Fittings shall be electroplated, malleable cast iron body, with cap nut, grounding ferrule, and connector body with insulated throat.

CABLES AND CONDUCTORS.--

Cables.--

Lane Cable and Treadle Cable shall be furnished and installed in accordance with the details shown on the plans.
Field tests for grounds and continuity for the lane cable and the treadle cable shall be preformed in the presence of the engineer.

Communication Cables shall consist of 2 pairs of No. 22 AWG stranded tinned copper with 0.203 millimeter minimum thickness polypropylene insulation. Each pair shall be individually shielded by an aluminum-polyester shield. The shielded pairs and a No. 24 AWG stranded tinned copper common drain wire shall be covered with a 0.483-millimeter minimum thickness chrome vinyl jacket. Conductor pairs shall be color coded black-red and green-white.

Conductors.--

Conductors shall be stranded copper wire with 600-volt insulation.

Conductor insulation types unless otherwise shown or specified, shall be as follows:

1. Conductors across hinges of control panel enclosures shall be Type MTW.
2. Conductors shall be type XHHW in wet and outdoor locations.
3. Conductors shall be type THHN in dry locations.

Wire connections and devices.--

Wire connections and devices shall be pressure or compression type, except that connectors for No. 10 AWG and smaller conductors in dry locations may be preinsulated spring-pressure type.

ELECTRICAL BOXES.--

Outlet, device and junction boxes.--

Unless otherwise shown or specified, boxes shall be cast metal and shall be the size and configuration best suited to the application indicated on the plans.

Multiple switches shall be installed in standard gang boxes, unless otherwise specified or shown on the plans.

Cast metal boxes shall be cast iron boxes with threaded hubs and shall be of the size and configuration best suited to the application shown on the plans.

Floor junction boxes recessed in concrete shall be heavy duty, cast metal with drilled and tapped conduit holes. Boxes shall be external flanged for flush cover. Covers shall be gasketed and attached with stainless steel screws. Boxes shall be hot dipped galvanized suitable for embedding into concrete.

Trench duct shall be 228.6-millimeter wide, 457.2-millimeter in length with cover plate. The depth shall be adjustable from 60.325-millimeter to 85.725-millimeter and shall be adjusted flush with the floor to allow easy roll in and out of the TRAC lane console. The cover plate shall have notches for cables as shown on the plans, and shall have grey vinyl tile trim.

Weatherproof junction boxes shall have cast metal covers with gaskets.
Sectional device plates will not be permitted.

Underground pull boxes.--

Pull boxes shall be high density reinforced concrete box with ultraviolet inhibitor polyethylene etched face anchored in concrete and fiberglass cover with hold down bolts. The polyethylene and fiberglass material shall be fire resistant and show no appreciable change in physical properties with exposure to the weather. No. 3 1/2 pull box shall be Brooks Products, No. 3 1/2; Christy Concrete Products, N9; or equal. No. 5 pull box shall be Brooks Products No. 5; Christy Concrete Products, N30; or equal.

Traffic rated pull boxes shall be high density reinforced concrete box with steel cover with hold down bolts and bonding strap. Pull box and cover shall be designed for H20 loading. No. 3 1/2 pull box shall be 250 mm by 430 mm and No. 5 pull box shall be 320 mm by 610 mm.

RECEPTACLES AND SWITCHES.--

Ground fault circuit interrupter receptacles, (GFCI).--
Ground fault circuit interrupter receptacles shall be NEMA Type 5-20R, feed-through type, ivory color, 3-wire, 20-ampere, 125-volt AC, grounding type, specification grade, duplex receptacle with ground fault interruption. Receptacle shall detect and trip at current leakage of 5 milliamperes and shall have front mounted test and reset buttons.

Duplex receptacles.--
Duplex receptacles shall be NEMA Type 5-20R, 3-wire, 20-ampere, 125-volt AC, safety grounding, ivory color, specification grade receptacle suitable for wiring with stranded conductors.

Alarm pushbutton.--
Alarm pushbutton switch shall be one normally open contact and a red mushroom type head with a diameter of not less than 3.5 mm and shall be mounted in the roll booth as shown on the plans.

Snap switches.--
Snap switches shall be 20-ampere, 120/277-volt AC, quiet type, specification grade, ivory color switch with silver cadmium alloy contacts. Switch shall be suitable for wiring with stranded conductors.

Three-way toggle switches.--
Three-way toggle switches shall be 20-ampere, 120/277-volt AC, quiet type, specification grade, ivory color switch with silver cadmium alloy contacts. Switch shall be suitable for wiring with stranded conductors.

Four-way toggle switches.--
Four-way switches shall be 20-ampere, 120/277-volt AC, quite type, specification grade, ivory color switch with silver cadmium alloy contacts. Switch shall be suitable for wiring with stranded conductors.

MISCELLANEOUS MATERIALS.--

Pull ropes.--
Pull ropes shall be nylon or polypropylene with a minimum tensile strength of 225 kg.

Anchorage devices.--
Anchorage devices shall be corrosion resistant, toggle bolts, wood screws, bolts, machine screws, studs, expansion shields, and expansion anchors and inserts.

Electrical supporting devices.--
Electrical supporting devices shall be one hole conduit clamps with clamp backs, hot-dipped galvanized, malleable cast iron.

Construction channel shall be 41 mm x 41 mm, 2.66 mm (12-gage) galvanized steel channel with 13 mm diameter bolt holes, 40 mm on center in the base of the channel.

Ground rod(s).--
Ground rod(s) shall be a 19 mm (minimum) galvanized or copper clad steel rod, 3 meters long.

Telephone outlet boxes.--

Telephone outlet boxes shall be 102 mm square boxes and plates with modular type telephone outlet. Boxes on stud walls shall have plaster ring.

Plates for flush mounting outlets in finished room shall be Type 430 stainless steel, 1 mm thick with satin finish.

PART 3.- EXECUTION

INSTALLATION.--

Conduit, general.--Rigid steel conduit shall be used unless otherwise shown on the plans or specified in these special provisions.

Unless otherwise specified or shown on the plans, flexible metal conduit shall be used to connect suspended lighting fixtures, motors, HVAC equipment, and other equipment subject to vibration in dry locations.

Unless otherwise specified or shown on the plans, liquid-tight flexible metal conduit shall be used to connect motors, HVAC equipment, and other equipment subject to vibration in wet locations.

Conduit installation.--Conduit trade sizes are shown on the plans. No deviation from the conduit size shown on the plans will be permitted without written permission from the Engineer.

Conduit shall be concealed unless otherwise shown on the plans.

Conduits shall be tightly covered and well protected during construction using metallic bushings and bushing "pennies" to seal open ends.

A pull rope shall be installed in all empty conduits. At least one meter of pull rope shall be doubled back into the conduit at each termination.

Locations of conduit runs shall be planned in advance of the installation and coordinated with the ductwork, plumbing, ceiling and wall construction in the same areas and shall not unnecessarily cross other conduits or pipe, nor prevent removal of ceiling tiles or panels, nor block access to mechanical or electrical equipment.

Where practical, conduits shall be installed in groups in parallel, vertical or horizontal runs and at elevations that avoid unnecessary offsets.

Exposed conduit shall be installed parallel and at right angles to the building lines.

All raceway systems shall be secured to the building structures using specified fasteners, clamps and hangers.

Single conduit runs shall be supported by using one hole pipe clamps. Where run horizontally on walls in damp or wet locations, conduit shall be installed with "clamp backs" to space conduit off the surface.

Multiple conduit runs shall be supported with construction channel secured to the building structure. Conduits shall be fastened to construction channel with channel compatible pipe clamps.

Raceways of different types shall be joined using specified couplings or transition fittings as recommended by the manufacturer.

Expansion couplings shall be installed where conduit crosses a building separation or expansion joint.

All floor and wall penetrations shall be sealed water-tight.

Conduit terminations.--Rigid steel conduits shall be securely fastened to cabinets, boxes and gutters using 2 locknuts and specified insulating metallic bushing. Conduit terminations at exposed weatherproof enclosures and cast outlet boxes shall be made watertight using specified hubs.

Grounding bushings with bonding jumpers shall be installed on all type of conduits terminating at concentric knockouts and on all conduits containing service conductors, grounding electrode conductor, and conductors feeding separate buildings.

Conductor and cable installation.--Conductors shall not be installed in conduit until all work of any nature that may cause injury is completed. Care shall be taken in pulling conductors that insulation is not damaged. An approved non-petroleum base and insulating type pulling compound shall be used as needed.

All cables shall be installed and tested in accordance with manufacturer's recommendations.

Splices and joints shall be insulated with insulation equivalent to that of the conductor.

Provide 155 mm of slack at each outlet and device connection. If the outlet or device is not at the end of a run of wire, connection shall be made with correctly colored pigtails tapped to the runs with splices as specified herein.

Branch circuit conductors in panelboards and load centers shall be neatly trained along a path from the breaker terminals to their exit point. The conductors shall have ample length to transverse the path without strain, but shall not be so long as to require coiling, doubling back, or cramming. The path shall transverse the panelboard gutter spaces without entering a gutter containing service conductors and, unless otherwise shown on the plans, without entering the gutter space of any panelboard feeder.

All pressure type connectors and lugs shall be retightened after the initial set.

Splices in underground pull boxes and similar locations shall be made watertight.

Junction boxes in furred or accessible ceiling spaces shall be identified with felt-tip pen denoting the circuits contained in the box.

Conductor identification.--The neutral and equipment grounding conductors shall be identified as follows:

Neutral conductor shall have a white or natural gray insulation except that conductors No. 4 and larger may be identified by distinctive white marker such as paint or white tape at each termination.

Equipment grounding conductor shall be bare or insulated. If insulated, equipment grounding conductors shall have green or green with one or more yellow stripes insulation over its entire length except that conductors No. 4 and larger may be permanently identified by distinctive green markers such as paint or green tape over its entire exposed insulation.

Feeder and branch circuit ungrounded conductors shall be color coded by continuously colored insulation, except conductors No. 6 AWG or larger may be color coded by colored tape at each connection and where accessible.

Where more than one branch circuit enters or leaves a conduit, panel, gutter, or junction box, each conductor shall be identified by its panelboard and circuit number. All control conductors including control conductors of manufacturer supplied and field wired control devices shall be identified at each termination with the wire numbers shown on the plans, approved working drawings, and as directed by the Engineer where deemed necessary. Identification shall be made with one of the following:

1. Adhesive backed paper or cloth wrap-around markers with clear, heat shrinkable tubing sealed over either type of marker.
2. Self-laminating wrap around type, printable, transparent, permanent heat bonding type thermoplastic film markers.
3. Pre-printed, white, heat-shrinkable tubing.

Each terminal block shall have a molded marking strip attached with screws. The identifying numbers of the terminating conductors, as shown on the plans or on the submittal drawings, shall be engraved in the marking strip.

Outlet, device and junction box installation.--Unless otherwise shown on the plans or specified in these special provisions, all boxes shall be cast metal. Weatherproof outlet, device and junction boxes shall have cast metal covers with gaskets. No unused openings shall be left in any box. Knockout seals shall be installed as required to close openings.

Outlet, device, and junction boxes shall be installed at the locations and elevations shown on the plans or specified herein. Adjustments to locations may be made as required by structural conditions and to suit coordination requirements of other trades.

Underground pull box installation.--Electrical pull box covers or lids shall be marked "ELECTRICAL." Telephone service pull box covers or lids shall have plain, unmarked covers.

The bottom of pull boxes shall be bedded in 155 mm of clean, crushed rock or gravel and shall be grouted with 40 mm thick grout prior to installation of conductors. Grout shall be sloped to a 25 mm PVC pipe drain hole. Conduit shall be sealed in place with grout.

Top of pull boxes shall be flush with surrounding grade or top of curb. In unpaved areas where pull box is not immediately adjacent to and protected by a concrete foundation, pole or other protective construction, the top of pull box shall be set at plus 30 mm above surrounding grade. Pull boxes shown on the plans in the vicinity of curbs shall be placed adjacent to the back of curb. Pull boxes shown on the plans adjacent to lighting standards shall be placed on the side of foundation facing away from traffic.

Ground rod(s) installation.--The ground rod(s) shall be driven vertically until the top is 155 mm above the surrounding surface. When vertical penetration of the ground rod cannot be obtained, an equivalent horizontal grounding system, approved by the Engineer, shall be installed.

Anchorages.--Hangers, brackets, conduit straps, supports, and electrical equipment shall be rigidly and securely fastened to surfaces by means of toggle bolts on hollow masonry; expansion shields and machine screws, or expansion anchors and studs or standard preset inserts on concrete or solid masonry; machine screws or bolts on metal surfaces; and wood or lag screws on wood construction.

Anchorage devices shall be installed in accordance with the anchorage manufacturer's recommendations.

Mounting heights.--Electrical system components shall be mounted as shown on the plans. The mounting height dimensions shall be measured above the finished floor to the bottom of the device or component.

12-16.03 ELECTRICAL EQUIPMENT

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing electrical equipment and related accessories in accordance with the details shown on the plans and these special provisions.

Related work.--Anchorage devices shall be as specified under "Basic Materials and Methods" elsewhere in this Section 12-16.

SUBMITTALS.--

Product data.--A list of materials and equipment to be installed, manufacturer's descriptive data, and such other data as may be requested by the Engineer shall be submitted for approval.

Manufacturer's descriptive data shall include complete description, performance data and installation instructions for the materials and equipment specified herein. Control and wiring diagrams, rough-in dimensions, and component layout shall be included where applicable. All control and power conductors on the shop drawings shall be identified with wire numbers.

PART 2.- PRODUCTS

Extinguishable message sign.--

Each extinguishable message sign shall be internally illuminated weather-tight and dust-tight unit which will produce a clearly visible message only when internally illuminated. The message shall consist of 250 mm high letters as shown on the plans.

The general design of each sign shall be as shown on the standard plans. Minor details or construction shown are typical and may be modified subject to approval by the Engineer.

The Contractor shall submit a reproducible copy and a “blue line” print of shop drawings to the Engineer for review prior to performing any work on the signs.

(A). HOUSING.--The housing shall be a maximum size, 2430 mm wide x 1220 mm high x 305 mm deep and shall be weathertight, dusttight, corrosion resistant and shall be made of materials as shown on the standard plans.

Provisions shall be made for ease of maintenance of all components.

Sign panels and housing window shall be made of acrylic plastic which, including any painted portions, shall be highly resistant to crazing, staining, discoloration, creep and warping, and the long range deleterious effects of vehicle fumes, direct sunlight, heat (up to 90 degrees C), water oils and aging.

The housing skin shall be made of Type 5052-H32 aluminum alloy sheet with a flat black polyester/polyurethane powder coating finish. The housing reinforcing and miscellaneous parts shall be made of suitable gages and types of aluminum, except external fasteners, machine screw parts, lock washers, etc., which shall be stainless steel.

Gaskets shall be uniform and even textured and shall be highly resistant to stiffening and setting and the long range deleterious effects of vehicle fumes, direct sunlight, heat (up to 70 degrees C.), water, oils and aging.

Terminal strips shall be used for input, output and tie point connections and shall be of the molded phenolic barrier type.

(B). BALLASTS, CONTROL RELAYS AND TERMINAL BLOCKS.--Ballast inductors shall meet the requirements of ANSI Standard: C82.1, “Fluorescent Lamp Ballasts.”

Heater transformers shall produce rated secondary voltage (+/-10 percent) at full load and at 1/3 load. The maximum temperature rise of the transformer coils shall not exceed 40 degrees C. above an ambient temperature of 40 degrees C.

Inductors and transformers shall have cores made of a suitable grade of silicon steel lamination material and shall have thorough resin impregnation.

Each mounting chassis shall be fabricated of 3.175 mm, Type 5052-H32 aluminum alloy sheet. All units shall be mounted on the chassis with plated brass or steel hardware, except for lock washers, which shall be beryllium copper, externally toothed.

Capacitors shall be rated 660-volts, 60 Hz, AC, for operation down to –20 degrees C. with capacity as shown on the plans and shall be oil filled, paper type, hermetically sealed with solder lug terminals. Capacitance shall be within +/-10 percent of rating at 25 degrees C. Each capacitor shall be capable of withstanding a current limited, DC, 15-second breakdown test at 25 degrees C. of 3,000 volts from each terminal to case. Minimum insulation leakage resistance from terminal to terminal, in megohms, shall be not less than 1,500 divided by capacitance in microfarads.

Each magnetic control relay shall be of the heavy-duty, power type with 120-volt, AC coil and double-pole, double-throw contacts with minimum rating of 2 amperes at 480 volts, 60 Hz, AC. The coil shall consume not more than 10 volt amperes with sealed armature.

The relay coil shall be designed to provide reliable service under the following conditions:

Maximum operating voltage: 10 percent over rated volts.
Ambient temperature: 60 degrees C.

The relay coil shall meet NEMA requirements for temperature rise and voltage breakdown.

Maximum dimensions of relay shall be: mounting base, 50 mm by 100 mm; overall height, 50 mm.

