

INFORMATION HANDOUT

For Contract No. 03-1F4204

At 03-But-99-40.5/40.8

Identified by

Project ID 0300001119

AGREEMENTS

California Department of Fish and Wildlife – Streambed Alteration Agreement

Notification No. 1600-2014-0021-R2

MATERIALS INFORMATION

Revised Foundation Report for Rock Creek Bridge (Widen), Bridge No. 12-0027, dated March 10, 2014

Final Hydraulic Report for Rock Creek, dated February 20, 2014

Asbestos and Lead-Containing Paint Survey Report

CALIFORNIA DEPARTMENT OF FISH AND WILDLIFE
NORTH CENTRAL REGION
1701 NIMBUS ROAD, SUITE A
RANCHO CORDOVA, CA 95670



STREAMBED ALTERATION AGREEMENT
NOTIFICATION NO. 1600-2014-0021-R2
Rock Creek

CALIFORNIA DEPARTMENT OF TRANSPORTATION
STATE ROUTE 99 – ROCK CREEK BRIDGE WIDENING PROJECT

This Streambed Alteration Agreement (Agreement) is entered into between the California Department of Fish and Wildlife (DFW) and California Department of Transportation (Caltrans) (Permittee) as represented by Rodney Murphy.

RECITALS

WHEREAS, pursuant to Fish and Game Code (FGC) section 1602, Permittee notified DFW on February 7, 2014 that Permittee intends to complete the project described herein.

WHEREAS, pursuant to FGC section 1603, DFW has determined that the project could substantially adversely affect existing fish or wildlife resources and has included measures in the Agreement necessary to protect those resources.

WHEREAS, Permittee has reviewed the Agreement and accepts its terms and conditions, including the measures to protect fish and wildlife resources.

NOW THEREFORE, Permittee agrees to complete the project in accordance with the Agreement.

PROJECT LOCATION

The project is located along State Route 99 (SR99) where it crosses Rock Creek in Butte County, State of California; Latitude 39.8193 North, Longitude -121.9224 East, U.S. Geological Survey (USGS) map Nord (Attachment A: Project Maps).

PROJECT DESCRIPTION

The California Department of Transportation (Caltrans) proposes to widen the Rock Creek Bridge (BR # 12-0027) along SR-99 (Post Mile 40.55/40.75) north of Chico in Butte County. The scope of work includes: widening the bridge deck so that it will have 8 ft. shoulders; widen the roadway approach shoulders to 8 ft.; and add ½ ton Rock Slope Protection (RSP) in the creek channel underneath the bridge placed around

bridge columns that will be in the active channel, for scour protection. Fourteen new bridge columns will be installed and the 28 existing columns will be removed. Bridge columns will be poured into pre-drilled holes, no pile driving will take place for this project. All work within the streambed will take place when the creek is dry.

All figures and minimization measures included in the Notification of Streambed Alteration No. 1600-2014-0021-R2 shall be implemented.

PROJECT IMPACTS

Existing fish or wildlife resources the project could substantially adversely affect include: riparian vegetation, nesting raptors and migratory birds, other aquatic and terrestrial plant and wildlife species, cold water fish species.

The adverse effects the project could have on the fish or wildlife resources identified above include: disturbance from project activity; direct take of terrestrial species and of non-fish aquatic species; disruption to nesting birds and other wildlife; and loss or impediment of terrestrial animal species travel routes due to temporary structures such as survey tape, erosion protection materials etc.

MEASURES TO PROTECT FISH AND WILDLIFE RESOURCES

1. Administrative Measures

Permittee shall meet each administrative requirement described below.

- 1.1 Documentation at Project Site. Permittee shall make the Agreement, any extensions and amendments to the Agreement, and all related notification materials and California Environmental Quality Act (CEQA) documents, readily available at the project site at all times and shall be presented to DFW personnel, or personnel from another state, federal, or local agency upon request.
- 1.2 Providing Agreement to Persons at Project Site. Permittee shall provide copies of the Agreement and any extensions and amendments to the Agreement to all persons who will be working on the project at the project site on behalf of Permittee, including but not limited to contractors, subcontractors, inspectors, and monitors.
- 1.3 Notification of Conflicting Provisions. Permittee shall notify DFW if Permittee determines or learns that a provision in the Agreement might conflict with a provision imposed on the project by another local, state, or federal agency. In that event, DFW shall contact Permittee to resolve any conflict.
- 1.4 Project Site Entry. Permittee agrees that DFW personnel may enter the project site at any time to verify compliance with the Agreement.

- 1.5 Does Not Authorize "Take." This Agreement does not authorize "take" of any listed species. Take is defined as hunt, pursue, catch, capture or kill or attempt to hunt, pursue, catch, capture, or kill. If there is potential for take of any listed species to occur, the Permittee shall consult with DFW as outlined in FGC Section 2081 and shall obtain the required state and federal threatened and endangered species permits.
- 1.6 Notification of Project Modification. Permittee agrees to notify DFW of any modifications made to the project plans submitted to DFW.

2. Avoidance and Minimization Measures

To avoid or minimize adverse impacts to fish and wildlife resources identified above, Permittee shall implement each measure listed below.

- 2.1 CEQA Compliance. Permittee shall implement and adhere to the mitigation measures in the Biological Resources section of the Mitigated Negative Declaration (SCH Number: 2012092008) adopted by the lead agency, Caltrans, for the Project pursuant to the California Environmental Quality Act (CEQA) on December 12, 2012 unless those mitigation measures are less protective of fish and wildlife or conflict with the conditions of this Agreement.
- 2.2 Work Period. The time period for completing the work within the active channel shall be restricted to periods when the creek is not flowing. Work within the active channel shall be confined to the period of June 15 to October 15. Revegetation, restoration and erosion control work is not confined to this time period.
- 2.3 Work Period in Dry Weather Only. Work within Rock Creek shall be restricted to periods of dry weather. Precipitation forecasts and potential increases in stream flow shall be considered when planning construction activities. Construction activities shall cease and all necessary erosion control measures shall be implemented prior to the onset of precipitation. Construction activities halted due to precipitation may resume when precipitation ceases and the National Weather Service 72 hour weather forecast indicates a 20% or less chance of precipitation, provided no work occurs in the stream bed if water is flowing. If a construction phase may cause the introduction of sediments into the stream: 1) no phase of the project shall be started in May or November of any year, unless all work for that phase and all associated erosion control measures are completed prior to the onset of precipitation; and 2) no phase of the project shall commence unless all equipment and materials are removed from the channel at least 12 hours prior to the onset of precipitation and all associated erosion control measures are in place prior to the onset of precipitation. No work shall occur during a dry-out period of 24 hours after the above referenced wet weather. Weather forecasts shall be documented upon request by DFW.

- 2.4 Work Period Modification. If Permittee needs more time to complete the project activity, the work may be permitted outside of the work period and extended on a day-to-day basis (or for some other set period of time) by DFW representative who reviewed the project, or if unavailable, through contact with the Regional office. Permittee shall submit a written request for a work period variance to DFW. The work period variance request shall: 1) describe the extent of work already completed; 2) detail the activities that remain to be completed; 3) detail the time required to complete each of the remaining activities; and 4) provide photographs of both the current work completed and the proposed site for continued work. The work period variance request should consider the effects of increased stream flows and rain delays. Work period variances are issued at the discretion of DFW. DFW will review the written request to work outside of the established work period. DFW reserves the right to require additional measures to protect fish and wildlife resources as a condition for granting the variance. DFW will have ten (10) calendar days to review the proposed work period variance.
- 2.5 Bird Nests. It is unlawful to take, possess, or needlessly destroy the nest or eggs of any bird except as otherwise provided by the Fish and Game Code. No trees that contain active nests of birds shall be disturbed until all eggs have hatched and young birds have fledged without prior consultation and approval of a DFW representative.
- 2.6 Swallow Nesting Considerations. Construction shall either occur outside of the swallow nesting period (March 15 through August 31), or the suitable bridge nesting habitat shall be excluded from avian access (via netting, plastic sheeting or similar material) by the Permittee before initiation of the breeding season to prevent nesting. The material shall remain in place until August 1 or until construction activities at the site are complete. The material shall be anchored such that swallows cannot attach their nests to the structure through gaps in the material. If swallows begin building nests on the structure after installation, the mud placed by the swallows shall be removed and the material's integrity repaired. In lieu of physical barriers to suitable bridge nesting habitat, nests can be knocked down daily with a pole/scrapper until birds cease nest building behavior. During this period, completed nests and nests with eggs are not to be removed.
- 2.7 Removal of Trees/Shrubs During Fall/Winter Months. To avoid potential impact to tree nesting birds, trees and shrubs designated for removal may be cut down during the time period of November 1 to February 15. Tree and shrub removal may commence provided that that no birds are nesting, or using the site as a rookery at the time of removal.
- 2.8 Vegetation Removal. Disturbance or removal of vegetation shall not exceed the minimum necessary to complete operations. Except for the trees specifically identified for removal in the notification, no native trees with a trunk diameter at breast height (DBH) in excess of four (4) inches shall be removed or damaged without prior consultation and approval of a DFW representative. Using hand tools

(clippers, chain saw, etc.), trees may be trimmed to the extent necessary to gain access to the work sites. All cleared material/vegetation shall be removed out of the riparian/stream zone.

- 2.9 Sediment Control. Precautions to minimize turbidity/siltation shall be taken into account during project planning and implementation. This may require the placement of silt fencing, coir logs, coir rolls, straw bale dikes, or other siltation barriers so that silt and/or other deleterious materials are not allowed to pass to downstream reaches. Materials composing the silt barrier shall not pose an entanglement risk to fish or wildlife such as monofilament mesh and non-biodegradable synthetic erosion blankets. Passage of sediment beyond the sediment barrier(s) is prohibited. If any sediment barrier fails to retain sediment, corrective measures shall be taken. The sediment barrier(s) shall be maintained in good operating condition throughout the construction period and the following rainy season. Maintenance includes, but is not limited to, removal of accumulated silt and/or replacement of damaged siltation barriers. The Permittee is responsible for the removal of non-biodegradable silt barriers (such as plastic silt fencing) after the disturbed areas have been stabilized with erosion control vegetation (usually after the first growing season). Upon DFW determination that turbidity/siltation levels resulting from project related activities constitute a threat to aquatic life, activities associated with the turbidity/siltation shall be halted until effective DFW approved control devices are installed or abatement procedures are initiated.
- 2.10 Seeding Requirement. Permittee shall restore all exposed/disturbed areas and access points within the work area, by seeding with a locally native seed mix, unless otherwise agreed upon by DFW. Seed mix shall be pre-approved by DFW. Revegetation shall be completed as soon as possible after construction activities in those areas cease. Seeding shall be broadcast straw, jute netting coconut fiber blanket or similar erosion control material.
- 2.11 Rock Slope Protection. Rock slope protection (RSP) materials shall consist of clean rock, competent for the application, sized and properly installed to resist washout. RSP is only authorized for placement around bridge columns within the active channel in order to prevent scour.
- 2.12 Pollution Control. Utilize Best Management Practices (BMPs) to prevent spills and leaks into water bodies. If maintenance or refueling of vehicles or equipment must occur on-site, use a designated area and/or a secondary containment, located away from drainage courses to prevent the runoff of storm water and the runoff of spills. Ensure that all vehicles and equipment are in good working order (no leaks). Place drip pans or absorbent materials under vehicles and equipment when not in use. Ensure that all construction areas have proper spill clean-up materials (absorbent pads, sealed containers, booms, etc.) to contain the movement of any spilled substances. Any other substances which could be hazardous to aquatic life, resulting from project related activities, shall be

prevented from contaminating the soil and/or entering the waters of the state. Any of these materials, placed within or where they may enter a stream or lake by the Applicant or any party working under contract or with the permission of the Permittee, shall be removed immediately. DFW shall be notified immediately by the Permittee of any spills and shall be consulted regarding clean-up procedures.

