



STATE OF CALIFORNIA  
DEPARTMENT OF TRANSPORTATION

**NOTICE TO BIDDERS  
AND  
SPECIAL PROVISIONS**

**FOR CONSTRUCTION ON STATE HIGHWAY IN DEL NORTE COUNTY ABOUT  
7 MILES NORTH OF GASQUET AT VARIOUS LOCATIONS FROM 0.5 MILE  
NORTH OF MIDDLE FORK SMITH RIVER BRIDGE 01-0019 TO 0.8 MILE NORTH  
OF MIDDLE FORK SMITH RIVER BRIDGE 01-0016**

**In District 01 On Route 199**

**Under**

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*Bid book dated August 12, 2013*

*Standard Specifications dated 2010*

*Project plans approved June 24, 2013*

*Standard Plans dated 2010*

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**Identified by**

**Contract No. 01-479404**

**01-DN-199-20.3/25.9**

**Project ID 0100000371**

**Federal-Aid Project**

**NHP-P199(033)E**

**Electronic Advertising Contract**



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# SPECIAL NOTICES

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- For federal-aid projects, the Department is modifying its DBE program.

**CONTRACT NO. 01-479404**

The special provisions contained herein have been prepared by or under the direction of the following Registered Persons.

**HIGHWAY AND TRAFFIC**

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# STANDARD PLANS LIST

The standard plan sheets applicable to this Contract include those listed below. The applicable revised standard plans (RSPs) listed below are included in the project plans.

A10A	Abbreviations (Sheet 1 of 2)
A10B	Abbreviations (Sheet 2 of 2)
A10C	Lines and Symbols (Sheet 1 of 3)
A10D	Lines and Symbols (Sheet 2 of 3)
A10E	Lines and Symbols (Sheet 3 of 3)
A10F	Legend - Soil (Sheet 1 of 2)
A10G	Legend - Soil (Sheet 2 of 2)
A10H	Legend - Rock
A20A	Pavement Markers and Traffic Lines, Typical Details
A20B	Pavement Markers and Traffic Lines, Typical Details
A20C	Pavement Markers and Traffic Lines, Typical Details
A20D	Pavement Markers and Traffic Lines, Typical Details
RSP A24E	Pavement Markings - Words, Limit and Yield Lines
A62A	Excavation and Backfill - Miscellaneous Details
A62B	Limits of Payment for Excavation and Backfill - Bridge Surcharge and Wall
A62C	Limits of Payment for Excavation and Backfill - Bridge
A62F	Excavation and Backfill - Metal and Plastic Culverts
A73A	Object Markers
A73B	Markers
A73C	Delineators, Channelizers and Barricades
A76A	Concrete Barrier Type 60
A77B1	Metal Beam Guard Railing - Standard Hardware
A77C1	Metal Beam Guard Railing - Wood Post and Wood Block Details
A77C2	Metal Beam Guard Railing - Steel Post and Notched Wood Block Details
A77C3	Metal Beam Guard Railing - Typical Line Post Embedment and Hinge Point Offset Details
A77C4	Metal Beam Guard Railing - Typical Railing Delineation and Dike Positioning Details
A77F1	Metal Beam Guard Railing - Typical Layouts for Structure Approach
A77H1	Metal Railing - End Anchor Assembly (Type SFT)
A77J4	Metal Beam Guard Railing - Transition Railing (Type WB)
A87A	Curbs and Driveways

A87B	Hot Mix Asphalt Dikes
RSP P74	Pavement Edge Treatments
RSP P75	Pavement Edge Treatments - Overlays
RSP P76	Pavement Edge Treatments - New Construction
RSP D73	Drainage Inlets
D74B	Drainage Inlets
D74C	Drainage Inlet Details
D75A	Steel Pipe Inlets
D75C	Pipe Inlets - Ladder and Trash Rack Details
RSP D77A	Grate Details No. 1
RSP D77B	Grate Details No. 2
D87A	Corrugated Metal Pipe Downdrain Details
D87B	Plastic Pipe Downdrain Details
D87C	Cable Anchorage System
D87D	Overside Drains
D94A	Metal and Plastic Flared End Sections
D97A	Corrugated Metal Pipe Coupling Details No. 1 - Annular Coupling Band Bar and Strap and Angle Connections
D97C	Corrugated Metal Pipe Coupling Details No. 3 - Helical and Universal Couplers
D97D	Corrugated Metal Pipe Coupling Details No. 4 - Hugger Coupling Bands
D97F	Corrugated Metal Pipe Coupling Details No. 6 - Positive Joint
D97G	Corrugated Metal Pipe Coupling Details No. 7 - Downdrain
H1	Landscape and Erosion Control - Abbreviations
H2	Landscape - Symbols
H51	Erosion Control Details - Fiber Roll and Compost Sock
H52	Rolled Erosion Control Product
T1A	Temporary Crash Cushion, Sand Filled (Unidirectional)
T1B	Temporary Crash Cushion, Sand Filled (Bidirectional)
T2	Temporary Crash Cushion, Sand Filled (Shoulder Installations)
T3A	Temporary Railing (Type K)
T3B	Temporary Railing (Type K)
RSP T13	Traffic Control System for Lane Closure on Two Lane Conventional Highways
T51	Temporary Water Pollution Control Details (Temporary Silt Fence)
T56	Temporary Water Pollution Control Details (Temporary Fiber Roll)
T58	Temporary Water Pollution Control Details (Temporary Construction Entrance)

T59	Temporary Water Pollution Control Details (Temporary Concrete Washout Facility)
T60	Temporary Water Pollution Control Details (Temporary Reinforced Silt Fence)
T61	Temporary Water Pollution Control Details (Temporary Drainage Inlet Protection)
T62	Temporary Water Pollution Control Details (Temporary Drainage Inlet Protection)
T63	Temporary Water Pollution Control Details (Temporary Drainage Inlet Protection)
T64	Temporary Water Pollution Control Details (Temporary Drainage Inlet Protection)
B0-1	Bridge Details
B0-3	Bridge Details
B0-5	Bridge Details
B0-13	Bridge Details
RSP B3-3A	Retaining Wall Type 1A (Case 1)
RSP B3-4A	Retaining Wall Type 5 (Case 1)
RSP B3-5	Retaining Wall Details No. 1
B3-6	Retaining Wall Details No. 2
B6-21	Joint Seals (Maximum Movement Rating = 2")
B7-7	Deck Drain - Type D-3
RSP B11-47	Cable Railing
RSP B11-60	Concrete Barrier Type 80 (Sheet 1 of 2)
RS1	Roadside Signs, Typical Installation Details No. 1
RS2	Roadside Signs - Wood Post, Typical Installation Details No. 2
RS4	Roadside Signs, Typical Installation Details No. 4
S93	Framing Details for Framed Single Sheet Aluminum Signs, Rectangular Shape
S94	Roadside Framed Single Sheet Aluminum Signs, Rectangular Shape
S95	Roadside Single Sheet Aluminum Signs, Diamond Shape
ES-1A	Electrical Systems (Legend, Notes and Abbreviations)
ES-1B	Electrical Systems (Legend, Notes and Abbreviations)
ES-1C	Electrical Systems (Legend, Notes and Abbreviations)
ES-2C	Electrical Systems (Service Equipment Notes, Type III Series)
ES-2D	Electrical Systems (Service Equipment Enclosure and Typical Wiring Diagram, Type III - A Series)
ES-3C	Electrical Systems (Controller Cabinet Foundation Details)
ES-4A	Electrical Systems (Signal Heads and Mountings)
ES-4C	Electrical Systems (Vehicular Signal Heads and Mountings)
ES-4D	Electrical Systems (Signal Mounting)
ES-4E	Electrical Systems (Signal Faces and Emergency Vehicle Detector Mountings)

ES-5A	Electrical Systems (Detectors)
ES-5B	Electrical Systems (Detectors)
ES-5D	Electrical Systems (Curb Termination and Handhole)
ES-6A	Electrical Systems (Lighting Standard, Types 15 and 21)
ES-6D	Electrical Systems (Lighting Standard, Types 15D and 21D, Double Luminaire Mast Arm)
RSP ES-8A	Electrical Systems (Pull Box)
RSP ES-10A	Electrical Systems (Isofootcandle Diagrams)
ES-13A	Electrical Systems (Splicing Details)
ES-13B	Electrical Systems (Fuse Rating, Kinking and Banding Detail)

### **CANCELED STANDARD PLANS LIST**

The standard plan sheets listed below are canceled and not applicable to this contract.

B3-1	Canceled on April 20, 2012
B3-2	Canceled on April 20, 2012
B3-3	Canceled on April 20, 2012
B3-4	Canceled on April 20, 2012
B3-7	Canceled on April 20, 2012
B3-8	Canceled on April 20, 2012
ES-8	Canceled on January 20, 2012
ES-10	Canceled on July 20, 2012

# NOTICE TO BIDDERS

Bids open Tuesday, October 1, 2013

Dated August 12, 2013

General work description: Widen shoulders with HMA over AB, replace bridge and const ret walls.

The Department will receive sealed bids for CONSTRUCTION ON STATE HIGHWAY IN DEL NORTE COUNTY ABOUT 7 MILES NORTH OF GASQUET AT VARIOUS LOCATIONS FROM 0.5 MILE NORTH OF MIDDLE FORK SMITH RIVER BRIDGE 01-0019 TO 0.8 MILE NORTH OF MIDDLE FORK SMITH RIVER BRIDGE 01-0016.

District-County-Route-Post Mile: 01-DN-199-20.3/25.9

Contract No. 01-479404

The Contractor must have either a Class A license or any combination of the following Class C licenses which constitutes a majority of the work: C-8, C-12.

The DBE Contract goal is 8 percent.

Federal-aid project no.:

NHP-P199(033)E

For the Federal training program, the number of trainees or apprentices is 7.

Bids must be on a unit price basis.

Complete the work within 360 working days.

The estimated cost of the project is \$11,700,000.

No prebid meeting is scheduled for this project.

The Department will receive bids until 2:00 p.m. on the bid open date at 1727 30th Street, Bidders' Exchange, MS 26, Sacramento, CA 95816. Bids received after this time will not be accepted. Department staff will direct the bidders to the bid opening.

The Department will open and publicly read the bids at the above location immediately after the specified closing time.

District office addresses are provided in the *Standard Specifications*.

Present bidders' inquiries to the Department and view the Department's responses at:

[http://www.dot.ca.gov/hq/esc/oe/project\\_status/bid\\_inq.html](http://www.dot.ca.gov/hq/esc/oe/project_status/bid_inq.html)

Questions about alleged patent ambiguity of the plans, specifications, or estimate must be asked before bid opening. After bid opening, the Department does not consider these questions as bid protests.

Submit your bid with bidder's security equal to at least 10 percent of the bid.

Prevailing wages are required on this Contract. The Director of the California Department of Industrial Relations determines the general prevailing wage rates. Obtain the wage rates at the DIR Web site, <http://www.dir.ca.gov>, or from the Department's Labor Compliance Office of the district in which the work is located.

The federal minimum wage rates for this Contract as determined by the United States Secretary of Labor are available at <http://www.dot.ca.gov/hq/esc/oe/federal-wages>.

If the minimum wage rates as determined by the United States Secretary of Labor differs from the general prevailing wage rates determined by the Director of the California Department of Industrial Relations for similar classifications of labor, the Contractor and subcontractors must not pay less than the higher wage rate. The Department does not accept lower State wage rates not specifically included in the federal minimum wage determinations. This includes helper, or other classifications based on hours of experience, or any other classification not appearing in the federal wage determinations. Where federal wage determinations do not contain the State wage rate determination otherwise available for use by the Contractor and subcontractors, the Contractor and subcontractors must not pay less than the federal minimum wage rate that most closely approximates the duties of the employees in question.

The Department has made available Notices of Suspension and Proposed Debarment from the Federal Highway Administration. For a copy of the notices, go to [http://www.dot.ca.gov/hq/esc/oe/contractor\\_info](http://www.dot.ca.gov/hq/esc/oe/contractor_info). Additional information is provided in the Excluded Parties List System at <https://www.epls.gov>.

Department of Transportation

WAT

### BID ITEM LIST

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
1	070030	LEAD COMPLIANCE PLAN	LS	LUMP SUM
2	080050	PROGRESS SCHEDULE (CRITICAL PATH METHOD)	LS	LUMP SUM
3	090100	TIME-RELATED OVERHEAD (WDAY)	WDAY	400
4	120090	CONSTRUCTION AREA SIGNS	LS	LUMP SUM
5	120100	TRAFFIC CONTROL SYSTEM	LS	LUMP SUM
6	120149	TEMPORARY PAVEMENT MARKING (PAINT)	SQFT	48
7	120159	TEMPORARY TRAFFIC STRIPE (PAINT)	LF	8,800
8	026136	TEMPORARY PORTABLE TRANSVERSE RUMBLE STRIP	LF	140
9	120165	CHANNELIZER (SURFACE MOUNTED)	EA	85
10	120199	TRAFFIC PLASTIC DRUM	EA	7
11	120200	FLASHING BEACON (PORTABLE)	EA	6
12	026137	TEMPORARY SIGNAL SYSTEM (LOCATION 1)	LS	LUMP SUM
13	026138	TEMPORARY SIGNAL SYSTEM (LOCATION 2)	LS	LUMP SUM
14	128652	PORTABLE CHANGEABLE MESSAGE SIGN (LS)	LS	LUMP SUM
15	129000	TEMPORARY RAILING (TYPE K)	LF	2,540
16	026139	ALTERNATIVE TEMPORARY CRASH CUSHIONS	EA	8
17	026140	RELOCATE ALTERNATIVE TEMPORARY CRASH CUSHION	EA	3
18	130100	JOB SITE MANAGEMENT	LS	LUMP SUM
19	130300	PREPARE STORM WATER POLLUTION PREVENTION PLAN	LS	LUMP SUM
20	130310	RAIN EVENT ACTION PLAN	EA	100

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
21	130320	STORM WATER SAMPLING AND ANALYSIS DAY	EA	150
22	130330	STORM WATER ANNUAL REPORT	EA	4
23	130505	MOVE-IN/MOVE-OUT (TEMPORARY EROSION CONTROL)	EA	3
24	130510	TEMPORARY MULCH	SQYD	240
25	130570	TEMPORARY COVER	SQYD	2,460
26	130610	TEMPORARY CHECK DAM	LF	480
27	130620	TEMPORARY DRAINAGE INLET PROTECTION	EA	8
28	130640	TEMPORARY FIBER ROLL	LF	1,090
29	130650	TEMPORARY GRAVEL BAG BERM	LF	140
30	130670	TEMPORARY REINFORCED SILT FENCE	LF	790
31	130680	TEMPORARY SILT FENCE	LF	540
32	130710	TEMPORARY CONSTRUCTION ENTRANCE	EA	4
33	130730	STREET SWEEPING	LS	LUMP SUM
34	130900	TEMPORARY CONCRETE WASHOUT	LS	LUMP SUM
35	026141	WATER QUALITY SAMPLING AND ANALYSIS DAY	EA	300
36	026142	WATER QUALITY MONITORING REPORT	EA	36
37	026143	WATER QUALITY ANNUAL REPORT	EA	4
38	140003	ASBESTOS COMPLIANCE PLAN	LS	LUMP SUM
39	141000	TEMPORARY FENCE (TYPE ESA)	LF	190
40	141103	REMOVE YELLOW THERMOPLASTIC TRAFFIC STRIPE (HAZARDOUS WASTE)	LF	3,970

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
41	141120	TREATED WOOD WASTE	LB	19,100
42	146001	CONTRACTOR-SUPPLIED BIOLOGIST (DAY)	WDAY	300
43	146003	NATURAL RESOURCE PROTECTION PLAN	LS	LUMP SUM
44	026144	CONTRACTOR SUPPLIED ARBORIST (DAY)	WDAY	5
45	149001	DUST CONTROL PLAN (NATURALLY OCCURRING ASBESTOS)	LS	LUMP SUM
46	150203	ABANDON CULVERT (EA)	EA	1
47	026145	ABANDON DOWNDRAIN	EA	1
48	150630	REMOVE MARKER	EA	2
49	150662	REMOVE METAL BEAM GUARD RAILING	LF	1,470
50	150711	REMOVE PAINTED TRAFFIC STRIPE	LF	1,390
51	150712	REMOVE PAINTED PAVEMENT MARKING	SQFT	24
52	150722	REMOVE PAVEMENT MARKER	EA	170
53	150740	REMOVE SIGN	EA	12
54	150801	REMOVE OVERSIDE DRAIN	EA	2
55	150808	REMOVE CULVERT (EA)	EA	5
56	150814	REMOVE DOWNDRAIN (EA)	EA	3
57	150820	REMOVE INLET	EA	6
58	150821	REMOVE HEADWALL	EA	4
59	026146	REMOVE CABLE ANCHORAGE SYSTEM	EA	1
60	150860	REMOVE BASE AND SURFACING	CY	2,940