Fuseholders shall be the panel mounting type rated at 250 volts, AC, complete with a 10 mm inch diameter by 38 mm inch length, slow blowing, cartridge type fuse.

Surge limiting and ballast resistors shall be ceramic coated, 20-watt, wirewound units. Resistor leads shall have plastic insulation rated 60 volts, AC, for operation at 90 degrees C.

All wiring connections from components shall be terminated on 2 molded phenolic, barrier type, terminal block assemblies rated at 15 amperes, 600 volts, AC. Terminal designations shall be marked as indicated on the plans.

(C). LAMPHOLDERS AND LAMPS.--Lampholders shall have silver plated contacts.

Lamps shall be of the extra-high output, rapid-start type with T-12 bulb of the length shown on the plans, cool-white color and plated contacts for operation up to 1,500 milliamperes.

(D). CONDUCTORS AND WIRING.--Ballast and sign conductors shall be No. 16 stranded copper wire and shall be labeled by UL as 105 degrees C. Appliance Wiring Material (AWM) for use at 600 volts. Ballast conductors shall be secured with easily removable, spring cross straps (not clamped, cabled or served) on the underside of the chassis. Color coding and terminal markings shall be as shown on the plans.

Lead ends shall be fitted with spade lugs.

(E) PLUG DISCONNECT.--Each plug disconnect shall consist of molded nylon plug and receptacle housings containing plug pins and individual sockets designed to be crimped to conductors and snapped into the housings. Housings shall have integral, molded, polarizing and locking devices. Minimum UL electrical rating shall be 10 amperes, 600 volts, AC. Pins and sockets shall be tin plated phosphor bronze secured to conductors using a ratchet type precision crimping tool.

(F) TESTING.--Tests shall verify that the following conditions exist:

Transformer output voltage - 480V ±10 percent.
Sign input current (daytime level) - 4 amp. maximum.
Lamp current each (daytime level) - 1.4 amp. ±15 percent (nightime level) 30 milliampere ±15 percent.
Cathode filament voltage - 3.6V ±10 percent and shall be supplied from a steady (non-flashing) source.

(G) SIGN OPERATION.--The sign shall operate as follows:

During daytime, the lamps shall operate at full rated brightness.
During nightime, the lamps shall be dimmed to approximately 1/35 of daytime brightness.
Starting and flashing (if required) shall be positive, without flickering, during daytime and nightime levels.

Traffic Signal Head.--

The traffic signal head shall be installed on the canopy structure above the toll lane as shown on the plans. Each traffic signal shall consist of two 304.8-millimeter signal heads, one having a red prismatic lens and one having a green prismatic lens. Each signal head shall consist of an aluminum body, gasketed hinged cover, integral serrated top and bottom locking rings, alzakaluminum hinged reflector, lamp receptacle, one 120-volt, 1,950-lumen, 6,000-hour clear traffic signal lamp, a molded phenolic terminal block and 304.8-millimeter diameter prismatic glass lens. Traffic signal heads shall be equiped with hoods.

Flashing Beacon Signal Head.--

The flashing beacon signal head shall be the same as the signal head, except 203.2-millimeter diameter amber lens, 203.2-millimeter long hood, and 655 lumen, 120-volt, 8,000-hour clear signal lamp. Flashing beacons shall be installed on the toll lane divider as shown on the plans.

Toll Booth Clock.--

The toll booth clock shall be furnished with a 228.6-millimeter round surface-mounted clock at the location shown on the plans. The clock shall be a minute impulse, self-regulated indicating type. The clock shall be electrically and operationally compatible with the existing master clock, and shall provide uniform and accurate system time.

Sergeant's console.--

The existing Sergeant's Console is a NEMA 12 enclosure as shown on the plans. The console shall be modified as stated herein and as shown on the plans. The existing control panel shall be removed and a new control panel shall be fabricated and installed in the same location. A new electrical mounting panel, minimum 14-gage sheet steel, shall be installed on the interior side of the front console door for mounting the new control relays. The modified console shall be rewired as shown on the plans. All new equipment shall be at the site and approved by the Engineer before any wires are disconnected from the existing console. The Sergeant's Console shall be out of operation for the rewiring process for no more than one eight hour shift on a given day determined by the Engineer. All interior control wires shall be 19-strand No. 14 MTW. Wires shall be neatly trained and bundled and wiring troughs shall be provided as needed. Wiring shall be arranged so that any piece of apparatus may be removed without disconnecting any wires except the leads to that piece of apparatus.

Control Relays.--

Control relays shall be 120-volt, AC, general purpose relay with 3-pole, double throw, 10-ampere contacts with pilot light. Relay shall be enclosed in clear plastic with 11-pin tube type plug base. Sockets shall be barrier type, 11-contact relay socket with 10-ampere contacts and screw terminals.

Selector Switches.--

Selector switches shall be 22 mm, 2-position, 10-ampere, 120-volt rotary switch mounted on the sergeant's console control panel as shown on the plans. Switch contacts shall have an inductive pilot duty rating of 60 amperes (make), 6 amperes (break) and 10 amperes (continuous) at 120-volt and 35 percent power factor. Selector switches SS1 through SS13 shall be single-pole. Selector switch SS14 shall be double-pole. Selector switch legend plates shall be marked "ON-OFF".

Pilot Lights.--.

Pilot lights shall be 120-volt, AC, light emitting diode (LED) type lamp. Pilot lights PL1 through PL10 shall be 30.5mm dual lamp square pilot lights. All other pilot lights shall be 22mm round pilot lights with colored plastic lens and screw cap. Pilot lights shall be mounted on the sergeant's console control panel with lens color as shown on the plans.

Pushbuttons.--

Pushbuttons shall be 22mm with one normally closed momentary contact. The contact shall have an inductive pilot duty rating of 60 amperes (make), 6 amperes (break) and 10 amperes (continuous) at 120-volt and 35 percent power factor. Pushbuttons shall have a legend plate marked "RESET".

Buzzer.--

Buzzer shall be 120 volt and have a maximum current consumption of 14 milliamperes. The buzzer shall have an output sound level of between 68 and 80 dB. The maximum diameter of each buzzer shall be 50mm and the audio frequency of each buzzer shall be different to distinguish the different alarm operations by the different buzzer sounds.

SWITCHES.--

Air conditioner disconnect switch.--

Air Conditioner Disconnect switch shall be 2-pole, 240-volt, AC, 60-ampere, fused, heavy duty safety switch in a NEMA-Type-4x. The fuses shall be sized to suit the air conditioning unit furnished. Two spare fuses shall be provided.

Exhaust fan disconnect switch.--

Exhaust fan disconnect switch shall be single-pole, 120-volt, 20-ampere, specification grade, AC switch in a cast metal box with standard galvanized cover.

INTERCOMMUNICATION SYSTEM.--

Intercom.--

Intercom shall be installed in the Toll Booth as shown on the plans and shall be connected to the existing intercom system.

Intercom shall consist of a 101.6-millimeter speaker, 20-ohm impedance, mounted in a cylindrical baffle and a “CALL” pushbutton. The baffle shall be designed to be surface mounted on a 101.6-millimeter outlet box from the ceiling of the Toll Booth.

The speaker grille shall be fabricated from perforated, cold rolled steel and shall be matte black. The baffle housing shall be 152.4-millimeters in diameter and 152.4-millimeters in depth, sturdy design and aluminum construction suitable for the intended use. The baffle housing shall be satin white. The baffle interior shall be coated with sound deadening material and lined with jute to prevent mechanical and acoustical resonance. Welded studs shall be provided to facilitate mounting a standard 101.6-millimeter speaker.

Call pushbutton shall be 120-volt AC, 3 ampere, momentary contact, normally open pushbutton switch with screw terminals.

Telephone system.--

A telephone set shall be installed in the Toll Booth as shown on the plans and shall be connected to the existing telecommunication system.

The telephone set in the Toll Booth shall be flush wall mount type. It shall consist of a handset with coiled retractile cord, cradle and high efficiency transmitter and receiver of commercial quality. The handset shall be made of tough, thermoplastic material in a sand beige color with matching color retractile cord. The handset cable shall be designed to hold the handset securely when not in use. The cradle shall be made of tough, thermoplastic material and shall contain the double-pole, double-throw easy action hookswitch assembly.

The handset cradle shall be mounted on a flush mount steel plate with brushed-satin chrome finish.

MISCELLANEOUS MATERIALS.--

Nameplates.--

Nameplates shall be laminated phenolic plastic with white core and black front and back. Nameplate inscription shall be in capitals letters etched through the outer layer of the nameplate material.

PART 3.- EXECUTION

INSTALLATION.--

Disconnect switches.--Disconnect switches shall be installed where shown on the plans.

Existing panelboards.--Provide new circuit breakers, where required to match existing type unless otherwise shown on the plans. Provide mounting hardware, bus straps, and related materials for proper circuit breaker installation. Provide new

panelboard identification nameplate with designation as shown for each panelboard. Remove existing nameplates where applicable. Provide new typewritten circuit directory reflecting changes made under the Contract.

Equipment identification.--Equipment shall be identified with nameplates fastened with self-tapping, cadmium-plated screws or nickel-plated bolts.

Nameplate inscriptions shall be as shown on the plans.

12-16.04 LIGHTING

GENERAL.--This work shall consist of furnishing, installing and connecting all lighting equipment in accordance with the details shown on the plans and these special provisions.

SUBMITTALS.--Manufacturer's descriptive information, photometric curves, catalog cuts, and installation instructions shall be submitted for approval.

PRODUCTS.--

Lighting fixture lamps.--

Lighting fixture lamps shall be type and size as shown on the plans. Lamps shall be General Electric, Phillips, Sylvania, or equal. Fluorescent lamps, unless otherwise noted, shall be 4100K tri-phosphor with a CRI of 70 or greater.

Ballasts.--

All fluorescent fixtures shall be equipped with high power factor ballasts suitable for the line voltage and for the type, size and number of lamps required by the fixture. Fluorescent ballasts shall be UL Listed, Class P and ETL Certified ballasts. All ballasts except 800-milliampere ballasts shall have sound rating A. Fluorescent ballasts except for 800-milliampere ballasts shall be high-frequency electronic ballasts with power factor greater then 0.95, ballast factor at least 0.87, total harmonic distortion less than 10 percent, crest factor less than or equal to 1.6, complying with ANSI C 62.41 Category A for surge protection, and FCC Part 18 for interference. Dimming ballasts shall be high frequency ballasts as specified above and shall be capable of dimming the light output from 100 percent to 20 percent of the rated light output.

Lighting fixtures.--

Lighting fixtures shall be as shown on the plans and as specified herein. Outdoor luminaires shall be listed and labeled "Fixture Suitable For Wet Locations."

Booth and desk lights.--

Ceiling-mounted fluorescent fixture with two 35-watt lamps, 800 MA ballast and one-piece, clear acrylic, wrap-around diffuser. The fixture shall be Benjamin Products, Catalog No. 45257-4; Halophane, Catalog No. WL2K035; or equal.

Canopy fixture.--

Canopy fixture shall be wet location 120-volt fluorescent fixture with two 2438 millimeter, 800 ma, high output lamps and 0 degree C ballast. The housing shall be high impact thermoplastic fully gasketed around the lens and at the conduit connecting points. The lens shall be high impact clear prismatic plastic attached to the housing by stainless steel clamps. The fixture shall be Day Brite Vaporlume, Catalog No. WR 81241; Benjamin Stream FLO, Catalog No. FAD-2425-8; Crouse-Hinds, Catalog No. NFW82110-BY; or equal.

H1.--

Outdoor, wall mounted, 35-watt, 120-volt high pressure sodium luminaire with integral ballast. The luminaire shall be Holophane, Catalog No.WP1A035HPBZ; General Electric, Catalog No. WL0351PE; or equal.

EXECUTION.--

LIGHTING FIXTURES.--Lighting fixtures shall be mounted securely in accordance with the manufacturer's recommendations. Mounting methods shall be suitable for the particular type of ceiling or support at each location.

The Contractor shall provide all supports, hangers, spacers, channels, fasteners and other hardware necessary to support the fixtures.

Fixtures shall be set at the mounting heights shown on the plans, except heights shown shall be adjusted to meet conditions.

BALLASTS.--All fluorescent fixtures shall be equipped with high power factor ballasts suitable for the line voltage and for the type, size and number of lamps required by fixture. The Contractor has the option to install low voltage dimming

control provided that the Contractor submit plans and specifications with appropriate revisions for the low voltage dimming control to the Engineers for approval prior to installation.

All ballasts used in unheated areas inside the building shall be -20°C ballasts or less.

12-16.05 CLOSED CIRCUIT TELEVISION SYSTEM

GENERAL.—The adjustable camera shall consist of the following components to function as described herein and shown on the plans.

a. camera with zoom lens
b. pan and tilt mechanism
c. remote control receiver
d. connectors,fittings,cables and other features necessary for a complete and operable system
e. mounting hardware as shown on the plans or as recommended by the camera manufacturer
f. weather tight enclosures
g. The fixed camera shall consist of the following:

camera in a weather tight enclosure
items as described in (d) through (e) above

12-16.05A CAMERA

The camera assemblies shall be assembled and tested in accordance with these specifications prior to delivery to the jobsite. Factory testing documentation shall be furnished to the Engineer. Camera assemblies shall be delivered to the jobsite as a complete unit.

The camera shall be 16.9 mm, solid-state CCD (charged coupled device) color camera and shall meet or exceed the following specifications:

a. Signal format shall be compatible with all applicable National Television Standards Committee (NTSC), have 525 horizontal lines at 60 Hz and 30 full frames per second.
b. Color balance shall be of the through-the-lens type, with less than 10 IRE units unbalance from 2850 to 5800 degrees Kelvin.
c. External synchronization with color lock and burst phase adjustment along with horizontal phase adjustment shall be available.
d. Imaging system shall have Charged Coupled Device (CCD) solid-state image sensor. The minimum number of picture elements shall be 768(H) by 493(V) pixels. The image area shall be 6.4 mm by 4.8 mm .
e. Video output shall be 1.0 volt peak-to-peak ±3.0 dB (75-ohm balanced).
f. Minimum scene illumination necessary for full video output shall be 6.5 lux and a minimum of .50 lux for 80 percent video measured at the faceplate.
g. Camera resolution shall be a minimum of 460 horizontal television lines and a minimum of 350 vertical television lines when measured from the camera video output.
h. Signal-to-noise ratio with Automatic Gain Control (AGC) off shall be equal to or greater than 48 dB, Committee Consultation International Radio (CCIR) weighting.
i. Gamma shall be 0.5.
j. Camera shall operate on 115VAC powered from the Camera Control Receiver.
k. Imaging system shall be designed to minimize the effect of blooming and smearing of high intensity light sources under nighttime viewing conditions.
l. Camera shall be equipped with a standard "C" type lens mount and shall be designed to operate and interconnect with industry standard auto iris lenses.
m. Automatic Gain Control (AGC) shall have a range of 20 dB with an internal peak-average adjustment.
n. Camera shall be capable of integrating multiple frames for increased sensitivity.

The camera shall be specifically designed to operate under low light conditions and shall function satisfactorily over a wide range of dynamic lighting conditions ranging from low light to full sunlight.

The camera shall have a built-in programmable electronic circuit character generator capable of superimposing a written camera identification message over the video display. The identification shall be programmable up to two lines of 24 characters per line. Text characters shall be white with a black border 28 horizontal TV lines in height. The camera identification shall be input via an RS-422 connector to the camera housing programmable from a computer or a dumb terminal. The character generator shall also superimpose a message indicating that the camera pressure is low if the pressure in the enclosure drops to 0.14 kilograms per square centimeter and below.

The camera lens shall be motorized with automatic iris and manual override. The iris shall be controlled directly by the camera in automatic mode and from the control receiver in manual mode. Each lens shall incorporate an intra-spot neutral density filter and shall meet the following requirements:

Adjustable camera:
Standard "C" type mount, 16.9 millimeter format.
Focal length of 12.5 to 75 mm providing a 6:1 power zoom ratio with F/1.4 stop.
Maximum aperture of no less than F1.0.
All lens motors shall be slipclutch protected to prevent damage due to overload.

Fixed camera:
A 50mm auto iris lens with an F/1.8 stop.

12-16.05B CAMERA ENCLOSURE

The camera and lens shall be contained in a sealed and pressurized cylindrical enclosure with outside dimension of approximately 559 mm (L) x 114 mm (Dia) maximum. The enclosure shall be provided with a mounting for attachment to the pan and tilt unit. The enclosure shall be fabricated from corrosion resistant aluminum and finished in a beige or neutral color of weather resistant enamel. The front of the enclosure shall be tempered glass. The enclosure shall be purged of air and pressurized with dry nitrogen to 5 ± 1 psi. A standard Schrader valve shall be located on the back plane of the environmental enclosure for pressurizing and purging. A decal shall be affixed permanently to the rear housing plate stating that the unit is pressurized and that safety precautions must be observed. A pressure relief valve, preset to 20 ± 5.0 psi shall be located on the back plate of the environmental enclosure. A safety pressure release bolt shall be incorporated such that the enclosure may not be opened without first releasing the internal pressure. Each housing shall have one package of desiccant placed in the enclosure. The camera connector for the environmental housing shall mount on the rear plate and seal to the plate with an O-Ring.