- 2.13 Designated Representative. Before initiating ground- or vegetation-disturbing project activities, Permittee shall designate a representative (Designated Representative) responsible for communications with the CDFW and overseeing compliance with this Agreement. The Permittee shall notify the CDFW in writing thirty (30) days prior to commencement of ground- or vegetation-disturbing activities of the Designated Representative's name, business address, and contact information. Permittee shall notify the CDFW in writing if a substitute Designated Representative is selected or identified at any time during the term of this Agreement.
- 2.14 Designated Biologist. At least thirty (30) days before initiating ground- or vegetation-disturbing activities, Permittee shall submit to the CDFW in writing the name, qualifications, business address, and contact information for a biological monitor (Designated Biologist). Permittee shall obtain the CDFW's written approval of the Designated Biologist prior to the commencement of project activities in the stream. The Designated Biologist shall be knowledgeable and experienced in the biology and natural history of local fish and wildlife resources present at the project site. The Designated Biologist shall be responsible for monitoring all project activities, including construction and any ground- or vegetation-disturbing activities in areas subject to this Agreement.
- 2.15 Designated Biologist Authority. The Designated Biologist shall have authority to immediately stop any activity that is not in compliance with this Agreement, and/or to order any reasonable measure to avoid or minimize impacts to fish and wildlife resources. Neither the Designated Biologist nor the CDFW shall be liable for any costs incurred as a result of compliance with this measure. This includes cease-work orders issued by the CDFW.

3. Reporting Measures

Permittee shall meet each reporting requirement described below.

- 3.1 The Permittee shall notify DFW within two working days of beginning work within the stream zone. Notification shall be submitted as instructed in Contact Information section below. Email notification is preferred.
- 3.2 Upon completion of the project activities described in this agreement, the project area shall be digitally photographed. Photographs shall be submitted to DFW within fifteen (15) days of project completion. Photographs and notification of

project completion shall be submitted as instructed in Contact Information section below. Email submittal is preferred.

CONTACT INFORMATION

Any communication that Permittee or DFW submits to the other shall be in writing and any communication or documentation shall be delivered to the address below by U.S. mail, fax, or email, or to such other address as Permittee or DFW specifies by written notice to the other.

To Permittee:

Rodney Murphy
California Department of Transportation
703 B Street
Marysville, CA 95901
Email: rodney_murphy@dot.ca.gov

To DFW:

Department of Fish and Wildlife
North Central Region
1701 Nimbus Road, Suite A
Rancho Cordova, CA 95670
Attn: Lake and Streambed Alteration Program – Tim Nosal
Notification #1600-2014-0021 R2

Fax: 916-358-2912
Email: r2lsa@wildlife.ca.gov

LIABILITY

Permittee shall be solely liable for any violations of the Agreement, whether committed by Permittee or any person acting on behalf of Permittee, including its officers, employees, representatives, agents or contractors and subcontractors, to complete the project or any activity related to it that the Agreement authorizes.

This Agreement does not constitute DFW's endorsement of, or require Permittee to proceed with the project. The decision to proceed with the project is Permittee's alone.

SUSPENSION AND REVOCATION

DFW may suspend or revoke in its entirety the Agreement if it determines that Permittee or any person acting on behalf of Permittee, including its officers, employees,

representatives, agents, or contractors and subcontractors, is not in compliance with the Agreement.

Before DFW suspends or revokes the Agreement, it shall provide Permittee written notice by certified or registered mail that it intends to suspend or revoke. The notice shall state the reason(s) for the proposed suspension or revocation, provide Permittee an opportunity to correct any deficiency before DFW suspends or revokes the Agreement, and include instructions to Permittee, if necessary, including but not limited to a directive to immediately cease the specific activity or activities that caused DFW to issue the notice.

ENFORCEMENT

Nothing in the Agreement precludes DFW from pursuing an enforcement action against Permittee instead of, or in addition to, suspending or revoking the Agreement.

Nothing in the Agreement limits or otherwise affects DFW's enforcement authority or that of its enforcement personnel.

OTHER LEGAL OBLIGATIONS

This Agreement does not relieve Permittee or any person acting on behalf of Permittee, including its officers, employees, representatives, agents, or contractors and subcontractors, from obtaining any other permits or authorizations that might be required under other federal, state, or local laws or regulations before beginning the project or an activity related to it.

This Agreement does not relieve Permittee or any person acting on behalf of Permittee, including its officers, employees, representatives, agents, or contractors and subcontractors, from complying with other applicable statutes in the FGC including, but not limited to, FGC sections 2050 *et seq.* (threatened and endangered species), 3503 (bird nests and eggs), 3503.5 (birds of prey), 5650 (water pollution), 5652 (refuse disposal into water), 5901 (fish passage), 5937 (sufficient water for fish), and 5948 (obstruction of stream).

Nothing in the Agreement authorizes Permittee or any person acting on behalf of Permittee, including its officers, employees, representatives, agents, or contractors and subcontractors, to trespass.

AMENDMENT

DFW may amend the Agreement at any time during its term if DFW determines the amendment is necessary to protect an existing fish or wildlife resource.

Permittee may amend the Agreement at any time during its term, provided the amendment is mutually agreed to in writing by DFW and Permittee. To request an amendment, Permittee shall submit to DFW a completed DFW "Request to Amend Lake or Streambed Alteration" form and include with the completed form payment of the corresponding amendment fee identified in DFW's current fee schedule (see Cal. Code Regs., tit. 14, § 699.5).

TRANSFER AND ASSIGNMENT

This Agreement may not be transferred or assigned to another entity, and any purported transfer or assignment of the Agreement to another entity shall not be valid or effective, unless the transfer or assignment is requested by Permittee in writing, as specified below, and thereafter DFW approves the transfer or assignment in writing.

The transfer or assignment of the Agreement to another entity shall constitute a minor amendment, and therefore to request a transfer or assignment, Permittee shall submit to DFW a completed DFW "Request to Amend Lake or Streambed Alteration" form and include with the completed form payment of the minor amendment fee identified in DFW's current fee schedule (see Cal. Code Regs., tit. 14, § 699.5).

EXTENSIONS

In accordance with FGC section 1605(b), Permittee may request one extension of the Agreement, provided the request is made prior to the expiration of the Agreement's term. To request an extension, Permittee shall submit to DFW a completed DFW "Request to Extend Lake or Streambed Alteration" form and include with the completed form payment of the extension fee identified in DFW's current fee schedule (see Cal. Code Regs., tit. 14, § 699.5). DFW shall process the extension request in accordance with FGC 1605(b) through (e).

If Permittee fails to submit a request to extend the Agreement prior to its expiration, Permittee must submit a new notification and notification fee before beginning or continuing the project the Agreement covers (Fish & G. Code, § 1605, subd. (f)).

EFFECTIVE DATE

The Agreement becomes effective on the date of DFW's signature, which shall be: 1) after Permittee's signature; 2) after DFW complies with all applicable requirements under the California Environmental Quality Act (CEQA); and 3) after payment of the applicable FGC section 711.4 filing fee listed at http://www.wildlife.ca.gov/habcon/ceqa/ceqa_changes.html.

TERM

This Agreement shall expire within five (5) years of DFW's signature, unless it is terminated or extended before then. All provisions in the Agreement shall remain in force throughout its term. Permittee shall remain responsible for implementing any provisions specified herein to protect fish and wildlife resources after the Agreement expires or is terminated, as FGC section 1605(a)(2) requires.

EXHIBIT

The document listed below is included as an exhibit to the Agreement.

Attachment A: Project Maps

AUTHORITY

If the person signing the Agreement (signatory) is doing so as a representative of Permittee, the signatory hereby acknowledges that he or she is doing so on Permittee's behalf and represents and warrants that he or she has the authority to legally bind Permittee to the provisions herein.

AUTHORIZATION

This Agreement authorizes only the project described herein. If Permittee begins or completes a project different from the project the Agreement authorizes, Permittee may be subject to civil or criminal prosecution for failing to notify DFW in accordance with FGC section 1602.

CONCURRENCE

The undersigned accepts and agrees to comply with all provisions contained herein.