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
61	026147	RECONSTRUCT GABION WALL	LS	LUMP SUM
62	026148	RELOCATE TEMPORARY RAILING (TYPE K)	LF	1,000
63	153103	COLD PLANE ASPHALT CONCRETE PAVEMENT	SQYD	1,320
64	157550	BRIDGE REMOVAL	LS	LUMP SUM
65	160102	CLEARING AND GRUBBING (LS)	LS	LUMP SUM
66	170101	DEVELOP WATER SUPPLY	LS	LUMP SUM
67	190101	ROADWAY EXCAVATION	CY	5,100
68	190109	ROADWAY EXCAVATION (NATURALLY OCCURRING ASBESTOS)	CY	800
69 (F)	026149	STRIPPING EXCAVATION	CY	1,192
70	026150	SELECTED MATERIAL (CY)	CY	180
71	190185	SHOULDER BACKING	TON	350
72 (F)	192003	STRUCTURE EXCAVATION (BRIDGE)	CY	727
73	192009	STRUCTURE EXCAVATION (NATURALLY OCCURRING ASBESTOS)	CY	62
74 (F)	192020	STRUCTURE EXCAVATION (TYPE D)	CY	595
75 (F)	192037	STRUCTURE EXCAVATION (RETAINING WALL)	CY	625
76 (F)	192049	STRUCTURE EXCAVATION (SOLDIER PILE WALL)	CY	236
77 (F)	193003	STRUCTURE BACKFILL (BRIDGE)	CY	735
78 (F)	193013	STRUCTURE BACKFILL (RETAINING WALL)	CY	283
79 (F)	193029	STRUCTURE BACKFILL (SOLDIER PILE WALL)	CY	152
80 (F)	193116	CONCRETE BACKFILL (SOLDIER PILE WALL)	CY	283

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
81 (F)	193118	CONCRETE BACKFILL	CY	273
82 (F)	193119	LEAN CONCRETE BACKFILL	CY	66
83	202011	MULCH	CY	62
84	210010	MOVE-IN/MOVE-OUT (EROSION CONTROL)	EA	4
85	210280	ROLLED EROSION CONTROL PRODUCT (BLANKET)	SQFT	8,260
86	210300	HYDROMULCH	SQFT	23,300
87	210350	FIBER ROLLS	LF	75
88	210420	STRAW	SQFT	23,300
89	210430	HYDROSEED	SQFT	52,460
90	210600	COMPOST	SQFT	23,300
91	210630	INCORPORATE MATERIALS	SQFT	21,800
92	260203	CLASS 2 AGGREGATE BASE (CY)	CY	1,890
93	390011	PREPAVING INERTIAL PROFILER	LS	LUMP SUM
94	390020	PREPAVING GRINDING DAY	EA	2
95	390132	HOT MIX ASPHALT (TYPE A)	TON	3,480
96	390134	HOT MIX ASPHALT (OPEN GRADED)	TON	1,710
97	390135	HOT MIX ASPHALT (LEVELING)	TON	790
98	391007	PAVING ASPHALT (BINDER, GEOSYNTHETIC PAVEMENT INTERLAYER)	TON	1.2
99	393004	GEOSYNTHETIC PAVEMENT INTERLAYER (PAVING FABRIC)	SQYD	1,070
100	026151	CENTERLINE RUMBLE STRIP	STA	44

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
101	394077	PLACE HOT MIX ASPHALT DIKE (TYPE F)	LF	460
102	397005	TACK COAT	TON	11
103	460210	GROUND ANCHOR (SUBHORIZONTAL)	EA	36
104	480300	TEMPORARY SUPPORT	LS	LUMP SUM
105	026152	TEMPORARY RETAINING WALL	SQFT	680
106	026153	STEEL SOLDIER PILE (C 15 X 50)	LF	1,820
107	044391	STEEL SOLDIER PILE (W 16 X 67)	LF	259
108	044392	STEEL SOLDIER PILE (W 18 X 106)	LF	260
109	044393	STEEL SOLDIER PILE (W 18 X 143)	LF	700
110	490316	STEEL SOLDIER PILE (HP 14 X 73)	LF	210
111	490400	24" DRILLED HOLE	LF	259
112	490403	30" DRILLED HOLE	LF	1,811
113	490601	16" CAST-IN-DRILLED-HOLE CONCRETE PILING	LF	336
114	490605	36" CAST-IN-DRILLED-HOLE CONCRETE PILING	LF	100
115	490606	42" CAST-IN-DRILLED-HOLE CONCRETE PILING	LF	303
116	490607	48" CAST-IN-DRILLED-HOLE CONCRETE PILING	LF	24
117	495000	MICROPILE	EA	55
118 (F)	510051	STRUCTURAL CONCRETE, BRIDGE FOOTING	CY	355
119 (F)	510053	STRUCTURAL CONCRETE, BRIDGE	CY	1,258
120 (F)	510060	STRUCTURAL CONCRETE, RETAINING WALL	CY	413

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
121 (F)	510072	STRUCTURAL CONCRETE, BARRIER SLAB	CY	138
122 (F)	510502	MINOR CONCRETE (MINOR STRUCTURE)	CY	24.3
123 (F)	510526	MINOR CONCRETE (BACKFILL)	CY	5.7
124	044394	RANDOM ROCK TEXTURE	SQFT	7,370
125	519081	JOINT SEAL (MR 1/2")	LF	20
126	519091	JOINT SEAL (MR 1 1/2")	LF	20.5
127	519100	JOINT SEAL (MR 2")	LF	40.5
128	519102	JOINT SEAL (TYPE AL)	LF	3
129 (F)	520102	BAR REINFORCING STEEL (BRIDGE)	LB	350,200
130 (F)	520103	BAR REINFORCING STEEL (RETAINING WALL)	LB	36,200
131 (F)	520106	BAR REINFORCING STEEL (EPOXY COATED)	LB	94,025
132 (F)	044395	BAR REINFORCING STEEL (BARRIER SLAB) (EPOXY COATED)	LB	14,930
133 (F)	520115	BAR REINFORCING STEEL (GALVANIZED)	LB	670
134	560248	FURNISH SINGLE SHEET ALUMINUM SIGN (0.063"-UNFRAMED)	SQFT	58
135	560249	FURNISH SINGLE SHEET ALUMINUM SIGN (0.080"-UNFRAMED)	SQFT	84
136	562001	METAL (ROADSIDE SIGN)	LB	50
137	566011	ROADSIDE SIGN - ONE POST	EA	13
138	568017	INSTALL ROADSIDE SIGN PANEL ON EXISTING POST	EA	1
139 (F)	575004	TIMBER LAGGING	MFBM	25.5
140	597601	PREPARE AND STAIN CONCRETE	SQFT	7,370

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
141	620100	18" ALTERNATIVE PIPE CULVERT	LF	65
142	620140	24" ALTERNATIVE PIPE CULVERT	LF	370
143	680283	3" PLASTIC PIPE UNDERDRAIN	LF	100
144	681132	GEOCOMPOSITE DRAIN	SQFT	800
145 (F)	682022	CLASS 1 PERMEABLE MATERIAL (BLANKET)	CY	2.4
146	692003	8" ENTRANCE TAPER	EA	1
147	692303	8" ANCHOR ASSEMBLY	EA	1
148	698020	8" ALTERNATIVE PIPE DOWNDRAIN	LF	12
149	698140	24" ALTERNATIVE PIPE DOWNDRAIN	LF	80
150	700639	36" CORRUGATED STEEL PIPE INLET (.109" THICK)	LF	7.1
151	705315	24" ALTERNATIVE FLARED END SECTION	EA	1
152 (F)	721015	ROCK SLOPE PROTECTION (LIGHT, METHOD B) (CY)	CY	15
153	721524	CONCRETED-ROCK SLOPE PROTECTION (1/4 T, METHOD A) (TON)	TON	500
154	026154	DOUBLE TWISTED WIRE MESH DRAPERY SYSTEM	SQFT	26,500
155	729011	ROCK SLOPE PROTECTION FABRIC (CLASS 8)	SQYD	38
156	730045	MINOR CONCRETE (GUTTER) (CY)	CY	10
157 (F)	750001	MISCELLANEOUS IRON AND STEEL	LB	3,499
158 (F)	750041	ISOLATION CASING	LB	1,555
159 (F)	750501	MISCELLANEOUS METAL (BRIDGE)	LB	438
160 (F)	750505	BRIDGE DECK DRAINAGE SYSTEM	LB	780

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
161	044396	BAT HABITAT	EA	4
162	820112	MARKER (CULVERT)	EA	16
163	820118	GUARD RAILING DELINEATOR	EA	31
164	832001	METAL BEAM GUARD RAILING	LF	380
165	832013	METAL BEAM GUARD RAILING (7' POST)	LF	820
166 (F)	839527	CABLE RAILING (MODIFIED)	LF	365
167	839541	TRANSITION RAILING (TYPE WB)	EA	8
168	839584	ALTERNATIVE IN-LINE TERMINAL SYSTEM	EA	9
169	839585	ALTERNATIVE FLARED TERMINAL SYSTEM	EA	2
170	839704	CONCRETE BARRIER (TYPE 60D)	LF	175
171 (F)	044397	CONCRETE BARRIER (TYPE 80 MODIFIED)	LF	995
172 (F)	044398	CONCRETE BARRIER (TYPE 80A MODIFIED)	LF	183
173	840504	4" THERMOPLASTIC TRAFFIC STRIPE	LF	18,300
174	840506	8" THERMOPLASTIC TRAFFIC STRIPE	LF	450
175	850122	PAVEMENT MARKER (RETROREFLECTIVE- RECESSED)	EA	400
176	999990	MOBILIZATION	LS	LUMP SUM

# SPECIAL PROVISIONS

## DIVISION I GENERAL PROVISIONS

### 1 GENERAL

**Add to section 1-1.01:**

#### Bid Items and Applicable Sections

Item code	Item description	Applicable section
026136	TEMPORARY PORTABLE TRANSVERSE RUMBLE STRIPS	12
026137	TEMPORARY SIGNAL SYSTEM (LOCATION 1)	12
026138	TEMPORARY SIGNAL SYSTEM (LOCATION 2)	12
026139	ALTERNATIVE TEMPORARY CRASH CUSHIONS	12
026140	RELOCATE ALTERNATIVE TEMPORARY CRASH CUSHION	12
026141	WATER QUALITY SAMPLING AND ANALYSIS DAY	13
026142	WATER QUALITY MONITORING REPORT	13
026143	WATER QUALITY ANNUAL REPORT	13
026144	CONTRACTOR SUPPLIED ARBORIST (DAY)	20
026145	ABANDON DOWNDRAIN	15
026146	REMOVE CABLE ANCHORAGE SYSTEM	15
026147	RECONSTRUCT GABION WALL	15
026148	RELOCATE TEMPORARY RAILING (TYPE K)	15
026149	STRIPPING EXCAVATION	19
026150	SELECTED MATERIAL (CY)	19
026151	CENTERLINE RUMBLE STRIP	39
026152	TEMPORARY RETAINING WALL	48
026153	STEEL SOLDIER PILE (C 15 X 50)	49
044391	STEEL SOLDIER PILE (W 16 X 67)	49
044392	STEEL SOLDIER PILE (W 18 X 106)	49
044393	STEEL SOLDIER PILE (W 18 X 143)	49
044394	RANDOM ROCK TEXTURE	51
044395	BAR REINFORCING STEEL (BARRIER SLAB) (EPOXY COATED)	52
026154	DOUBLE TWISTED WIRE MESH DRAPERY SYSTEM	72
044396	BAT HABITAT	78
044397	CONCRETE BARRIER (TYPE 80 MODIFIED)	83
044398	CONCRETE BARRIER (TYPE 80A MODIFIED)	83

**Add to section 1-1.09:**

This project is in a freeze-thaw area.



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## 5 CONTROL OF WORK

### Add to section 5-1.09A:

The Department encourages the project team to exhaust the use of partnering in dispute resolution before engagement of an objective third party.

For certain disputes, a facilitated partnering session or facilitated dispute resolution session may be appropriate and effective in clarifying issues and resolving all or part of a dispute.

To afford the project team enough time to plan and hold the session, a maximum of 20 days may be added to the DRB referral time following the Engineer's response to a *Supplemental Potential Claim Record*.

To allow this additional referral time, the project team must document its agreement and intention in the dispute resolution plan of the partnering charter. The team may further document agreement of any associated criteria to be met for use of the additional referral time.

If the session is not held, the DRB referral time remains in effect as specified in section 5-1.43.

### Add to section 5-1.20A:

During the progress of the work under this Contract, work under the following contracts may be in progress at or near the job site of this Contract:

#### Coincident or Adjacent Contracts

Contract no.	County–Route–Post Mile	Location	Type of work
01-0B3104	DN-199-21.7	Near Gasquet	Reconstruct Roadway
01-0B3204	DN-199-24.6	Near Gasquet	Construct Retaining Wall
01-0B3304	DN-199-26.3	Near Gasquet	Reconstruct Roadway
01-4500U4	DN-199-22.7/26.5	Near Gasquet	Widen Roadway

AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA

## 6 CONTROL OF MATERIALS

### Add to section 6-2.03:

The Department furnishes you with:

- Loop detector sensor units
- Model 170 controller assembly, including controller unit, completely wired controller cabinet, and detector sensor units

The Department furnishes you with completely wired controller cabinets with auxiliary equipment but without controller unit at the Bracut Maintenance Yard at 6100 North Highway 101. At least 48 hours before you pick up the materials, inform the Engineer of what you will pick up and when you will pick it up.

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## 7 LEGAL RELATIONS AND RESPONSIBILITY TO THE PUBLIC

Replace section 7-1.02K(6)(j)(iii) with:

### 7-1.02K(6)(j)(iii) Earth Material Containing Lead

Section 7-1.02K(6)(j)(iii) includes specifications for handling, removing, and disposing of earth material containing lead.

Submit a lead compliance plan.

Lead is present in earth material on the job site. The average lead concentrations are below 1,000 mg/kg total lead and below 5 mg/L soluble lead. The material on the job site:

1. Is not a hazardous waste
2. Does not require disposal at a permitted landfill or solid waste disposal facility

Lead has been detected in material to a depth of 1.0 in unpaved areas of the highway. Levels of lead found on the job site range from less than 2.5 to 150 mg/kg total lead with an average concentration of 15.22 mg/kg total lead as analyzed by EPA test method 6010 or EPA test method 7000 series and based upon a 95 percent upper confidence limit. Levels of lead found within the project limits have a predicted average soluble concentration of up to 3.6 mg/L as analyzed by the California Waste Extraction Test and based upon a 95 percent upper confidence limit.

Handle the material under all applicable laws, rules, and regulations, including those of the following agencies:

1. Cal/OSHA
2. CA RWQCB, Region1—North Coast
3. CA Department of Toxic Substances Control

If the material is disposed of:

1. Disclose the lead concentration of the material to the receiving property owner when obtaining authorization for disposal on the property
2. Obtain the receiving property owner's acknowledgment of lead concentration disclosure in the written authorization for disposal
3. You are responsible for any additional sampling and analysis required by the receiving property owner

If you choose to dispose of the material at a commercial landfill:

1. Transport it to a Class III or Class II landfill appropriately permitted to receive the material
2. You are responsible for identifying the appropriately permitted landfill to receive the material and for all associated trucking and disposal costs, including any additional sampling and analysis required by the receiving landfill





Sign panels for construction project funding signs must be framed, single sheet aluminum panels complying with section 56-2.

The background on construction project funding signs must be Type II retroreflective sheeting on the Authorized Material List for signing and delineation materials.

The legend must be retroreflective, except for nonreflective black letters and numerals. The colors blue and orange must comply with PR Color no. 3 and no. 6, respectively, as specified in the Federal Highway Administration's *Color Tolerance Chart*.

The legend for the type of project on construction project funding signs must read as follows:

HIGHWAY CONSTRUCTION

The legend for the types of funding on construction project funding signs must read as follows and in the following order:

FEDERAL HIGHWAY TRUST FUNDS

STATE HIGHWAY FUNDS

The Engineer will provide the year of completion for the legend on construction project funding signs. Furnish and install a sign overlay for the year of completion within 10 working days of notification.

The size of the legend on construction project funding signs must be as described. Do not add any additional information unless authorized.

### **12-2.03 CONSTRUCTION**

Install 2 Type 1 construction project funding signs at the locations designated by the Engineer before starting major work activities visible to highway users.

When authorized, remove and dispose of construction project funding signs upon completion of the project.

### **12-2.04 PAYMENT**

Not Used

**Replace section 12-3.05 with:**

### **12-3.05 PORTABLE FLASHING BEACONS**

#### **12-3.05A General**

Section 12-3.05 includes specifications for installing, removing, and moving portable flashing beacons.

Each portable flashing beacon must have:

1. Standard and base
2. Lighting unit
3. Flasher unit
4. Battery power source

Assemble units to form a complete, self-contained, flashing beacon that can be delivered to the job site and placed into immediate operation.

#### **12-3.05B Materials**

The lens for the beacon lighting unit must have a visible diameter of 12 inches. The lens must be glass or plastic as specified in ANSI D-10.1 for a yellow traffic signal lens.

Provide a minimum 8-inch-long visor and a backplate for the beacon lighting unit. Visors are not required during the hours of darkness.

The flasher unit must provide 50 to 60 flashes per minute with 250- to 350-milliseconds dwell time.

The standard must be adjustable to provide variable mounting of the lighting unit from 6 to 10 feet, measured from the bottom of the base to the center of the lens, with provisions for securing the standard at the desired height. Securely attach the standard to the base and provide enough length of multi-conductor, neoprene jacketed cable as required for the full vertical height.