The complete camera assembly shall meet or exceed the following environmental specifications:

Tempersture	40 to +50 degrees C
Humidity	100% MIL-E-5400T, para 3.2.24.4
Altitude	0 to 3, 048 meter, MIL-E-5400T, para 3.2.24.2
Vibration	Swept 5-60 Hz, 2.08-mm amplitude: (15 Gs at 60 Hz) Random 60- 1000 Hz .5 Gs RMS
Shock	15 Gs, 3 axis. MIL-E-5400T, para 3.2.24.6
Sand and Dust	MIL-E-5400T, para 3.2.24.7
Fungus Salt	MIL-E-5400T, para 3.2.24.8
Atmosphere	MIL-E-5400T, para 3.2.24.9
Explosion	MIL-E-5400T, para 3.2.24.10
EMI	FCC Rules, part 15, subpart J, for Class A devices
Acoustic Noise	+150 dB

A sunshield shall be supplied and installed on the camera to protect the camera from direct sunlight.

12-16.05C PAN AND TILT UNIT

The pan and tilt unit for all color video cameras shall be designed for use in all outdoor weather conditions.

The pan and tilt drive shall be a medium-duty, counterbalanced unit designed for operation with loads up to 15.8 kilograms. It shall include drive motors with factory lubricated, hardened-steel gear trains that require no field lubrication, and quad ring seals with Teflon wear surfaces for protection in severe conditions. Operational stability shall be provided by self energizing brakes integral with the drive motors, or by solenoid-operated brakes.

Input voltage shall be provided by the remote control receiver. Input shall be 115V,AC. Input power shall be 21VA pan, 21VA tilt.

The size of the unit shall be approximately 279 mm high, 229 mm wide with a base of approximately 3870 mm square.

All parts shall be corrosion-protected steel or aluminum. Finish shall be off-white baked enamel. Pan bearings shall be heavy duty ball bearings. Tilt bearings shall be oil tight bronze bushing. Gears shall be hardened steel. Seals shall be quad ring and gasket seals for all weather environmental protection. Angular travel shall be: Pan, 350 degrees maximum; Tilt, ±90 degrees. Ambient temperature range shall be -30 degrees C to + 10 degrees C. Overload protection shall be motor thermal on tilt, motor clutch protection on pan.

12-16.05D REMOTE PAN/TILT AND ZOOM LENS CONTROL RECEIVER

A remote control receiver shall be installed near each pan/tilt and zoom lens camera.

Remote control command shall be sent by the existing remote control transmitter over coaxial cables.

The receiver assembly shall have the following features:

Control	On//Off toggle
Signal input/output	1.00V p-p composite video
Pan/tilt output	24V AC
Lens outputs	12V DC switched on command from the remote transmitter so as to allow the zoom and focus lens functions to operate over their entire range at the command of the operator.
Connectors (For Pan/Tilt unit and camera)	37 pin AMP type.
Connectors (For video)	BNC type
Supply voltage	117V AC, 60 Hz (±10 %)
Camera power	117V AC
Construction	Weatherproof NEMA 4 enclosure
Size	approx. 102 mm high, 279 mm wide, 254 deep.

12-16.05E PAN/TILT AND CAMERA CONTROL CABLE ASSEMBLY

The pan/tilt and camera control cable assembly from the camera housing and the pan/tilt mechanism to the remote control receiver shall be of the type recommended by the camera manufacturer and all connections shall be done using Y connectors.

12-16.05F POWER CABLE

Units or assemblies which require 120 volt AC power for operation shall be furnished with a 0.91 m long, 3-conductor power cable permanently affixed to its chassis or enclosure at one end in a manner so that it is fully protected from contact with metal edges. The chassis or the enclosure shall have a strain relief device to prevent strain on each internal connection to the power cable.

12-16.05G VIDEO COAXIAL CABLE (RG59/U and RG6/U)

The video cable shall be UL approved coaxial type with one No. 22 solid bare copper conductor, bare copper shield and with an outer PVC jacket.Video coaxial cable shall be type RG59/U for cameras located up to 230 m maximum away from the monitor. Type RG6/U shall be used feed cameras up to 460 m away.

12-16.05H INSTALLATION

Cameras and monitors shall be adjusted for both day and night operations. The contractor shall submit the installation and mounting details of the cameras.

12-16.05I EXECUTION

The system shall be installed at locations shown on the plans and by methods recommended by the manufacturers. The Contractor shall provide all supports, fasteners and other hardware necessary to support the units.

12-16.06 FIRE ALARM AND DETECTION SYSTEM

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing a complete and operational fire alarm and detection system in accordance with the details shown of the plans and these special provisions.

The system shall include all materials, whether mentioned or not, but are necessary for the complete and operational fire alarm and detection system.

SYSTEM DESCRIPTION.--

Design Requirements.--The fire alarm and detection system shall be a low voltage, direct current, zoned, closed circuit, electrically supervised, and Class A fire alarm and detection system. The system shall consist of fire alarm control panel, manual pull stations, smoke detectors, end-of-line resistors, audio-visual devices, and all other necessary appurtenances.

The alarm system components shall be listed by U.L. for NFPA, or F.M. and the California State Fire Marshal.

SUBMITTALS.--

Product data.--Manufacturer's descriptive information and installation instructions shall be submitted for approval.

Working drawings shall show equipment locations, wiring schematics, details, panel configuration and sizes and a point-to-point schematics of circuits and zone schedules. Include front elevations, cabinet dimensions, type of mounting, doors, barriers, catalog number of locks, and finishes for all terminal cabinets. Show interfaces to equipment furnished by other, identifying numbers of wires and termination requirements. Responsibility for each end of the interfaces shall be noted.

Complete sequence of operations of the system.

Complete system wiring diagrams for components connected to the system.

A copy of any state or local fire alarm system equipment approvals.

Installation instructions shall include brand name and catalog reference of equipment supplied, wiring diagrams, battery calculations, voltage drop calculations, riser diagrams and floor plans showing all devices and conduit and conductor sizes.

PART 2.- PRODUCTS

Fire alarm control panel.--

Fire alarm control panel shall be surface-mounted, locking cabinet, completely self-contained control panel suitable for 120-volt, AC, input power with separate terminals for all external wires and end-of-line resistors installed within the control panel.

The control panel shall conform to the following requirements:

Compatible with Radionics 6000 or 6500 receiver or equivalent;
Two zones;
Digital dialer communicator connected to California Department of Transportation (Caltrans) Traffic Management Center (TMC) via interface which is supplied by other;
Audible trouble signal, silencing switch and trouble pilot light;
Solid state, modular construction;
Fan shut down relays;
24-hour standby batteries, battery charger with automatic transfer on loss of utility company power and retransfer upon restitution of utility power;
Indicating lights for normal power failure, battery power failure, audible alarm, and silencing switch;
Low battery reporting.

Manual pull station.--

Manual pull station shall be single-action, non-coded, closed circuit, pull down type pull station mounted on a standard electrical outlet box. The manual pull station actuating contact shall function continuously until reset. The pull station shall have provisions for fire drill and testing and shall have integral LED light to indicate operation of the pull station.

Smoke detector.--

Smoke detector shall be photoelectric type detector with plug-in detector head. The smoke detector shall have integral LED light to indicate operation of the smoke detector.

Audio-visual device.--

Audio-visual device shall be vibrating type horn with flashing light and adjustable volume control with maximum audible output of 90 dB at 3 meters from the horn. Frequency of audio visual flash shall be not less than one flash per second.

PART 3.- EXECUTION.--

INSTALLATION.--

General.--The fire alarm system shall be installed in accordance with the manufacturer's recommendations. No modification of the recommended alarm system type, components type, or replacement shall be made without prior written approval from the Engineer and as indicated on the plans in accordance with approved working drawings, in accordance with all codes and regulations.

Conduit and conductors.--Fire alarm system wiring shall be installed in conduits conforming to the requirements of "Basic Materials and Methods" elsewhere in these special provisions. Conduit size shall be as recommended by the fire

alarm system manufacturers except that conduits shall be not less than 16 mm diameter, trade size. Within the TOS HUB, conduits shall be exposed conduit.

Verify and provide all required interconnections between the fire alarm control panel and the remote equipment via modem and interfacing unit.

Fire alarm systems shall be installed in accordance with instruction and direction as provided by the manufacturer.

Fire alarm signal shall be transmitted via interface to California Department of Transportation (Caltrans) Traffic Management Center (TMC) and State Fire Marshal.

Conductors and cables for the fire alarm system shall be as recommended by the fire alarm system manufacturer.

FIELD QUALITY CONTROL.--

Testing.--The operational test for the fire alarm system shall be performed by the Contractor in the presence of the Engineer. The operational tests shall demonstrate that all functions of the system operate in the manner described in the manufacturer's literature and that the system is stable under normal vibration and shocks to components. The Contractor shall notify the Engineer in writing not less than 10 days in advance of performing the operational tests.

Monitoring.--The contractor shall provide monitoring services for the facility for one year after the acceptance of the contract. The services shall include a toll-free telephone line connecting to the 24-hour on call monitoring station. Monitoring station shall contact designated site representative in the event of alarm and dispatch an immediate on-site response to the alarm location if the site representative cannot be reached or verification of the cause of the alarm cannot be determined.

Monitoring services after the first year will be handled by the State.

DEMONSTRATION.--

Training.--The Contractor shall provide one hour of on-site training on the use, operation, and, maintenance of the system for not more than 8 designated State employees. The Contractor shall notify the Engineer in writing not less than 10 days in advance of proposed training class.

12-16.07 INTRUSION ALARM SYSTEM

PART 1.- GENERAL

SUMMARY.--

Scope.--This work shall consist of furnishing and installing a complete and operational intrusion alarm system in accordance with the details shown on the plans and these special provisions.

The system shall include all materials, whether mentioned or not, that are necessary for a complete and operational intrusion alarm system. The security system shall be capable of transmitting an alarm signal via California Department of Transportation (Caltrans) interface and telephone wires to the Caltrans Traffic Management Center (TMC).

SYSTEM DESCRIPTION.--

Design requirements.--The intrusion alarm system shall be a low voltage, direct current, zoned alarm system, and shall consist of a control panel, magnetic contact switches, combination detectors, multiple switch contact monitors, glass break discriminators, and manual keypad stations. Each zone shall be "supervised, Class B circuit." The end of line resistor shall be installed in the control panel.

The alarm system shall self-test and report status of individual zones.

The alarm system shall provide an automatically rechargeable back-up power supply system in case of building power interruption.

The alarm system components shall be U.L. or F.M. Listed. The system proposed shall be approved by the Federal Communication Commission (FCC).

SUBMITTALS.--

Product data.--Manufacturer's descriptive information and installation instructions shall be submitted for approval.

Working drawings shall be submitted showing complete layout of the system, conduit and wiring diagrams, connection diagrams, number and location of detectors. Catalog data shall be submitted for all manufactured components.

Installation instructions shall include manufacturer and catalog reference, and model number of equipment to be furnished, conduit and conductor sizes, wiring diagram, and floor plan showing locations of multiple switch contact monitor and devices.

QUALITY ASSURANCE.--

Installer qualification.--The installer of the security alarm system shall be licensed by the State Department of Consumer Affairs, Bureau of Collection and Investigative Services. License numbers and expiration dates shall be included on all correspondence.

PART 2.- PRODUCTS

Security monitor control panel, MS.--

The control panel shall be a surface-mounted, locking cabinet, completely self-contained control panel suitable for 120-volt, AC, input power with separate terminals for all external wires.

The control panel shall meet the following requirements:

Compatible with Radionics 6000 or 6500 receiver or equivalent;
Four zones;
Digital dialer communicator;
12-volt auxiliary power supply;
Rechargeable battery (8 hour minimum);
Battery charger;
Low battery reporting;
Silent alarm signaling;
System connected to RJ31X or RJ38X telephone jack to transmit the alarm signal to California Department of Transportation (Caltrans) Traffic Management Center (TMC) ;
Line test every twenty-four (24) hours
120-volt, AC, input
Front accessible control and indication digital keypad.

Magnetic contact switch.--

Magnetic door switch for pedestrian door shall be a 2-section, self-lock mounting type switch, and shall be compatible with the material of the door on which it is installed. The switch shall be epoxied in the switch housing. Magnetic contact switches shall be the type capable of being concealed on the top of the door frame.
Switch shall be housed in a non-magnetic case.

Digital keypad.--

The remote digital keypad shall be flush mounted on a sheet metal box. Each digital keypad shall have two separate SPDT outputs contact with selectable timings (10, 30, 35 seconds); multiple programmable codes, and 3 LED indicating lights for loop status, system status and shunt status. Each digital keypad shall operate on DC power and contacts shall be rated one ampere at minimum 12-volt DC. Each digital keypad shall be wired to the control panel to turn on or turn off the entire system from each location. Digital keypad shall have 12-key with alarmed and ready lights and audible warning signal.

Multiple switch contact monitor.--

The multiple switch contact monitor shall have six (6) contact points: four (4) monitored contacts, one (1) supervised tamper and one (1) relay output contact. Multiple switch contact monitor can be located anywhere on the distributed system.

Combination detector.--

Combination detectors shall be low voltage, wall-mounted, wide angle microwave or passive infrared detectors with a detection pattern appropriate to cover areas indicated on the plans. Model must be specified on proposed installation layout. The detector shall have an LED indicating light.

PART 3. EXECUTION.--

INSTALLATION.--

General.--The intrusion alarm system shall be installed in accordance with the manufacturer's recommendations.

The switch section without wires shall be recessed flush into the top edge of the door at the approximate center of the door, and the switch section with wires shall be recessed flush in the top section of the door frame. The two sections of the switch shall be mounted directly opposite each other to provide maximum sensitivity. The wiring from each magnetic switch shall be run to the control panel in the zone dedicated for the intrusion alarm circuit.

Combination detector shall be mounted at not less than 2.3 meters above finished floor at locations shown on the plans.

Conduit and conductors.--All intrusion alarm system wiring shall be installed in conduit system conforming to the requirements under "Basic Materials and Methods" elsewhere in these special provisions. Conduit size shall be as recommended by the intrusion alarm manufacturer, except that conduits shall be not less than 16 mm diameter.

All conductors and cables for the intrusion alarm system wiring shall be as recommended by the intrusion alarm system manufacturer.

FIELD QUALITY CONTROL.--

Testing.--The operational test for the intrusion alarm system shall be performed by the Contractor in the presence of the Engineer and California Highway Patrol. The operational tests shall demonstrate that all functions of the system operate in the manner described in the manufacturer's literature and the alarm shall be received in the California Department of Transportation (Caltrans) Traffic Management Center (TMC). The Contractor shall notify the Engineer in writing not less than 10 days in advance of performing the operational tests.

Monitoring.--The contractor shall provide monitoring services for the facility for one year after the acceptance of the contract. The services shall include a toll-free telephone line connecting to the 24-hour on call monitoring station. Monitoring station shall contact designated site representative in the event of alarm and dispatch an immediate on-site response to the alarm location if the site representative cannot be reached or verification of the cause of the alarm cannot be determined.

Monitoring services after the first year will be handled by the State.

DEMONSTRATION.--

Training.--The Contractor shall provide one hour of on-site training on the use, operation, and maintenance of the system for not more than 8 designated State employees. The Contractor shall notify the Engineer in writing not less than 10 days in advance of proposed training class.

SECTION 13. RAILROAD RELATIONS AND INSURANCE

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SECTION 13-1. RELATIONS WITH RAILROAD COMPANY

13-1.01 General.--The term "Railroad" shall be understood to mean the Union Pacific Railroad Company.

It is expected that the Railroad will cooperate with the contractor to the end that the work may be handled in an efficient manner. However, except for the additional compensation provided for hereinafter for delays in completion of specific unit of work to be performed by the Railroad, the Contractor shall have no claim for damages, extension of time, or extra compensation in the event his work is held up by any of the work to be performed by the Railroad.

13-1.02 Railroad Requirements.--The Contractor shall cooperate with Railroad where work is over or under the tracks , or within the limits of Railroad property, in order to expedite the work and to avoid interference with the operation of railroad equipment.

The Contractor shall notify James H. Smith, Manager Public Projects, Engineering Department Room 1001 Southern Pacific Building, One Market Plaza, San Francisco, CA. 94105, Phone (415) 541-1421, FAX (415) 541-1483 and the State's Engineer, in writing, at least five (10) working days before performing any work on, or adjacent to the property or tracks of Railroad. One copy of the written notice shall also be sent to D. P. Jumper, Director of Public Projects, also at the above address.

The Contractor shall comply with the rules and regulations of Railroad or the instructions of it's representatives in relation to the proper manner of protecting the tracks and property of Railroad and the traffic moving on such tracks, as well as the wires, signals and other property of Railroad, its tenants or licensees, at and in the vicinity of the work during the period of construction.