FOR CALTRANS

Rodney Murphy
Project Manager

Date

FOR DEPARTMENT OF FISH AND WILDLIFE

Tina Bartlett
Regional Manager

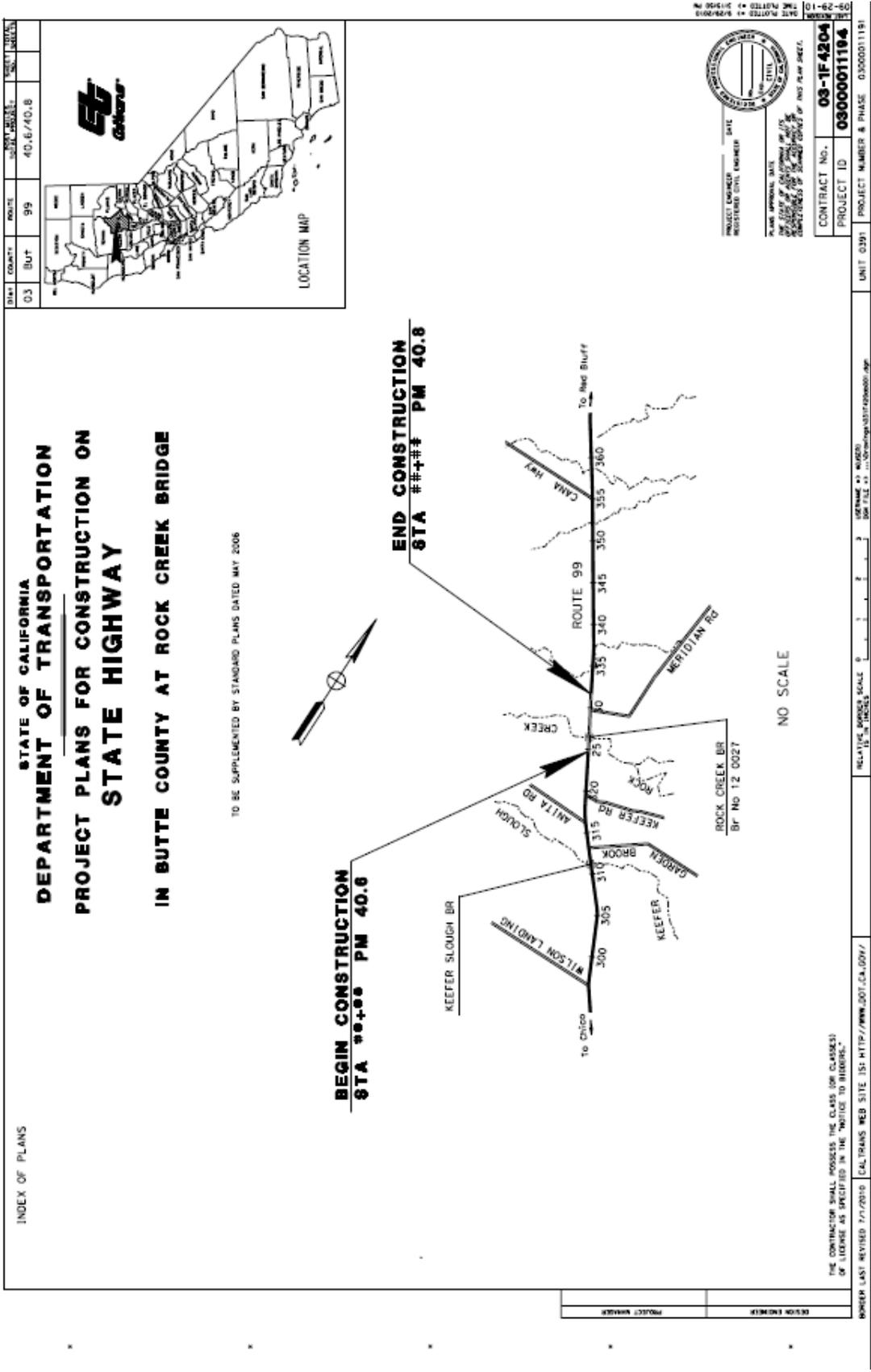
Date

Prepared by: Tim Nosal
Environmental Scientist

Attachment A:
Project Maps

Caltrans: State Route 99 Rock Creek Bridge Widening Project
Butte County

LSA#1600-2014-0021-R2



Memorandum

*Serious Drought.
Help Save Water!*

To: MR. DOUGLAS DUNRUD
Chief,
Design Branch 14
Office of Bridge Design-South
Division of Engineering Services

Attention: Mr. John Lane

Date: March 10, 2014

File: 03-BUT-99- PM 40.65
Rock Creek Bridge (Widen)
Br. No. 12-0027
03-1F4201
(Proj. #0300001119)

From: DEPARTMENT OF TRANSPORTATION
Division of Engineering Services
Geotechnical Services – MS 5
Office of Geotechnical Design – North

Subject: Revised Foundation Report for Rock Creek Bridge (Widen)

Introduction

The following recommendations are in response to a request for revised foundation recommendations made by the Office of Bridge Design North – Branch 14 for the proposed Rock Creek Bridge (Widen), Br. No. 12-0027. The following report supersedes our original Rock Creek Bridge (Widen) report dated December 16, 2013. This report is based on revised foundation loads provided by Mr. John Lane in an email dated January 13, 2014.

Foundation recommendations in this report are based on a subsurface investigation completed in September 1999 along with a review of the available General Plan, existing As-Built Plans, Bridge Inspection Reports and available geologic literature and mapping.

The log of test borings originally prepared in metric units (revision date 10-31-00) have been converted to English units (revision date 3-4-14). The log of test borings using English units supersede the previous log of test borings prepared using metric units.

Elevations used in this report are based on the NAVD 88 vertical datum, unless otherwise noted.

The following Department of Transportation, Caltrans records and resources were used for this Foundation Report:

- General Plan for Rock Creek Bridge Widening (Br. No. 12-0027) dated August 27, 2013.
- Foundation Recommendations for Rock Creek Bridge (Widen) (Br. No. 12-0027) dated August 25, 2000.
- Preliminary Foundation Report (PFR) for Rock Creek Bridge (Br. No. 12-0027), dated April 4, 2013.
- Final Hydraulic Report for Rock Creek Bridge dated September 30, 2013.
- Log of Test Borings (LOTB) drilled at the site in September 1999.

Project Description

The existing bridge was built in 1950. The bridge was built as a 6-span continuous Reinforced Concrete (RC) slab on RC 4-column bents and bent style abutments with 4 RC columns. All supports are founded on spread footings.

We understand that the existing structure has a history of scour issues dating back to 1958, and the proposed widening will eliminate the need for any scour countermeasures.

According to the General Plan dated 8/27/13, the structure is to be widened by about 5 feet (ft.) on both the right and left sides. The widening will be supported on one additional column at each support location for both the left and right sides. Driven steel HP 10x57 and 3-foot diameter CIDH piles have been considered for foundation support at the abutment and pier locations for the proposed new structure.

Summary of Site Geology and Subsurface Conditions

Regional Area Geology

The project site is located within the Great Valley geomorphic province. Based on the Geologic Map of the Chico Quadrangle, California (Saucedo and Wagner 1992) the foundation material consists of the Pleistocene Modesto Formation (Qm), which consists of alluvial deposits of gravel, sand, silt, and clay.

Subsurface Conditions

According to the 1999 Log of Test Borings, the subsurface material at the site generally consists of up to 20 ft of gravel and cobbles with a tight silt matrix overlying medium dense silty sand, sandy silt and gravel interbedded with stiff to very stiff clay and sandy clay.

Groundwater

Groundwater was encountered between elevation 150.6 ft and elevation 151.2 ft during the September 1999 field investigation. Groundwater elevations can be expected to vary due to seasonal fluctuations.

Corrosion Evaluation

According to the Foundation Recommendations for the Rock Creek Bridge dated August 25, 2000, "Representative soil samples taken during the foundation investigation were tested for corrosion potential. The results of the laboratory tests indicated this site to be non-corrosive, per Caltrans standards."

Scour

According to the "Final Hydraulic Report for Rock Creek Bridge dated September 30, 2013, a pier scour analysis was performed for the proposed widening of the existing structure. The estimated total scour shown in the final hydraulic report is tabulated below:

Table 1

Estimated total scour based on the Final Hydraulic Report dated September 30, 2013.

	Scour	Scour Elevation
Abutment 1	0.0 ft	N/A
Pier 2	0.0 ft	N/A
Pier 3	6.1 ft	170.1 ft
Pier 4	9.6 ft	158.3 ft
Pier 5	9.1 ft	160.1 ft
Pier 6	5.9 ft	170.1 ft
Abutment 7	0.0 ft	N/A

Structure Hydraulics recommends design of new foundations for the proposed widening, so that no foundation support (lateral or vertical) as a result of soil loss due to possible future scour as shown in the table above will be compromised.

Seismic Recommendations

Based on Caltrans ARS Online Version 2.3.06, the nearest active fault to the project site is the Great Valley 01 fault (Fault ID No. 64) located southwest of the bridge site. The fault is referred as a reverse fault with a 15 degree dip to the west. The maximum moment magnitude (MMax) of the fault is estimated to be 6.7 and the rupture distance from the fault plane to the bridge site is approximately 22.8 miles.

According to Caltrans Methodology for Developing Design Response Spectrum for Use in Seismic Design Recommendations, November 2012, the highest spectral accelerations are obtained by any or a combination of the following three methods for the subject bridge.

1. The statewide minimum deterministic spectrum requirements with MMax of 6.5, vertical strike-slip event with a rupture distance of 7.5 miles,
2. A deterministic spectrum based on the nearest active fault as shown on the ARS Online Tool (Version 2.2.06),
3. A probabilistic spectrum based on the U.S. Geologic Survey (USGS) 5% Probability of Exceedance in 50 years (975 year return period).

Based on the soil data shown on the LOTB dated September 1999, a shear wave velocity (V_{S30}) was estimated using SPT blow counts and correlation formula for granular soils. The estimated V_{S30} is 890 feet per second at the site.

Using the estimated V_{S30} , the ARS obtained by Method 3 controls design at the site. Therefore, the recommended Design ARS (attached) is the probabilistic spectrum based on the USGS 5% Probability of Exceedance in 50 years (975 year return period) with a Peak Ground Acceleration (PGA) being estimated to be 0.25g.

Our Office performed a liquefaction analysis and the results indicate that liquefaction potential is low at the bridge site during seismic events. Potential for surface rupture due to fault movement is considered insignificant since there are no known faults projecting toward or passing directly through the bridge site.

As-Built Foundation Data

As-Built General Plan and Foundation Plan sheets titled "Bridge Across Rock Creek" (Br. No. 12-0027) (circa 1951) indicate that the bridge foundations consist of spread footings at all support locations. The design footing pressure for this structure is shown as 3 tsf on the As-Built Foundation Plan (circa 1951). A summary of the existing foundation data is presented below in Table 2

Table 2
As-Built Spread Footing Data for Rock Creek Bridge (Br. No. 12-0027)

Support Location	Design Footing Pressure ⁽²⁾ (tsf)	Bottom of Footing Elevation ⁽¹⁾ (ft)
Abutment 1	3.0	92.0
Pier 2	3.0	90.0
Pier 3	3.0	87.0
Pier 4	3.0	83.0
Pier 5	3.0	83.0
Pier 6	3.0	85.0
Abutment 7	3.0	90.5

Notes:

- 1. Bottom of Footing Elevations were obtained from the "As-Built" Bridge Across Rock Creek General Plan circa 1951 and are based on a benchmark with an assumed elevation of 100 ft.*
- 2. Design Footing Pressure was obtained from the "As-Built" Bridge Across Rock Creek Foundation Plan circa 1951.*

Foundation Recommendations

It is our opinion that driven Steel HP 10x57 and 3-foot diameter CIDH piles are suitable for foundation support for the proposed widening at the abutment and pier locations. Recommendations for the piles are provided below.

The recommended pile tip elevations are based on the cut-off elevations and factored loads provided by the Office of Bridge Design, Branch 14 in emails from Mr. John Lane dated November 26th 2013 and January 13th 2014. Refer to Table 3 for the Abutment Foundation Design Recommendations, Table 4 for the Pier Foundation Design Recommendations and Table 5 for the Pile Data Table.

Table 3
Abutment Foundation Design Recommendations for Rock Creek Bridge (Widen)
(Br. No. 12-0027)

Abutment Foundation Design Recommendations									
Support Location	Pile Type	Cut-off Elevation (ft)	LRFD Service-I Limit State Load (kips) per Support		LRFD Service-I Limit State Total Load (kips) per Pile (Compression)	Nominal Resistance (kips)	Design Tip Elevations (ft)	Specified Tip Elevation (ft)	Nominal Driving Resistance Required (kips)
			Total	Permanent					
Abut. 1	HP 10x57	184.57	200	130	100	200	121 (a)	121	250
Abut. 7	HP 10x57	184.3	200	130	100	200	121 (a)	121	230

Notes:

- 1) Design tip elevations are controlled by: (a) Compression.

Table 4
Pier Foundation Design Recommendations for Rock Creek Bridge (Widen)
(Br. No. 12-0027)

Pier Foundations Design Recommendations										
Support Location	Pile Type	Cut-off Elevation (ft)	Service-I Limit State Load per Support (kips)	Total Permissible Support Settlement (inches)	Required Factored Nominal Resistance (kips)				Design Tip Elevations (ft)	Specified Tip Elevation (ft)
					Strength Limit		Extreme Event			
					Comp. ($\phi=0.7$)	Tension ($\phi=0.7$)	Comp. ($\phi=1$)	Tension ($\phi=1$)		
Pier 2	36" CIDH	180	N/A	1	540	0	N/A	N/A	106 (a-I)	106
Pier 3	36" CIDH	168.9	N/A	1	580	0	N/A	N/A	97 (a-I)	97
Pier 4	36" CIDH	160.5	N/A	1	580	0	N/A	N/A	84 (a-I)	84
Pier 5	36" CIDH	162.5	N/A	1	580	0	N/A	N/A	84 (a-I)	84
Pier 6	36" CIDH	171.1	N/A	1	540	0	N/A	N/A	101 (a-I)	101

Notes:

- 1) Design tip elevations are controlled by: (a-I) Compression (Strength Limit).
- 2) The specified tip elevation shall not be raised above the design tip elevations for Lateral Load.