The base must be large enough to accommodate a minimum of two 12-V automotive-type storage batteries, and must be of such shape and mass that the beacon will not roll in the event it is struck by a vehicle or pushed over.

The lamp must be rated at 25 W for operation on 12-V battery current.

The flashing beacon assembly must be weatherproof and must be capable of operating a minimum of 150 hours between battery recharging or other routine maintenance.

The standard and base must be finished with 2 applications of commercial-quality, orange enamel similar in color to color no. 12473 of Federal Standard 595B. The interior of the visor and the front face of the backplate must be finished with 2 applications of commercial-quality flat black enamel.

#### **12-3.05C Construction**

Moving flashing beacons from location to location as ordered after initial placement is change order work.

Immediately repair and repaint, or replace flashing beacons in their original locations when they are displaced or not in an upright position from any cause.

The Department does not pay for repair or replacement of portable flashing beacons.

#### **12-3.05D Payment**

Portable flashing beacons are measured once at each location.

#### **Add to section 12-3.12C:**

Start displaying the message on the portable changeable message sign 15 minutes before closing the lane.

Place the portable changeable message sign in advance of the 1st warning sign for each:

1. Stationary lane closure
2. Shoulder closure

For 5 days, starting on the day of signal activation, place 1 portable changeable message sign in each direction of travel and display the following message: "SIGNAL AHEAD -- PREPARE TO STOP."

#### **Replace section 12-3.15 with:**

#### **12-3.15 ALTERNATIVE TEMPORARY CRASH CUSHION**

##### **12-3.15A GENERAL**

Alternative temporary crash cushion must be furnished and installed as shown on the plans and under these special provisions.

Whenever work activity exposes traffic to a fixed obstacle, protect the traffic from the obstacle with an alternative temporary crash cushion. The alternative temporary crash cushion must be in place before opening traffic lanes adjacent to the obstacle.

##### **12-3.15B MATERIALS**

Alternatives for temporary crash cushion systems must consist of one of the following Department authorized equal.

1. CRASH CUSHION (TYPE ADIEM) – Crash cushion (Type ADIEM) must be an ADIEM 350 manufactured by Trinity Highway Products, LLC located in Centerville, Utah. The ADIEM 350 can be obtained from Trinity Highway Products, LLC, 950 West 400 South, Centerville, UT 84014, telephone (800) 772-7976.
2. CRASH CUSHION (QuadGuard CZ System) – Crash cushion (QuadGuard CZ System) must be from Energy Absorption Systems, Inc. The QuadGuard CZ system can be obtained from the distributor, Traffic Control Service, Inc., 1818 E. Orangethorp, Fullerton, CA 92831, telephone (800) 222-8274.
3. CRASH CUSHION (TYPE ABSORB) – Crash cushion (Type Absorb) must be an ABSORB 350, 9-element system, as manufactured by Barrier Systems, Inc. The ABSORB 350, 9 element system can be obtained from the distributor, Statewide Traffic Safety and Signs, telephone (800) 559-7080.

### **12-3.15C SUBMITTALS**

Submit one copy of the manufacturer's plan and parts list for the alternative temporary crash cushion to be installed.

Submit a certificate of compliance for alternative temporary crash cushion.

### **12-3.15D CONSTRUCTION**

Alternative temporary crash cushion must be installed under the manufacturer's instructions and these specifications.

Maintain in place alternative temporary crash cushion at each location including times when work is not actively in progress. Remove alternative temporary crash cushion when no longer required as determined by the Engineer.

Immediately repair alternative temporary crash cushion damaged by your activities or by public traffic. Remove and replace alternative temporary crash cushions damaged beyond repair by your activities or by public traffic as determined by the Engineer.

Attach a Type R or Type P marker panel to the front of the alternative temporary crash cushion if the closest point of the alternative temporary crash cushion system is within 12 feet of the travelled way. Firmly fasten the marker panel to the alternative temporary crash cushion system with commercial quality hardware or by other authorized methods.

Relocate alternative temporary crash cushion to locations shown on plans. Relocation must occur within one day.

Remove alternative temporary crash cushion including marker panels, at the time of Contract acceptance. Do not install alternative temporary crash cushion in the permanent work.

### **12-3.15E PAYMENT**

The Department does not pay for alternative temporary crash cushion placed under section 7-1.04 or placed in excess of the number described.

Relocate alternative temporary crash cushion is paid as a separate bid item.

A lateral move of an alternative temporary crash cushion, not shown or ordered is change order work.

Repair or replacement of alternative temporary crash cushion damaged by public traffic is change order work.

**Replace section 12-3.16 with:**

### **12-3.16 TEMPORARY SIGNAL SYSTEM**

#### **12-3.16A General**

Installing temporary signal system (TSS) consists of installing, maintaining and removing temporary traffic signal, lighting, and photovoltaic powered flashing beacons for traffic control.

Install the TSS system as shown on the sheet labeled *E*. The sheet title matches the bid item description.

The Department will furnish 2 Model 170E traffic signal controller assembly, including wired cabinet, controller unit, and loop detector sensor units.

Furnish other materials and equipment for a TSS, including photovoltaic powered flashing beacons, signal heads, mast arms, luminaires, wood poles, conductors, light emitted diode (LED) signal modules, generators, uninterruptible power supply (UPS), automatic transfer switch (ATS) and hardware.

Material and equipment used in the TSS may be new or used but must be suitable for the intended use. Detector lead-cables and loop wires must be new.

Orient each signal face to be clearly visible to traffic approaching from the direction that the signal is intended to control.

Backplates and visors on used equipment must be primed and painted black

### **12-3.16B Operation**

TSS must operate at nominal 120 V(ac). Lighting must operate at 120 V(ac).

Unless otherwise directed, the system must operate on a continuous, 24-hour basis except when it is necessary that traffic be controlled by flaggers. When traffic is controlled by flaggers, the temporary flashing beacons must remain in operation and the signals placed on all red flash, or the signal turned off, as approved.

The Department will perform timing for the TSS.

### **12-3.16C Maintaining Temporary Signal System**

Except for the controller assembly, you are responsible for maintaining the TSS.

If components in the TSS are damaged, displaced, or cease to operate or function as specified from any cause during the progress of the work, immediately repair or replace the components, then restore to the original condition. Components include signs, generator, ATS, luminaires, photovoltaic powered flashing beacons, and signal equipment.

If the TSS is out of operation, provide flaggers immediately, at your expense, to maintain traffic control until the traffic signals are returned to service.

### **12-3.16D Light Emitting Diode Signal Module**

Comply with section 86-4.01D.

Use LED signal module as the light source for the 12-inch section.

### **12-3.16E Conduit**

At locations where conduit is required to be installed under pavement and if a delay to vehicles will not exceed 5 minutes, conduit may be installed by the trenching in pavement method as specified in section 86-2.05C.

### **12-3.16F Conductors and Wiring**

Conductors must be the types specified in section 86-2.08 or Type UF cable of the size and number of conductors shown. The minimum conductor size must be no. 12.

If conductors are placed across paved areas, place in a conduit.

Conductors placed outside of paved areas must be placed in conduit. If Type 1 or 2 conduit is used, the minimum depth must be 12 inches. If Type 3 conduit is used, the minimum depth must be 18 inches.

Conductors placed across structures must be placed in a Type 1, 2, or 3 conduit. Install the conduit on the outside face of the railing and secure by a method determined by the Engineer.

Conductors to be installed on wood poles or posts are to be placed in conduit. Conduit below 10 feet above ground must be Type 1 conduit or Type 3-Schedule 80 conduit.

Unless otherwise shown, weatherheads must be installed on top of vertical conduits on wood poles and posts.

Conductors to a terminal compartment or signal head on a pole may be spliced to through conductors of the same phase in a pull box adjacent to the pole. Do not splice conductors or cables except in pull boxes or in NEMA Type 3R enclosures.

### **12-3.16G Bonding and Grounding**

Comply with section 86-2.10.

Provide effective grounding for the generator.

### **12-3.16H Service**

#### **12-3.16H(1) General**

Use a generator system with an additional generator as a backup to provide power for the TSS.

#### **12-3.16H(2) Commercial Power**

Not Used

#### **12-3.16H(3) Generator**

Generators must be 120/240 V(ac), 60 Hz, 5 kW minimum, continuous duty type which will provide an applied voltage of  $120 \pm 5$  V at  $60 \pm 1.0$  Hz. Generators may be powered by LPG engines. Engines must have automatic oil feed. Generator systems must be equipped to provide automatic start-stop operation, with a 12 V(dc) starting system. The backup generator must be equipped with a battery charger that is powered by the line voltage. Generator output circuits must have overcurrent protection with a maximum setting of 30 A or as shown.

Generators, power source equipment, fuel sources, fuel storage containers and fuel lines must be located a minimum of 10 feet outside the edge of traveled way, be secure and be protected by temporary railing (Type K).

Fuel storage must be sufficient for times when the generator system operates unattended.

Engines must be equipped with approved spark arrestors.

#### **12-3.16H(4) Generator Operation**

Provide 2 generators. A single generator must operate the system. In the event of a failure to supply voltage for the system, the 2nd generator must start automatically and transfer the system load upon reaching operating voltage.

#### **12-3.16H(5) Automatic Transfer Switch**

An automatic transfer switch must provide the following functions:

1. Line voltage monitoring and in the event of a power outage signal the generator to start.
2. Engine start delay, adjustable from 0 to 6 seconds, to prevent starting if the power outage is only momentary and an engine stop delay, adjustable from 0 to 8 minutes, to allow the generator set to run unloaded to cool before shut down.
3. Transfer delay adjustable from 0 to 30 seconds to allow the generator to stabilize before connecting to the load and retransfer delay adjustable from 0 to 30 minutes to allow the line voltage to stabilize.
4. "Load-No Load" switch to allow a test with or without load.
5. "Normal-Test" switch that will start and run the generator in the "Test" position. "Normal" position must return the generator to automatic operation.
6. Battery charger powered by the normal line voltage.
7. Generator voltage sensor that signals for a transfer if the generator output is ready.

Provide a mechanical interlock to prevent application of power to the load from both sources and to prevent backfeeding from the generator to the line.

The automatic transfer switch must be rated at 100 A, 120/240 V(ac), 3-wire, single phase and be compatible with the generator furnished.

### **12-3.16I Uninterruptible Power Supply**

Provide an On-Line mode type uninterruptible power supply (UPS) that supplies power to operate the temporary signal system in the event of loss of primary power and during the transfer to the backup generator. The UPS must have sufficient capacity to operate the TSS for a minimum of 5 minutes. The UPS must automatically provide power to the TSS upon loss of primary power and automatically transfers the load to the backup generator when the UPS voltage sensor senses that the backup generator output is ready.

The UPS must be 2.0 kW minimum, 120 V(ac), single phase and capable of supplying 20 A minimum.

The UPS must not be installed in the signal controller cabinet.

### **12-3.16J Testing and Scheduling of Work**

At least 2 days prior to the start of the preliminary functional field testing, perform the continuity, ground and insulation resistance tests and generator transfer test. Record and provide all results of the field tests to the Engineer.

Submit a proposed schedule for a preliminary functional test and anticipated initial turn-on. This schedule must provide a minimum five days notice prior to a preliminary functional test date after complete installation of the TSS, and a minimum ten days in advance notice of an anticipated traffic signal turn-on. Preliminary functional test date is subject to availability of Department Electrical Personnel to observe and assist.

Perform a preliminary functional test to show the TSS, including the loop detection and generator transfer system is installed and operates correctly.

Upon satisfactory preliminary testing of the TSS in which all deficiencies have been corrected, request scheduling of the initial turn-on of the TSS. This schedule request must provide a minimum five days notice for turn-on. This turn on date is subject to availability of Department Electrical Personnel to assist with turn-on.

Do not put the TSS in operation until the Engineer has approved the preliminary functional test of the system. Do not perform initial turn-on after 1300 or on lane closure restriction days per section 12-4.04. Remove equipment from the roadway and do not interfere with the turn-on.

Arrange to have signal technicians present at the time the system is preliminary field-tested and when the equipment is turned on.

After turn-on, test the generator and transfer system operation on a weekly basis. The test must demonstrate that the TSS and generator transfer system operate satisfactorily when the primary power source is shut off.

### **12-3.16K Department-Furnished Controller Assembly**

Construct the controller cabinet foundation as shown for Model 332 cabinets, including furnishing and installing anchor bolts. Install the controller cabinet on the foundation and make field wiring connections to the terminal blocks in the controller cabinet.

Completely wired controller cabinets, with auxiliary equipment, but without controller unit, are at the Department Maintenance Station in Eureka, 6100 North Highway 101.

A listing of field conductor terminations in each Department-furnished controller cabinet will be furnished to you at the job site.

The Department forces will maintain all controller assemblies.

### **12-3.16L Detectors**

Loop detector sensor units are Department-furnished as part of the controller assembly.

Loop detector lead-in cable must be Type B.

Comply with section 86-5.01A.

Detector lead-in cables must run continuously from the termination pull box to the control cabinet terminals and shall not be spliced.

Loop wire is Type 2.

Type A loops consist of four turns of conductor.

Slots must be filled with elastomeric sealant or hot-melt rubberized asphalt sealant.

Multiple loops to a single detector lead-in cable are connected in series.

### **12-3.16M Photovoltaic Powered Flashing Beacon**

#### **12-3.16M(1) General**

Photovoltaic powered flashing beacon indication is a wood poles, mounted flashing beacon, sign panel, photovoltaic panels, batteries as shown and operates at 12 V(dc).

Wood poles must be of the size shown and comply with section 86-2.12.

Flashing beacon indication is a 12 inch circular section with a yellow light emitting diode (LED) assembly source (20 W nominal) and has visors and backplates.

#### **12-3.16M(2) Submittals**

Submit to the Engineer details of the proposed photovoltaic system for approval, prior to installation.

Details include 12-inch amber LED beacon specifications, photovoltaic panel specifications, voltage regulator specifications, battery specifications.

#### **12-3.16M(3) Photovoltaic System Details**

Photovoltaic panels, voltage regulation assembly and batteries are sized to provide for 120 hours of continuous operation without sunlight for charging.

You may remotely locate photovoltaic panels, and voltage regulation assembly.

### **12-3.16N Maintaining Visibility**

Clear, remove and dispose of tree branches and brush that may affect accessibility to cabinets, enclosures and generators, and that may affect the visibility of traffic signals, flashing beacons and signs. Maintain visibility until removal of TSS.

### **12-3.16O Salvaging TSS**

Upon completion of the work requiring TSS, salvage Department-furnished components of the TSS and deliver to the Department Maintenance Station near at 6100 North Highway 101, Eureka CA 95503.

### **12-3.16P Completion and Restoration**

Backfill pole holes.

The following materials may be abandoned in place when no longer required:

1. Conductors placed in slots across paved areas
2. Direct buried cables, installed 24 inches or more below the ground surface

#### **Add to section 12-3:**

### **12-3.18 TEMPORARY PORTABLE TRANSVERSE RUMBLE STRIPS**

#### **12-3.18A General**

##### **12-3.18A(1) Summary**

Section 12-3.18 includes specifications for installing, removing, and moving temporary portable transverse rumble strips (TPTRS).

##### **12-3.18A(2) Submittals**

Submit a certificate of compliance for TPTRS.

##### **12-3.18B Materials**

TPTRS must be made of flexible, engineered polymer materials.

TPTRS must have a non-slip surface texture

Each rumble strip must measure approximately 11 feet long, 12 inches wide and 13/16 inch thick.

Each rumble strip must have a 12 degree bevel on the leading edge.

TPTRS must be black in color.

**12-3.18C Construction**

Place and maintain TPTRS across the traveled way as shown. Do not place TPTRS across surfaces used by bicycles.

**12-3.18D Payment**

No separate or additional payment will be made for relocating, repositioning or removing TPTRS.

**Add to section 12-4.02A:**

Notify the Engineer at least 15 days prior to starting any work that will reduce the traveled way to less than 16 feet.

Designated holidays are as shown in the following table:

<b>Designated Holidays</b>	
Holiday	Date observed
New Year's Day	January 1st
Washington's Birthday	3rd Monday in February
Memorial Day	Last Monday in May
Independence Day	July 4th
Labor Day	1st Monday in September
Veterans Day	November 11th
Thanksgiving Day	4th Thursday in November
Christmas Day	December 25th

If a designated holiday falls on a Sunday, the following Monday is a designated holiday. If November 11th falls on a Saturday, the preceding Friday is a designated holiday.

Special days are:

Event	Event Date	Special Days
Sea Cruise	First Weekend in October	Friday through Monday

Under a 1-way reversing traffic control operation, traffic may be stopped in 1 direction for periods not to exceed five minutes. After each stoppage, all accumulated traffic for that direction must pass through the work zone before another stoppage is made.

Not more than three stationary lane closures will be allowed in each direction of travel at one time. Concurrent stationary closures must be spaced no closer than one mile apart.

During excavation, installation of culverts or temporary signal system, the road may be closed and traffic stopped for periods not to exceed 10 minutes. After 1 closure is made, all accumulated traffic must pass through the work zone before another closure is allowed.

Personal vehicles of your employees must not be parked on the traveled way or shoulders, including sections closed to traffic.