The Contractor shall perform his work in such manner and at such times as shall not endanger, or interfere with the safe operation of the tracks and property of Railroad and traffic moving on such tracks, as well as wires, signals and other property of Railroad its tenants or licensees, at or in the vicinity of the work.

The Contractor shall take protective measures necessary to keep railroad facilities, including track ballast, free of sand, or debris, resulting from his operations. Any damage to railroad facilities resulting from Contractor's operations will be repaired or replaced by Railroad and the cost of such repairs or replacement shall be deducted from the contractor's progress and final pay estimates.

When it applies the Contractor shall comply with the Railroad Common Standard(C.S.) 1741 when working beneath the tracks.

The Contractor shall not pile or store any materials, nor park any equipment when not in use, closer to the center of the nearest track, than permitted by the following permanent clearances:

18'-0"	Horizontally from centerline of track
23'-0"	Vertically above top of rail

The placement above top of rail of false work, forms, bracing or other construction supports, placement or driving of piles, shall not be closer to the center of the nearest railroad track than permitted by the following temporary construction clearances:

12'-0"	Horizontally from centerline of track
21'-0"	Vertically above top of rail

The temporary horizontal and vertical construction clearance above provided will not be permitted until authorization by the Public Utilities Commission. It is anticipated that authorization will be received not later than fifteen days after the approval of the contract by the Attorney General. In the event authorization is not received by the time specified, and, if in the opinion of the Engineer, the Contractor's operations are delayed or interfered with by reason of authorization not being received by the said time, the State will compensate the Contractor for such delay to the extent provided in Section 8-1.09, "Right of Way Delays," of the Standard Specification and not otherwise.

Walkways with railing shall be constructed by Contractor over open excavation areas when in close proximity of tracks, and railings shall not be closer than 8'-6" horizontally from center line of the nearest track, if tangent, or 9'-6" if curved.

Any infringement on the above temporary construction clearances due to the contractor's operations shall be submitted to Railroad for approval by way of State's Engineer, and shall not be undertaken until approved by Railroad and until the Engineer has obtained any necessary authorization from the Public Utilities Commission for the infringement. No extension of time or extra compensation will be allowed in the event the Contractor's work is delayed pending Railroad approval, and Public Utilities Commission authorization.

When the temporary vertical clearance is less than 22'-6" above top of rail, Railroad shall have the option of installing tell-tales or other protective devices Railroad deems necessary for protection of railroad trainmen or traffic.

Six sets of working drawings showing details of construction affecting railroad tracks and property, including those for falsework or shoring of excavations near tracks not included in the contract plans shall be furnished by the Contractor to the Engineer; and the Contractor shall not begin such work until notified by the State's Engineer that such plans have been approved. In no case shall the Contractor be relieved of responsibility for results obtained by the use of said plans.

The Contractor shall notify the Engineer in writing, at least 25 calendar days but no more than 40 days in advance of the starting date of installing temporary work with less than permanent clearance at each structure site. The Contractor will not be permitted to proceed with work across railroad tracks unless the requirement has been met. No extension of time or extra compensation will be allowed in the event the Contractor's work is delayed because of his failure to comply with the requirements in this paragraph.

Except in connection with construction of grade separation structures on premises of Railroad, no private crossings at grade over tracks of Railroad for the purpose of hauling earth, rock, paving or other materials will be permitted. If the Contractor, for the purpose of constructing highway-railway grade separation structures, including construction ramps thereto, desires to move his equipment or materials across Railroad's tracks, he shall obtain permission from Railroad; and, should it be required, the Contractor shall execute a private crossing agreement. The crossing installation for the use of the Contractor, together with any protective devices, if required, shall be at the expense of the Contractor. Contractor shall furnish his own employees as flagmen to control movements of vehicles on the private roadway and shall take all measures necessary to prevent the use of such roadway by unauthorized persons and vehicles.

In advance of any blasting, the Contractor shall notify Railroad in order that proper flagging protection may be provided.

The Contractor shall, upon completion of the work covered by this contract, to be performed by Contractor upon premises or over or beneath the tracks of Railroad, promptly remove from the premises of Railroad, all of Contractor's tools, implements and other materials, whether brought upon said premises by said Contractor or any Subcontractor, employee or agent of Contractor or of any Subcontractor, and cause said premises to be left in a clean and presentable condition.

Edges of jacking or boring pit excavations shall be kept a minimum of 15 feet from center of nearest track.

The top of the nearest pipe to be installed under the track shall be at least 3 feet below the base of ties and care is to be exercised so as not to damage any underground facilities of Railroad.

13-1.03 Protection of Railroad Facilities:-

(1) Railroad representatives, conductors, flagmen or watchmen will be provided by Railroad to protect its facilities, property and movements of its trains or engines.

(a) When any part of any equipment is standing or being operated within 10 feet, measured horizontally from centerline of any track on which trains may operate, or when any erection or construction activities are in progress within such limits, regardless of elevation above or below track.

(b) For any excavation below elevation of track subgrade if, in the opinion of Railroad's representative, track or other railroad facilities may be subject to settlement or movement

(c) During any clearing, grubbing, grading, or blasting in proximity to Railroad which, in the opinion of Railroad's representative, may endanger railroad facilities or operations.

(d) During any of the Contractor's operations when, in the opinion of Railroad's representatives, railroad facilities, including, but not limited to, tracks, buildings, signals, wire lines or pipe lines, may be endangered.

(2) The cost of flagging and inspection provided by Railroad during the period of constructing that portion of the project located on or near Railroad property, as deemed necessary for the protection of Railroad's facilities and trains, will be borne by the State for a period of 100 consecutive calendar days beginning on the date work commences on or near property of Railroad. The Contractor shall pay to the State liquidated damages in the sum of $500 per day for each day in excess of the above 100 days the Contractor works on or near Railroad property, and which requires flagging protection of Railroad's facilities and trains.

13-1.04 Work by Railroad Company:-No work by the Railroad, other than flagging and inspection is anticipated.

13-1.05 Legal Relations:-The provisions of this section, "Relations with Railroad Company" and the provisions of the following section, "Railroad Protective Insurance," of these special provisions shall inure directly to the benefit of Railroad.

13-1.06 Indemnification of Railroad; Fiber Optics:-The contractor or contractors, including its or their subcontractors, shall protect, indemnify and hold harmless Union Pacific Railroad Company and any other railroad company occupying or using Union Pacific's property or line of railroad, against all loss, liability and damage to any property arising from activities of the contractor or its subcontractors or agents involving work associated with the applicable State project that involves Union Pacific property or right-of-way.

The contractor shall telephone Union pacific at 1-800-336-9193(a 24-hour number) to determine if fiber optic cable is buried anywhere on the property to be used by the contractor. If fiber optic cable is buried on Union Pacific's property, the contractor will telephone the telecommunications company(ies) involved, arrange for a cable locator, and make arrangements for relocation or other protection of the fiber optic cable prior to beginning any work on Union Pacific's property. The contractor agrees to indemnify and hold Union Pacific harmless against and from all costs, liability and expense whatsoever actually incurred arising out of or in any way contributed to by any negligent act or omission of the contractor or its subcontractors or agents for any damage to or destruction of any telecommunications system by the contractor or its subcontractors or agents.

SECTION 13-2. RAILROAD PROTECTIVE INSURANCE

The term "Railroad" shall be understood to mean the Union Pacific Railroad Company.

In addition to any other form of insurance or bonds required under the terms of the contract and specifications, the Contractor will be required to carry insurance of the kinds and in the amounts hereinafter specified.

Such insurance shall be approved by the Railroad before any work is performed on Railroad's property and shall be carried until all work required to be performed on or adjacent to the Railroad's property under the terms of the contract is satisfactorily completed as determined by the Engineer, and thereafter until all tools, equipment and materials have been removed from Railroad's property and such property is left in a clean and presentable condition.

The insurance herein required shall be obtained by the successful bidder and he shall furnish the Office of Office Engineer, Division of Construction, Department of Transportation, State of California, P.O. Box 942874, Sacramento, California 94274-0001, with two completed certificates, in the form attached hereto, signed by the insurance company or its authorized agent or representative, reflecting the existence of each of the policies required by 1 and 2 below including coverage for X, C and U and completed operations hazards, the original policy of insurance and one certified copy thereof required by 3 below. Engineer will convey one of the certificates of policy certifying 1 and 2 and the original policy of insurance required by 3 to Railroad upon receipt from successful bidder. Engineer will notify successful bidder whether Railroad approves the insurance policies.

Certificate of insurance shall guarantee that the policy under 1 and 2 will not be amended, altered, modified or canceled insofar as the coverage contemplated hereunder is concerned, without at least thirty (30) days notice mailed by registered mail to the Engineer and to Railroad.

Full compensation for all premiums which the Contractor is required to pay on all the insurance described hereinafter shall be considered as included in the prices paid for the various items of work to be performed under the contract, and no additional allowance will be made therefor or for additional premiums which may be required by extensions of the policies of insurance.

The approximate ratio of the estimated cost of the work over or under or within 50 feet of Railroad's tracks to the total estimated cost is 1.00 Approximate daily train traffic is 0 passenger trains and 2 freight trains.

1. Contractor's Public Liability and Property
Damage Liability Insurance

The Contractor shall, with respect to the operations he performs within or adjacent to Railroad's property, carry regular Contractor's Public Liability and Property Damage Liability.

Insurance providing for the same limits as specified for Railroad's Protective Public Liability and Property Damage Liability insurance to be furnished for and in behalf of Railroad as hereinafter provided.

If any part of the work within or adjacent to Railroad's property is subcontracted, the Contractor in addition to carrying the above insurance, shall provide the above insurance on behalf of the subcontractors to cover their operations.

2. Contractor's Protective Public Liability and Property
Damage Liability Insurance

The Contractor shall, with respect to the operations performed for him by subcontractors who do work within or adjacent to Railroad's property, carry in his own behalf regular Contractor's Protective Public Liability and Property Damage Liability Insurance providing for the same limits as specified for Railroad's Protective Public Liability and Property Damage Liability Insurance to be furnished for and on behalf of Railroad as hereinafter provided.

3. Railroad's Protective Public Liability and Property
Damage Liability Insurance

The Contractor shall, with respect to the operations he performs within or adjacent to Railroad's property or that of any of his subcontractors who do work within or adjacent to Railroad's property perform, have issued and furnished individual policies of insurance in the Railroad Protective Liability Form as hereinafter specified. Each such policy shall provide limits of liability as specified in said Railroad Protective Liability Form.

Railroad Protective Liability Form.

_____
(Name of Insurance Company)

DECLARATIONS

Item 1. Named Insured:

The first policy shall name as insured:
Union Pacific Railroad Company
5500 Ferguson Drive
Los Angeles, California 90022

Item 2. Policy Period: From _____to _____ 12:01 a.m., Standard Time, at the designated job site as stated herein.

Item 3. The insurance afforded is only with respect to such of the following coverages as are indicated in Item 6 by specific premium charge or charges. The limit of the company's liability against such coverage or coverages shall be as stated herein, subject to all the terms of this policy having reference thereto.

Limits of Liability

Coverages		Each Occurence	Aggregate
A B C	Bodily injury liability Property damage liability and Physical damage to property	$ 2,000,000 Combined Single Limit	$ 6,000,000 for coverages A, B, & C

Item 4. Name and Address of Contractor:

Item 5. Name and Address of Governmental Authority for whom the work by the Contractor is being performed: State of California, acting by and through its Department of Transportation, P.O. Box 942874, Sacramento, California 94274-0001.

Item 6. Designation of the Job Site and Description of Work:

FOR CONSTRUCTION ON_____
_____
_____
_____
_____

Premium Bases	Rates per $100 of Cost		Advance Premiums	
	Coverage A	Coverages B & C	Coverage A	Coverages B & C
Contract Cost	$	$	$	$
Rental Cost	$	$	$	$

Countersigned_____19_____by_____

_____
Title

POLICY

_____
(Name of Insurance Company)

A _____ insurance company, herein called the company, agrees with the insured, named in the declarations made a part hereof, in consideration of the payment of the premium and in reliance upon the statements in the declaration made by the named insured and subject to all of the terms of this policy:

INSURING AGREEMENTS

I. Coverage A--Bodily Injury Liability.

To pay on behalf of the insured all sums which the insured shall become legally obligated to pay as damages because of bodily injury, sickness, or disease, including death at any time resulting therefrom, hereinafter called "bodily injury," either (1) sustained by any person arising out of acts or omissions at the designated job site which are related to or are in connection with the work described in Item 6 of the

declarations, or (2) sustained at the designated job site by the Contractor or any employee of the Contractor, or by any employee of the Governmental Authority specified in Item 5 of the Declarations, or by any designated employee of the insured whether or not arising out of such acts or omissions.

Coverage B--Property Damage Liability.

To pay on behalf of the insured all sums which the insured shall become legally obligated to pay as damages because of physical injury to or destruction of property, including loss of use of any property due to such injury or destruction, hereinafter called "property damage," arising out of acts or omissions at the designated job site which are related to or are in connection with the work described in Item 6 of the declarations.

Coverage C--Physical Damage to Property.

To pay for direct and accidental loss of or damage to rolling stock and their contents, mechanical construction equipment, or motive power equipment, hereinafter called "loss," arising out of acts or omissions at the designated job site which are related to or are in connection with the work described in Item 6 of the declarations; provided such property is owned by the named insured or is leased or entrusted to the named insured under a lease or trust agreement.

II. Definitions.

(a) Insured.--The unqualified word "insured" includes the named insured and also includes any executive officer, director or stockholder thereof while acting within the scope of his duties as such.

(b) Contractor.--The word "contractor" means the Contractor designated in Item 4 of the declarations and includes all subcontractors of said Contractor but shall not include the named insured.

(c) Designated employee of the insured.--The words "designated employee of the insured" mean:

(1) any supervisory employee of the insured at the job site,

(2) any employee of the insured while operating, attached to or engaged on work trains or other railroad equipment at the job site which are assigned exclusively to the Contractor, or

(3) any employee of the insured not within (1) or (2) who is specifically loaned or assigned to the work of the Contractor for prevention of accidents or protection of property, the cost of whose services is borne specifically by the Contractor or by governmental authority.

(d) Contract.--The word "contract" means any contract or agreement to carry a person or property for a consideration or any lease, trust or interchange contract or agreement respecting motive power, rolling stock or mechanical construction equipment.

III. Defense, Settlement, Supplementary Payments.

With respect to such insurance as is afforded by this policy under Coverages A and B, the company shall:

(a) defend any suit against the insured alleging such bodily injury or property damage and seeking damages which are payable under the terms of this policy, even if any of the allegations of the suit are groundless, false or fraudulent; but the company may make such investigation and settlement of any claim or suit as it deems expedient;

(b) pay, in addition to the applicable limits of liability:

(1) all expenses incurred by the company, all costs taxed against the insured in any such suit and all interest on the entire amount of any judgment therein which accrues after entry of the judgment and before the company has paid or tendered or deposited in court that part of the judgment which does not exceed the limit of the company's liability thereon;

(2) premiums on appeal bonds required in any such suit, premiums on bonds to release attachments for an amount not in excess of the applicable limit of liability of this policy, but without obligation to apply for or furnish any such bonds;

(3) expenses incurred by the insured for such immediate medical and surgical relief to others as shall be imperative at the time of the occurrence;

(4) all reasonable expenses, other than loss of earnings, incurred by the insured at the company's request.

IV. Policy Period, Territory.

This policy applies only to occurrences and losses during the policy period and within the United States of America, its territories or possessions, or Canada.

EXCLUSIONS

This policy does not apply:

(a) to liability assumed by the insured under any contract or agreement except a contract as defined herein;

(b) to bodily injury or property damage caused intentionally by or at the direction of the insured;

(c) to bodily injury, property damage or loss which occurs after notification to the named insured of the acceptance of the work by the governmental authority, other than bodily injury, property damage or loss resulting from the existence or removal of tools, uninstalled equipment and abandoned or unused materials;

(d) under Coverages A (1), B and C, to bodily injury, property damage or loss, the sole proximate cause of which is an act or omission of any insured other than acts or omissions of any designated employee of any insured;

(e) under Coverage A, to any obligation for which the insured or any carrier as his insurer may be held liable under any workmen's compensation, unemployment compensation or disability benefits law, or under any similar law; provided that the Federal Employers' Liability Act, U.S. Code (1946), Title 45, Sections 51-60, as amended, shall for the purposes of this insurance be deemed not to be any similar law;

(f) under Coverage B, to injury to or destruction of property (1) owned by the named insured or (2) leased or entrusted to the named insured under a lease or trust agreement.

(g) 1. Under any liability coverage, to injury, sickness, disease, death or destruction

(a) with respect to which an insured under the policy is also an insured under a nuclear energy liability policy issued by Nuclear Energy Liability Insurance Association, Mutual Atomic Energy Liability Underwriters or Nuclear Insurance Association of Canada, or would be an insured under any such policy but for its termination upon exhaustion of its limit of liability; or

(b) resulting from the hazardous properties of nuclear material and with respect to which (1) any person or organization is required to maintain financial protection pursuant to the Atomic Energy Act of 1954, or any law amendatory thereof, or (2) the insured is, or had this policy not been issued would be, entitled to indemnity from the United States of America, or any agency thereof, under any agreement entered into by the United States of America, or any agency thereof, with any person or organization.