Table 5
Pile Data Table for Rock Creek Bridge (Widen) (Br. No. 12-0027)

Pile Data Table						
Location	Pile Type	Nominal Resistance (kips)		Design Tip Elevation (ft)	Specified Tip Elevation (ft)	Nominal Driving Resistance (kips)
		Compression	Tension			
Abut. 1	HP 10x57	200	0	124 (1,4)	121	250
Pier 2	36" CIDH	780	0	105 (2,4)	106	N/A
Pier 3	36" CIDH	830	0	99 (2,4)	97	N/A
Pier 4	36" CIDH	830	0	85 (2,4)	84	N/A
Pier 5	36" CIDH	830	0	92 (2,4)	84	N/A
Pier 6	36" CIDH	780	0	101 (2,4)	101	N/A
Abut. 7	HP 10x57	200	0	124 (1,4)	121	230

Notes:

- 1) *Design tip elevations for Abutments are controlled by: (a) Compression.*
- 2) *Design tip elevations for Piers are controlled by: (a) Compression.*
- 3) *Scour potential exists to Elev. 170.1 @ Pier 3, 158.3 @ Pier 4, 160.1 @ Pier 5 and 170.1 @ Pier 6.*
- 4) *The specified tip elevation shall not be raised above the design tip elevations for Lateral Load.*
- 5) *The load carrying capacity of the CIDH piles are based on skin friction. The CIDH piles have not been designed to develop end bearing.*

General Notes to Designer

1. If lateral demands exist on the support piles, the structural design engineer shall indicate on the plans, in the pile data table, the design pile tip elevations required to meet the lateral load demands. If the specified pile tip elevations given in the above pile data table are not adequate for lateral load demands, the Office of Geotechnical Design-North, Branch A shall be contacted for further recommendations.
2. The load carrying capacity of the CIDH piles are based on skin friction. The CIDH piles have not been designed to develop end bearing.

Construction Considerations

1. Groundwater was encountered during drilling of test borings and will be encountered during excavation for CIDH concrete piles. The Contractor should anticipate placing concrete using the slurry displacement (wet) method.
2. Hard cobbles were encountered in the 1999 borings to approximately elevation 167 feet and should be anticipated during excavation for CIDH piles at Pier 2, Pier 3, Pier 5 and Pier 6 support locations.
3. Hard driving due to cobbles should be anticipated to approximately elevation 167 feet at Abutment 1 and Abutment 7 support locations.
4. Caving conditions as indicated on the LOTB were encountered during drilling of test borings. Unstable soils and caving conditions will likely be encountered during construction. Temporary casing may be necessary to construct CIDH concrete piles at Pier 2 to Pier 6.
5. If temporary casing is used during installation of CIDH concrete piles, it shall be removed while the concrete is being placed in order to develop the assumed pile capacity.
6. If the CIDH piles are constructed in the wet condition as anticipated, gamma-gamma testing will be required.

The recommendations contained in this report are based on specific project information regarding design loads, foundation dimensions and structure locations provided by the OBDS, Branch 14. If any conceptual changes are made during final project design, the Office of Geotechnical Design - North, Branch A should review those changes to determine if the foundation recommendations provided in this report are still applicable. Any questions regarding the recommendations in this report should be directed to Tim Alderman at (916) 227-1035, Tom Song at (916) 227-1057 or Reid Buell at (916) 227-1012, of the Office of Geotechnical Design-North.

Project Information

“Project Information,” discloses to bidders and contractors a list of pertinent information available for their inspection prior to bid opening. The following is information originating from Geotechnical Services:

Data and information attached with the project plans are:

- A. Log of Test Borings for Rock Creek Bridge (Widen), Bridge No. 12-0027.

Data and information included in the Information Handout provided to the bidders and contractors are:

- A. Revised Foundation Report for Rock Creek Bridge (Widen), Bridge No. 12-0027, dated March 10, 2014.

Report by:



TIM ALDERMAN
Engineering Geologist
Office of Geotechnical Design-North

Report by:

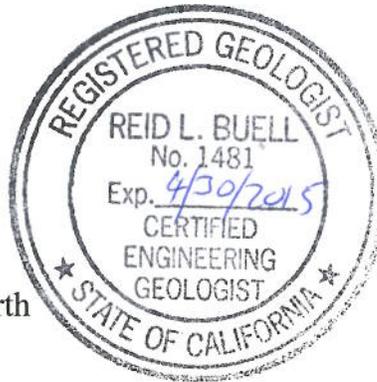


THOMAS N. SONG
Transportation Engineer
Office of Geotechnical Design-North

Supervised by:



REID BUELL, C.E.G. NO. 1481
Senior Engineering Geologist
Office of Geotechnical Design-North



Attachments: ARS Curve

- cc: OGDN File
GS File Room
District Project Manager
Structure Construction RE Pending File
Structure OE
DME

Rock Creek Bridge

Bridge No. 12-0027
EA No. 03-1F4201
EFIS No. 0300001119

Latitude 39.8194
Longitude -121.9224
Vs30 890 ft/sec

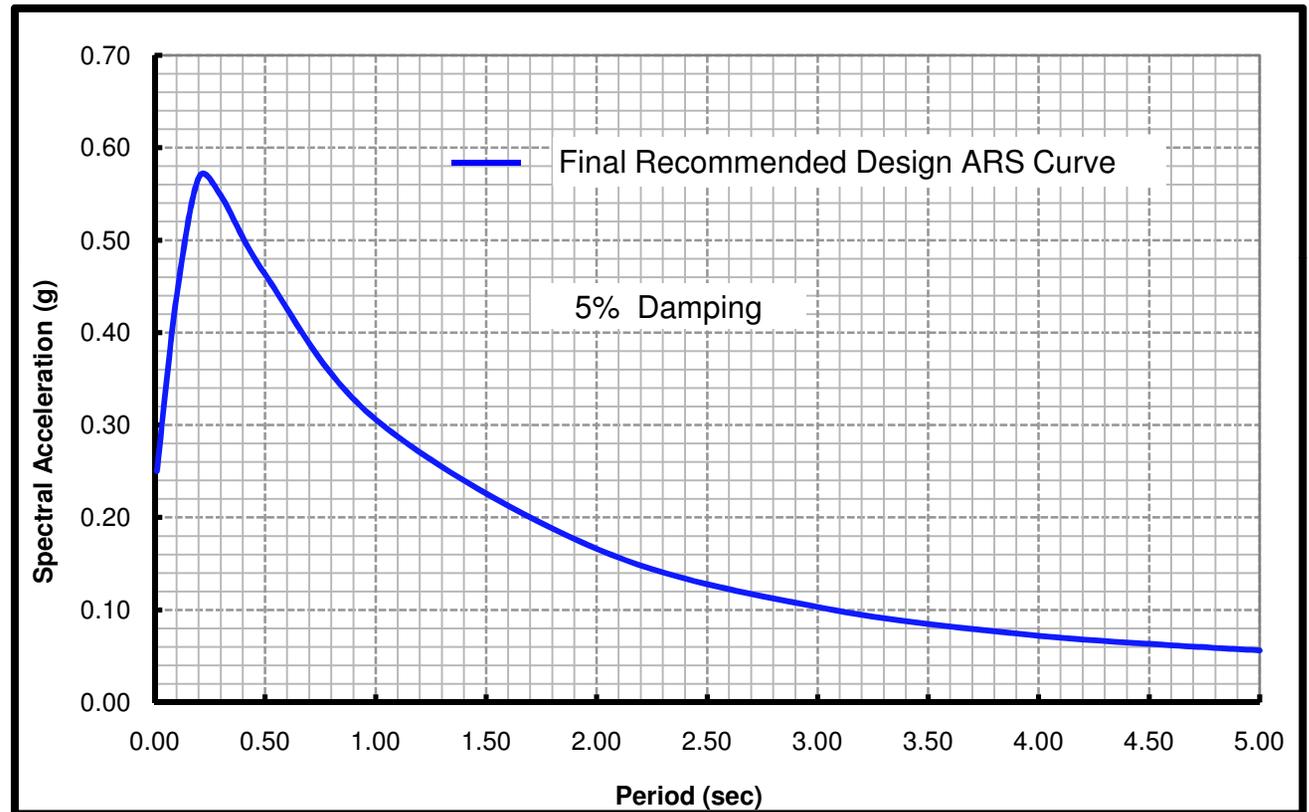
Controlling ARS 5% Probability of Exceedence in 50 years

Nearest Active Fault Data

Fault Name Great Valley 01
Fault ID # 64
Fault Type Rev.
M_{max} 6.7
Dip 15° W
R_{rup} 22.8 miles

Final Recommended ARS Data

Period (s)	SA (g)
0.01	0.25
0.1	0.44
0.2	0.568
0.3	0.548
0.5	0.463
1	0.306
2	0.166
3	0.103
4	0.072
5	0.056



Memorandum

To: Douglas Dunrad, Chief
Office of Bridge Design Services
Design Branch 14
Att: John Lane

Date: February 20, 2014

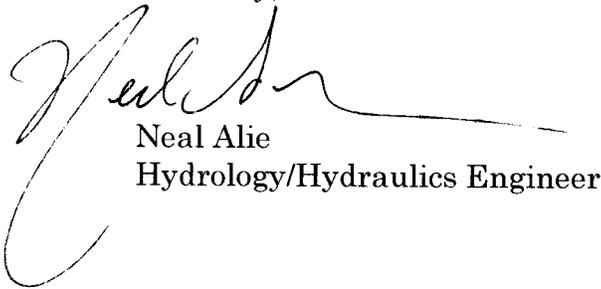
File: Rock Creek Bridge
Br. No. 12-0027
03-But-99-PM40.65
EA 03-1F4200
Project ID 0300001119

From: Neal Alie
Division of Engineering Services
Office of Design and Technical Services
Structure Hydraulics and Hydrology

Subject: Final Hydraulic Report for Rock Creek Bridge

Attached for your records is the Final Hydraulic Report for the
above referenced project. If you have any questions, please contact me
at (916) 227-0444 or my mobile at (916) 224-9640.