If work occurs or when work vehicles or equipment are parked within 6 feet of a traffic lane, close the shoulder area with fluorescent orange traffic cones or portable delineators. Place the cones or delineators on a taper in advance of the parked vehicles or equipment and along the edge of the pavement at 25-foot intervals to a point not less than 25 feet past the last vehicle or piece of equipment. Use at least 9 cones or delineators for the taper. Use a W20-1, "Road Work Ahead," W21-5b, "Right/Left Shoulder Closed Ahead," or C24(CA), "Shoulder Work Ahead," sign mounted on a crashworthy, portable sign support with flags. The sign must be placed as ordered by the Engineer and at least 48 by 48 inches in size. If a cone or delineator is displaced or overturned, immediately restore the device to its original position or location.

**Replace "Reserved" in section 12-4.04 with:**

<b>Lane Closure Restriction for Designated Holidays and Special Days</b>										
Thu	Fri	Sat	Sun	Mon	Tues	Wed	Thu	Fri	Sat	Sun
xx	<b>H</b> xx									
	<b>SD</b> xx									
	xx	<b>H</b> xx								
		<b>SD</b> xx								
	xx		<b>H</b> xx	xx						
			<b>SD</b> xx							
	xx			<b>H</b> xx						
				<b>SD</b> xx						
				xx	<b>H</b> xx					
					xx	<b>H</b> xx				
						xx	<b>H</b> xx	xx		
Legend:										
	Refer to lane requirement charts									
xx	The full width of the traveled way must be open for use by traffic.									
<b>H</b>	Designated holiday									
<b>SD</b>	Special day									

**Replace "Reserved" in section 12-4.05F with:**

<b>Chart no. 1 Conventional Highway Lane Requirements</b>																									
County: Del Norte	Route/Direction: 199 NB/SB												PM: 20.3/25.9												
From hour to hour	24	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mondays through Thursdays	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R
Fridays	R	R	R	R	R	R	R	R	R	R	R	R	R	R											
Saturdays																									
Sundays																					R	R	R	R	R

**Legend:**

R	Provide at least one 10 foot through traffic lane for use by both directions of travel (Reversing Control). The maximum lane closure length is 2,000 feet.
	Except during the use of a temporary signal system, no lane/shoulder closures allowed.

**REMARKS:** Except during the use of a temporary signal system, the full width of the traveled way must be open for use by public traffic when construction activities are not in progress.

**Replace section 12-5 with:**

**12-5 TRAFFIC CONTROL SYSTEM FOR LANE CLOSURE**

**12-5.01 GENERAL**

Section 12-5 includes specifications for closing traffic lanes with stationary lane closures on 2-lane, 2-way highways. The traffic control system for a lane closure must comply with the details shown.

Traffic control system includes signs.

**12-5.02 MATERIALS**

Not Used

**12-5.03 CONSTRUCTION**

Whenever components of the traffic control system are displaced or cease to operate or function as specified from any cause, immediately repair the components to the original condition or replace the components and restore the components to the original location.

For a stationary lane closure made only for the work period, remove the components of the traffic control system from the traveled way and shoulder, except for portable delineators placed along open trenches or excavation adjacent to the traveled way at the end of each work period. You may store the components at selected central locations designated by the Engineer within the limits of the highway.

Additional advance flaggers are required. All flaggers will have continuous radio contact with personnel in the work zone.

For traffic under 1-way control on unpaved areas, the cones shown along the centerline need not be placed.

You may use a pilot car to control traffic. If a pilot car is used for traffic control, the cones shown along the centerline need not be placed. The pilot car must have radio contact with personnel in the work area. Operate the pilot car through the traffic control zone at a speed not greater than 25 miles per hour.

#### **12-5.04 PAYMENT**

Traffic control system for lane closure is paid for as traffic control system. Flagging costs are paid for as specified in section 12-1.03.

The requirements in section 4-1.05 for payment adjustment do not apply to traffic control system. Adjustments in compensation for traffic control system will be made for an increase or decrease in traffic control work if ordered and will be made on the basis of the cost of the necessary increased or decreased traffic control. The adjustment will be made on a force account basis for increased work and estimated on the same basis in the case of decreased work.

A traffic control system required by change order work is paid for as a part of the change order work.

**Replace section 12-8 with:**

#### **12-8 TEMPORARY PAVEMENT DELINEATION**

##### **12-8.01 GENERAL**

Section 12-8 includes specifications for placing, applying, maintaining, and removing temporary pavement delineation.

Temporary signing for no-passing zones must comply with section 12-3.06.

Temporary painted traffic stripes and painted pavement markings used for temporary delineation must comply with section 84-3.

##### **12-8.02 MATERIALS**

###### **12-8.02A General**

Not Used

###### **12-8.02B Temporary Lane Line and Centerline Delineation**

Temporary pavement markers must be the same color as the lane line or centerline markers being replaced. Temporary pavement markers must be temporary pavement markers on the Authorized Material List for short-term day/night use, 14 days or less, or long-term day/night use, 180 days or less. Place temporary pavement markers under the manufacturer's instructions.

###### **12-8.02C Temporary Edge Line Delineation**

Not Used

###### **12-8.02D Temporary Traffic Stripe Tape**

Not Used

###### **12-8.02E Temporary Traffic Stripe Paint**

Not Used

###### **12-8.02F Temporary Pavement Marking Tape**

Not Used

###### **12-8.02G Temporary Pavement Marking Paint**

You may use one of the types of temporary removable pavement marking tape or permanent pavement marking tape on the Authorized Material List instead of temporary pavement marking paint.

###### **12- 8.02H Temporary Pavement Markers**

Not Used

##### **12-8.03 CONSTRUCTION**

### **12-8.03A General**

Wherever work activities obliterate pavement delineation, place temporary or permanent pavement delineation before opening the traveled way to traffic. Place lane line and centerline pavement delineation for traveled ways open to traffic. On multilane roadways, freeways and expressways, place edge line delineation for traveled ways open to traffic.

Establish the alignment for the temporary pavement delineation including required lines or markers. Surfaces to receive an application of paint or removable traffic tape must be dry and free of dirt and loose material. Do not apply temporary pavement delineation over existing pavement delineation or other temporary pavement delineation. Maintain temporary pavement delineation until it is superseded or you replace it with a new pattern of temporary pavement delineation or permanent pavement delineation.

When the Engineer determines the temporary pavement delineation is no longer required for the direction of traffic, remove the temporary pavement markers, underlying adhesive, and removable traffic tape from the final layer of surfacing and from the existing pavement to remain in place. Remove temporary pavement delineation that conflicts with any subsequent or new traffic pattern for the area.

### **12-8.03B Temporary Lane line and Centerline Delineation**

Whenever lane lines or centerlines are obliterated and temporary pavement delineation to replace the lines is not shown, the minimum lane line and centerline delineation must consist of temporary pavement markers placed longitudinally at intervals not exceeding 24 feet. For temporary pavement markers on the Authorized Material List for long-term day/night use, 180 days or less, cement the markers to the surfacing with the adhesive recommended by the manufacturer except do not use epoxy adhesive to place the pavement markers in areas where removal of the markers will be required.

For temporary lane line or centerline delineation consisting entirely of temporary pavement markers on the Authorized Material List for short-term day/night use, 14 days or less, place the markers longitudinally at intervals not exceeding 24 feet. Do not use the markers for more than 14 days on lanes opened to traffic. Place the permanent pavement delineation before the end of the 14 days. If the permanent pavement delineation is not placed within the 14 days, replace the temporary pavement markers with additional temporary pavement delineation equivalent to the pattern specified or shown for the permanent pavement delineation for the area. The Department does not pay for the additional temporary pavement delineation.

Where no-passing centerline pavement delineation is obliterated, install the following temporary no-passing zone signs before opening lanes to traffic. Install a W20-1, "Road Work Ahead," sign from 1,000 feet to 2,000 feet in advance of a no-passing zone. Install a R4-1, "Do Not Pass," sign at the beginning of a no-passing zone and at 2,000-foot intervals within the no-passing zone. For continuous zones longer than 2 miles, install a W7-3a or W71(CA), "Next \_\_\_ Miles," sign beneath the W20-1 sign. Install a R4-2, "Pass With Care," sign at the end of the no-passing zone. The Engineer determines the exact location of temporary no-passing zone signs. Maintain the temporary no-passing zone signs in place until you place the permanent no-passing centerline pavement delineation. Remove the temporary no-passing zone signs when the Engineer determines they are no longer required for the direction of traffic.

### **12-8.03C Temporary Edge Line Delineation**

You may apply temporary painted traffic stripe where removal of a 4-inch wide traffic stripe is not required.

The Engineer determines the lateral offset for traffic cones, portable delineators, and channelizers used for temporary edge line delineation. If traffic cones or portable delineators are used for temporary pavement delineation for edge lines, maintain the cones or delineators during hours of the day when the cones or delineators are being used for temporary edge line delineation.

Channelizers used for temporary edge line delineation must be an orange surface-mounted type. Cement channelizer bases to the pavement under section 85 for cementing pavement markers to pavement except do not use epoxy adhesive to place channelizers on the top layer of the pavement. Channelizers must be one of the 36-inch, surface-mounted types on the Authorized Material List.

Remove the temporary edge line delineation when the Engineer determines it is no longer required for the direction of traffic.

**12-8.03D Temporary Traffic Stripe Tape**

Not Used

**12-8.03E Temporary Traffic Stripe Paint**

Apply 1 or 2 coats of temporary traffic stripe paint for new or existing pavement. Apply one coat of temporary traffic stripe paint on the Middle Fork Smith River Bridge (Br No. 01-0080)

The painted temporary traffic stripe must be complete in place at the location shown before opening the traveled way to traffic.

**12-8.03F Temporary Pavement Marking Tape**

Not Used

**12-8.03G Temporary Pavement Marking Paint**

Apply and maintain temporary pavement markings consisting of painted pavement markings at the locations shown. The painted temporary pavement marking must be complete in place at the location shown before opening the traveled way to traffic. Removal of painted temporary pavement marking is not required.

Apply 1 or 2 coats of temporary pavement marking paint for new or existing pavement.

**12- 8.03H Temporary Pavement Markers**

Not Used

**12-8.04 PAYMENT**

Not Used

**Add to section 12:**

**12-9 TRAFFIC MANAGEMENT SUPERVISOR**

**12-9.01 GENERAL**

**12-9.01A Summary**

Section 12-9 includes specifications for Traffic Management Supervisor (TMS).

Provide a minimum of one TMS.

Comply with section 12-4.

**12-9.01B Definitions**

**traffic management supervisor:** The person solely responsible for managing work zone traffic control activities.

**corridor:** A designated route, routes or portions of routes that the Department has determined for maximum traffic delay.

**TCS:** Traffic Control Supervisor

**TMS:** Traffic Management Supervisor

**ATSSA:** American Traffic Safety Services Association

**12-9.01C Submittals**

The TMS must submit a daily traffic management report within 24 hours after the end of each working day that traffic control is in place.

The daily traffic management report must include:

1. A traffic log with:
  - 1.1. Contract Number
  - 1.2. County, Route and postmile
  - 1.3. Names of TMS managing the work zone traffic control
  - 1.4. Date of traffic control operations
  - 1.5. Length and location of lane closures
  - 1.6. Delay times when traveling through the work zone
  - 1.7. Length of traffic queues
2. A daily diary with:
  - 2.1. Identification of job site activities
  - 2.2. Instances of conflict or problems within the work zone
  - 2.3. Corrective measures taken
  - 2.4. Impacts to passage of pedestrians and bicyclists through the work zone
  - 2.5. Impacts to business and residential access
  - 2.6. Impacts to truck passage through the work zone
  - 2.7. Problems arising from unpaved conditions
  - 2.8. Problems arising from temporary detours

### **12-9.01D Quality Control and Assurance**

#### **12-9.01D(1) Qualifications**

A TMS must have two of the following:

1. A valid TCS certificate issued by the ATSSA or a similarly accredited agency
2. Successful completion of an intermediate or advanced work zone traffic control training course
3. A minimum of two years of full-time field experience in work zone traffic control

#### **12-9.02 MATERIALS**

Not Used.

#### **12-9.03 CONSTRUCTION**

The TMS must:

1. Serve as the point of contact for all traffic management issues
2. Ensure contract compliance related to work zone traffic control
3. Be present during all work shifts that require traffic control and available by phone during all other times
4. Attend the preconstruction conference and other meetings involving traffic control
5. Coordinate all work zone traffic control operations for the duration of the contract (including subcontractors, and suppliers) to ensure that all work zone traffic control is in place and fully operational prior to the commencement of any work
6. Immediately report delays exceeding the 10 minutes maximum traffic delay or traffic queue stoppage to the Engineer and take corrective action.
  - 6.1. Use a satellite phone where cell phone coverage is not available.
7. Monitor traffic queues, immediately report to the Engineer when a queue extends beyond the first warning sign for a lane closure, and take corrective action
8. Perform daily reviews of work zone traffic control during job site activities and document in a log
9. Notify the Engineer of accidents within or adjacent to the traffic control zone
10. Cooperate and coordinate traffic control with contractors working on adjacent projects
11. Prepare and submit a traffic control plan to provide emergency service vehicles access through the work zone

There is a total maximum traffic delay of 90 minutes for the corridor of Routes 197 PM 0.00 to 7.08 and 199 from PM 4.37 to 36.41. During the progress of traffic control under this Contract, it is anticipated that the following contracts may be in progress along this corridor:

**Corridor Contracts**

Contract no.	County–Route–Post Mile	City	Type of work
01-0C5104	DN-199-Var	Various along Route 199	Install High Friction Surface Treatment
01-454904	DN-197-3.2/4.0	Near Hiouchi	Widen Roadway
01-481104	DN-197-4.5	Near Hiouchi	Widen Roadway
010B2604	DN-199-8.1/8.3	Near Hiouchi	Widen Roadway
01-0B3104	DN-199-21.7	Near Gasquet	Reconstruct Roadway
01-0B3204	DN-199-24.6	Near Gasquet	Construct Retaining Wall
01-0B3304	DN-199-26.3	Near Gasquet	Reconstruct Roadway
01-4500U4	DN-199-22.7/26.5	Near Gasquet	Widen Roadway

It will be the responsibility of the TMS to coordinate with traffic control performed on the above contracts to prevent traffic delays exceeding the maximum time of 90 minutes.

If delays exceeding 30 minutes occur during traffic control on two consecutive days all work will be suspended per section 8-1.06. There will also be suspensions on all of the above corridor contracts as deemed necessary by the Department. No further traffic control is allowed until the Engineer has reviewed and accepted a work plan submitted to you that insures that future delays will not exceed 30 minutes. You are not entitled to payment for time-related overhead or right of way delays during these suspensions.

**12-9.04 PAYMENT**

The payment for traffic control system includes traffic management supervisor.

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**13 WATER POLLUTION CONTROL**

**Add to section 13-3.01A:**

The project is risk level 3.

**Add to section 13**

**13-11 WATER QUALITY MONITORING**

**13-11.01 GENERAL**

**13-11.01A Summary** Section 13-11 includes specifications for monitoring of water quality during in-water work. Water quality monitoring for in-water work includes collecting samples and reporting results.

This job site lies within the boundaries of the North Coast Regional Water Quality Control Board (RWQCB).

The receiving water for this job is Middle Fork Smith River.

**13-11.01B Definitions**

**WQM:** Water Quality Monitor. The WQM collects water quality sampling data and provides reports to the Engineer.

**WQO:** Water Quality Objective.

### **13-11.01C Submittals**

Within 7 days after contract approval, submit WQM qualifications including training and experience in collecting and analyzing water quality samples.

#### **13-11.01C(1) Water Quality Sampling and Analysis Day**

Water Quality Sampling and Analysis Day includes activities such as preparation, collection, analysis, and reporting of water quality samples.

#### **13-11.01C(2) Water Quality Monitoring Report**

Water Quality Monitoring Report includes:

1. Visual monitoring
2. Preparing and submitting Water Quality Objective (WQO) Exceedance Report
3. Monitoring and reporting inspection results
4. Obtaining monitoring report acceptance
5. Preparing and submitting reports required by RWQCB

The WQM must prepare a monthly monitoring report. Submit the monthly monitoring report by the 7th of the month for monitoring work conducted during the previous month. The report must include:

1. Visual monitoring inspection reports
2. If in-water work was done, include the following field sampling results and inspections:
  - 2.1. Analytical methods, reporting units, and detection limits
  - 2.2. Date, location, time of sampling, visual observation, photos, and measurements
  - 2.3. Estimate of water flow
  - 2.4. Calibration logs for field monitoring equipment
3. If storm events generate visible runoff, include visual monitoring results and inspections:
  - 3.1. Date, location, and time of visual observation
  - 3.2. Photos of areas disturbed by project activities including excess materials disposal areas
  - 3.3. Photos showing disturbed soil areas and documenting compliance for erosion control and revegetation measures including soil stabilization and sediment control BMPs
4. Summary of exceedance
5. Summary of corrective actions

The WQM must prepare other RWQCB reports when:

1. Conducting in-water work
2. Work activities cause a discharge of materials reaching receiving waters
3. Work activities cause a discharge resulting in the creation of a visible plume in receiving waters

Follow the monthly monitoring report requirements for other RWQCB reports. The other RWQCB reports must be submitted within 3 days of beginning in-water work or discovery of a discharge and continue every 2 weeks. Suspend the other RWQCB reports 2 weeks after concluding in-water work or correction of the discharge.