2. Under any medical payments coverage, or under any Supplementary Payments provision relating to immediate medical or surgical relief, to expenses incurred with respect to bodily injury, sickness, disease or death resulting from the hazardous properties of nuclear material and arising out of the operation of a nuclear facility by any person or organization.

3. Under any liability coverage, to injury, sickness, disease, death or destruction resulting from the hazardous properties of nuclear material, if

(a) the nuclear material (1) is at any nuclear facility owned by, or operated by or on behalf of, an insured or (2) has been discharged or dispersed therefrom;

(b) the nuclear material is contained in spent fuel or waste at any time possessed, handled, used, processed, stored, transported or disposed of by or on behalf of an insured; or

(c) the injury, sickness, disease, death or destruction arises out of the furnishing by an insured of services, materials, parts or equipment in connection with the planning, construction, maintenance, operation or use of any nuclear facility, but if such facility is located within the United States of America, its territories or possessions or Canada, this exclusion (c) applies only to injury to or destruction of property at such nuclear facility.

4. As used in this exclusion:

"hazardous properties" include radioactive, toxic or explosive properties;

"nuclear material" means source material, special nuclear material or byproduct material;

"source material", "special nuclear material", and "byproduct material" have the meanings given them in the Atomic Energy Act of 1954 or in any law amendatory thereof; "spent fuel" means any fuel element or fuel component, solid or liquid, which has been used or exposed to radiation in a nuclear reactor;

"waste" means any waste material (1) containing byproduct material and (2) resulting from the operation by any person or organization of any nuclear facility included within the definition of nuclear facility under paragraph (a) or (b) thereof;
"nuclear facility" means

(a) any nuclear reactor,

(b) any equipment or device designed or used for (1) separating the isotopes of uranium or plutonium, (2) processing or utilizing spent fuel, or (3) handling, processing or packaging waste,

(c) any equipment or device used for the processing, fabricating or alloying of special nuclear material if at any time the total amount of such material in the custody of the insured at the premises where such equipment or device is located consists of or contains more than 25 grams of plutonium or uranium 233 or any combination thereof, or more than 250 grams of uranium 235,

(d) any structure, basin, excavation, premises or place prepared or used for the storage or disposal of waste, and includes the site on which any of the foregoing is located, all operations conducted on such site and all premises used for such operations;

"nuclear reactor" means any apparatus designed or used to sustain nuclear fission in a self-supporting chain reaction or to contain a critical mass of fissionable material;

with respect to injury to or destruction of property, the word "injury" or "destruction" includes all forms of radioactive contamination of property.

(h) under Coverage C, to loss due to nuclear reaction, nuclear radiation or radioactive contamination, or to any act or condition incident to any of the foregoing.

CONDITIONS

(The conditions, except conditions 3, 4, 5, 7, 8, 9, 10, 11 and 12, apply to all coverages. Conditions 3, 4, 5, 7, 8, 9, 10, 11 and 12, apply only to the coverage noted thereunder.)

1. Premium.--The premium bases and rates for the hazards described in the declarations are stated therein. Premium bases and rates for hazards not so described are those applicable in accordance with the manuals in use by the company.

The term "contract cost" means the total cost of all work described in Item 6 of the declarations.

The term "rental cost" means the total cost to the Contractor for rental of work trains or other railroad equipment, including the remuneration of all employees of the insured while operating, attached to or engaged thereon.

The advance premium stated in the declarations is an estimated premium only. Upon termination of this policy the earned premium shall be computed in accordance with the company's rules, rates, rating plans, premiums and minimum premiums applicable to this insurance. If the earned premium thus computed exceeds the estimated advance premium paid, the company shall look to the Contractor specified in the declarations for any such excess; if less, the company shall return to the said Contractor the unearned portion paid.

In no event shall payment of premium be an obligation of the named insured.

2. Inspection.--The named insured shall make available to the company records of information relating to the subject matter of this insurance.

The company shall be permitted to inspect all operations in connection with the work described in Item 6 of the declarations.

3. Limits of Liability, Coverage A.--The limit of bodily injury liability stated in the declarations as applicable to "each person" is the limit of the company's liability for all damages, including damages for care and loss of services, arising out of bodily injury sustained by one person as the result of any one occurrence; the limit of such liability stated in the declarations as applicable to "each occurrence" is, subject to the above provision respecting each person, the total limit of the company's liability for all such damage arising out of bodily injury sustained by two or more persons as the result of any one occurrence.

4. Limits of Liability, Coverages B and C.--The limit of liability under Coverages B and C stated in the declarations as applicable to "each occurrence" is the total limit of the company's liability for all damages and all loss under Coverage B and C combined arising out of physical injury to, destruction or loss of all property of one or more persons or organizations, including the loss of use of any property due to such injury or destruction under Coverage B, as the result of any one occurrence.

Subject to the above provision respecting "each occurrence," the limit of liability under Coverages B and C stated in the declarations as "aggregate" is the total limit of the company's liability for all damages and all loss under Coverages B and C combined arising out of physical injury to, destruction or loss of property, including the loss of use of any property due to such injury or destruction under Coverage B.

Under Coverage C, the limit of the company's liability for loss shall not exceed the actual cash value of the property, or if the loss is of a part thereof the actual cash value of such part, at time of loss, nor what it would then cost to repair or replace the property or such part thereof with other of like kind and quality.

5. Severalty of Interests, Coverages A and B.-- The term "the insured" is used severally and not collectively, but the inclusion herein of more than one insured shall not operate to increase the limits of the company's liability.

6. Notice.--In the event of an occurrence or loss, written notice containing particulars sufficient to identify the insured and also reasonably obtainable information with respect to the time, place and circumstances thereof, and the names and addresses of the injured and of available witnesses, shall be given by or for the insured to the company or any of its authorized agents as soon as practicable. If claim is made or suit is brought against the insured, he shall immediately forward to the company every demand, notice, summons or other process received by him or his representative.

7. Assistance and Cooperation of the Insured, Coverages A and B.--The insured shall cooperate with the company and, upon the company's request, attend hearings and trials and assist in making settlements, securing and giving evidence, obtaining the attendance of witnesses and in the conduct of suits. The insured shall not, except at his own cost, voluntarily make any payment, assume any obligation or incur any

expense other than for such immediate medical and surgical relief to others as shall be imperative at the time of accident.

8. Action Against Company, Coverages A and B.--No action shall lie against the company unless, as a condition precedent thereto, the insured shall have fully complied with all the terms of this policy, nor until the amount of the insured's obligation to pay shall have been finally determined either by judgment against the insured after actual trial or by written agreement of the insured, the claimant and the company.

Any person or organization or the legal representative thereof who has secured such judgment or written agreement shall thereafter be entitled to recover under this policy to the extent of the insurance afforded by this policy. No person or organization shall have any right under this policy to join the company as a party to any action against the insured to determine the insured's liability. Bankruptcy or insolvency of the insured or of the insured's estate shall not relieve the company of any of its obligations hereunder.

Coverage C.--No action shall lie against the company unless, as a condition precedent thereto, there shall have been full compliance with all the terms of this policy nor until 30 days after proof of loss is filed and the amount of loss is determined as provided in this policy.

9. Insured's Duties in Event of Loss, Coverage C.--In the event of loss the insured shall:

(a) protect the property, whether or not the loss is covered by this policy, and any further loss due to the insured's failure to protect shall not be recoverable under this policy; reasonable expenses incurred in affording such protection shall be deemed incurred at the company's request;

(b) file with the company, as soon as practicable after loss, his sworn proof of loss in such form and including such information as the company may reasonably require and shall, upon the company's request, exhibit the damaged property.

10. Appraisal, Coverage C.--If the insured and the company fail to agree as to the amount of loss, either may, within 60 days after the proof of loss is filed, demand an appraisal of the loss. In such event the insured and the company shall each select a competent appraiser, and the appraisers shall select a competent and disinterested umpire. The appraisers shall state separately the actual cash value and the amount of loss and failing to agree shall submit their differences to the umpire. An award in writing of any two shall determine the amount of loss. The insured and the company shall each pay his chosen appraiser and shall bear equally the other expenses of the appraisal and umpire.

The company shall not be held to have waived any of its rights by any act relating to appraisal.

11. Payment of Loss, Coverage C.--The company may pay for the loss in money but there shall be no abandonment of the damaged property to the company.

12. No Benefit to Bailee, Coverage C.--The insurance afforded by this policy shall not inure directly or indirectly to the benefit of any carrier or bailee, other than the named insured, liable for loss to the property.

13. Subrogation.--In the event of any payment under this policy, the company shall be subrogated to all the insured's rights of recovery therefor against any person or organization and the insured shall execute and deliver instruments and papers and do whatever else is necessary to secure such rights. The insured shall do nothing after loss to prejudice such rights.

14. Application of Insurance.--The insurance afforded by this policy is primary insurance.

15. Three Year Policy.--A policy period of three years is comprised of three consecutive annual periods. Computation and adjustment of earned premium shall be made at the end of each annual period. Aggregate limits of liability as stated in this policy shall apply separately to each annual period.

16. Changes.--Notice to any agent or knowledge possessed by any agent or by any other person shall not effect a waiver or a change in any part of this policy or estop the company from asserting any right under the terms of this policy; nor shall the terms of this policy be waived or changed, except by endorsement issued to form a part of this policy.

17. Assignment.--Assignment of interest under this policy shall not bind the company until its consent is endorsed hereon.

18. Cancellation.--This policy may be canceled by the named insured by mailing to the company written notice stating when thereafter the cancellation shall be effective. This policy may be canceled by the company by mailing to the named insured, Contractor and governmental authority at the respective addresses shown in this policy written notice stating when not less than 30 days thereafter such cancellation shall be effective. The mailing of notice as aforesaid shall be sufficient proof of notice. The effective date and hour of cancellation stated in the notice shall become the end of the policy period. Delivery of such written notice either by the named insured or by the company shall be equivalent to mailing.

If the named insured cancels, earned premium shall be computed in accordance with the customary short rate table and procedure. If the company cancels, earned premium shall be computed pro rata. Premium adjustment may be made either at the time cancellation is effected or as soon as practicable after cancellation becomes effective, but payment or tender of unearned premium is not a condition of cancellation.

19. Declaration.--By acceptance of this policy the named insured agrees that such statements in the declarations as are made by him are his agreements and representations, that this policy is issued in reliance upon the truth of such representations and that this policy embodies all agreements existing between himself and the company or any of its agents relating to this insurance.

In witness whereof, the _____ Insurance Company has caused this policy to be signed by its president and a secretary at _____, and countersigned on the declarations page by a duly authorized agent of the company.

(Facsimile of Signature) (Facsimile of Signature)

_____ _____

Secretary President

CERTIFICATE OF INSURANCE

This is to certify to:

RAILROAD FILE NO.:
PUC #: L-22.00-A(Mt. Eden(Widen))

(1) Office Engineer, Division of Construction
California Department of Transportation
State of California
P. O. Box 942874, Sacramento, California 94274-0001

(2) and to the following Railroad Company

that such insurance as is afforded by the policy or policies described below for bodily injury liability and property damage liability is in full force and effect as of the date of this certificate and covers the following contractor as a named insured with respect to liability for damages arising out of operations performed by or for the named insured in connection with the contract or work described below.

1. Named Insured and Address

This is to certify that policies of insurance listed below have been issued to the insured named above and are in force at this time. Notwithstanding any requirement, term or condition of any contract or other document with respect to which this certificate may be issued or may pertain, the insurance afforded by the policies described herein is subject to all the terms, exclusions and conditions of such policies.

2. Description of Work

Contract No. _____

3. Coverages	Policy Expiration Date	Limits of Liability Each Occurrence	Aggregate
Contractor's Bodily Injury Liability and Property Damage Liability			
Umbrella or Excess Liability			

All of the coverages include coverage for the completed operations hazard, and X, C and U exposures.

Name of Insurance Company by Coverage

Coverages	Company	Policy Number
Bodily Injury Liability		
Property Damage Liability		
Umbrella or Excess Liability		

4. The policy or policies described above will not be amended, altered, modified or cancelled until thirty (30) days after written notice thereof has been given by registered mail to the (1) Office Engineer, Division of Construction, Department of Transportation, and (2) the Railroad named as certificate holder in this certificate.

Certificate Date:

For _____
(Insurance Company)

By _____
(Authorized Agent or Representative)

State of California
Department of Transportation
DH-0S-A104(10-28-88)

SECTION 14. FEDERAL REQUIREMENTS FOR FEDERAL-AID CONSTRUCTION PROJECTS

GENERAL .—The work herein proposed will be financed in whole or in part with Federal funds, and therefore all of the statutes, rules and regulations promulgated by the Federal Government and applicable to work financed in whole or in part with Federal funds will apply to such work. The "Required Contract Provisions, Federal-Aid Construction Contracts, "Form FHWA 1273, are included in this Section 14. Whenever in said required contract provisions references are made to "SHA contracting officer", "SHA resident engineer", or "authorized representative of the SHA", such references shall be construed to mean "Engineer" as defined in Section 1-1.18 of the Standard Specifications.

PERFORMANCE OF PREVIOUS CONTRACT .—In addition to the provisions in Section II, "Nondiscrimination," and Section VII, "Subletting or Assigning the Contract," of the required contract provisions, the Contractor shall comply with the following:

The bidder shall execute the CERTIFICATION WITH REGARD TO THE PERFORMANCE OF PREVIOUS CONTRACTS OR SUBCONTRACTS SUBJECT TO THE EQUAL OPPORTUNITY CLAUSE AND THE FILING OF REQUIRED REPORTS located in the proposal. No request for subletting or assigning any portion of the contract in excess of $10,000 will be considered under the provisions of Section VII of the required contract provisions unless such request is accompanied by the CERTIFICATION referred to above, executed by the proposed subcontractor.

NON-COLLUSION PROVISION .—The provisions in this section are applicable to all contracts except contracts for Federal Aid Secondary projects.

Title 23, United States Code, Section 112, requires as a condition precedent to approval by the Federal Highway Administrator of the contract for this work that each bidder file a sworn statement executed by, or on behalf of, the person, firm, association, or corporation to whom such contract is to be awarded, certifying that such person, firm, association, or corporation has not, either directly or indirectly, entered into any agreement, participated in any collusion, or otherwise taken any action in restraint of free competitive bidding in connection with the submitted bid. A form to make the non-collusion affidavit statement required by Section 112 as a certification under penalty of perjury rather than as a sworn statement as permitted by 28, USC, Sec. 1746, is included in the proposal.

PARTICIPATION BY MINORITY BUSINESS ENTERPRISES IN SUBCONTRACING .—Part 23, Title 49, Code of Federal Regulations applies to this Federal-aid project. Pertinent sections of said Code are incorporated in part or in its entirety within other sections of these special provisions.

Schedule B—Information for Determining Joint Venture Eligibility

(This form need not be filled in if all joint venture firms are minority owned.)

1. Name of joint venture _____
2. Address of joint venture _____
3. Phone number of joint venture _____
4. Identify the firms which comprise the joint venture. (The MBE partner must complete Schedule A.) _____
   _____

   a. Describe the role of the MBE firm in the joint venture. _____
      _____
   b. Describe very briefly the experience and business qualifications of each non-MBE joint venturer: _____
      _____

5. Nature of the joint venture's business _____
   _____
6. Provide a copy of the joint venture agreement.
7. What is the claimed percentage of MBE ownership? _____
8. Ownership of joint venture: (This need not be filled in if described in the joint venture agreement, provided by question 6.).

   a. Profit and loss sharing.
   b. Capital contributions, including equipment.
   c. Other applicable ownership interests.

9. Control of and participation in this contract. Identify by name, race, sex, and "firm" those individuals (and their titles) who are responsible for day-to-day management and policy decision making, including, but not limited to, those with prime responsibility for:

a. Financial decisions _____

b. Management decisions, such as:

(1) Estimating _____

(2). Marketing and sales _____

(3). Hiring and firing of management personnel _____

_____

(4) Purchasing of major items or supplies _____

_____

c. Supervision of field operations _____

Note.—If, after filing this Schedule B and before the completion of the joint venture's work on the contract covered by this regulation, there is any significant change in the information submitted, the joint venture must inform the grantee, either directly or through the prime contractor if the joint venture is a subcontractor.

Affidavit

"The undersigned swear that the foregoing statements are correct and include all material information necessary to identify and explain the terms and operation of our joint venture and the intended participation by each joint venturer in the undertaking. Further, the undersigned covenant and agree to provide to grantee current, complete and accurate information regarding actual joint venture work and the payment therefor and any proposed changes in any of the joint venture arrangements and to permit the audit and examination of the books, records and files of the joint venture, or those of each joint venturer relevant to the joint venture, by authorized representatives of the grantee or the Federal funding agency. Any material misrepresentation will be grounds for terminating any contract which may be awarded and for initiating action under Federal or State laws concerning false statements."