Sincerely,



Neal Alie
Hydrology/Hydraulics Engineer

cc: Steve Ng

State of California – Department of Transportation
Division of Engineering Services
Structure Design Services

Structure Hydraulics and Hydrology

FINAL HYDRAULIC REPORT

Rock Creek

Located on State Route 99 over Rock Creek in the County of Butte

Bridge No. 12-0027

Bridge widening \ Scour Countermeasure

02-BUT-099- PM 40.65

EA 03-1F420K/ 0300001119

February 20, 2014

WRITTEN BY:

Neal Alie

REVIEWED BY:

Ronald McGaugh

This report has been prepared under my direction as the professional engineer in responsible charge of the work, in accordance with the provisions of the Professional Engineers Act of the State of California



REGISTERED ENGINEER

C 56398

REGISTRATION NUMBER



Hydrology/Hydraulic Report

General

The Rock Creek Bridge, (Br. No. 12-0027) is located on State Route 99 in Butte County approximately 7 miles north of the city of Chico and 90 miles north of Sacramento. The bridge was constructed in 1951 and is approximately 146.0 feet long and 32.5 feet wide with non-standard shoulders. The existing bridge is comprised of continuous reinforced concrete (RC) slab spans on RC 4-column bents and bent style abutments with 4 RC columns with curtain walls all on spread footings.

According to the maintenance records, this structure has a history of scour problems dating back to 1958. There was a history of mining in this area and the bridge has experienced approximately 3 feet of degradation between 1951 and 1998, but has stabilized since. The bridge also has local pier scour with Pier 4 being the most severe with the footing currently exposed. There is a history of debris being caught up at the pier columns. There is erosion at the toe of the grouted rock slope paving at the Pier 3 side of span 3. There is a scour hole upstream of Pier 4, approximately 50.0 feet long, 20 feet wide and 3 feet deep.

In 2003 the Bridge's scour potential was assessed in accordance with FHWA Technical Advisory T5140.23, "Evaluating Scour at Bridges" and within current Caltrans guidelines, the bridge was determined to be scour critical. The item 113 code was changed to 3, "Bridge foundations determined to be unstable for calculated scour conditions."

Structure Design is proposing to widen both sides of Rock Creek Bridge with 36 inch CIDH Piles to create 8.0 foot shoulders. The proposed widening would act as a fully support structure eliminating the need for any scour countermeasure.

This report makes extensive reference to the (1) Caltrans Bridge Maintenance Reports, (2) General plans and profiles submitted by structures, (3) Caltrans As-Built Plans (4) Structure Hydraulics Hydrology/Hydraulics Report, October 26 1998, (5) Army Corps, Rock Creek-Keefer Slough Initial Assessment, February 1999

Elevations for the hydraulic model used in this report are based on NAVD 88.

Drainage Basin

Rock Creek drains approximately 44.1 square miles. The watershed is located on the Eastern slopes of the Sierras partially encompassing portions of Lassen National forest and the counties of Butte and Tehama. Watershed elevations range from approximately 1700 feet at the higher elevations to approximately 100 feet at the site. This watershed seems to have a high debris yield, and slight fluctuating channel aggradation and degradation. Local land owners constructed levees along portions of Rock Creek, but neither the Department of Water Resources or the Corps of Engineers maintain those levees.

In the lower elevations of the watershed approximately 1000 feet upstream of the bridge, the embankments of Highway 99 along with the levees trap the slow moving overland flow which eventually drains through the bridge opening. Channel slope was estimated at 0.37% just before entry into the structure, Manning's roughness coefficient was estimated at 0.035 to 0.037 in the vicinity of the project site. Average annual precipitation based on the Oregon Climate Service Prism Program(Annual normal's from 1981 to 2010) is about 43.5 inches per year.

Discharge

Since this watershed is ungagged, the National Stream flow Statistics program (NSS) was used to calculate a 100-year discharge of approximately 7400 cfs. Previous discharge values were either based on smaller or larger watersheds or high water marks. The US Army Corps study flow discharges were not based on a 100 year frequency so those flow values were not used. At approximately 7500 feet upstream of the bridge site Rock Creek and Keefer Slough intersect. The U.S. Army Corps of Engineers Hydrologic Engineering Center-River Analysis System (HEC-RAS) program was used to model the split flow. For the 100-year frequency it was determined that approximately 2000 cfs flows from Rock Creek to Keefer Slough. Although there is a split in flow reducing the total discharge in Rock Creek, a Bulking Factor of 1.1 to 1.5 due to debris load should be applied, resulting in a more conservative 100-year discharge of 7400 cfs.

Stage, Velocity and Waterway

The U.S. Army Corps of Engineers Hydrologic Engineering Center-River Analysis System (HEC-RAS) program was used to perform a one-dimensional hydraulic analysis to calculate the water surface elevations and velocity for the existing structure and for the proposed widening. The General Plans and As-Builts were referenced to acquire the planned deck elevation height.

The proposed freeboard is measured from the water surface elevation to the lowest chord of the soffit of the structure. The parameters used to model the existing structure included a 50 and 100-year discharge of 5589 and 7400 cfs respectively; a manning's

roughness coefficient of (0.034 to 0.05) and a channel slope of .037. The average velocity and the stage for the 50 and 100-year discharges at the upstream face of the bridge are given below.

Existing Bridge

	WSEL	Average Velocity	Available Freeboard
50-year Design Flood 5589 cfs	182.30 ft	6.12 fps	4.50 ft
100-year Base Flood 7400 cfs	183.85 ft	6.23 fps	2.95 ft

The minimum soffit elevation of the existing structure is 186.80 feet. The existing structure has adequate freeboard and waterway for both the design flood (50-year discharge) and the base flood (100-year discharge).

Proposed widening with 36 inch CIDH Piles

	WSEL	Average Velocity	Available Freeboard
50-year Design Flood 5589 cfs	182.38 ft	6.04 fps	4.42 ft
100-year Base Flood 7400 cfs	183.89 ft	6.18 fps	2.91 ft

The proposed widening has a minimal effect on the water surface elevation and velocity. Structure Hydraulics recommends maintaining at minimum the existing soffit elevation.

Streambed and Scour

The existing channel through the structure is on a bend of a slight curve. For the natural channel bottom, from observation and photographs the streambed is comprised of sand and some underlying gravel. A geology report completed in the summer of 1998 stated the stream is composed of dense to loose silt for the top 11 feet and dense silty sand for the next 50 feet of depth.

A pier scour analysis was performed for the proposed widening of the existing structure and the total scour is tabulated below:

	Scour	Scour Elevation
Abutment 1	0.0 ft	N/A
Pier 2	0.0 ft	N/A
Pier 3	6.1 ft	170.1 ft
Pier 4	9.6 ft	158.3 ft
Pier 5	9.1 ft	160.1 ft
Pier 6	5.9 ft	170.1 ft
Abutment 7	0.0 ft	N/A

The proposed widening will act as a fully support structure eliminating the need for any scour countermeasure. Structure Hydraulics recommends that all new foundations for the proposed widening should be designed assuming no ground support (lateral or vertical) as a result of soil loss due to possible future scour calculated above.

Drift

According to the maintenance Records, there is a history of drift consisting of trees and tree branches accumulating at Piers 4, 5 and 6. The accumulation of drift at the piers could increase the effective pier diameter increasing the local pier scour. It is recommended that Caltrans Maintenance remove any accumulation of drift.

Bank Protection

Rock slope protection will be designed by the District to protect the roadway approach fills, if required. The average velocity (See “Stage, Velocity and Waterway” section above) is provided to assist the District hydraulic engineers in the design of bank protection if necessary.

Summary Information for the Bridge Designer

Proposed Widening

HYDROLOGIC SUMMARY FOR Br. No. 12-0027			
Drainage Area: 44.1 square miles			
	Design Flood	Base Flood	Overtopping Flood/Flood of Record
Frequency	50-Year	100-Year	N/A
Discharge	5589 cfs	7400 cfs	N/A
Water Surface Elevation at Bridge	182.38 ft	183.89 ft	N/A
<p>Flood plain data are based upon information available when the plans were prepared and are shown to meet federal requirements. The accuracy of said information is not warranted by the State and interested or affected parties should make their own investigation.</p>			

ASBESTOS AND LEAD-CONTAINING PAINT SURVEY REPORT



**Rock Creek Bridge 12-0027
03-BUT-99 PM 40.65
Butte County, California**

PREPARED FOR:

**CALIFORNIA DEPARTMENT OF TRANSPORTATION
DISTRICT 3
703 B STREET, P.O. BOX 911
MARYSVILLE, CALIFORNIA 95901**



PREPARED BY:

**GEOCON CONSULTANTS, INC.
3160 GOLD VALLEY DRIVE, SUITE 800
RANCHO CORDOVA, CALIFORNIA 95742**



**GEOCON PROJECT NO. S9805-01-20
TASK ORDER NO. 20
E-FIS 03-0000-1119-1 (EA 03-1F4201)
CONTRACT NO. 03A2132**

FEBRUARY 2014



Project No. S9805-01-20

February 26, 2014

Alicia Beyer, Task Order Manager
Caltrans District 3
703 B Street
Marysville, California 95901

Subject: ASBESTOS AND LEAD-CONTAINING PAINT SURVEY REPORT
03-BUT-99, PM 40.65, ROCK CREEK BRIDGE NO. 12-0027
BUTTE COUNTY, CALIFORNIA
CONTRACT NO. 03A2132, E-FIS 03 0000 1119 1, EA 03-1F4201
TASK ORDER NO. 20

Dear Ms. Beyer:

In accordance with California Department of Transportation Contract No. 03A2132 and Task Order No. 20, we have performed an asbestos and lead-containing paint survey of the Rock Creek Bridge in Butte County, California. Our scope of services included surveying the structure for suspect asbestos-containing materials and lead-containing paint, collecting bulk samples, and submitting the samples to laboratories for analyses.

The accompanying report summarizes the services performed and laboratory analysis.

The contents of this report reflect the views of Geocon Consultants, Inc., who are responsible for the facts and accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the State of California or the Federal Highway Administration. This report does not constitute a standard, specification, or regulation.

Please contact us if you have questions concerning the contents of this report or if we may be of further service.

Sincerely,

GEOCON CONSULTANTS, INC.

David A. Watts, CAC No. 98-2404
Senior Project Scientist

John E. Juhend, PE, CEG
Principal/Senior Engineer

(2 + 2 CDs) Addressee

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FIGURES

1. Vicinity Map
2. Site Plan

PHOTOGRAPHS (1 through 3)

TABLES

1. Summary of Asbestos Analytical Results
2. Summary of Paint Analytical Results – Total and Soluble Lead

APPENDICES

- A. Analytical Laboratory Reports and Chain-of-custody Documentation

ASBESTOS AND LEAD-CONTAINING PAINT SURVEY REPORT

1.0 INTRODUCTION

This asbestos and lead-containing paint (LCP) survey report was prepared by Geocon Consultants, Inc. under Caltrans Contract No. 03A2132, Task Order No. 20 (TO-20).

1.1 Project Description

The project consists of the Rock Creek Bridge (12-0027) at Post Mile 40.65 on Highway 99 in Butte County, California. We performed asbestos and LCP survey activities at the project location. The project location is depicted on the Vicinity Map, Figure 1, and Site Plan, Figure 2.

1.2 General Objectives

The purpose of the scope of services outlined in TO-20 was to determine the presence and quantity of asbestos and LCP at the project location prior to bridge replacement activities. The information obtained from this investigation will be used by Caltrans for waste profiling, determining California Occupational Safety and Health Administration (Cal/OSHA) applicability, and coordinating asbestos and LCP disturbance activities.

It was not Geocon's intent during this inspection to conduct an evaluation of lead-based paint hazards in accordance with U.S. Department of Housing and Urban Development (HUD) guidelines.