#### **13-11.01C(3) Water Quality Annual Report**

Water Quality Annual Report preparation includes certifications, monitoring and inspection results, and obtaining Water Quality Annual Report acceptance.

Prepare a Water Quality Annual Report for the reporting period from July 1st to June 30th:

1. If construction occurs from July 1st through June 30th, submit the report no later than July 15th for the prior reporting period
  2. If construction ends before June 30th, submit the report within 15 days after contract acceptance
- Submit the Water Quality Annual Report as follows:

1. Submit 2 copies of the Water Quality Annual Report and allow 10 days for the Engineer's review. If revisions are required, the Engineer provides comments and specifies the date that the review stopped.
2. Change and resubmit the Water Quality Annual Report within 5 days of receipt of the Engineer's comments. The Engineer's review resumes when the complete Water Quality Annual Report is resubmitted.
3. When the Engineer accepts the Water Quality Annual Report, insert the WQM signed certification, the WPC Manager's signed certification, and the Engineer's signed certification.

Submit an electronic copy and 2 printed copies of the accepted Water Quality Annual Report.

Submit an electronic copy and 2 printed copies of Monitoring Reports as required:

1. Water Quality Monitoring Reports
2. Other reports required by the RWQCB

### **13-11.01D Quality Control and Assurance**

#### **13-11.01D(1) Training**

Training for personnel to collect water quality samples must include:

1. SAP review
2. Health and safety review
3. Sampling simulations

#### **13-11.01D(2) Water Quality Monitor (WQM)**

The WQM must have the same qualifications as the WPC Manager including the requirements for QSP described in the Permit (Order No. 2009-009-DWQ, NPDES No. CAS000002).

The WQM must have training and experience in collecting and analyzing water quality samples.

The WQM may be the same person as the WPC Manager.

#### **13-11.01D(3) Implementation Requirements**

##### **13-11.01D(3)(a) Visual Monitoring**

The WQM must perform:

1. Visual inspections for storm events
2. Non-storm water discharge visual inspections as follows:
  - 2.1. Observe receiving waters:
    - 2.1.1. 24 hours before beginning in-water work
    - 2.1.2. At least four times daily during in-water work activities
  - 2.2. Observe receiving waters for the presence of floating and suspended materials, sheen on the surface, discoloration, turbidity, odors, and sources of observed pollutants
  - 2.3. Observe the job site for the presence of authorized and unauthorized non-storm water discharges and their sources. Unauthorized discharges to surface waters include:
    - 2.3.1. Soil, silt, and sand
    - 2.3.2. Bark, sawdust, and slash
    - 2.3.3. Rubbish and debris
    - 2.3.4. Cement, concrete, and concrete washings
    - 2.3.5. Oil and petroleum products
    - 2.3.6. Welding slag
    - 2.3.7. Other organic or earthen materials

The WQM must prepare visual inspection reports that include the following:

1. Name of personnel performing the inspection, inspection date and date inspection report completed.
2. Storm and weather conditions
3. Locations and observations
4. Corrective actions taken

Retain visual inspections reports at the job site.

**13-11.01D(3)(b) Water Quality Sampling Day**

Perform water quality sampling whenever a project activity, conducted within waters of the State, has the potential to mobilize sediment or alter background conditions within waters of the State. Perform surface water quality sampling when:

1. Conducting in-water work
2. Work activities result in materials reaching receiving waters
3. Work activities result in the creation of a visible plume in receiving waters

This project is subject to the following WQOs:

Parameter	Test Method	Detection Limit (Min)	Unit	Water Quality Objective
pH	Field test with calibrated portable instrument	0.2	pH units	Lower NAL = 6.5 Upper NAL = 8.5 And any change greater than 0.5 units above natural background
Turbidity	Field test with calibrated portable instrument	1	NTU	20 percent above natural background

**13-11.01D(4) Reporting Requirements**

If there is an unauthorized discharge, the WQM must immediately notify the Engineer within 6 hours.

**13-11.01D(4)(a) WQO Exceedance Report**

If a WQO is exceeded, the WQM must:

1. Notify the Engineer by phone or electronic media within 30 minutes of WQO is exceeded
2. Submit a WQO Exceedance Report within 6 hours of WQO is exceeded

The report must:

1. Include the following field sampling results and inspections:
  - 1.1. Analytical methods, reporting units, and detection limits
  - 1.2. Date, location, time of sampling, visual observation, photos, and measurements
  - 1.3. Estimate of water flow
2. Description of BMPs and corrective actions taken to manage WQO exceedance

**13-11.01D(4)(b) Water Quality Annual Report**

The CSWQM must prepare a Water Quality Annual Report. The report must:

1. Use an approved report format
2. Include project information including description, location, and receiving waters
3. Include water quality monitoring information including:
  - 3.1. Summary and evaluation of sampling and analysis results including laboratory reports
  - 3.2. Analytical methods, reporting units, detections limits for analytical parameters
  - 3.3. Summary of corrective actions
  - 3.4. Identification of corrective actions or compliance activities that were not implemented
  - 3.5. Summary of exceedance
  - 3.6. Names of individuals performing water quality inspections and sampling
  - 3.7. Logistical information for inspections and sampling including location, date, time, and precipitation
  - 3.8. Visual observations and sample collection records

4. Include photos documenting compliance for:
  - 4.1. Disturbed soil areas created by work activities
  - 4.2. Erosion control and revegetation measures including soil stabilization and sediment control BMPs
  - 4.3. Completed work
5. Include records of water quality permit compliance training and meetings

### **13-11.02 MATERIALS**

Not Used

### **13-11.03 CONSTRUCTION**

#### **13-11.03A General**

Not Used

#### **13-11.03B Water Quality Sampling Day**

At least 24 hours before beginning in-water work:

1. Establish locations for water quality sampling:
  - 1.1. Upstream of the effluent discharge point or location of in-water work by no more than 100 feet.
  - 1.2. Effluent discharge point including location of in-water work
  - 1.3. Downstream of the effluent discharge point or location of in-water work between 75 and 100 feet
2. Conduct water quality sampling to document background conditions for upstream, effluent, and downstream locations. Sample for each WQO described above.
3. Estimate water flow

Whenever conducting in-water work including, conduct water quality sampling:

1. At least four times daily for each water quality objective
2. At upstream, effluent, and downstream locations

If sample results exceed a WQO, immediately notify the Engineer within 30 minutes and do the following:

1. Conduct water quality sampling every hour until measurements comply with WQOs
2. Measure the distance from the effluent location to the downstream extent of the exceedance
3. Obtain photos of the tributary upstream, downstream, and at the location of in-water work
4. If BMPs are installed, repaired, or modified to control the source of the exceedance, monitor the activity and document with samples, photos, and a brief summary

You are not required to physically collect samples under the following conditions:

1. During dangerous weather conditions such as flooding or electrical storms
2. Outside of normal working hours

If downstream samples show increased levels and indicate a possible WQO exceedance, assess WPC practices, site conditions, and surrounding influences to determine the probable cause for the increase.

Whenever assigned field personnel take samples, comply with the equipment manufacturer's recommendation for collection, analysis methods, and equipment calibration.

Retain calibration logs at the job site.

Retain water quality sampling documentation and analytical results at the job site.

### **13-11.04 PAYMENT**

Not Used



#### 14-6.02C(5) Protection Measures

Within species protection area "SPA1" (Entire Project Area as shown), implement the following protection measures

1. For protection of migratory birds, no removal of trees or shrubs is allowed from February 1 to September 15 of any year or comply with section 14-6.05C(2).
2. Survey the entire project limits for osprey nests as follows:
  - 2.1. Conduct database and ground-based nest search for new osprey nests within 0.5 miles of Route 199 PM 20.5 to 25.65 within four days before starting construction activities each year. If a nest is detected, do not begin any construction activity and contact the Engineer within 24 hours.
  - 2.2. Following the initial ground-based osprey nest search, surveys of nest sites/territories must consist of three visits lasting at least 2 hours and separated by at least 7 days. The first visit must occur after April 1 and the last visit must occur after May 1, unless occupancy is observed. Provide osprey nest survey results to the Engineer within 24 hours prior to beginning project activities.
  - 2.3. For construction activities within 0.5 miles of occupied nests, conduct monitoring between March 1 and August 31 to determine if nearby construction activities adversely affect nesting ospreys or their young. For construction activities further than 0.5 miles from occupied nests, collect information on the start of nesting behavior, fledging dates, and productivity of osprey nests to help refine the critical nesting period.
  - 2.4. If ospreys appear to be disturbed by construction activities at any time, immediately suspend operations causing the disturbance and contact the Engineer.
3. If vegetation (tree and/or shrub) removal will take place during the migratory bird breeding season, between February 1 and September 15 of any year, a Migratory Bird Protection Program (MBPP) will be submitted to the Department and California Department of Fish and Wildlife (CDFW) for review at least 30 days prior to construction, including staging, clearing and grubbing, as specified in the CDFW Lake and Streambed Alteration Agreement (1600-2013-0059-RI) Condition # 2.6 available in section 2-1.06B.

Implement the following protection measures to control the spread of root disease caused by the fungal pathogen, *Phytophthora lateralis*:

1. Prior to earthwork cut down all Port Orford Cedars and dispose in areas approved by the Engineer.
2. Excess earthwork material must be disposed in areas within 20 miles of the project limits and approved by the Engineer.
3. All equipment, vehicles, and personnel footwear must be cleaned with a bleach solution approved by the Engineer prior to and after any ground disturbing activities
4. Establish wash stations for cleaning equipment, vehicles and personnel footwear with the bleach solution. Comply with section 13-4 for disposal of the bleach solution.

Within species protection area "SPA 2" (Location 2, PM 23.92-24.08, as shown), implement the following protection measures:

Survey the project limits for location 2 for Northern Spotted Owl within four days before starting construction activities.

Conduct Activity Center Search surveys for the resident Northern Spotted Owl pair (Dollar Bend pair) to determine the current location of their activity center. If, after surveys, it is determined that the Dollar Bend pair territory activity center has moved to within 825 feet of US 199 do not begin any construction activity and contact the Engineer within 24 hours.

Within species protection area, "Tree Protection Areas" (Location 2 "B" 504+90, 510+25 and 512+90 as shown), implement the following protection measures during earth disturbing operations within a circular area whose radius is three times the diameter at breast height (dbh) of each tree (structural root zone):

1. A contractor supplied arborist complying with section 20-2.03E will be present to monitor ground disturbing operations in each structural root zone. If the Arborist determines potential for root damage is imminent, this operation will be suspended. Resumption of the operation will be determined by the engineer and the arborist.

2. Excavation within each structural root zone will be performed by first loosening the soil with a selective pneumatic excavator, then removing the soil with hand tools or other methods approved by the arborist and the engineer. Set aside the top six inches of native material for backfill. Use of mechanized equipment will only be allowed for culvert operations as determined by the arborist and the engineer.
3. Within the structural root zone roots that need to be cut must be cut cleanly with sharp instruments to promote healing. Roots larger than two inches in diameter will not be cut without approval of the arborist

**14-6.02C(6) Monitoring Schedule**

Not used.

**14-6.02D Payment**

Not Used

**Replace section 14-6.05 with:**

**14-6.05 CONTRACTOR-SUPPLIED BIOLOGIST**

**14-6.05A General**

**14-6.05A(1) Summary**

Section 14-6.05 includes specifications for providing a Contractor-supplied biologist to monitor construction and other activities to protect regulated species that may be harmed during construction activities.

**14-6.05A(2) Submittals**

**14-6.05A(2)(a) Qualifications**

Within 7 days after Contract approval, submit each biologist's name, resume, and statement of qualifications. Allow 10 days for review. If the submittal is incomplete, the Engineer will provide comments. Within 7 days after receiving the Engineer's comments, update and resubmit qualifications data. Do not start construction activities until the Contractor-supplied biologist is authorized.

**14-6.05A(2)(b) Protocols**

Not Used.

**14-6.05A(2)(c) Pre-Construction Survey Report**

Submit a pre-construction survey report within 10 days before starting construction activities.

**14-6.05A(2)(d) Initial Monitoring Report**

Submit an Initial Monitoring Report that includes, at a minimum, the requirements for Monitoring Report submittals within 48 hours after starting ground-disturbing activities.

**14-6.05A(2)(e) Monitoring Report**

Submit Monitoring Reports according to the schedule in the Natural Resources Protection Plan.

**14-6.05A(2)(f) Incident Report**

Submit an Incident Report within 24 hours of the incident.

**14-6.05A(2)(g) Annual Monitoring Report**

Submit no later than December 1 during each year of construction.

**14-6.05A(2)(h) Final Monitoring Report**

Submit no later than 30 days after completion of the project.

**14-6.05A(3) Quality Control and Assurance**

**14-6.05A(3)(a) Qualifications**

A biologist must meet PLAC requirements. Provide required qualifications for transmittal to regulatory agencies. All project specific authorizations must be current and valid for the duration of the project.

Biologists who perform specialized activities must have demonstrated field experience working with the species or performing the specialized task. Biologists who perform specialized activities must meet the following minimum requirements:

Specialized activity/species	Requirements
Migratory Bird Nest Surveys	Has previously conducted Migratory Bird Nest Surveys
Northern Spotted Owl Activity Center Survey	Meets "Owl Surveyor" qualifications in the current USFWS Northern Spotted Owl Survey Protocol.

**14-6.05A(3)(b) Protocols**

Not Used.

**14-6.05B Materials**

Not Used

**14-6.05C Construction**

**14-6.05C(1) General**

Not Used

**14-6.05C(2) Pre-construction Survey**

Survey the work area for regulated species within four days before starting construction activities.

**14-6.05C(3) Protective Radius**

Not Used.

**14-6.05C(4) Monitoring Schedule**

Monitoring must comply with the schedule in section 14-6.07.

**14-6.05C(5) Monitoring Duties**

Implement monitoring as specified in section 14-6.07.

**14-6.05C(6) Notification and Reporting**

All reports must include the following:

1. PLAC requirement implementation
2. Name(s) of the biologist(s) conducting biological activity
3. Date(s) and time(s) of monitoring
4. Locations and activities monitored
5. Representative photographs
6. Findings
7. If regulated species are observed, reports must recommend actions to protect the regulated species
8. Name of the biologist who prepared the report
9. Signature of the biologist certifying the accuracy of the report

The Pre-Construction Survey Report includes one of the following:

1. Detailed observations and locations where regulated species were observed
2. Statement that no regulated species were observed by each biologist

The Incident Report includes:

1. Description of any take incident
2. Species name and number taken
3. Details of required notifications with contact information
4. Corrective actions proposed or taken
5. Disposition of taken species

The Annual Monitoring Report includes:

1. Construction beginning and ending dates
2. Identification of project impacts on the species covered in the plan
3. Species protection measures with protection measure implementation details
4. Incidental take details, including species name, number taken, people contacted, contact information, and disposition of taken species
5. An assessment of the effectiveness of the species protection measures to mitigate project impacts
6. Recommendations to improve efficiency of protection measures to mitigate impacts to regulated species

The Final Monitoring Report must be a cumulative report following the format of the Annual Monitoring Report.

**14-6.05D Payment**

Not Used

**Replace section 14-6.06 with:**

**14-6.06 SPECIES PROTECTION AREA**

**14-6.06A General**

**14-6.06A(1) Summary**

Section 14-6.06 includes specifications for areas that have species protection requirements.

Species protection areas (SPAs) within the project limits are shown:

<b>Species Protection Areas</b>		
Identification	Location	Trees
SPA 1	Entire project limits	NA
SPA 2	Location 2: PM 23.92 to 24.08	NA
Tree Protection Areas	Location 2: Areas surrounding 3 trees	38 inch dbh, 24 ft right of "B" 504+90 36 inch dbh, 37 ft right of "B" 510+25 35 inch dbh, 35 ft right of "B" 512+90

**14-6.06B Materials**

Not Used

**14-6.06C Construction**

Not Used

**14-6.06D Payment**

Not Used

**Replace section 14-6.07 with:**

**14-6.07 NATURAL RESOURCE PROTECTION PLAN**

**14-6.07A General**

**14-6.07A(1) Summary**

Section 14-6.07 includes specifications for preparing a plan to protect biological resources.

Prepare a natural resource protection plan that defines measures you will take to maintain project compliance with all environmental laws, regulations, and PLAC requirements.

Comply with section 14-6.02.

**14-6.07A(2) Submittals**

Submit the natural resource protection plan within 21 days after Contract approval. Allow 7 days for the Engineer's review. If the submittal is incomplete, the Engineer will provide comments. Within 7 days after receiving the Engineer's comments, update and resubmit the natural resource protection plan.

**14-6.07A(3) Quality Control and Assurance**

The natural resource protection plan must be prepared and signed by a biologist knowledgeable of the species or habitats discussed and address species protection measures.