Name of Firm	Name of Firm
Signature	Signature
Name	Name
Title	Title
Date	Date

Date _____

State of _____

County of _____

On this ____ day of _____, 19 __, before me appeared (Name) _____, to me personally known, who, being duly sworn, did execute the foregoing affidavit, and did state that he or she was properly authorized by (Name of firm) _____ to execute the affidavit and did so as his or her free act and deed.

Notary Public _____

Commission expires _____

[Seal]

Date _____

State of _____

County of _____

On this ___ day of _____, 19 __, before me appeared (Name) _____ to me personally known, who, being duly sworn, did execute the foregoing affidavit, and did state that he or she was properly authorized by (Name of firm) _____ to execute the affidavit and did so as his or her free act and deed.

Notary Public _____

Commission expires _____

[Seal]

REQUIRED CONTRACT PROVISIONS
FEDERAL-AID CONSTRUCTION CONTRACTS

I. GENERAL

1. These contract provisions shall apply to all work performed on the contract by the contractor's own organization and with the assistance of workers under the contractor's immediate superintendence and to all work performed on the contract by piecework, station work, or by subcontract.

2. Except as otherwise provided for in each section, the contractor shall insert in each subcontract all of the stipulations contained in these Required Contract Provisions, and further require their inclusion in any lower tier subcontract or purchase order that may in turn be made. The Required Contract Provisions shall not be incorporated by reference in any case. The prime contractor shall be responsible for compliance by any subcontractor or lower tier subcontractor with these Required Contract Provisions.

3. A breach of any of the stipulations contained in these Required Contract Provisions shall be sufficient grounds for termination of the contract.

4. A breach of the following clauses of the Required Contract Provisions may also be grounds for debarment as provided in 29 CFR 5.12:

Section I, paragraph 2;
Section IV, paragraphs 1, 2, 3, 4, and 7;
Section V, paragraphs 1 and 2a through 2g.

5. Disputes arising out of the labor standards provisions of Section IV (except paragraph 5) and Section V of these Required Contract Provisions shall not be subject to the general disputes clause of this contract. Such disputes shall be resolved in accordance with the procedures of the U.S. Department of Labor (DOL) as set forth in 29 CFR 5, 6, and 7. Disputes within the meaning of this clause include disputes between the contractor (or any of its subcontractors) and the contracting agency, the DOL, or the contractor's employees or their representatives.

6. **Selection of Labor:** During the performance of this contract, the contractor shall not:

a. discriminate against labor from any other State, possession, or territory of the United States (except for employment preference for Appalachian contracts, when applicable, as specified in Attachment A), or

b. employ convict labor for any purpose within the limits of the project unless it is labor performed by convicts who are on parole, supervised release, or probation.

II. NONDISCRIMINATION

(Applicable to all Federal-aid construction contracts and to all related subcontracts of $10,000 or more.)

1. **Equal Employment Opportunity:** Equal employment opportunity (EEO) requirements not to discriminate and to take affirmative action to assure equal opportunity as set forth under laws, executive orders, rules, regulations (28 CFR 35, 29 CFR 1630, and 41 CFR 60) and orders of the Secretary of Labor as modified by the provisions prescribed herein, and imposed pursuant to 23 U.S.C. 140 shall constitute the EEO and specific affirmative action standards for the contractor's project activities under this contract. The Equal Opportunity Construction Contract Specifications set forth under 41 CFR 60-4.3 and the provisions of the American Disabilities Act of 1990 (42 U.S.C. 12101 et seq.) set forth under 28 CFR 35 and 29 CFR 1630 are incorporated by reference in this contract. In the execution of this contract, the contractor agrees to comply with the following minimum specific requirement activities of EEO:

a. The contractor will work with the State highway agency (SHA) and the Federal Government in carrying out EEO obligations and in their review of his/her activities under the contract.

b. The contractor will accept as his operating policy the following statement:

"It is the policy of this Company to assure that applicants are employed, and that employees are treated during employment, without regard to their race, religion, sex, color, national origin, age or disability. Such action shall include: employment, upgrading, demotion, or transfer; recruitment or recruitment advertising; layoff or termination; rates of pay or other forms of compensation; and selection for training, including apprenticeship, preapprenticeship, and/or on-the-job training."

2. **EEO Officer:** The contractor will designate and make known to the SHA contracting officers an EEO Officer who will have the responsibility for and must be capable of effectively administering and promoting an active contractor program of EEO and who must be assigned adequate authority and responsibility to do so.

3. **Dissemination of Policy:** All members of the contractor's staff who are authorized to hire, supervise, promote, and discharge employees, or who recommend such action, or who are substantially involved in such action, will be made fully cognizant of, and will implement, the contractor's EEO policy and contractual responsibilities to provide EEO in each grade and classification of employment. To ensure that the above agreement will be met, the following actions will be taken as a minimum:

a. Periodic meetings of supervisory and personnel office employees will be conducted before the start of work and then not less often than once every six months, at which time the contractor's EEO policy and its implementation will be reviewed and explained. The meetings will be conducted by the EEO Officer.

b. All new supervisory or personnel office employees will be given a thorough indoctrination by the EEO Officer, covering all major aspects of the contractor's EEO obligations within thirty days following their reporting for duty with the contractor.

c. All personnel who are engaged in direct recruitment for the project will be instructed by the EEO Officer in the contractor's procedures for locating and hiring minority group employees.

d. Notices and posters setting forth the contractor's EEO policy will be placed in areas readily accessible to employees, applicants for employment and potential employees.

e. The contractor's EEO policy and the procedures to implement such policy will be brought to the attention of employees by means of meetings, employee handbooks, or other appropriate means.

4. **Recruitment:** When advertising for employees, the contractor will include in all advertisements for employees the notation: "An Equal Opportunity Employer." All such advertisements will be placed in publications having a large circulation among minority groups in the area from which the project work force would normally be derived.

a. The contractor will, unless precluded by a valid bargaining agreement, conduct systematic and direct recruitment through public and private employee referral sources likely to yield qualified minority group applicants. To meet this requirement, the contractor will identify sources of potential minority group employees, and establish with such identified sources procedures whereby minority group applicants may be referred to the contractor for employment consideration.

b. In the event the contractor has a valid bargaining agreement providing for exclusive hiring hall referrals, he is expected to observe the provisions of that agreement to the extent that the system permits the contractor's compliance with EEO contract provisions. (The DOL has held that where implementation of such agreements have the effect of discriminating against minorities or women, or obligates the contractor to do the same, such implementation violates Executive Order 11246, as amended.)

c. The contractor will encourage his present employees to refer minority group applicants for employment. Information and procedures with regard to referring minority group applicants will be discussed with employees.

5. **Personnel Actions:** Wages, working conditions, and employee benefits shall be established and administered, and personnel actions of every type, including hiring, upgrading, promotion, transfer, demotion, layoff, and termination, shall be taken without regard to race, color, religion, sex, national origin, age or disability. The following procedures shall be followed:

a. The contractor will conduct periodic inspections of project sites to insure that working conditions and employee facilities do not indicate discriminatory treatment of project site personnel.

b. The contractor will periodically evaluate the spread of wages paid within each classification to determine any evidence of discriminatory wage practices.

c. The contractor will periodically review selected personnel actions in depth to determine whether there is evidence of discrimination. Where evidence is found, the contractor will promptly take corrective action. If the review indicates that the discrimination may extend beyond the actions reviewed, such corrective action shall include all affected persons.

d. The contractor will promptly investigate all complaints of alleged discrimination made to the contractor in connection with his obligations under this contract, will attempt to resolve such complaints, and will take appropriate corrective action within a reasonable time. If the investigation indicates that the discrimination may affect persons other than the complainant, such corrective action shall include such other persons. Upon completion of each investigation, the contractor will inform every complainant of all of his avenues of appeal.

6. Training and Promotion:

a. The contractor will assist in locating, qualifying, and increasing the skills of minority group and women employees, and applicants for employment.

b. Consistent with the contractor's work force requirements and as permissible under Federal and State regulations, the contractor shall make full use of training programs, i.e., apprenticeship, and on-the-job training programs for the geographical area of contract performance. Where feasible, 25 percent of apprentices or trainees in each occupation shall be in their first year of apprenticeship or training. In the event a special provision for training is provided under this contract, this subparagraph will be superseded as indicated in the special provision.

c. The contractor will advise employees and applicants for employment of available training programs and entrance requirements for each.

d. The contractor will periodically review the training and promotion potential of minority group and women employees and will encourage eligible employees to apply for such training and promotion.

7. **Unions:** If the contractor relies in whole or in part upon unions as a source of employees, the contractor will use his/her best efforts to obtain the cooperation of such unions to increase opportunities for minority groups and women within the unions, and to effect referrals by such unions of minority and female employees. Actions by the contractor either directly or through a contractor's association acting as agent will include the procedures set forth below:

a. The contractor will use best efforts to develop, in cooperation with the unions, joint training programs aimed toward qualifying more minority group members and women for membership in the unions and increasing the skills of minority group employees and women so that they may qualify for higher paying employment.

b. The contractor will use best efforts to incorporate an EEO clause into each union agreement to the end that such union will be contractually bound to refer applicants without regard to their race, color, religion, sex, national origin, age or disability.

c. The contractor is to obtain information as to the referral practices and policies of the labor union except that to the extent such information is within the exclusive possession of the labor union and such labor union refuses to furnish such information to the contractor, the contractor shall so certify to the SHA and shall set forth what efforts have been made to obtain such information.

d. In the event the union is unable to provide the contractor with a reasonable flow of minority and women referrals within the time limit set forth in the collective bargaining agreement, the contractor will, through independent recruitment efforts, fill the employment vacancies without regard to race, color, religion, sex, national origin, age or disability; making full efforts to obtain qualified and/or qualifiable minority group persons and women. (The DOL has held that it shall be no excuse that the union with which the contractor has a collective bargaining agreement providing for exclusive referral failed to refer minority employees.) In the event the union referral practice prevents the contractor from meeting the obligations pursuant to Executive Order 11246, as amended, and these special provisions, such contractor shall immediately notify the SHA.

8. Selection of Subcontractors, Procurement of Materials and Leasing of Equipment: The contractor shall not discriminate on the grounds of race, color, religion, sex, national origin, age or disability in the selection and retention of subcontractors, including procurement of materials and leases of equipment.

a. The contractor shall notify all potential subcontractors and suppliers of his/her EEO obligations under this contract.

b. Disadvantaged business enterprises (DBE), as defined in 49 CFR 23, shall have equal opportunity to compete for and perform subcontracts which the contractor enters into pursuant to this contract. The contractor will use his best efforts to solicit bids from and to utilize DBE subcontractors or subcontractors with meaningful minority group and female representation among their employees. Contractors shall obtain lists of DBE construction firms from SHA personnel.

c. The contractor will use his best efforts to ensure subcontractor compliance with their EEO obligations.

9. **Records and Reports:** The contractor shall keep such records as necessary to document compliance with the EEO requirements. Such records shall be retained for a period of three years following completion of the contract work and shall be available at reasonable times and places for inspection by authorized representatives of the SHA and the FHWA.

a. The records kept by the contractor shall document the following:

(1) The number of minority and non-minority group members and women employed in each work classification on the project;

(2) The progress and efforts being made in cooperation with unions, when applicable, to increase employment opportunities for minorities and women;

(3) The progress and efforts being made in locating, hiring, training, qualifying, and upgrading minority and female employees; and

(4) The progress and efforts being made in securing the services of DBE subcontractors or subcontractors with meaningful minority and female representation among their employees.

b. The contractors will submit an annual report to the SHA each July for the duration of the project, indicating the number of minority, women, and non-minority group employees currently engaged in each work classification required by the contract work. This information is to be reported on Form FHWA-1391. If on-the-job training is being required by special provision, the contractor will be required to collect and report training data.

III. NONSEGREGATED FACILITIES

(Applicable to all Federal-aid construction contracts and to all related subcontracts of $10,000 or more.)

a. By submission of this bid, the execution of this contract or subcontract, or the consummation of this material supply agreement or purchase order, as appropriate, the bidder, Federal-aid construction contractor, subcontractor, material supplier, or vendor, as appropriate, certifies that the firm does not maintain or provide for its employees any segregated facilities at any of its establishments, and that the firm does not permit its employees to perform their services at any location, under its control, where segregated facilities are maintained. The firm agrees that a breach of this certification is a violation of the EEO provisions of this contract. The firm further certifies that no employee will be denied access to adequate facilities on the basis of sex or disability.

b. As used in this certification, the term "segregated facilities" means any waiting rooms, work areas, restrooms and washrooms, restaurants and other eating areas, time clocks, locker rooms, and other storage or dressing areas, parking lots, drinking fountains, recreation or entertainment areas, transportation, and housing facilities provided for employees which are segregated by explicit directive, or are, in fact, segregated on the basis of race, color, religion, national origin, age or disability, because of habit, local custom, or otherwise. The only exception will be for the disabled when the demands for accessibility override (e.g. disabled parking).

c. The contractor agrees that it has obtained or will obtain identical certification from proposed subcontractors or material suppliers prior to award of subcontracts or consummation of material supply agreements of $10,000 or more and that it will retain such certifications in its files.

IV. PAYMENT OF PREDETERMINED MINIMUM WAGE

(Applicable to all Federal-aid construction contracts exceeding $2,000 and to all related subcontracts, except for projects located on roadways classified as local roads or rural minor collectors, which are exempt.)

1. General:

a. All mechanics and laborers employed or working upon the site of the work will be paid unconditionally and not less often than once a week and without subsequent deduction or rebate on any account [except such payroll deductions as are permitted by regulations (29 CFR 3)] issued by the Secretary of Labor under the Copeland Act (40 U.S.C. 276c) the full amounts of wages and bona fide fringe benefits (or cash equivalents thereof) due at time of payment. The payment shall be computed at wage rates not less than those contained in the wage determination of the Secretary of Labor (hereinafter "the wage determination") which is attached hereto and made a part hereof, regardless of any contractual relationship which may be alleged to exist between the contractor or its subcontractors and such laborers and mechanics. The wage determination (including any additional classifications and wage rates conformed under paragraph 2 of this Section IV and the DOL poster (WH-1321) or Form FHWA-1495) shall be

posted at all times by the contractor and its subcontractors at the site of the work in a prominent and accessible place where it can be easily seen by the workers. For the purpose of this Section, contributions made or costs reasonably anticipated for bona fide fringe benefits under Section 1(b)(2) of the Davis-Bacon Act (40 U.S.C. 276a) on behalf of laborers or mechanics are considered wages paid to such laborers or mechanics, subject to the provisions of Section IV, paragraph 3b, hereof. Also, for the purpose of this Section, regular contributions made or costs incurred for more than a weekly period (but not less often than quarterly) under plans, funds, or programs, which cover the particular weekly period, are deemed to be constructively made or incurred during such weekly period. Such laborers and mechanics shall be paid the appropriate wage rate and fringe benefits on the wage determination for the classification of work actually performed, without regard to skill, except as provided in paragraphs 4 and 5 of this Section IV.

b. Laborers or mechanics performing work in more than one classification may be compensated at the rate specified for each classification for the time actually worked therein, provided, that the employer's payroll records accurately set forth the time spent in each classification in which work is performed.

c. All rulings and interpretations of the Davis-Bacon Act and related acts contained in 29 CFR 1, 3, and 5 are herein incorporated by reference in this contract.

2. Classification:

a. The SHA contracting officer shall require that any class of laborers or mechanics employed under the contract, which is not listed in the wage determination, shall be classified in conformance with the wage determination.

b. The contracting officer shall approve an additional classification, wage rate and fringe benefits only when the following criteria have been met:

(1) the work to be performed by the additional classification requested is not performed by a classification in the wage determination;

(2) the additional classification is utilized in the area by the construction industry;

(3) the proposed wage rate, including any bona fide fringe benefits, bears a reasonable relationship to the wage rates contained in the wage determination; and

(4) with respect to helpers, when such a classification prevails in the area in which the work is performed.

c. If the contractor or subcontractors, as appropriate, the laborers and mechanics (if known) to be employed in the additional classification or their representatives, and the contracting officer agree on the classification and wage rate (including the amount designated for fringe benefits where appropriate), a report of the action taken shall be sent by the contracting officer to the DOL, Administrator of the Wage and Hour Division, Employment Standards Administration, Washington, D.C. 20210. The Wage and Hour Administrator, or an authorized representative, will approve, modify, or disapprove every additional classification action within 30 days of receipt and so advise the contracting officer or will notify the contracting officer within the 30-day period that additional time is necessary.

d. In the event the contractor or subcontractors, as appropriate, the laborers or mechanics to be employed in the additional classification or their representatives, and the contracting officer do not agree on the proposed classification and wage rate (including the amount designated for fringe benefits, where appropriate), the contracting officer shall refer the questions, including the views of all interested parties and the recommendation of the contracting officer, to the Wage and Hour Administrator for determination. Said Administrator, or an authorized representative, will issue a determination within 30 days of receipt and so advise the contracting officer or will notify the contracting officer within the 30-day period that additional time is necessary

e. The wage rate (including fringe benefits where appropriate) determined pursuant to paragraph 2c or 2d of this Section IV shall be paid to all workers performing work in the additional classification from the first day on which work is performed in the classification.