2.0 BACKGROUND

2.1 Asbestos

The Code of Federal Regulations (CFR), 40 CFR 61, Subpart M, National Emissions Standards for Hazardous Air Pollutants (NESHAP) and Federal Occupational Safety and Health Administration (FED OSHA) classify asbestos-containing material (ACM) as any material or product that contains *greater than* 1% asbestos. Nonfriable ACM is classified by NESHAP as either Category I or Category II material defined as follows:

- **Category I** – asbestos-containing packings, gaskets, resilient floor coverings, and asphalt roofing products.
- **Category II** – all remaining types of nonfriable asbestos-containing material not included in Category I that when dry, cannot be crumbled, pulverized, or reduced to powder by hand pressure.

Regulated asbestos-containing material (RACM), a hazardous waste when friable, is classified as any manufactured material that contains *greater than* 1% asbestos by dry weight *and* is:

- Friable (can be crumbled, pulverized, or reduced to powder by hand pressure); or

- Category I material that has become friable; or
- Category I material that has been subjected to sanding, grinding, cutting, or abrading; or
- Category II nonfriable material that has a high probability of becoming crumbled, pulverized, or reduced to a powder during demolition or renovation activities.

Activities that disturb materials containing *any* amount of asbestos are subject to certain requirements of the Cal/OSHA asbestos standard contained in Title 8 of the California Code of Regulations (CCR) §1529. Typically, removal or disturbance of more than 100 square feet of material containing more than 0.1% asbestos must be performed by a registered asbestos abatement contractor, but associated waste labeling is not required if the material contains 1% or less asbestos. When the asbestos content of a material exceeds 1%, virtually all requirements of the standard become effective.

Materials containing more than 1% asbestos are also subject to NESHAP regulations (40 CFR Part 61, Subpart M). RACM (friable ACM and nonfriable ACM that will become friable during demolition operations) must be removed from structures prior to demolition. Certain nonfriable ACM and materials containing 1% or less asbestos may remain in structures during demolition; however, there are waste handling/disposal issues and Cal/OSHA work requirements that must be addressed. Contractors are responsible for segregating and characterizing waste streams prior to disposal.

With respect to potential worker exposure, notification, and registration requirements, Cal/OSHA defines asbestos-containing construction material (ACCM) as construction material that contains more than 0.1% asbestos (Title 8, CCR 341.6).

2.2 Lead Paint

Construction activities (including demolition) that disturb materials or paints containing *any* amount of lead are subject to certain requirements of the Cal/OSHA lead standard contained in Title 8, CCR, §1532.1. Deteriorated paint is defined by Title 17, CCR, Division 1, Chapter 8, §35022 as a surface coating that is cracking, chalking, flaking, chipping, peeling, non-intact, failed, or otherwise separating from a substrate. Demolition of a deteriorated LCP component would require waste characterization and appropriate disposal. Intact LCP on a component is currently accepted by most landfills and recycling facilities; however, contractors are responsible for segregating and characterizing waste streams prior to disposal.

For a solid waste containing lead, the waste is classified as California hazardous when: 1) the representative total lead content equals or exceeds the respective Total Threshold Limit Concentration (TTLC) of 1,000 milligrams per kilogram (mg/kg); or 2) the representative soluble lead content equals or exceeds the respective Soluble Threshold Limit Concentration (STLC) of 5 milligrams per liter (mg/l) based on the standard Waste Extraction Test (WET). A waste has the potential for exceeding the lead STLC when the waste's total lead content is greater than or equal to ten times the respective STLC

value since the WET uses a 1:10 dilution ratio. Hence, when total lead is detected at a concentration greater than or equal to 50 mg/kg, and assuming that 100 percent of the total lead is soluble, soluble lead analysis is required. Lead-containing waste is classified as “Resource, Conservation, and Recovery Act” (RCRA) hazardous, or Federal hazardous, when the representative soluble lead content equals or exceeds the Federal regulatory level of 5 mg/l based on the Toxicity Characteristic Leaching Procedure (TCLP).

The above regulatory criteria are based on chemical concentrations. Wastes may also be classified as hazardous based on other criteria such as ignitability; however, for the purposes of this investigation, toxicity (i.e., lead concentration) is the primary factor considered for waste classification since waste generated during the construction activities would not likely warrant testing for ignitability or other criteria. Waste that is classified as either California-hazardous or RCRA-hazardous requires management as a hazardous waste.

Potential hazards exist to workers who remove or cut through LCP coatings during demolition. Dust containing hazardous concentrations of lead may be generated during scraping or cutting materials coated with lead-containing paint. Torching of these materials may produce lead oxide fumes. Therefore, air monitoring and/or respiratory protection may be required during the demolition of materials coated with LCP. Guidelines regarding regulatory provisions for construction work where workers may be exposed to lead are presented in Title 8, CCR, §1532.1.

2.3 Architectural Drawings and Previous Survey Activities

We reviewed structure as-built plans provided by Caltrans prior to field activities. We did not observe specifications or notes regarding the use of asbestos-containing materials or lead paint in the architectural plans provided. Previous asbestos survey reports were not available for our review.

3.0 SCOPE OF SERVICES

Mr. David Watts, a California-Certified Asbestos Consultant (CAC), certification No. 98-2404 (expiration September 16, 2014), and Certified Lead Paint Inspector/Assessor and Project Monitor with the California Department of Public Health Services (DPH), certification numbers I-1734 and M-1734 (expiration December 4, 2014), performed the asbestos and LCP survey at the project location on January 22, 2014.

3.1 Asbestos

Suspect ACM were grouped into homogeneous areas with representative samples randomly collected from each. In addition, each potential ACM was evaluated for friability. A total of two bulk asbestos samples representing one suspect component (concrete) were collected.

Our procedures for inspection and sampling in accordance with TO-20 are discussed below:

- Collected bulk asbestos samples after first wetting friable materials with a light mist of water. The samples were then cut from the substrate and transferred to labeled containers. Note that when multiple samples were collected, the sampling locations were distributed throughout the homogeneous area (spaces where the material was observed).
- Relinquished bulk asbestos samples under standard chain-of-custody protocol to EMSL Analytical, Inc., a California-licensed and Caltrans-approved subcontractor, for asbestos analysis in accordance with United States Environmental Protection Agency (EPA) Test Method 600/R-93/116 using polarized light microscopy (PLM). EMSL Analytical, Inc. is a laboratory accredited by the National Institute of Standards and Technology National Voluntary Laboratory Accreditation Program (NIST-NVLAP) for bulk asbestos fiber analysis. The laboratory analyses were requested on a turnaround period of five days.

Sample group identification numbers, material descriptions, approximate quantities, friability assessments, and photo references are summarized on Table 1. Approximate sample locations are presented on Figure 2. Materials represented by the samples collected are shown in the attached photographs.

3.2 Lead Paint

A total of six bulk paint samples were collected from suspect LCP observed at the project location. Mr. Watts field-composited the suspect LCP samples into three paint schemes prior to submittal to the laboratory. We did not observe deteriorated LCP during our survey. Our sampling procedures in accordance with TO-20 are discussed below:

- Collected bulk samples of suspect LCP using techniques presented in HUD guidelines. In addition, the painted areas were evaluated for evidence of deterioration such as flaking or cracking.
- Relinquished bulk LCP samples under standard chain-of-custody protocol to Advanced Technology Laboratories, a California-licensed and Caltrans-approved subcontractor, for total and soluble lead analysis in accordance with EPA Test Method 6010B. Advanced Technology Laboratories is accredited by the DPH for lead analysis. The laboratory analyses were requested on a turnaround period of five days.

Paint sample identification numbers, descriptions, peeling and flaking quantities, and photo references are summarized on Table 2. Approximate sample locations are presented on Figure 2. Materials represented by the samples collected are shown in the attached photographs.

4.0 INVESTIGATIVE RESULTS

4.1 Asbestos Analytical Results

No asbestos was detected in samples of the suspect concrete materials collected during our survey. A summary of the analytical laboratory test results for asbestos is presented on Table 1. Reproductions of the laboratory report and chain-of-custody documentation are in Appendix A.

4.2 Paint Analytical Results

A sample representing intact gray paint used on the bridge barriers and barrier posts exhibited a total lead concentration of 210,000 mg/kg and a TCLP lead concentration of 59 mg/l.

A sample representing intact yellow traffic striping exhibited a total lead concentration of 4.9 mg/kg.

A sample representing intact white traffic striping exhibited a total lead concentration of 6.0 mg/kg.

A summary of the analytical laboratory test results for paint is presented on Table 2. Reproductions of the laboratory reports and chain-of-custody documentation are presented in Appendix A.

5.0 RECOMMENDATIONS

Based on our findings, we recommend the following:

5.1 Asbestos

Since no asbestos was detected in the concrete samples collected during our survey, the Cal/OSHA asbestos standard does not apply for planned activities. In addition, renovation debris would not be considered a California hazardous waste based on asbestos content. Written notification to the U.S. EPA Region IX and the California Air Resources Board is required ten working days prior to commencement of *any* demolition activity (whether asbestos is present or not).

5.2 Lead Paint

Gray barrier system paint sampled during our survey would be classified as California and Federal hazardous based on lead content if stripped, blasted, or otherwise separated from the substrate.

Traffic striping sampled during our survey would not be considered California or Federal hazardous based on lead content.

We recommend that all paints at the project location be treated as lead-containing for purpose of determining the applicability of the Cal/OSHA lead standard during maintenance, renovation, and demolition activities. This recommendation is based on LCP sample results and the fact that lead was a common ingredient of paints manufactured before 1978 and is still an ingredient of some paints. In accordance with Title 8, CCR, §1532.1(p), written notification to the nearest Cal/OSHA district office is required at least 24 hours prior to certain lead-related work. Compliance and training requirements regarding construction activities where workers may be exposed to lead are presented in Title 8, CCR, §1532.1, subsections (e) and (l), respectively.

Disturbance, packaging, storage, transporting, and disposing of material containing lead paint at hazardous levels must conform to applicable local, California, and Federal regulations. The removal, transportation, placement, handling, and disposal of LCP must result in no visible dust.

6.0 REPORT LIMITATIONS

The asbestos and LCP survey was conducted in conformance with generally accepted standards of practice for identifying and evaluating asbestos and LCP in structures. The survey addressed only the structure identified in Section 1.1. Due to the nature of structure surveys, asbestos and LCP use, and laboratory analytical limitations, some ACM or LCP at the project location may not have been identified. Spaces such as cavities, voids, crawlspaces, and pipe chases may have been concealed to our investigator. Previous renovation work may have concealed or covered spaces or materials or may have partially demolished materials and left debris in inaccessible areas. Additionally, renovation activities may have partially replaced ACM with indistinguishable non-ACM. Asbestos and/or LCP may exist in areas of the structure that were not accessible or sampled in conjunction with this TO.

During renovation or demolition operations, suspect materials may be uncovered which are different from those accessible for sampling during this assessment. Personnel in charge of renovation/demolition should be alerted to note materials uncovered during such activities that differ substantially from those included in this or previous assessment reports. If suspect ACM and/or LCP are found, additional sampling and analysis should be performed to determine if the materials contain asbestos or lead.