The natural resource protection plan includes:

1. List of species and habitats addressed in the plan.
2. List of protocols for species protection surveys, with full protocols in an appendix.
3. Protection measures for regulated species likely to occur in the project site.
4. Protective radii for regulated species encounters.
5. Implementation plan for protection measures, including monitoring schedule
6. Monitoring duties.
  - 6.1. Monitor to assure that no equipment or any construction related material enters the Middle Fork Smith River wetted channel.
  - 6.2. Monitor to assure that any construction platforms required for bridge construction and demolition platforms required for bridge demolition at the Middle Fork Smith River Bridge (Br. No. 01-0015) as shown will be above the wetted channel of the Middle Fork Smith River.
  - 6.3. Monitor to assure that all bridge construction and bridge demolition platforms will be placed no sooner than June 15 of any construction year.
  - 6.4. Monitor to assure that all bridge construction platforms and bridge demolition platforms will be removed 1 week prior to a National Weather Service NOAA forecast for measurable precipitation after October 15 of any construction year or as described in the conditions of the PLACs.
  - 6.5. Immediately notify the Engineer of any take of regulated species.
  - 6.6. Prepare, submit, and sign notifications and reports.
7. Justification for each instance where protection measures and an implementation plan are not necessary for a regulated species
8. Schedule for inspecting protection measures
9. Schedule for maintaining protection measures
10. Schedule for submittal of monitoring reports
11. Response plan for instances where regulated species are encountered
12. Migratory Bird Protection Plan as specified in the PLAC, Department of Fish and Wildlife 1600 Streambed Alteration Agreement Condition 2.6 available in section 2-1.06B.

**14-6.07B Materials**

Not Used

**14-6.07C Construction**

Not Used

#### **14-6.07D Payment**

Not Used

#### **Add to section 14-9.02A**

Notify the Air Pollution Control District (APCD) or Air Quality Management District (AQMD) identified below as required by the National Emission Standards for Hazardous Air Pollutants (NESHAP) at 40 CFR Part 61, Subpart M, and California Health and Safety Code section 39658(b)(1). Notification must take place no less than 14 days before starting demolition or renovation activities as defined in the NESHAP regulations. Notification forms and other information are available from:

North Coast Unified Air Quality Management  
District, 2300 Myrtle Ave.  
Eureka, Ca 95501

<http://www.ncuaqmd.org/>

Mail or otherwise deliver the original notification form with any necessary attachments to:

North Coast Unified Air Quality Management  
District, 2300 Myrtle Ave.  
Eureka, Ca 95501

Notify other local permit agencies and utility companies before starting any demolition activities. Submit a copy of the notification form and attachments as an informational submittal before starting demolition or renovation activities.

#### **Replace section 14-11.05 with**

#### **14-11.05 NATURALLY OCCURRING ASBESTOS**

##### **14-11.05A General**

Section 14-11.05 includes specifications for managing Naturally Occurring Asbestos (NOA), serpentine and ultramafic rock. One or more of these materials are present within the job site. NOA is used as defined under 17 CA Code of Regs § 93105.

Comply with the Airborne Toxic Control Measures (ATCM) during all earthwork activities on the job site.

NOA exists in all native earth and rock materials at Location 1. Dispose of all native earth and rock materials excavated at Location 1 at a California Environmental Protection Agency or a State of Oregon Department of Environmental Quality landfill permitted to accept NOA material...

The tested levels of NOA range from 2.0 to 25.75 percent asbestos, with an average of 14.6 percent as analyzed by California Air Resources Board (CARB) Test Method 435.

The site investigation report is available as specified in section 2-1.06B.

##### **14-11.05A(1) Notifications**

Notify the Air Pollution Control District (APCD) or Air Quality Management District (AQMD) in writing at least 15 days before starting work that disturbs NOA. Submit proof of notification and any exemption. Keep a copy at the job site.

##### **14-11.05A(2) Submittals**

##### **14-11.05A(2)(a) Daily Ambient Air Monitoring Report**

When required by local APCD or AQMD, perform daily ambient air monitoring on the job site. If daily ambient monitoring is required, submit a written air monitoring report to the Engineer every month. The report must include:

1. Air monitoring results
2. Analysis of results from the prior month
3. Name and location of the laboratory where the analysis was performed
4. Assessment of exposures of workers or the public
5. Descriptions of the type of air monitoring equipment
6. Sampling frequency

#### **14-11.05A(2)(b) Dust Control Plan**

The APCD or AQMD may require review and approval of the dust control plan (DCP) and fee payment. If required, submit the DCP approved by APCD or AQMD. Otherwise submit the DCP.

On job sites that require blasting, the DCP must include the use of blasting mats or cover material not containing NOA.

#### **14-11.05A(2)(c) Asbestos Compliance Plan**

Submit the asbestos compliance plan (ACP) to prevent or minimize worker exposure to asbestos. The ACP must be signed by a CIH certified in Comprehensive Practice by the American Board of Industrial Hygiene.

The ACP must comply with the following regulations:

1. 8 CA Code of Regs, § 1529, (Asbestos) and § 5192, (Hazardous Waste Operations and Emergency Response)
2. Occupational Safety and Health Guidance Manual published by the National Institute of Occupational Safety and Health (NIOSH)
3. Occupational Safety and Health Administration (OSHA), including addenda issued up to and including the date of advertisement of the Contract

Include the following information in the ACP:

1. Identification of personnel designated to be on site
2. Job hazard analysis for work assignments
3. Summary of potential risks
4. Worker exposure air monitoring plan
5. Description of personal protective equipment
6. Delineation of work zones on the job site
7. Decontamination procedures
8. General safe work practices
9. Site security measures
10. Emergency response plans
11. Description of worker training

#### **14-11.05A(2)(d) Sampling and Analysis Plan**

#### **Not Used 14-11.05A(2)(e) Fill Material Documentation**

Submit documentation that fill material to be used as cover is asbestos free as defined by ATCM.

#### **14-11.05A(2)(f) NOA Burial Location Report**

Within 5 business days of completing placement of NOA at the burial location, submit a report for that burial location, including the form titled "Burial Location of Soil Containing Naturally Occurring Asbestos" and electronic geospatial vector data shape files of the top and bottom perimeters of the burial location to the Engineer and to:

NOA@dot.ca.gov

The Engineer will notify you within 5 business days of receipt if accepted. If the report is rejected, you have 5 business days to submit a corrected report.

#### **14-11.05A(2)(g) Disposal Documentation**

Submit 1 copy each as an information submittal:

1. Bill of lading
2. Acknowledgement of receipt of material containing NOA from receiving party or landfill facility

For surplus NOA sent to a landfill facility also submit 1 copy each as an information submittal:

1. Landfill receipts showing the concentration of asbestos
2. Certified weight tickets showing the amount of disposal material containing NOA that was sent to the facility

If additional test results are required by the owner of the landfill facility, submit them as an information submittal.

#### **14-11.05A(3) Quality Control and Assurance**

Manage NOA under State laws and regulations and county and municipal ordinances and regulations. Laws and regulations that govern this work include:

1. 8 CA Code of Regs § 1529 (Asbestos) and § 5192 (Hazardous Waste Operations and Emergency Response)
2. 17 CA Code of Regs § 93105 and § 93106
3. 22 CA Code of Regs, Div 4,5, Chp 10
4. Health & Safety Code, Division 20, Chp 6.5 (Hazardous Waste Control)

Manage NOA under the rules and regulations of the following agencies:

1. US EPA
2. DTSC
3. CDPH
4. Cal/OSHA
5. CARB
6. Oregon DEQ
7. North Coast Unified Air District

#### **14-11.05A(4) Training**

Before performing work in areas with material containing NOA, personnel who have not had the worker training must complete a safety training program that complies with the ACP. The safety training program must meet the requirements of 8 CA Code of Regs §1529, (Asbestos), and § 5192 (b)(4)(B), (Hazardous Waste Operations and Emergency Response). Provide the Engineer written certification of completion of safety training for each trainee before performing work in areas containing NOA.

Provide training, personal protective equipment, and washing facilities for 3 Department employees.

#### **14-11.05B Materials**

Not Used

#### **14-11.05C Construction**

##### **14-11.05C(1) General**

Prevent visible dust emission during excavation, stockpiling, transportation, or placement of NOA under section 14-9.03 and 17 CA Code of Regs § 93105(d)(1)(B).

Control dust in areas with NOA using measures that include the following:

1. Stabilize unpaved areas subject to vehicular traffic by keeping adequately wetted, treated with a chemical dust palliative, or covered with material that contains less than 0.25 percent asbestos.
2. The speed of vehicles and equipment traveling across unpaved areas must not be more than 15 mph unless the road surface and surrounding area is sufficiently stabilized to prevent vehicles and equipment going faster from causing dust that is visible from crossing job site limits.
3. Stockpiles and disturbed areas not subject to vehicular traffic must be located in the plan and stabilized by being kept adequately wetted, treated with a chemical dust palliative, or covered with material that contains less than 0.25 percent asbestos.
4. Conduct activities so that no dirt or mud tracking is visible on any paved roadway open to the public.

Do not leave NOA with asbestos content of 0.25 percent or higher exposed on the surface if disturbed during construction activities. Stabilize these areas by keeping them wetted or by treating them with a chemical dust palliative. Cover disturbed NOA permanently placed during construction activities with a 3-inch minimum layer of asbestos-free material.

Survey the location of the bottom and top perimeters of each area where you bury NOA.

The survey must be performed by or under the direction of either:

1. Land surveyor licensed under the Bus & Prof Code, Chp 15 (commencing with § 8700)
2. Civil engineer licensed before January 1, 1982 under the Bus & Prof Code, Chp 7 (commencing with § 6700)

Survey 10 points to determine each burial location horizontally and vertically within the specified accuracies and to create closed polygons of the perimeters of the bottom and top of the burial location. If 10 points are not sufficient to define the polygon, add additional points until the polygon is defined. Establish the position of the bottom and top perimeters before placing subsequent layers of material that obstruct the location.

Report each burial location in California State Plane Coordinates in US Survey feet within the appropriate zone of the California Coordinate System of 1983 (CCS83) and in latitude and longitude. Horizontal positions must be referenced to CCS83 (epoch 2007.00 or later National Geodetic Survey [NGS] or California Spatial Reference Center [CSRC] published epoch) to an accuracy of 3 feet horizontally. Identify the points to an accuracy of 1 foot vertically. Reference elevations of the bottom and top of the burial locations to North American Vertical Datum of 1988 (NAVD88) . Report accuracy of spatial data in US Survey feet under Federal Geographic Data Committee (FGDC)-STD-007.1-1998.

Material containing NOA may be temporarily stockpiled until it is transported and disposed of or used on site. Limit stockpile locations to areas that contain NOA within the job site. Cover temporary stockpiles with polyethylene sheeting of 10 mil minimum thickness or stabilize stockpiles by other methods permitted under 17 CA Code of Regs § 93105(d)(1)(3). Temporarily stockpiled material containing NOA is not selected material as specified in section 19-2.03D.

On job sites that require blasting, minimize the emission of NOA with the use of blasting mats or cover material not containing NOA. Sample and analyze cover material after blasting to determine if it contains NOA. Cover material not containing NOA after blasting is your property. Dispose of cover material containing NOA as specified.

#### **14-11.05C(2) Material Transportation and Disposal**

Do not dispose of material containing NOA in a surfacing application as defined in 17 CA Code of Regs § 93106, Asbestos Airborne Toxic Control Measure for Surfacing Applications.

Transport surplus NOA containing greater than or equal to 1.0 percent asbestos to an appropriately permitted landfill. You are responsible for identifying the appropriately permitted landfill to receive the NOA. Surplus material containing less than 1.0 percent NOA may be disposed under Section 5-1.20B(4). In all cases of transporting and disposing of excess material containing NOA:

1. Use warning signs that the surplus material contains NOA.
2. Provide written notification of asbestos content to the party receiving the material, as defined in 17 CA Code of Regs § 93105(d)(3).
3. Obtain written acknowledgement, from the property owner or the landfill facility, that the surplus contains NOA.

Material containing NOA excavated from outside the limits of payment for verified bid items is the property of the Contractor and must be disposed of at an approved facility.

#### **14-11.05C(3) Close-out**

After you have completed managing NOA you have no further responsibility for the NOA in place within the job site. You will not be considered a generator of the hazardous material and no further action is required.

#### **14-11.05D Payment**

Not Used

**Replace section 14-11.07 with:**

#### **14-11.07 REMOVE YELLOW TRAFFIC STRIPE AND PAVEMENT MARKING WITH HAZARDOUS WASTE RESIDUE**

##### **14-11.07A General**

##### **14-11.07A(1) Summary**

Section 14-11.07 includes specifications for removing existing yellow thermoplastic traffic stripe. The residue from the removal of this material is a Department-generated hazardous waste.

Residue from removal of yellow thermoplastic traffic stripe contains lead chromate. The average lead concentration is at least 1,000 mg/kg total lead or 5 mg/l soluble lead. When applied to the roadway, the yellow thermoplastic traffic stripe contained as much as 2.6 percent lead. Residue produced from the removal of this yellow thermoplastic traffic stripe contains heavy metals in concentrations that exceed thresholds established by the Health & Safety Code and 22 CA Code of Regs. For bidding purposes, assume the residue is not regulated under the Federal Resource Conservation and Recovery Act (RCRA), 42 USC § 6901 et seq.

Work associated with disposal of hazardous waste residue regulated under RCRA as determined by test results is change order work.

Yellow thermoplastic may produce toxic fumes when heated.

##### **14-11.07A(2) Submittals**

##### **14-11.07A(2)(a) General**

Reserved

##### **14-11.07A(2)(b) Lead Compliance Plan**

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

##### **14-11.07A(2)(c) Work Plan**

Submit a work plan for the removal, containment, storage, and disposal of yellow thermoplastic and yellow painted traffic stripe and pavement marking. The work plan must include:

1. Objective of the operation
2. Removal equipment
3. Procedures for removal and collection of yellow thermoplastic traffic stripe residue, including dust
4. Type of hazardous waste storage containers
5. Container storage location and how it will be secured
6. Hazardous waste sampling protocol and QA/QC requirements and procedures
7. Qualifications of sampling personnel
8. Analytical lab that will perform the analyses
9. DTSC registration certificate and CA Highway Patrol (CHP) Biennial Inspection of Terminals (BIT) Program compliance documentation of the hazardous waste hauler that will transport the hazardous waste
10. Disposal site that will accept the hazardous waste residue

The Engineer will review the work plan within 5 business days of receipt.

Do not perform work that generates hazardous waste residue until the work plan has been authorized.

Correct any rejected work plan and resubmit a corrected work plan within 5 business days of notification by the Engineer. A new review period of 5 business days will begin from date of resubmittal.

##### **14-11.07A(2)(d) Analytical Test Results**

Submit analytical test results of the residue from removal of yellow thermoplastic traffic stripe, including chain of custody documentation, for review and acceptance before:

1. Requesting the Engineer's signature on the waste profile requested by the disposal facility
2. Requesting the Engineer obtain an US EPA Generator Identification Number for disposal
3. Removing the residue from the site

**14-11.07A(2)(e) U.S. Environmental Protection Agency Identification Number Request**

Submit a request for the US EPA Generator Identification Number when the Engineer accepts analytical test results documenting that residue from removal of yellow thermoplastic traffic stripe is a hazardous waste.

**14-11.07A(2)(f) Disposal Documentation**

Submit documentation of proper disposal from the receiving landfill within 5 business days of residue transport from the project.

**14-11.07B Materials**

Not Used

**14-11.07C Construction**

Where grinding or other authorized methods are used to remove yellow thermoplastic traffic stripe that will produce a hazardous waste residue, immediately contain and collect the removed residue, including dust. Use a HEPA filter-equipped vacuum attachment operated concurrently with the removal operations or other equally effective approved methods for collection of the residue.

Make necessary arrangements to test the yellow thermoplastic hazardous waste residue as required by the disposal facility and these special provisions. Testing must include:

1. Total lead by US EPA Method 6010B
2. Total chromium by US EPA Method 6010B
3. Soluble lead by California Waste Extraction Test (CA WET)
4. Soluble chromium by CA WET
5. Soluble lead by Toxicity Characteristic Leaching Procedure (TCLP)
6. Soluble chromium by TCLP

From the first 220 gal of hazardous waste or portion thereof if less than 220 gal of hazardous waste are produced, a minimum of 4 randomly selected samples must be taken and analyzed individually. Samples must not be composited. From each additional 880 gal of hazardous waste or portion thereof if less than 880 gal are produced, a minimum of 1 additional random sample must be taken and analyzed. Use chain of custody procedures consistent with chapter 9 of US EPA Test Methods for Evaluating Solid Waste, Physical/Chemical Methods (SW-846) while transporting samples from the project to the laboratory. Each sample must be homogenized before analysis by the laboratory performing the analyses. A sample aliquot sufficient to cover the amount necessary for the total and the soluble analyses must then be taken. This aliquot must be homogenized a 2nd time and the total and soluble analyses run on this aliquot. The homogenization process must not include grinding of the samples. Submit the name and location of the disposal facility that will be accepting the hazardous waste and the analytical laboratory along with the testing requirements not less than 5 business days before the start of removal of yellow thermoplastic traffic stripe. The analytical laboratory must be certified by the California Department of Public Health (CDPH) Environmental Laboratory Accreditation Program (ELAP) for all analyses to be performed.

After the Engineer accepts the analytical test results, dispose of yellow thermoplastic hazardous waste residue at a Class 1 disposal facility located in California under the requirements of the disposal facility operator within 90 days after accumulating 220 pounds of residue and dust.