3. Payment of Fringe Benefits:

a. Whenever the minimum wage rate prescribed in the contract for a class of laborers or mechanics includes a fringe benefit which is not expressed as an hourly rate, the contractor or subcontractors, as appropriate, shall either pay the benefit as stated in the wage determination or shall pay another bona fide fringe benefit or an hourly case equivalent thereof.

b. If the contractor or subcontractor, as appropriate, does not make payments to a trustee or other third person, he/she may consider as a part of the wages of any laborer or mechanic the amount of any costs reasonably anticipated in providing bona fide fringe benefits under a plan or program, provided, that the Secretary of Labor has found, upon the written request of the contractor, that the applicable standards of the Davis-Bacon Act have been met. The Secretary of Labor may require the contractor to set aside in a separate account assets for the meeting of obligations under the plan or program.

4. Apprentices and Trainees (Programs of the U.S. DOL) and Helpers:

a. Apprentices:

(1) Apprentices will be permitted to work at less than the predetermined rate for the work they performed when they are employed pursuant to and individually registered in a bona fide apprenticeship program registered with the DOL, Employment and Training Administration, Bureau of Apprenticeship and Training, or with a State apprenticeship agency recognized by the Bureau, or if a person is employed in his/her first 90 days of probationary employment as an apprentice in such an apprenticeship program, who is not individually registered in the program, but who has been certified by the Bureau of Apprenticeship and Training or a State apprenticeship agency (where appropriate) to be eligible for probationary employment as an apprentice.

(2) The allowable ratio of apprentices to journeyman-level employees on the job site in any craft classification shall not be greater than the ratio permitted to the contractor as to the entire work force under the registered program. Any employee listed on a payroll at an apprentice wage rate, who is not registered or otherwise employed as stated above, shall be paid not less than the applicable wage rate listed in the wage determination for the classification of work actually performed. In addition, any apprentice performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed. Where a contractor or subcontractor is performing construction on a project in a locality other than that in which its program is registered, the ratios and wage rates (expressed in percentages of the journeyman-level hourly rate) specified in the contractor's or subcontractor's registered program shall be observed.

(3) Every apprentice must be paid at not less than the rate specified in the registered program for the apprentice's level of progress, expressed as a percentage of the journeyman-level hourly rate specified in the applicable wage determination. Apprentices shall be paid fringe benefits in accordance with the provisions of the apprenticeship program. If the apprenticeship program does not specify fringe benefits, apprentices must be paid the full amount of fringe benefits listed on the wage determination for the applicable classification. If the Administrator for the Wage and Hour Division determines that a different practice prevails for the applicable apprentice classification, fringes shall be paid in accordance with that determination.

(4) In the event the Bureau of Apprenticeship and Training, or a State apprenticeship agency recognized by the Bureau, withdraws approval of an apprenticeship program, the contractor or subcontractor will no longer be permitted to utilize apprentices at less than the applicable predetermined rate for the comparable work performed by regular employees until an acceptable program is approved.

b. Trainees:

(1) Except as provided in 29 CFR 5.16, trainees will not be permitted to work at less than the predetermined rate for the work performed unless they are employed pursuant to and individually registered in a program which has received prior approval, evidenced by formal certification by the DOL, Employment and Training Administration.

(2) The ratio of trainees to journeyman-level employees on the job site shall not be greater than permitted under the plan approved by the Employment and Training Administration. Any employee listed on the payroll at a trainee rate who is not registered and participating in a training plan approved by the Employment and Training Administration shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed. In addition, any trainee performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed.

(3) Every trainee must be paid at not less than the rate specified in the approved program for his/her level of progress, expressed as a percentage of the journeyman-level hourly rate specified in the applicable wage determination. Trainees shall be paid fringe benefits in accordance with the provisions of the trainee program. If the trainee program does not mention fringe benefits, trainees shall be paid the full amount of fringe benefits listed on the wage determination unless the Administrator of the Wage and Hour Division determines that there

is an apprenticeship program associated with the corresponding journeyman-level wage rate on the wage determination which provides for less than full fringe benefits for apprentices, in which case such trainees shall receive the same fringe benefits as apprentices.

(4) In the event the Employment and Training Administration withdraws approval of a training program, the contractor or subcontractor will no longer be permitted to utilize trainees at less than the applicable predetermined rate for the work performed until an acceptable program is approved.

c. Helpers**:**

Helpers will be permitted to work on a project if the helper classification is specified and defined on the applicable wage determination or is approved pursuant to the conformance procedure set forth in Section IV.2. Any worker listed on a payroll at a helper wage rate, who is not a helper under an approved definition, shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed.

5. Apprentices and Trainees (Programs of the U.S. DOT):

Apprentices and trainees working under apprenticeship and skill training programs which have been certified by the Secretary of Transportation as promoting EEO in connection with Federal-aid highway construction programs are not subject to the requirements of paragraph 4 of this Section IV. The straight time hourly wage rates for apprentices and trainees under such programs will be established by the particular programs. The ratio of apprentices and trainees to journeymen shall not be greater than permitted by the terms of the particular program.

6. Withholding:

The SHA shall upon its own action or upon written request of an authorized representative of the DOL withhold, or cause to be withheld, from the contractor or subcontractor under this contract or any other Federal contract with the same prime contractor, or any other Federally-assisted contract subject to Davis-Bacon prevailing wage requirements which is held by the same prime contractor, as much of the accrued payments or advances as may be considered necessary to pay laborers and mechanics, including apprentices, trainees, and helpers, employed by the contractor or any subcontractor the full amount of wages required by the contract. In the event of failure to pay any laborer or mechanic, including any apprentice, trainee, or helper, employed or working on the site of the work, all or part of the wages required by the contract, the SHA contracting officer may, after written notice to the contractor, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds until such violations have ceased.

7. Overtime Requirements:

No contractor or subcontractor contracting for any part of the contract work which may require or involve the employment of laborers, mechanics, watchmen, or guards (including apprentices, trainees, and helpers described in paragraphs 4 and 5 above) shall require or permit any laborer, mechanic, watchman, or guard in any workweek in which he/she is employed on such work, to work in excess of 40 hours in such workweek unless such laborer, mechanic, watchman, or guard receives compensation at a rate not less than one-and-one-half times his/her basic rate of pay for all hours worked in excess of 40 hours in such workweek.

8. Violation:

Liability for Unpaid Wages; Liquidated Damages: In the event of any violation of the clause set forth in paragraph 7 above, the contractor and any subcontractor responsible thereof shall be liable to the affected employee for his/her unpaid wages. In addition, such contractor and subcontractor shall be liable to the United States (in the case of work done under contract for the District of Columbia or a territory, to such District or to such territory) for liquidated damages. Such liquidated damages shall be computed with respect to each individual laborer, mechanic, watchman, or guard employed in violation of the clause set forth in paragraph 7, in the sum of $10 for each calendar day on which such employee was required or permitted to work in excess of the standard work week of 40 hours without payment of the overtime wages required by the clause set forth in paragraph 7.

9. Withholding for Unpaid Wages and Liquidated Damages:

The SHA shall upon its own action or upon written request of any authorized representative of the DOL withhold, or cause to be withheld, from any monies payable on account of work performed by the contractor or subcontractor under any such contract or any other Federal contract with the same prime contractor, or any other Federally-assisted contract subject to the Contract Work Hours and Safety Standards Act, which is held by the same prime contractor, such sums as may be determined to be necessary to satisfy any liabilities of such contractor or subcontractor for unpaid wages and liquidated damages as provided in the clause set forth in paragraph 8 above.

V. STATEMENTS AND PAYROLLS

(Applicable to all Federal-aid construction contracts exceeding $2,000 and to all related subcontracts, except for projects located on roadways classified as local roads or rural collectors, which are exempt.)

1. Compliance with Copeland Regulations (29 CFR 3):

   The contractor shall comply with the Copeland Regulations of the Secretary of Labor which are herein incorporated by reference.

2. Payrolls and Payroll Records:

   a. Payrolls and basic records relating thereto shall be maintained by the contractor and each subcontractor during the course of the work and preserved for a period of 3 years from the date of completion of the contract for all laborers, mechanics, apprentices, trainees, watchmen, helpers, and guards working at the site of the work.

   b. The payroll records shall contain the name, social security number, and address of each such employee; his or her correct classification; hourly rates of wages paid (including rates of contributions or costs anticipated for bona fide fringe benefits or cash equivalent thereof the types described in Section 1(b)(2)(B) of the Davis Bacon Act); daily and weekly number of hours worked; deductions made; and actual wages paid. In addition, for Appalachian contracts, the payroll records shall contain a notation indicating whether the employee does, or does not, normally reside in the labor area as defined in Attachment A, paragraph 1. Whenever the Secretary of Labor, pursuant to Section IV, paragraph 3b, has found that the wages of any laborer or mechanic include the amount of any costs reasonably anticipated in providing benefits under a plan or program described in Section 1(b)(2)(B) of the Davis Bacon Act, the contractor and each subcontractor shall maintain records which show that the commitment to provide such benefits is enforceable, that the plan or program is financially repossible, that the plan or program has been communicated in writing to the laborers or mechanics affected, and show the cost anticipated or the actual cost incurred in providing benefits. Contractors or subcontractors employing apprentices or trainees under approved programs shall maintain written evidence of the registration of apprentices and trainees, and ratios and wage rates prescribed in the applicable programs.

   c. Each contractor and subcontractor shall furnish, each week in which any contract work is performed, to the SHA resident engineer a payroll of wages paid each of its employees (including apprentices, trainees, and helpers, described in Section IV, paragraphs 4 and 5, and watchmen and guards engaged on work during the preceding weekly payroll period). The payroll submitted shall set out accurately and completely all of the information required to be maintained under paragraph 2b of this Section V. This information may be submitted in any form desired. Optional Form WH-347 is available for this purpose and may be purchased from the Superintendent of Documents (Federal stock number 029-005-0014-1), U.S. Government Printing Office, Washington, D.C. 20402. The prime contractor is responsible for the submission of copies of payrolls by all subcontractors.

   d. Each payroll submitted shall be accompanied by a "Statement of Compliance," signed by the contractor or subcontractor or his/her agent who pays or supervises the payment of the persons employed under the contract and shall certify the following:

      (1) that the payroll for the payroll period contains the information required to be maintained under paragraph 2b of this Section V and that such information is correct and complete;

      (2) that such laborer or mechanic (including each helper, apprentice, and trainee) employed on the contract during the payroll period has been paid the full weekly wages earned, without rebate, either directly or indirectly, and that no deductions have been made either directly or indirectly from the full wages earned, other than permissible deductions as set forth in the Regulations, 29 CFR 3;

      (3) that each laborer or mechanic has been paid not less that the applicable wage rate and fringe benefits or cash equivalent for the classification of worked performed, as specified in the applicable wage determination incorporated into the contract.

   e. The weekly submission of a properly executed certification set forth on the reverse side of Optional Form WH-347 shall satisfy the requirement for submission of the "Statement of Compliance" required by paragraph 2d of this Section V.

   f. The falsification of any of the above certifications may subject the contractor to civil or criminal prosecution under 18 U.S.C. 1001 and 31 U.S.C. 231.

g. The contractor or subcontractor shall make the records required under paragraph 2b of this Section V available for inspection, copying, or transcription by authorized representatives of the SHA, the FHWA, or the DOL, and shall permit such representatives to interview employees during working hours on the job. If the contractor or subcontractor fails to submit the required records or to make them available, the SHA, the FHWA, the DOL, or all may, after written notice to the contractor, sponsor, applicant, or owner, take such actions as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds. Furthermore, failure to submit the required records upon request or to make such records available may be grounds fordebarment action pursuant to 29 CFR 5.12.

VI. RECORD OF MATERIALS, SUPPLIES, AND LABOR

1. On all Federal-aid contracts on the National Highway System, except those which provide solely for the installation of protective devices at railroad grade crossings, those which are constructed on a force account or direct labor basis, highway beautification contracts, and contracts for which the total final construction cost for roadway and bridge is less than $1,000,000 (23 CFR 635) the contractor shall:

   a. Become familiar with the list of specific materials and supplies contained in Form FHWA-47, "Statement of Materials and Labor Used by Contractor of Highway Construction Involving Federal Funds," prior to the commencement of work under this contract.

   b. Maintain a record of the total cost of all materials and supplies purchased for and incorporated in the work, and also of the quantities of those specific materials and supplies listed on Form FHWA-47, and in the units shown on Form FHWA-47.

   c. Furnish, upon the completion of the contract, to the SHA resident engineer on Form FHWA-47 together with the data required in paragraph 1b relative to materials and supplies, a final labor summary of all contract work indicating the total hours worked and the total amount earned.

2. At the prime contractor's option, either a single report covering all contract work or separate reports for the contractor and for each subcontract shall be submitted.

VII. SUBLETTING OR ASSIGNING THE CONTRACT

1. The contractor shall perform with its own organization contract work amounting to not less than 30 percent (or a greater percentage if specified elsewhere in the contract) of the total original contract price, excluding any specialty items designated by the State. Specialty items may be performed by subcontract and the amount of any such specialty items performed may be deducted from the total original contract price before computing the amount of work required to be performed by the contractor's own organization (23 CFR 635).

   a. "Its own organization" shall be construed to include only workers employed and paid directly by the prime contractor and equipment owned or rented by the prime contractor, with or without operators. Such term does not include employees or equipment of a subcontractor, assignee, or agent of the prime contractor.

   b. "Specialty Items" shall be construed to be limited to work that requires highly specialized knowledge, abilities, or equipment not ordinarily available in the type of contracting organizations qualified and expected to bid on the contract as a whole and in general are to be limited to minor components of the overall contract.

2. The contract amount upon which the requirements set forth in paragraph 1 of Section VII is computed includes the cost of material and manufactured products which are to be purchased or produced by the contractor under the contract provisions.

3. The contractor shall furnish (a) a competent superintendent or supervisor who is employed by the firm, has full authority to direct performance of the work in accordance with the contract requirements, and is in charge of all construction operations (regardless of who performs the work) and (b) such other of its own organizational resources (supervision, management, and engineering services) as the SHA contracting officer determines is necessary to assure the performance of the contract.

4. No portion of the contract shall be sublet, assigned or otherwise disposed of except with the written consent of the SHA contracting officer, or authorized representative, and such consent when given shall not be construed to relieve the contractor of any responsibility for the fulfillment of the contract. Written consent will be given only after the SHA has assured that each subcontract is evidenced in writing and that it contains all pertinent provisions and requirements of the prime contract.

VIII. SAFETY: ACCIDENT PREVENTION

1. In the performance of this contract the contractor shall comply with all applicable Federal, State, and local laws governing safety, health, and sanitation (23 CFR 635). The contractor shall provide all safeguards, safety devices and protective equipment and take any other needed actions as it determines, or as the SHA contracting officer may determine, to be reasonably necessary to protect the life and health of employees on the job and the safety of the public and to protect property in connection with the performance of the work covered by the contract.

2. It is a condition of this contract, and shall be made a condition of each subcontract, which the contractor enters into pursuant to this contract, that the contractor and any subcontractor shall not permit any employee, in performance of the contract, to work in surroundings or under conditions which are unsanitary, hazardous or dangerous to his/her health or safety, as determined under construction safety and health standards (29 CFR 1926) promulgated by the Secretary of Labor, in accordance with Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C. 333).

3. Pursuant to 29 CFR 1926.3, it is a condition of this contract that the Secretary of Labor or authorized representative thereof, shall have right of entry to any site of contract performance to inspect or investigate the matter of compliance with the construction safety and health standards and to carry out the duties of the Secretary under Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C. 333).

IX. FALSE STATEMENTS CONCERNING HIGHWAY PROJECTS

In order to assure high quality and durable construction in conformity with approved plans and specifications and a high degree of reliability on statements and representations made by engineers, contractors, suppliers, and workers on Federal-aid highway projects, it is essential that all persons concerned with the project perform their functions as carefully, thoroughly, and honestly as possible. Willful falsification, distortion, or misrepresentation with respect to any facts related to the project is a violation of Federal law. To prevent any misunderstanding regarding the seriousness of these and similar acts, the following notice shall be posted on each Federal-aid highway project (23 CFR 635) in one or more places where it is readily available to all persons concerned with the project:

Notice To All Personnel Engaged On Federal-Aid Highway Projects

18 U.S.C. 1020 READS AS FOLLOWS:

"Whoever being an officer, agent, or employee of the United States, or any State or Territory, or whoever, whether a person, association, firm, or corporation, knowingly makes any false statement, false representation, or false report as to the character, quality, quantity, or cost of the material used or to be used, or the quantity or quality of the work performed or to be performed, or the cost thereof in connection with the submission of plans, maps, specifications, contracts, or costs of construction on any highway or related project submitted for approval to the Secretary of Transportation; or

Whoever knowingly makes any false statement, false representation, false report or false claim with respect to the character, quality, quantity, or cost of any work performed or to be performed, or materials furnished or to be furnished, in connection with the construction of any highway or related project approved by the Secretary of Transportation; or

Whoever knowingly makes any false statement or false representation as to material fact in any statement, certificate, or report submitted pursuant to provisions of the Federal-aid Roads Act approved July 1, 1916, (39 Stat. 355), as amended and supplemented;

Shall be fined not more that $10,000 or imprisoned not more than 5 years or both."