This report has been prepared exclusively for Caltrans. The information contained herein is only valid as of the date of the report and will require an update to reflect additional information obtained.

This report is not a comprehensive site characterization and should not be construed as such. The findings as presented in this report are predicated on the results of the limited sampling and laboratory testing performed. In addition, the information obtained is not intended to address potential impacts related to sources other than those specified herein. Therefore, the report should be deemed conclusive with respect to only the information obtained. We make no warranty, express or implied, with respect to the content of this report or any subsequent reports, correspondence or consultation. Geocon strived to perform the services summarized herein in accordance with the local standard of care in the geographic region at the time the services were rendered.

The contents of this report reflect the views of the author who is responsible for the facts and accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the State of California or the Federal Highway Administration. This report does not constitute a standard, specification, or regulation.



Photo 1 – Rock Creek Bridge (12-0027) at PM 40.65 on Highway 99 in Butte County, California



Photo 2 – Metal drainpipe (non-suspect) below bridge deck



Photo 3 – Bridge span and columns



GEOCON
CONSULTANTS, INC.

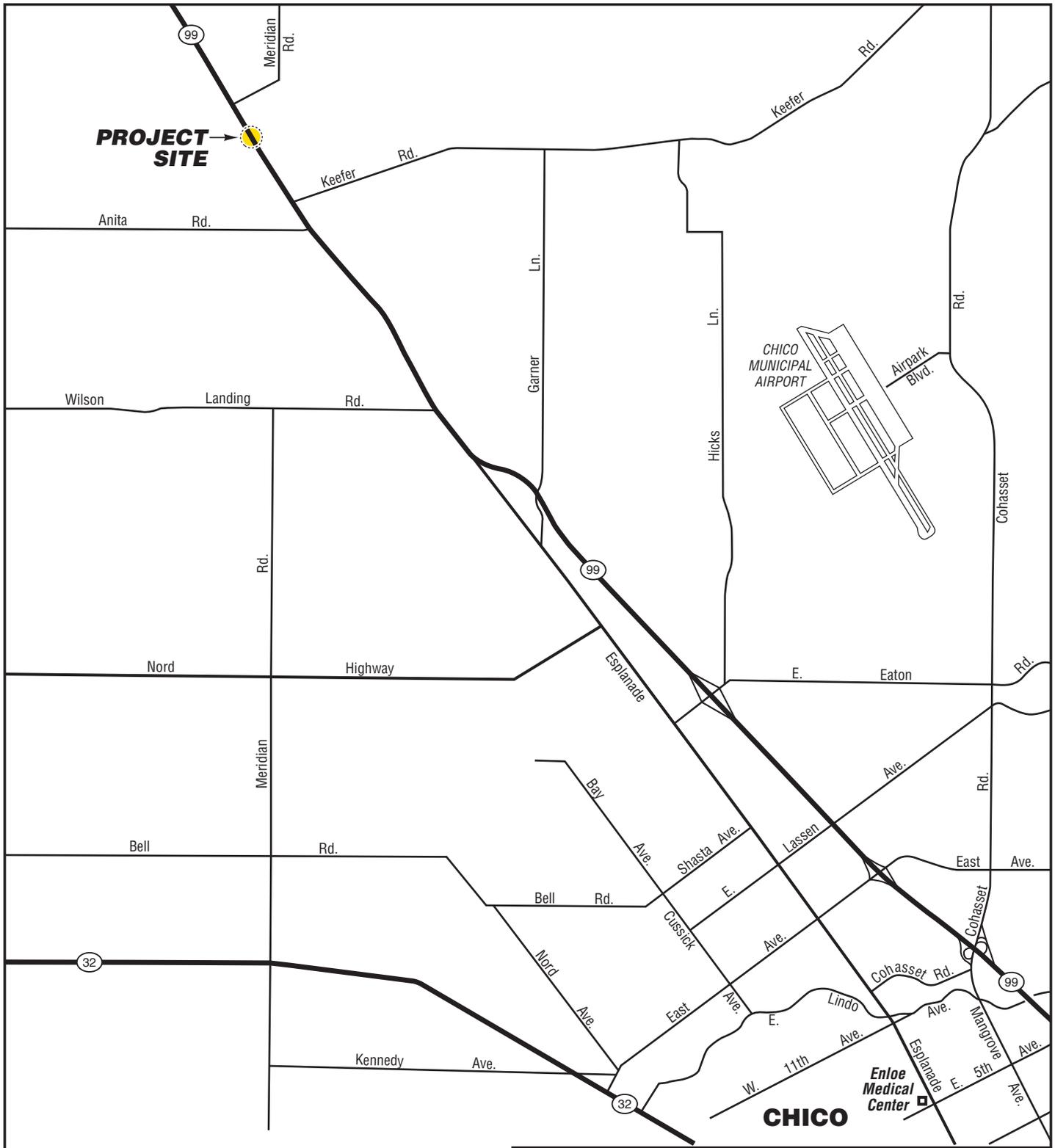
3160 GOLD VALLEY DR – SUITE 800 – RANCHO CORDOVA, CA 95742
PHONE 916.852.9118 – FAX 916.852.9132

PHOTOGRAPHS 1, 2, & 3

Rock Creek Bridge
Butte County, California

S9805-01-20

February 2014



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CONSULTANTS, INC.

3160 GOLD VALLEY DR - SUITE 800 - RANCHO CORDOVA, CA 95742
PHONE 916.852.9118 - FAX 916.852.9132

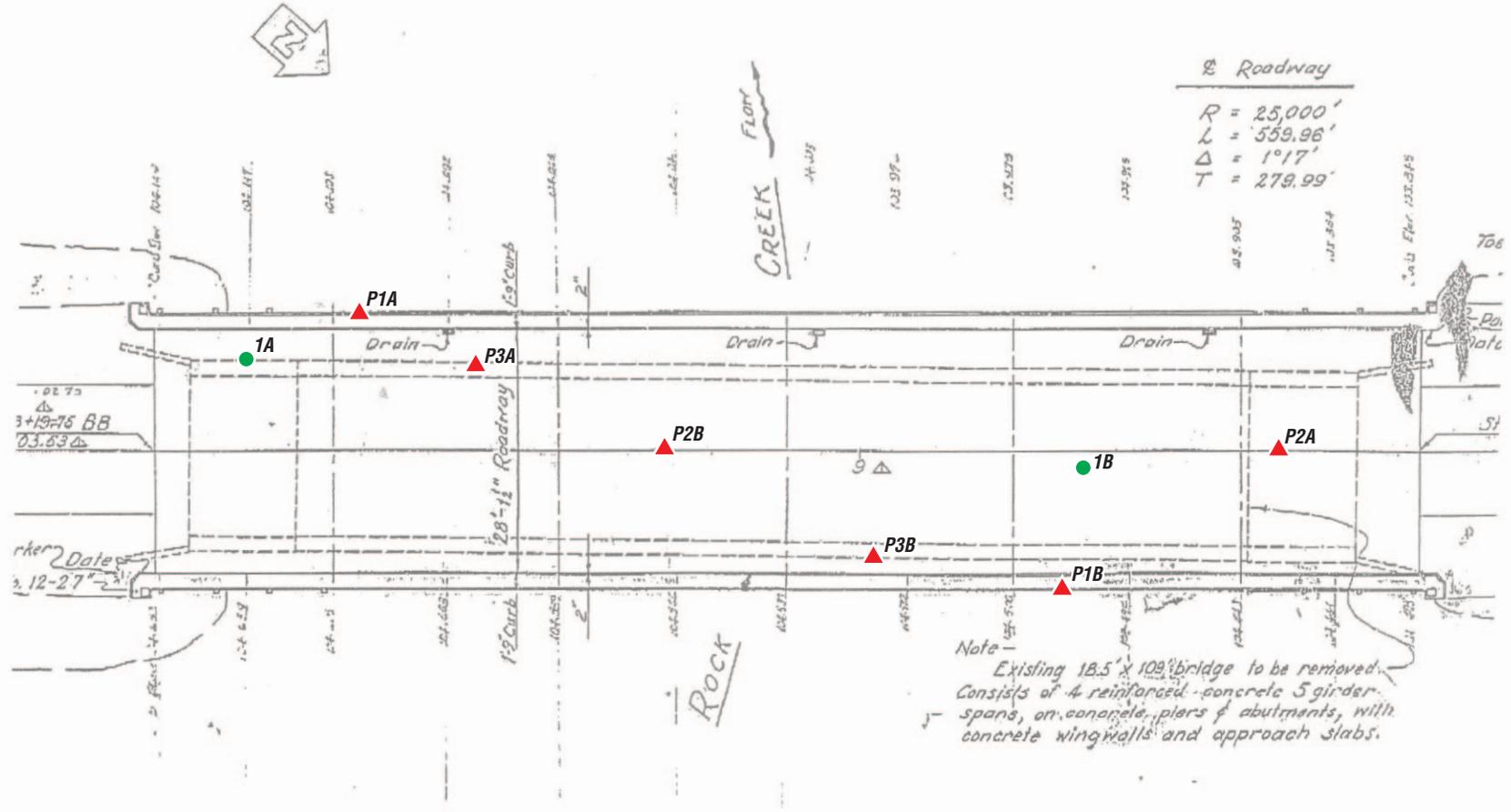
Rock Creek Bridge No. 12-0027, 03-BUT-99 PM 40.65

GEOCON Proj. No. S9805-01-20
Task Order No. 20
E-FIS 06-0000-1119-1
EA 03-1F4201
Caltrans Contract 03A2132

VICINITY MAP

February 2014

Figure 1



$R = 25,000'$
 $L = 559.96'$
 $\Delta = 1^\circ 17'$
 $T = 279.99'$

Note -
 Existing 18.5' x 109' bridge to be removed.
 Consists of 4 reinforced concrete 5 girder spans, on concrete piers & abutments, with concrete wingwalls and approach slabs.