If less than 220 pounds of hazardous waste residue and dust is generated in total, dispose of it within 90 days after the start of accumulation of the residue and dust.

The Engineer will sign all manifests as the generator within 2 business days of receiving and accepting the analytical test results and receiving your request for the US EPA Generator Identification Number. Use a transporter with a current DTSC registration certificate and that is in compliance with the CHP BIT Program when transporting hazardous waste.

#### **14-11.07D Payment**

Payment for a lead compliance plan is not included in the payment for environmental stewardship work.

If analytical test results demonstrate that the residue is a non-hazardous waste and the Engineer agrees, dispose of the residue at an appropriately permitted CA Class II or CA Class III facility. The Department does not adjust payment for this disposal.

#### **Replace section 14-11.09 with:**

#### **14-11.09 TREATED WOOD WASTE**

##### **14-11.09A General**

##### **14-11.09A(1) Summary**

Section 14-11.09 includes specifications for handling, storing, transporting, and disposing of treated wood waste (TWW).

Wood removed from metal beam guard railing is TWW. Manage TWW under 22 CA Code of Regs, Div. 4.5, Chp. 34.

##### **14-11.09A(2) Submittals**

For disposal of TWW, submit as an informational submittal a copy of each completed shipping record and weight receipt within 5 business days.

##### **14-11.09B Materials**

Not Used

##### **14-11.09C Construction**

##### **14-11.09C(1) General**

##### **14-11.09C(2) Training**

Provide training to personnel who handle TWW or may come in contact with TWW. Training must include:

1. All applicable requirements of 8 CA Code of Regs
2. Procedures for identifying and segregating TWW
3. Safe handling practices
4. Requirements of 22 CA Code of Regs, Div. 4.5, Chp. 34
5. Proper disposal methods

Maintain records of personnel training for 3 years.

##### **14-11.09C(3) Storage**

Store TWW before disposal using the following methods:

1. Elevate on blocks above a foreseeable run-on elevation and protect from precipitation for no more than 90 days.
2. Place on a containment surface or pad protected from run-on and precipitation for no more than 180 days.
3. Place in water-resistant containers designed for shipping or solid waste collection for no more than 1 year.
4. Place in a storage building as defined in 22 CA Code of Regs, Div. 4.5, Chp. 34, § 67386.6(a)(2)(C).

Prevent unauthorized access to TWW using a secured enclosure such as a locked chain link fenced area or a lockable shipping container located within the job site.

Resize and segregate TWW at a location where debris from the operation including sawdust and chips can be contained. Collect and manage the debris as TWW.

Provide water-resistant labels that comply with 22 CA Code of Regs, Div. 4.5, Chp. 34, §67386.5, to clearly mark and identify TWW and accumulation areas. Labels must include:

1. Caltrans, District number, Construction, Construction Contract number
2. District office address
3. Engineer's name, address, and telephone number
4. Contractor's contact name, address and telephone number
5. Date placed in storage

**14-11.09C(4) Transporting and Disposal**

Before transporting TWW, obtain an agreement from the receiving facility that the TWW will be accepted. Protect shipments of TWW from loss and exposure to precipitation. For projects with 10,000 pounds or more of TWW, request a US EPA Generator Identification Number from the Engineer at least 5 business days before the first shipment. Each shipment must be accompanied by a shipping record such as a bill of lading or invoice that includes:

1. Caltrans with district number
2. Construction Contract number
3. District office address
4. Engineer's name, address, and telephone number
5. Contractor's contact name and telephone number
6. Receiving facility name and address
7. Waste description: Treated Wood Waste with preservative type if known or unknown/mixture
8. Project location
9. Estimated quantity of shipment by weight or volume
10. Date of transport
11. Date of receipt by the receiving TWW facility
12. Weight of shipment as measured by the receiving TWW facility
13. For projects with 10,000 pounds or more of TWW include the USA EPA Generator Identification Number.

The shipping record must be at least a 4-part carbon or carbonless 8 1/2 by 11-inch form to allow retention of copies by the Engineer, transporter, and disposal facility.

Dispose of TWW at an approved TWW facility. A list of currently approved TWW facilities is available at:

<http://www.dtsc.ca.gov/HazardousWaste/upload/lanfillapr11pdated1.pdf>

Dispose of TWW within:

1. 90 days of generation if stored on blocks
2. 180 days of generation if stored on a containment surface or pad
3. 1 year of generation if stored in a water-resistant container, or within 90 days after the container is full, whichever is shorter
4. 1 year of generation if storing in a storage building as defined in 22 CA Code of Regs, Div. 4.5, Chp. 34, § 67386.6(a)(2)(C)

**14-11.09D Payment**

Not Used

**Replace section 14-11 with:**

**14-11.11 MANAGEMENT OF ASBESTOS CONTAINING MATERIALS**

**14-11.11A General**

**14-11.11A(1) Summary**

Section 14-11.11 includes specifications for , removal, and disposal of asbestos-containing material (ACM). Friable ACM generated as part of this work is Department-generated hazardous waste under 14-11.02F.

**14-11.11A(2) Definitions**

**asbestos:** Includes chrysotile, amosite, crocidolite, tremolite, anthrophyllite, actinolite and any of these minerals that has been chemically treated and/or altered.

**asbestos-containing material (ACM):** Any building material, including asbestos cement pipe containing commercial asbestos in an amount greater than 1% by weight, area, or count.

**certified asbestos consultant (CAC):** An asbestos consultant certified by Cal/OSHA under 8 CA Code of Regs § 341.15 and 1529.

**friable ACM:** Any material containing more than 1 percent asbestos by area that hand pressure can crumble, pulverize or reduce to powder when dry".

**non-friable ACM:** Asbestos fibers are tightly bound into the matrix of the material and should not become an airborne hazard as long as the material remains intact and undamaged, and is not sawed, sanded, drilled or otherwise abraded during removal (Asbestos Hazard Emergency Response Act (AHERA)).

#### **14-11.11A(3) Asbestos Survey Results**

Asbestos was detected in joint fill materials, sheet packing, and concrete in the Smith River Middle Fork Bridge (Br. No. 1-0015). Portions of the survey report are included in the "Information Handout." The complete report entitled "Asbestos and Lead-Containing Paint Survey Report, July 2009" is available as specified in section 2-1.06B.

#### **14-11.11A(4) Submittals**

##### **14-11.11A(4)a Asbestos Surveying Work Plan for Sampling**

Not Used

##### **14-11.11A(4)b Asbestos Sampling and Analysis Report**

Not Used

##### **14-11.11A(4)c Air Quality Management District (AQMD) or Air Pollution Control District (APCD) Notification of Demolition**

Submit a copy of the NESHAP notification form and attachments, required under section 14-9.02, before submittal to the AQMD or APCD.

##### **14-11.11A(4)d Asbestos Compliance Plan**

Prepare an Asbestos Compliance Plan (ACP) to prevent or minimize exposure to asbestos. The ACP must be signed by a CIH before submission to the Engineer for review and authorization. Submit the ACP to the Engineer at least 15 business days before beginning work in areas containing or suspected to contain asbestos. The ACP must comply with section 7-1.01A, "Labor Code Requirements" of the Standard Specifications and contain as a minimum:

1. Identification of key personnel for the project
2. Scope of Work and equipment that will be used
3. Job hazard analysis for work assignments
4. Summary of risk assessment
5. Personal protective equipment
6. Delineation of work zones on-site
7. Decontamination procedures
8. General safe work practices
9. Security measures
10. Emergency response plans
11. Worker training

##### **14-11.11A(4)e Removal Work Plan**

Prepare a work plan for the removal, storage, transportation and disposal of ACM. Removal and management of ACM must be performed by a contractor registered under Labor Code § 6501.5 and certified under Bus & Prof Code § 7058.6

Asbestos removal procedures must include:

1. Installing asbestos warning signs at perimeters of abatement work areas
2. Wetting asbestos materials with sprayers
3. Containing large volumes of asbestos materials in disposal bins for temporary storage until removed from the site
4. Providing manifests for disposal upon completion for the Engineer to sign

5. Providing transporters registered to transport hazardous waste in the state of California under the Health and Safety Code Ch 6.5, Div 20 and 22 CA Code of Regs, Div 4.5
6. Disposing of asbestos materials at a disposal facility permitted by the California Environmental Protection Agency
7. Compliance with federal, state, and local requirements for asbestos work, transport, and disposal

**14-11.11A(4)f Certification of Completion of Safety Training**

Submit a certification of completion of safety training for all trained personnel before starting work in areas containing or suspected to contain asbestos.

**14-11.11A(4)g Waste Shipment Records**

Submit a copy of all waste shipment records within 35 days after shipment.

**14-11.11A(5) Quality Control and Assurance**

**14-11.11A(5)a Qualifications**

The person in charge of asbestos inspection and abatement planning must be a CAC.

The person in charge of asbestos removal must be registered under Labor Code § 6501.5 and certified under Bus & Prof Code § 7058.6.

**14-11.11A(5)b Regulatory Requirements**

Codes which govern removal and disposal of materials containing asbestos include:

1. CA Health and Safety Code, Division 20, Chapter 6.5, Hazardous Waste Control
2. 8 CA Code of Regs, General Industry Safety Order 5208 Asbestos
3. 8 CA Code of Reg, § 1529 and 341
4. 22 CA Code of Regs, Division 4.5
5. Cal/OSHA, Part 26 (amended), of 29 CFR
6. 40 CFR, Part 61, subpart M

**14-11.11B Materials**

Not used

**14-11.11C Construction**

Notify Cal/OSHA of changes in work locations or conditions under 8 CA Code of Regs § 341.9.

Before starting work in areas containing or suspected to contain asbestos, provide safety training that meets the requirements of 8 CA Code of Regs § 1529 to personnel who have no prior training or are not current in their training status, including State personnel.

Provide training, personal protective equipment, and medical surveillance required by the Asbestos Compliance Plan to 3 State personnel.

**14-11.11C(1) Asbestos Inspection (Removal of ACM from Bridge)**

**Not Used 14-11.11C(2) Unanticipated Suspected ACM discovered During Demolition or Excavation**

If unanticipated suspected ACM is discovered during demolition, the portion of the work that involves the unanticipated suspected ACM must be performed by or under the direction of CAC. Test the suspected ACM in compliance with USEPA “Asbestos/NESHAP Regulated Asbestos Containing Materials Guidance.”

Notify the APCD or the AQMD of changes to removal or demolition plans, including discovery of ACM during demolition, within 2 business days of the change.

**14-11.11C(3) Removal**

Comply with 8 CA Code of Regs § 1529 and § 341. Remove friable ACM using the wetting method. Remove and handle all non-friable ACM to prevent breakage. The removal of ACM encased in concrete or other similar structural material is not required before demolition, but the ACM must be adequately wetted whenever exposed during demolition. Prevent visible emissions from all ACM removal activities.

Mark all regulated work areas with the following or equivalent warning:

**DANGER  
ASBESTOS  
CANCER AND LUNG DISEASE HAZARD  
AUTHORIZED PERSONNEL ONLY**

**14-11.11C(4) Packaging**

Comply with 22 CA Code of Regs, Div 4.5, Chapter 12, Article 3 requirements for packaging and labeling removed ACM. Place removed ACM in approved containers (double ply, 0.06-inch minimum thickness, plastic bags) with caution labels affixed to bags. Caution labels must have conspicuous, legible lettering, that spells out the following or equivalent warning:

**DANGER  
CONTAINS ASBESTOS FIBERS  
AVOID CREATING DUST  
CANCER AND LUNG DISEASE HAZARD**

Place removed materials containing asbestos directly into a covered, lockable roll off or drop box that has the same caution label affixed on all sides.

**14-11.11C(5) Transportation**

All haulers of friable ACM must have current registration with DTSC for transporting hazardous waste and must have a U.S. Environmental Protection Agency Identification Number (U.S. EPA I.D. Number). All vehicles used to transport hazardous waste material must carry a valid registration during transport. Transport non-friable (non-hazardous waste) ACM to the disposal facility with a shipping document or waste shipment record.

**14-11.11C(6) Disposal**

Dispose of friable and non-friable waste containing asbestos at a disposal facility permitted to accept the waste and that meets all the requirements specified by federal, state and local regulations. Notify the proper authorities at the disposal site in advance of delivery of ACM.

**14-11.11D Payment**

Not Used

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**15 EXISTING FACILITIES**

**Replace section 15-1.03B with:**

**15-1.03B Residue Containing Lead from Paint and Thermoplastic**

Residue from cold planing contains lead from paint and thermoplastic. The average lead concentrations are less than 1,000 mg/kg total lead and 5 mg/L soluble lead. This residue:

1. Is a nonhazardous waste
2. Does not contain heavy metals in concentrations that exceed thresholds established by the Health and Safety Code and 22 CA Code of Regs
3. Is not regulated under the Federal Resource Conservation and Recovery Act (RCRA), 42 USC § 6901 et seq.
4. Is generated by cold planing as shown.

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

Payment for a lead compliance plan is not included in the payment for existing facilities work.

Payment for handling, removal, and disposal of grinding or cold planing residue that is a nonhazardous waste is included in the payment for the type of removal work involved.

**Replace section 15-2.02B(3) with:**

**15-2.02B(3) Cold Planing Asphalt Concrete Pavement**

**15-2.02B(3)(a) General**

Schedule cold planing activities so that not more than 7 days elapses between the time the pavement is cold planed and the HMA is placed.

**15-2.02B(3)(b) Materials**

Use the same quality of HMA for temporary tapers that is used for the HMA overlay or comply with the specifications for minor HMA in section 39.

**15-2.02B(3)(c) Construction**

**15-2.02B(3)(c)(i) General**

Do not use a heating device to soften the pavement.

The cold planing machine must be:

1. Equipped with a cutter head width that matches the planing width. If the cutter head width is wider than the cold plane area shown, submit to the Engineer a request for using a wider cutter head. Do not cold plane unless the Engineer approves your request.
2. Equipped with automatic controls for the longitudinal grade and transverse slope of the cutter head and:
  - 2.1. If a ski device is used, it must be at least 30 feet long, rigid, and a 1-piece unit. The entire length must be used in activating the sensor.
  - 2.2. If referencing from existing pavement, the cold planing machine must be controlled by a self-contained grade reference system. The system must be used at or near the centerline of the roadway. On the adjacent pass with the cold planing machine, a joint-matching shoe may be used.
3. Equipped to effectively control dust generated by the planing operation
4. Operated so that no fumes or smoke is produced.

Replace broken, missing, or worn machine teeth.

**15-2.02B(3)(c)(ii) Grade Control and Surface Smoothness**

Furnish, install, and maintain grade and transverse slope references.

The depth, length, width, and shape of the cut must be as shown or as ordered. The final cut must result in a neat and uniform surface. Do not damage the remaining surface.

The completed surface of the planed asphalt concrete pavement must not vary more than 0.02 foot when measured with a 12-foot straightedge parallel with the centerline. With the straightedge at right angles to the centerline, the transverse slope of the planed surface must not vary more than 0.03 foot.

Where lanes are open to traffic, the drop-off of between adjacent lanes must not be more than 0.15 foot.

**15-2.02B(3)(c)(iii) Temporary HMA Tapers**

If a drop-off between the existing pavement and the planed area at transverse joints cannot be avoided before opening to traffic, construct a temporary HMA taper. The HMA temporary taper must be:

1. Placed to the level of the existing pavement and tapered on a slope of 200:1 (horizontal:vertical) or flatter to the level of the planed area
2. Compacted by any method that will produce a smooth riding surface

Completely remove temporary tapers before placing permanent surfacing.

**15-2.02B(3)(c)(iv) Remove Planed Material**

Remove cold planed material concurrent with planing activities so that removal does not lag more than 50 feet behind the planer.

**15-2.02B(3)(d) Payment**

Payment for removal of pavement markers, thermoplastic traffic stripe, painted traffic stripe, and pavement marking within the area of cold planing is included in the payment for cold plane asphalt concrete pavement of the types shown in the Bid Item List.

**Replace section 15-2.02C(2) with:**

**15-2.02C(2) Remove Traffic Stripes and Pavement Markings Containing Lead**

Residue from removing traffic stripes and pavement markings contains lead from the thermoplastic. The average lead concentrations are less than 1,000 mg/kg total lead and 5 mg/L soluble lead. This residue:

1. Is a nonhazardous waste
2. Does not contain heavy metals in concentrations that exceed thresholds established by the Health and Safety Code and 22 CA Code of Regs
3. Is not regulated under the Federal Resource Conservation and Recovery Act (RCRA), 42 USC § 6901 et seq.

Submit a lead compliance plan under section 7-1.02K(6)(j)(ii).

Payment for a lead compliance plan is not included in the payment for existing facilities work.

Payment for handling, removal, and disposal of pavement residue that is a nonhazardous waste is included in the payment for the type of removal work involved.

**Replace section 15-2.04F with:**

**15-2.04F Reconstruct Gabion Walls**

Gabion walls must be removed, refabricated, and installed as described.

Removing portion of gabion walls must comply with section 15-4.

Excavation and backfill must comply with section 19-3.

Submit 2 sets of shop drawings showing details of the refabrication of removed material and the fabrication of new material.

Refabrication of removed material, fabrication of new material, and installation of the gabion walls must comply with section 72-16.