X. IMPLEMENTATION OF CLEAN AIR ACT AND FEDERAL WATER POLLUTION CONTROL ACT

(Applicable to all Federal-aid construction contracts and to all related subcontracts of $100,000 or more.)

By submission of this bid or the execution of this contract, or subcontract, as appropriate, the bidder, Federal-aid construction contractor, or subcontractor, as appropriate, will be deemed to have stipulated as follows:

1. That any facility that is or will be utilized in the performance of this contract, unless such contract is exempt under the Clean Air Act, as amended (42 U.S.C. 1857 et seq., as amended by Pub.L. 91-604), and under the Federal Water Pollution Control Act, as amended (33 U.S.C. 1251 et seq., as amended by Pub.L. 92-500), Executive Order 11738, and regulations in implementation thereof (40 CFR 15) is not listed, on the date of contract award, on the U.S. Environmental Protection Agency (EPA) List of Violating Facilities pursuant to 40 CFR 15.20.

2. That the firm agrees to comply and remain in compliance with all the requirements of Section 114 of the Clean Air Act and Section 308 of the Federal Water Pollution Control Act and all regulations and guidelines listed thereunder.

3. That the firm shall promptly notify the SHA of the receipt of any communication from the Director, Office of Federal Activities, EPA, indicating that a facility that is or will be utilized for the contract is under consideration to be listed on the EPA List of Violating Facilities.

4. That the firm agrees to include or cause to be included the requirements of paragraph 1 through 4 of this Section X in every nonexempt subcontract, and further agrees to take such action as the government may direct as a means of enforcing such requirements.

XI. CERTIFICATION REGARDING DEBARMENT, SUSPENSION, INELIGIBILITY AND VOLUNTARY EXCLUSION

1. Instructions for Certification - Primary Covered Transactions:

(Applicable to all Federal-aid contracts - 49 CFR 29)

a. By signing and submitting this proposal, the prospective primary participant is providing the certification set out below.

b. The inability of a person to provide the certification set out below will not necessarily result in denial of participation in this covered transaction. The prospective participant shall submit an explanation of why it cannot provide the certification set out below. The certification or explanation will be considered in connection with the department or agency's determination whether to enter into this transaction. However, failure of the prospective primary participant to furnish a certification or an explanation shall disqualify such a person from participation in this transaction.

c. The certification in this clause is a material representation of fact upon which reliance was placed when the department or agency determined to enter into this transaction. If it is later determined that the prospective primary participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the department or agency may terminate this transaction for cause of default.

d. The prospective primary participant shall provide immediate written notice to the department or agency to whom this proposal is submitted if any time the prospective primary participant learns that its certification was erroneous when submitted or has become erroneous by reason of changed circumstances.

e. The terms "covered transaction," "debarred," "suspended," "ineligible," "lower tier covered transaction," "participant," "person," "primary covered transaction," "principal," "proposal," and "voluntarily excluded," as used in this clause, have the meanings set out in the Definitions and Coverage sections of rules implementing Executive Order 12549. You may contact the department or agency to which this proposal is submitted for assistance in obtaining a copy of those regulations.

f. The prospective primary participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency entering into this transaction.

g. The prospective primary participant further agrees by submitting this proposal that it will include the clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transaction," provided by the department or agency entering into this covered transaction, without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions.

h. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant may decide the method and frequency by which it determines the eligibility of its principals. Each participant may, but is not required to, check the nonprocurement portion of the "Lists of Parties Excluded From Federal Procurement or Nonprocurement Programs" (Nonprocurement List) which is compiled by the General Services Administration.

i. Nothing contained in the foregoing shall be construed to require establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of participant is

not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

j. Except for transactions authorized under paragraph f of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the department or agency may terminate this transaction for cause or default.

Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion — Primary Covered Transactions

1. The prospective primary participant certifies to the best of its knowledge and belief, that it and its principals:

   a. Are not presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from covered transactions by any Federal department or agency;

   b. Have not within a 3-year period preceding this proposal been convicted of or had a civil judgement rendered against them for commission of fraud or a criminal offense in connection with obtaining, attempting to obtain, or performing a public (Federal, State or local) transaction or contract under a public transaction; violation of Federal or State antitrust statutes or commission of embezzlement, theft, forgery, bribery, falsification or destruction of records, making false statements, or receiving stolen property;

   c. Are not presently indicted for or otherwise criminally or civilly charged by a governmental entity (Federal, State or local) with commission of any of the offenses enumerated in paragraph 1b of this certification; and

   d. Have not within a 3-year period preceding this application/proposal had one or more public transactions (Federal, State or local) terminated for cause or default.

2. Where the prospective primary participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

2. Instructions for Certification - Lower Tier Covered Transactions:

(Applicable to all subcontracts, purchase orders and other lower tier transactions of $25,000 or more - 49 CFR 29)

a. By signing and submitting this proposal, the prospective lower tier is providing the certification set out below.

b. The certification in this clause is a material representation of fact upon which reliance was placed when this transaction was entered into. If it is later determined that the prospective lower tier participant knowingly rendered an erroneous certification, inaddition to other remedies available to the Federal Government, the department or agency with which this transaction originated may pursue available remedies, including suspension and/or debarment.

c. The prospective lower tier participant shall provide immediate written notice to the person to which this proposal is submitted if at any time the prospective lower tier participant learns that its certification was erroneous by reason of changed circumstances.

d. The terms "covered transaction," "debarred," "suspended," "ineligible," "primary covered transaction," "participant," "person," "principal," "proposal," and "voluntarily excluded," as used in this clause, have the meanings set out in the Definitions and Coverage sections of rules implementing Executive Order 12549. You may contact the person to which this proposal is submitted for assistance in obtaining a copy of those regulations.

e. The prospective lower tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency with which this transaction originated.

f. The prospective lower tier participant further agrees by submitting this proposal that it will include this clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transaction," without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions.

g. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant may decide the method and frequency by which it determines the eligibility of its principals. Each participant may, but is not required to, check the Nonprocurement List.

h. Nothing contained in the foregoing shall be construed to require establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

i. Except for transactions authorized under paragraph e of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the department or agency with which this transaction originated may pursue available remedies, including suspension and/or debarment.

Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion — Lower Tier Covered Transactions

1. The prospective lower tier participant certifies, by submission of this proposal, that neither it nor its principals is presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participation in this transaction by any Federal department or agency.

2. Where the prospective lower tier participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

XII. CERTIFICATION REGARDING USE OF CONTRACT FUNDS FOR LOBBYING

(Applicable to all Federal-aid construction contracts and to all related subcontracts which exceed $100,000 - 49 CFR 20)

1. The prospective participant certifies, by signing and submitting this bid or proposal, to the best of his or her knowledge and belief, that:

   a. No Federal appropriated funds have been paid or will be paid, by or on behalf of the undersigned, to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with the awarding of any Federal contract, the making of any Federal grant, the making of any Federal loan, the entering into of any cooperative agreement, and the extension, continuation, renewal, amendment, or modification of any Federal contract, grant, loan, or cooperative agreement.

   b. If any funds other than Federal appropriated funds have been paid or will be paid to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connec tion with this Federal contract, grant, loan, or cooperative agreement, the undersigned shall complete and submit Standard Form-LLL, "Disclosure Form to Report Lobbying," in accordance with its instructions.

2. This certification is a material representation of fact upon which reliance was placed when this transaction was made or entered into. Submission of this certification is a prerequisite for making or entering into this transaction imposed by 31 U.S.C. 1352. Any person who fails to file the required certification shall be subject to a civil penalty of not less than $10,000 and not more than $100,000 for each such failure.

3. The prospective participant also agrees by submitting his or her bid or proposal that he or she shall require that the language of this certification be included in all lower tier subcontracts, which exceed $100,000 and that all such recipients shall certify and disclose accordingly.

FEDERAL-AID FEMALE AND MINORITY GOALS

In accordance with Section II, "Nondiscrimination," of "Required Contract Provisions Federal-aid Construction Contracts" the following are the goals for female utilization:

Goal for Women (applies nationwide).....(percent) 6.9

The following are goals for minority utilization:

CALIFORNIA ECONOMIC AREA

		Goal (Percent)
174	**Redding, CA:**	
	Non-SMSA Counties	6.8
	CA Lassen; CA Modoc;CA Plumas;CA Shasta; CA Siskiyou; CA Tehama.	
175	**Eureka, CA**	
	Non-SMSA Counties	6.6
	CA Del Norte; CA Humboldt; CA Trinity.	
176	**San Francisco-Oakland-San Jose, CA:**	
	SMSA Counties:	
	7120 Salinas-Seaside-Monterey, CA	28.9
	CA Monterey.	
	7360 San Francisco-Oakland	25.6
	CA Alameda; CA Contra Costa; CA Marin; CA San Francisco; CA San Mateo.	
	7400 San Jose, CA	19.6
	CA Santa Clara.	
	7485 Santa Cruz, CA.	14.9
	CA Santa Cruz.	
	7500 Santa Rosa, CA	9.1
	CA Sonoma.	
	8720 Vallejo-Fairfield- Napa, CA	17.1
	CA Napa; CA Solano	
	Non-SMSA Counties	23.2
	CA Lake; CA Mendocino; CA San Benito	
177	**Sacramento, CA** :	
	SMSA Counties:	
	6920 Sacramento, CA	16.1
	CA Placer; CA Sacramento; CA Yolo.	
	Non-SMSA Counties	14.3
	CA Butte; CA Colusa; CA El Dorado; CA Glenn; CA Nevada; CA Sierra; CA Sutter; CA Yuba.	
178	**Stockton-Modesto, CA:**	
	SMSA Counties:	
	5170 Modesto, CA	12.3
	CA Stanislaus.	
	8120 Stockton, CA	24.3
	CA San Joaquin.	
	Non-SMSA Counties	19.8
	CA Alpine; CA Amador; CA Calaveras; CA Mariposa;CA Merced; CA Tuolumne.	

		Goal (Percent)
179	**Fresno-Bakersfield, CA**	
	SMSA Counties:	
	0680 Bakersfield, CA	19.1
	CA Kern.	
	2840 Fresno, CA	26.1
	CA Fresno.	
	Non-SMSA Counties	23.6
	CA Kings; CA Madera; CA Tulare.	
180	**Los Angeles, CA:**	
	SMSA Counties:	
	0360 Anaheim-Santa Ana-Garden Grove, CA	11.9
	CA Orange.	
	4480 Los Angeles-Long Beach, CA	28.3
	CA Los Angeles.	
	6000 Oxnard-Simi Valley-Ventura, CA	21.5
	CA Ventura.	
	6780 Riverside-San Bernardino-Ontario, CA.	19.0
	CA Riverside; CA San Bernardino.	
	7480 Santa Barbara-Santa Maria-Lompoc, CA	19.7
	CA Santa Barbara.	
	Non-SMSA Counties	24.6
	CA Inyo; CA Mono; CA San Luis Obispo.	
181	**San Diego, CA:**	
	SMSA Counties	
	7320 San Diego, CA.	16.9
	CA San Diego.	
	Non-SMSA Counties	18.2
	CA Imperial.	

In addition to the reporting requirements set forth elsewhere in this contract the Contractor and subcontractors holding subcontracts, not including material suppliers, of $10,000 or more, shall submit for every month of July during which work is performed, employment data as contained under Form FHWA PR-1391 (Appendix C to 23 CFR, Part 230), and in accordance with the instructions included thereon.

FEDERAL REQUIREMENT TRAINING SPECIAL PROVISIONS

As part of the Contractor's equal employment opportunity affirmative action program, training shall be provided as follows:

The Contractor shall provide on-the-job training to develop full journeymen in the types of trades or job classification involved.

The goal for the number of trainees or apprentices to be trained under the requirements of this special provision will be 12.

In the event the Contractor subcontracts a portion of the contract work, he shall determine how many, if any, of the trainees or apprentices are to be trained by the subcontractor, provided however, that the Contractor shall retain the primary responsibility for meeting the training requirements imposed by this special provision. The Contractor shall also insure that this Training Special Provision is made applicable to such subcontract. Where feasible, 25 percent of trainees or apprentices in each occupation shall be in their first year of apprenticeship or training.

The number of trainees or apprentices shall be distributed among the work classifications on the basis of the Contractor's needs and the availability of journeymen in the various classifications within a reasonable area of recruitment. Prior to commencing work, the Contractor shall submit to the Department for approval the number of trainees or apprentices to be trained in each selected classification and training program to be used. Furthermore, the Contractor shall specify the starting time for training in each of the classifications. The Contractor will be credited for each trainee or apprentice employed by him on the contract work who is currently enrolled or becomes enrolled in an approved program and will be reimbursed for such trainees or apprentices as provided hereinafter.

Training and upgrading of minorities and women toward journeymen status is a primary objective of this Training Special Provision. Accordingly, the Contractor shall make every effort to enroll minority and women trainees or apprentices (e.g., by conducting systematic and direct recruitment through public and private sources likely to yield minority and women trainees or apprentices) to the extent such persons are available within a reasonable area of recruitment. The Contractor will be responsible for demonstrating the steps that he has taken in pursuance thereof, prior to a determination as to whether the Contractor is in compliance with this Training Special Provision. This training commitment is not intended, and shall not be used, to discriminate against any applicant for training, whether a member of a minority group or not.

No employee shall be employed as a trainee or apprentice in any classification in which he has successfully completed a training course leading to journeyman status or in which he has been employed as a journeyman. The Contractor should satisfy this requirement by including appropriate questions in the employee application or by other suitable means. Regardless of the method used the Contractor's records should document the findings in each case.

The minimum length and type of training for each classification will be as established in the training program selected by the Contractor and approved by both the Department and the Federal Highway Administration. The Department and the Federal Highway Administration will approve a program if it is reasonably calculated to meet the equal employment opportunity obligations of the Contractor and to qualify the average trainee or apprentice for journeyman status in the classification concerned by the end of the training period. Furthermore, apprenticeship programs registered with the U.S. Department of Labor, Bureau of Apprenticeship and Training, or with the State of California, Department of Industrial Relations, Division of Apprenticeship Standards recognized by the Bureau and training programs approved but not necessarily sponsored by the U.S. Department of Labor, Manpower Administration, Bureau of Apprenticeship and Training shall also be considered acceptable provided it is being administered in a manner consistent with the equal employment obligations of Federal-aid highway construction contracts. Approval or acceptance of a training program shall be obtained from the State prior to commencing work on the classification covered by the program. It is the intention of these provisions that training is to be provided in the construction crafts rather than clerk-typists or secretarial-type positions. Training is permissible in lower level management positions such as office engineers, estimators, timekeepers, etc., where the training is oriented toward construction applications. Training in the laborer classification may be permitted provided that significant and meaningful training is provided and approved by the division office. Some offsite training is permissible as long as the training is an integral part of an approved training program and does not comprise a significant part of the overall training.

Except as otherwise noted below, the Contractor will be reimbursed 80 cents per hour of training given an employee on this contract in accordance with an approved training program. As approved by the Engineer, reimbursement will be made for training of persons in excess of the number specified herein. This reimbursement will be made even though the Contractor receives additional training program funds from other sources, provided such other source does not specifically prohibit the Contractor from receiving other reimbursement. Reimbursement for offsite training indicated above may only be made to the Contractor where he does one or more of the following and the trainees or apprentices are concurrently employed on a Federal-aid project; contributes to the cost of the training, provides the instruction to the trainee or apprentice or pays the trainee's or apprentice's wages during the offsite training period.

No payment shall be made to the Contractor if either the failure to provide the required training, or the failure to hire the trainee or apprentice as a journeyman, is caused by the Contractor and evidences a lack of good faith on the part of the Contractor in meeting the requirements of this Training Special Provision. It is normally expected that a trainee or apprentice will begin his training on the project as soon as feasible after start of work utilizing the skill involved and remain on the project as long as training opportunities exist in his work classification or until he has completed his training program. It is not required that all trainees or apprentices be on board for the entire length of the contract. A Contractor will have fulfilled his responsibilities under this Training Special Provision if he has provided acceptable training to the number of trainees or

apprentices specified. The number trained shall be determined on the basis of the total number enrolled on the contract for a significant period.

Only trainees or apprentices registered in a program approved by the State of California's State Administrator of Apprenticeship may be employed on the project and said trainees or apprentices shall be paid the standard wage specified under the regulations of the craft or trade at which they are employed.

The Contractor shall furnish the trainee or apprentice a copy of the program he will follow in providing the training. The Contractor shall provide each trainee or apprentice with a certification showing the type and length of training satisfactorily completed.