LEGEND:

- Approximate Asbestos Sample Location
- ▲ Approximate Paint Sample Location



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Rock Creek Bridge No. 12-0027, 03-BUT-99 PM 40.65

GEOCON Proj. No. S9805-01-20
 Task Order No. 20
 E-FIS 06-0000-1119-1
 EA 03-1F4201
 Caltrans Contract 03A2132

SITE PLAN

February 2014 Figure 2

TABLE 1
SUMMARY OF ASBESTOS ANALYTICAL RESULTS
ROCK CREEK BRIDGE NO. 12-0027
CONTRACT 03A2132, TASK ORDER NO. 20
03-BUTTE-99, POST MILE 40.65
BUTTE COUNTY, CALIFORNIA

Polarized Light Microscopy (PLM) - EPA Test Method 600/R-93/116

Sample Group No.	Description of Material	Approximate Quantity	Friable	Site Photo	Asbestos Content
1	Concrete	NA	NA	1 through 3	ND

Notes:

NA = Not applicable (no asbestos detected)

ND = Not detected

TABLE 2
SUMMARY OF PAINT ANALYTICAL RESULTS - TOTAL AND SOLUBLE LEAD
ROCK CREEK BRIDGE (12-0027)
CONTRACT 03A2132, TASK ORDER NO. 20
03-BUTTE-99, POST MILE 40.65
BUTTE COUNTY, CALIFORNIA

Paint Sample No.	Paint Description	Approximate Quantity Peeling/Flaking	Site Photos	Total Lead (mg/kg)	TCLP Lead (mg/l)
P1A/B	Gray paint (barriers and barrier posts)	Intact	1	210,000	59
P2A/B	Yellow traffic striping	Intact	1	4.9	---
P3A/B	White traffic striping	Intact	1	6.0	---

Notes:

mg/kg = milligrams per kilogram (EPA Test Method 6010B)

mg/l = milligrams per liter

TCLP = Toxicity Characteristic Leaching Procedure (EPA Test Method 7420)

--- = Not analyzed

APPENDIX

A



EMSL Analytical, Inc

2235 Polvorosa Ave , Suite 230, San Leandro, CA 94577

Phone/Fax: (510) 895-3675 / (510) 895-3680

<http://www.EMSL.com>

sanleandrolab@emsl.com

EMSL Order:	091401023
CustomerID:	GECN21
CustomerPO:	S9805-01-20
ProjectID:	03A2132

Attn: **Dave Watts**
Geocon Consultants, Inc.
6671 Brisa Street

Livermore, CA 94550

Phone: (925) 371-5900
 Fax: (925) 371-5915
 Received: 01/23/14 9:00 AM
 Analysis Date: 1/29/2014
 Collected: 1/22/2014

Project: **S9805-01-20 / ROCK CREEK BRIDGE DEMO**

Test Report: Asbestos Analysis of Bulk Materials via EPA 600/R-93/116 Method using Polarized Light Microscopy

Sample	Description	Appearance	Non-Asbestos		Asbestos
			% Fibrous	% Non-Fibrous	% Type
1A-Concrete <i>091401023-0001</i>		Gray Non-Fibrous Homogeneous		100% Non-fibrous (other)	None Detected
1B-Concrete <i>091401023-0002</i>		Gray Non-Fibrous Homogeneous		100% Non-fibrous (other)	None Detected

Analyst(s) _____
 Matthew Batongbacal (2)


 Baojia Ke, Laboratory Manager
 or other approved signatory

EMSL maintains liability limited to cost of analysis. This report relates only to the samples reported and may not be reproduced, except in full, without written approval by EMSL. EMSL bears no responsibility for sample collection activities or analytical method limitations. Interpretation and use of test results are the responsibility of the client. This report must not be used by the client to claim product certification, approval, or endorsement by NVLAP, NIST or any agency of the federal government. Non-friable organically bound materials present a problem matrix and therefore EMSL recommends gravimetric reduction prior to analysis. Samples received in good condition unless otherwise noted. Estimated accuracy, precision and uncertainty data available upon request. Unless requested by the client, building materials manufactured with multiple layers (i.e. linoleum, wallboard, etc.) are reported as a single sample. Reporting limit is 1%
 Samples analyzed by EMSL Analytical, Inc San Leandro, CA NVLAP Lab Code 101048-3, WA C884

Initial report from 01/29/2014 15:17:24



EMSL ANALYTICAL, INC.
LABORATORY PRODUCTS TRAINING

Asbestos Chain of Custody

EMSL Order Number (Lab Use Only):

091401023

03A2132

EMSL ANALYTICAL, INC.
2235 POLVOROSA DR., STE. 230
SAN LEANDRO, CA 94577
PHONE: (510) 895-3675
FAX: (510) 895-3680

Company: GEOCON		EMSL-Bill to: <input checked="" type="checkbox"/> Same <input type="checkbox"/> Different If Bill to is Different note instructions in Comments**	
Street: 6671 BRIST ST.		Third Party Billing requires written authorization from third party	
City: LIVERMORE	State/Province: CA	Zip/Postal Code: 94550	Country: USA
Report To (Name): D. WAITS		Fax #: 925-371-5915	
Telephone #: 925-371-5900		Email Address: WAITS@GEOCONINC.COM	
Project Name/Number: ROCK CREEK BRIDGE DEMO		59805-01-20	
Please Provide Results: <input type="checkbox"/> Fax <input checked="" type="checkbox"/> Email		Purchase Order: _____ U.S. State Samples Taken: _____	

Turnaround Time (TAT) Options* - Please Check

3 Hour
 6 Hour
 24 Hour
 48 Hour
 72 Hour
 96 Hour
 1 Week
 2 Week

*For TEM Air 3 hours/6 hours, please call ahead to schedule. There is a premium charge for 3 Hour TEM AHERA or EPA Level II TAT. You will be asked to sign an authorization form for this service. Analysis completed in accordance with EMSL's Terms and Conditions located in the Analytical Price Guide.

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January 30, 2014

Dave Watts
Geocon Consultants, Inc.
6671 Brisa Street
Livermore, CA 94550
Tel: (925) 961-5273
Fax: (925) 371-5915



Re: ATL Work Order Number : 1400206

Client Reference : ROCK CREEK BRIDGE DEMO, S9805-01-20

Enclosed are the results for sample(s) received on January 23, 2014 by Advanced Technology Laboratories. The sample(s) are tested for the parameters as indicated on the enclosed chain of custody in accordance with applicable laboratory certifications. The laboratory results contained in this report specifically pertains to the sample(s) submitted.

Thank you for the opportunity to serve the needs of your company. If you have any questions, please feel free to contact me or your Project Manager.

Sincerely,

Eddie Rodriguez
Laboratory Director

The cover letter and the case narrative are an integral part of this analytical report and its absence renders the report invalid. Test results contained within this data package meet the requirements of the National Environmental Laboratory Accreditation Conference and/or applicable state-specific certification programs. The report cannot be reproduced without written permission from the client and Advanced Technology Laboratories.



Certificate of Analysis

Geocon Consultants, Inc.
6671 Brisa Street
Livermore , CA 94550

Project Number : ROCK CREEK BRIDGE DEMO, S9805-
Report To : Dave Watts
Reported : 01/30/2014

SUMMARY OF SAMPLES

Sample ID	Laboratory ID	Matrix	Date Sampled	Date Received
P1A/B	1400206-01	Paint	1/22/14 0:00	1/23/14 10:15
P2A/B	1400206-02	Paint	1/22/14 0:00	1/23/14 10:15
P3A/B	1400206-03	Paint	1/22/14 0:00	1/23/14 10:15



Certificate of Analysis

Geocon Consultants, Inc.
6671 Brisa Street
Livermore , CA 94550

Project Number : ROCK CREEK BRIDGE DEMO, S9805-
Report To : Dave Watts
Reported : 01/30/2014

Client Sample ID P1A/B

Lab ID: 1400206-01

Total Metals by ICP-AES EPA 6010B

Analyst: AG

Analyte	Result (mg/kg)	PQL (mg/kg)	MDL (mg/kg)	Dilution	Batch	Prepared	Date/Time Analyzed	Notes
Lead	210000	200	NA	100	B4A0387	01/28/2014	01/28/14 13:59	



Certificate of Analysis

Geocon Consultants, Inc.
6671 Brisa Street
Livermore, CA 94550

Project Number : ROCK CREEK BRIDGE DEMO, S9805-
Report To : Dave Watts
Reported : 01/30/2014

Client Sample ID P2A/B

Lab ID: 1400206-02

Total Metals by ICP-AES EPA 6010B

Analyst: AG

Analyte	Result (mg/kg)	PQL (mg/kg)	MDL (mg/kg)	Dilution	Batch	Prepared	Date/Time Analyzed	Notes
Lead	4.9	2.0	NA	1	B4A0387	01/28/2014	01/28/14 14:02	



Certificate of Analysis

Geocon Consultants, Inc.

6671 Brisa Street

Livermore, CA 94550

Project Number : ROCK CREEK BRIDGE DEMO, S9805-

Report To : Dave Watts

Reported : 01/30/2014

Notes and Definitions

ND	Analyte is not detected at or above the Practical Quantitation Limit (PQL). When client requests quantitation against MDL, analyte is not detected at or above the Method Detection Limit (MDL)
PQL	Practical Quantitation Limit
MDL	Method Detection Limit
NR	Not Reported
RPD	Relative Percent Difference
CA1	CA-NELAP (CDPH)
CA2	CA-ELAP (CDPH)
OR1	OR-NELAP (OSPHL)
TX1	TX-NELAP (TCEQ)

Notes:

- (1) The reported MDL and PQL are based on prep ratio variation and analytical dilution.
- (2) The suffix [2C] of specific analytes signifies that the reported result is taken from the instrument's second column.
- (3) Results are wet unless otherwise specified.

February 06, 2014

Dave Watts
Geocon Consultants, Inc.
6671 Brisa Street
Livermore, CA 94550
Tel: (925) 961-5273
Fax: (925) 371-5915

ACCREDITED IN ACCORDANCE WITH

ELAP No.: 1838
NELAP No.: 02107CA
CSDLAC No.: 10196
ORELAP No.: CA300003
TCEQ No.: T104704502

Re: ATL Work Order Number : 1400206

Client Reference : ROCK CREEK BRIDGE DEMO, S9805-01-20

Enclosed are the results for sample(s) received on January 23, 2014 by Advanced Technology Laboratories. The sample(s) are tested for the parameters as indicated on the enclosed chain of custody in accordance with applicable laboratory certifications. The laboratory results contained in this report specifically pertains to the sample(s) submitted.

Thank you for the opportunity to serve the needs of your company. If you have any questions, please feel free to contact me or your Project Manager.

Sincerely,



Eddie Rodriguez
Laboratory Director

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Certificate of Analysis

Geocon Consultants, Inc.
6671 Brisa Street
Livermore , CA 94550

Project Number : ROCK CREEK BRIDGE DEMO, S9805-
Report To : Dave Watts
Reported : 02/06/2014

SUMMARY OF SAMPLES

Sample ID	Laboratory ID	Matrix	Date Sampled	Date Received
P1A/B	1400206-01	Paint	1/22/14 0:00	1/23/14 10:15



Certificate of Analysis

Geocon Consultants, Inc.
6671 Brisa Street
Livermore, CA 94550

Project Number : ROCK CREEK BRIDGE DEMO, S9805-
Report To : Dave Watts
Reported : 02/06/2014

Notes and Definitions

M2	Matrix spike recovery outside of acceptance limit due to possible matrix interference. The analytical batch was validated by the laboratory control sample.
D2	Sample required dilution due to high concentration of non-target analyte.
ND	Analyte is not detected at or above the Practical Quantitation Limit (PQL). When client requests quantitation against MDL, analyte is not detected at or above the Method Detection Limit (MDL)
PQL	Practical Quantitation Limit
MDL	Method Detection Limit
NR	Not Reported
RPD	Relative Percent Difference
CA1	CA-NELAP (CDPH)
CA2	CA-ELAP (CDPH)
OR1	OR-NELAP (OSPHL)
TX1	TX-NELAP (TCEQ)

- Notes:
- (1) The reported MDL and PQL are based on prep ratio variation and analytical dilution.
 - (2) The suffix [2C] of specific analytes signifies that. The reported result is taken from the instrument's second column.
 - (3) Results are wet unless otherwise specified.

Diane Galvan

From: David Watts [watts@geoconinc.com]
Sent: Thursday, January 30, 2014 4:42 PM
To: Diane Galvan
Subject: Re: Results/EDD/Invoice - ROCK CREEK BRIDGE DEMO (1400206)

Tclp on P1. Same tat. Thx.

Sent from my iPhone