**Replace section 15-2.05C with:**

**15-2.05C Abandon Culverts and Downdrains**

**15-2.05C(1) General**

Abandon culverts or downdrains by removing portions of the culverts or downdrains, filling the inside, and backfilling the depressions and trenches to grade. As an alternative to abandoning a culvert or downdrain, you may remove the culvert or downdrain, dispose of it, and backfill.

Notify the Engineer before abandoning a culvert or downdrain.

**15-2.05C(2) Materials**

Openings into existing structures that are to remain in place must be plugged with minor concrete under section 90.

**15-2.05C(3) Construction**

Wherever culverts or downdrain intersect side slopes, remove them to a depth of at least 3 feet. Measure the depth normal to the plane of the finished side slope. Abandon the remaining portion of the culvert or downdrain.

Culverts or downdrain that are 12 inches or more in diameter must be completely filled by authorized methods. Backfill with sand that is clean, free draining, and free from roots and other deleterious substances. As an alternative to sand, you may backfill with one of the following:

1. Controlled low-strength material under section 19-3.02F
2. Slurry cement backfill under section 19-3.02D

Ends of culverts and downdrain must be securely closed by a 6-inch-thick, tight-fitting plug or wall of commercial-quality concrete.

**15-2.05C(4) Payment**

If backfilling inside the culvert or downdrain is required, payment for backfilling inside the culverts or downdrains is included in the payment for abandon culvert or abandon downdrain. Payment for backfilling outside the culvert or downdrain is included in the payment for abandon culvert or abandon downdrain.

**Replace section 15-2.09E with:**

Relocate temporary railing (Type K) as shown.

Conform to section 12-3.08.

Each move shown is measured and paid for.

**Add to section 15-4.01A(2):**

The Department's review time for work plans for removing specific structures or portions of structures is shown in the following table:

Structure name	Review time
Middle Fork Smith River Bridge (Replace)	30 days

**Add to section 15-4.01C(1):**

Remove the following bridge:

Bridge no.	Description of work
01-0080	Remove bridge in stages as shown

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**DIVISION III GRADING**

**16 CLEARING AND GRUBBING**

**Replace the 4th paragraph in section 16-1.03A with:**

Clear and grub vegetation only within the excavation and embankment slope lines.

**Add to section 16-1.03A:**

All removed trees 16 inches diameter at breast height (dbh) or greater must be cut in 20 - 40-foot sections with root wads intact where feasible. If trees are removed during the rain season (October 15<sup>th</sup> to June 15<sup>th</sup>) leave 20 to 40-feet of trunk standing for later dry season excavation and removal with attached root wad. Move these cut sections of trees within five days to a location within 50 miles as specified by the Engineer.

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**19 EARTHWORK**

**Replace the 1st paragraph of section 19-3.02G with:**

Concrete backfill encasing steel soldier piles below the lagging and placed in overexcavated areas where shown must comply with section 90-1 and contain at least 505 pounds of cementitious material per cubic yard.

**Add to section 19-3.02:**

**19-3.02I Permeable Material**

Permeable material must be Class 1, Type B, complying with section 68-2.02F.

**Add to section 19-3.03B(1):**

For footings at locations with structure excavation (Type D), ground or surface water is expected to be encountered but seal course concrete is not described.

**Replace the 1st paragraph of section 19-3.03B(2) with:**

In cuts, excavate for and construct soldier pile walls in lifts from the top down.

**Add to section 19-3.04:**

Structure excavation for footings at locations not shown as structure excavation (Type D) and where ground or surface water is encountered is paid for as structure excavation (bridge).

Pervious backfill material placed within the limits of payment for bridges is paid for as structure backfill (bridge). Pervious backfill material placed within the limits of payment for retaining walls is paid for as structure backfill (retaining wall).

Permeable material placed within the limits of payment for retaining walls is paid for as structure backfill (retaining wall).

**Replace section 19-4 with:  
19-4 ROCK EXCAVATION (CONTROLLED BLASTING)**

**19-4.01 GENERAL**

**19-4.01A Summary**

Section 19-4 includes specifications for performing rock excavation with controlled blasting.

You may use hydraulic splitters, pneumatic hammers, controlled blasting, or other roadway excavation techniques authorized to fracture rock and construct stable final rock cut faces.

Comply with section 12.

Comply with federal, state, and local blasting regulations. Regulations containing specific Cal-OSHA requirements for blasting activities include 8 CA Code of Regs, Ch 4, Subchapter 7, Group 18, "Explosive Materials." Regulations for explosives containing percholate materials include 22 CA Code of Regs, Division 4.5, Ch 33, "Best Management Practices for Percholate Materials."

You are liable for damages resulting from blasting activities.

**19-4.01B Definitions**

**controlled blasting:** Use of explosives and blasting accessories in predetermined spaced and aligned drill holes to limit blast vibrations, noise from airblast overpressure, and flyrock.

**flyrock:** Rock that becomes airborne due to blasting.

**near field blasting:** Blasting within 30 feet of a critical structure.

**19-4.01C Submittals**

**19-4.01C(1) General**

Submit 3 copies of the blasting safety plan and each controlled blasting plan. After each plan is authorized, submit 3 additional copies of each authorized plan.

**19-4.01C(2) Blasting Safety Plan**

Submit a blasting safety plan. The plan must include:

1. References to applicable federal, state, and local codes and regulations
2. Copies of permits required for blasting activities
3. Business name, contractor license number, address, and telephone number of the blasting subcontractor
4. Proof of current liability insurance and bonding
5. Name, address, telephone number, copies of applicable licenses, and resume of:
  - 5.1. Blaster-in-charge
  - 5.2. Personnel responsible for controlled blast design, loading, and conducting the blasting operation
  - 5.3. Safety officer for blasting subcontractor
  - 5.4. Blast monitoring consultant
  - 5.5. Blasting consultant
6. Name, address, and telephone number of the local fire station and law enforcement agencies
7. Detailed description of:
  - 7.1. Location where explosives will be stored
  - 7.2. Security measures to protect and limit access to the explosives
  - 7.3. Transportation means for explosives
  - 7.4. List of personnel permitted to handle the explosives
8. Exclusion zone and limited-entry zone for nonblast related operations and personnel surrounding loading and blasting operations
9. Details of warning signals used to alert employees on the job site of an impending blast and to indicate the blast is completed and the area is safe to enter
10. How blasting operations will be conducted
11. Measures to protect blasting operations and personnel from lightning

12. Emergency evacuation procedures for areas where explosives may be present
13. How misfires will be recognized, handled, and resolved including:
  - 13.1. Who will be notified
  - 13.2. How blast zone will be secured until misfire is resolved
  - 13.3. Identification of equipment that may be needed to resolve misfires
14. Details of signs to be used around blasting zones including:
  - 14.1. Timing of when signs will be posted relative to a specific blast
  - 14.2. Name and telephone number of person responsible for placing signs
  - 14.3. Roadway signs for compliance with Chapter 6, Typical Application 2, of the California MUTCD.
15. Traffic control details for:
  - 15.1. Loading and blasting operations
  - 15.2. Misfire event or other blast related phenomenon that causes a transportation corridor to remain closed to the public
16. Description of possible noxious gas generation and details of safeguards to be used to protect employees, work zones adjacent to the shot, private property, and the public
17. Procedure to report and resolve complaints for blast related accidents
18. Copies of each MSDS and manufacturer data sheets of explosives, caps, primers, initiators, and other compounds

#### **19-4.01C(3) Controlled Blasting Plan**

Submit a controlled blasting plan for each blast. The plan must include details on how each blast will be controlled and the following:

1. Blast identification by numerical and chronological sequence
2. Location, referenced to stationing, offset distance, date, and time of blast
3. Drawings showing drill hole pattern, spacing, burden, and initiation sequence
4. Typical cross-sections through zone to be blasted
5. Groundwater level, if present, within the prism to be blasted
5. Initiation-sequence diagram showing the actual firing time of each delay
6. Type of material to be blasted
7. Number of drill holes
8. Diameter, depth, and spacing of holes
9. Height or length of stemming
10. Types and characteristics of explosives used, including explosive's density, relative strength, and date of manufacture
11. Type of caps and delay periods used and their date of manufacture
12. Total amount of explosives used
13. Total amount of explosives detonating within any 8 millisecond period
14. Powder factor (pounds of explosive per cubic yard of material blasted)
15. Method of firing
16. Direction and distance to nearest building or structure
17. Type and method of instrumentation
18. Location and placement of instruments
19. Measures to limit air noise and flyrock
20. Measures to limit overbreak
21. Name of blasting subcontractor
22. Name and signature of blaster-in-charge
23. Drawings showing spacing and proximity of shot guards to blast location

Changes to the controlled blasting plan made to adjust for site conditions must be submitted for review before implementing.

#### **19-4.01D Quality Control and Assurance**

##### **19-4.01D(1) General**

Not Used

#### **19-4.01D(2) Blaster-In-Charge**

Assign a blaster-in-charge responsible for supervising all blasting activities. The blaster-in-charge must have 10 years of experience in performing or supervising similar blasting activities and must be a licensed blaster.

#### **19-4.01D(3) Blast Monitoring Consultant**

Assign a blast monitoring consultant to monitor blasting generated vibrations and noise near buildings and structures that may be subject to damage. The monitoring consultant must be responsible for collecting and interpreting vibration and noise data. The blast monitoring consultant must:

1. Not be employed by the blasting contractor or other subcontractor on the project
2. Have a minimum of a 2-year Associate's Degree in science or engineering
3. Have at least 5 years of documented experience in collecting and interpreting ground vibrations and noise data

#### **19-4.01D(4) Blasting Consultant**

Assign a blasting consultant to oversee near field blasting activities. The blasting consultant must:

1. Be an engineer or geologist who is licensed in the State
2. Have 10 years of experience providing specialized blasting services in near field blasting
3. Not be employed by the blasting contractor, explosive manufacturer, or explosive distributor
4. Submit a resume of credentials and a list of projects worked on

#### **19-4.01D(5) Preblast Surveys**

At least 15 days before starting blasting activities, prepare a preblast survey of all buildings and structures within 330 feet of blasting activities and submit it with the controlled blasting plan. The preblast survey must include a written report, sketches, and photos or a videotape with date and time displayed on the image. The preblast survey must include:

1. Name of the person making the inspection
2. Name of property owner and occupants
3. Property address
4. Date and time of the inspection
5. Description of the structure or other improvement including culverts and bridges
6. Detailed description of existing condition of walls, ceiling, and floor of each interior room including attic and basement
7. Detailed description of existing condition of foundations, exterior walls, roofs, doors, windows, and porches
8. Detailed description of existing condition of garages, outbuildings, sidewalks, driveways, and swimming pools
9. Detailed listing of highway sign posts, light fixtures, and overhead power lines
10. Survey of wells or other private water supplies including total depth and existing water surface levels
11. Identification of sites conducting procedures, processes, or operations that may be sensitive to blasting activities
12. Scaled map or aerial photo showing the location of structures and properties surveyed and location of all proposed blasting sites

If blasting activities are suspended for a period of 45 days or more, perform another preblast survey and submit it at least 15 days before resuming blasting activities.

After blasting activities are completed, prepare and submit a postblast survey of the same buildings and structures as in the preblast survey. The postblast survey must include all items included in the preblast survey.

#### **19-4.01D(6) Vibration and Noise Monitoring**

Vibration levels must be kept below peak particle velocity of 2 inches per second at the nearest building or structure.

Noise from airblast overpressure levels must be kept below 128 dB (C-network or Linear network) at the nearest building

Ground vibrations and noise created from blasting must be controlled by using properly designed delay sequencing and charge weights for shots.

Provide 3 seismographs to be available for deployment that are appropriate for controlled blasting activities and capable of:

1. Recording particle velocities for 3 mutually perpendicular components of vibration and instantaneous resultant peak vector sum in the range generally found with controlled blasting.
2. Continuously measuring, recording, and reporting vibrations along 3 primary axes.
3. Measuring and recording vibration frequencies ranging from 2 to 300 Hz.
4. Providing a printed record of each event showing a plot of peak particle velocity versus vibration frequencies.
5. Measuring and recording airblast noise levels. The noise transducer must be detachable from the main unit to allow placing at elevations with a clear line of sight between transducer and blast.

Record each blast shot using approved seismographs and prepare a vibration and noise monitoring report. The report must include:

1. Identification of instruments used
2. Name of blast monitoring consultant
3. Distance and direction of recording stations from blast area
4. Type of ground at recording station and material on which instrument sits
5. Maximum particle velocity in each component and resultant peak particle velocity of each shot
6. Copy of seismograph readings with date and signature of blast monitoring consultant
7. Noise levels recorded in dB (C-network or Linear network) units

#### **19-4.01D(7) Video Recording of Blasts**

Video-record each blast. The video-recording must be taken from a safe location with a clear view of the blast area, activities, and progression. Identify each video or section of video with an index to identify each blast. Submit a copy of each video in DVD-Video format.

#### **19-4.01D(8) Blasting Complaints**

Accurately document each complaint. Notify the Engineer immediately of a complaint received or at the start of the next day's work shift. Complaint documentation must include:

1. Name and address of complainant
2. Date, time, and nature of complaint
3. Dated photo or videotape of physical damage
4. Name of person receiving complaint
5. Record of complaint investigation conducted
6. Resolution of complaint

#### **19-4.01D(9) Postblast Reports**

Document each shot in a postblast report. The postblast report must include all data required in the controlled blasting plan for that shot and the following:

1. Description of site conditions, loading, and time of blast
2. Description of weather conditions at time of blast including wind direction and cloud cover
3. Drillers boring record
4. Copy of vibration and noise monitoring report
5. Copy of documented complaints arising from the blast

Submit the postblast report within 48 hours of the blast.

#### **19-4.02 MATERIALS**

Not Used

**19-4.03 CONSTRUCTION**

At least 7 days before starting or resuming blasting activities, notify occupants of the local buildings within 330 feet of the blasting area in writing. Verbally notify occupants of pending blasting activities on the day of blasting.

Do not perform blasts within 1,200 feet of concrete placed within 72 hours.

Before firing any blast, confirm that groundwater conditions are consistent with shot design and explosive type to be used.

Before firing any blast in areas where flyrock may result in personal injury or damage to property or the work, cover the rock to be blasted with blasting mats, soil, or other equally serviceable material to prevent flyrock.

If blasting causes flyrock, suspend blasting activities. The blasting consultant must review the site to determine the cause of the flyrock problem and provide an amendment to the controlled blasting plan that prevents flyrock.

Do not use drill cuttings as stemming in controlled blasting operations.

**19-4.04 PAYMENT**

Rock excavation is measured and paid as roadway excavation in section 19-2.04.

**Replace section 19-9 with:  
19-9 SHOULDER BACKING**

**19-9.01 GENERAL**

**19-9.01A Summary**

Section 19-9 includes specifications for constructing shoulder backing adjacent to the edge of new pavement surfacing.

**19-9.02 MATERIALS**

Shoulder backing must be clean and consist of one or any combination of the following materials:

1. Broken stone
2. Crushed gravel
3. Natural rough surfaced gravel
4. Sand

Shoulder backing must comply with the grading requirements for the sieve sizes shown in the following table:

Sieve sizes	Percentage passing
2"	100
1"	75–100
3/4"	65–100
No. 4	35–60
No. 30	10–35
No. 200	5–15

Sand equivalent for shoulder backing must be from 10 to 35.

If a combination of broken stone, crushed gravel, natural rough surfaced gravel, and sand is used, shoulder backing must comply with the quality requirements shown in the following table:







Where IP testing is required, identify areas of localized roughness. Areas of localized roughness must be identified using the ProVAL smoothness assurance analysis by calculating continuous IRI for each wheel path with a 25-foot interval using a 250 mm filter.

Interpret references to "must-grinds" as "localized roughness" and "PI<sub>0</sub>" as "MRI" in the RSS for section 39.

### **39-1.12B Submittals**

At least 5 business days before start of initial profiling or changing profiler or operator, submit:

1. IP certification issued by Texas Transportation Institute. The certification must be not more than 12 months old.
2. Operator certification for the IP issued by Texas Transportation Institute. The certification must be not more than 36 months old.
3. List of manufacturer's recommended test procedures for IP calibration and verification.

Within 2 business days after cross correlation testing, submit ProVAL profiler certification analysis report for cross correlation test results performed on test section to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

Within 2 business days after each day of inertial profiling, submit profile data to the Engineer and to the electronic mailbox address:

smoothness@dot.ca.gov

Profiling data must include:

1. Raw profile data for each lane.
2. ProVAL ride quality analysis report for IRIs of left and right wheel paths of each lane. Submit in pdf file format.
3. ProVAL ride quality analysis report for MRIs of each lane. Submit in pdf file format.
4. ProVAL smoothness assurance analysis report for IRIs of left wheel path. Submit in pdf file format.
5. ProVAL smoothness assurance analysis report for IRIs of right wheel path. Submit in pdf file format.
6. GPS data file for each lane in GPS exchange. Submit in GPS eXchange file format.
7. Manufacturer's recommended IP calibration and verification tests results.
8. AASHTO IP calibration and verification test results including bounce, block, and distance measurement instrument (DMI).

Submit the raw profile data in unfiltered electronic pavement profile file (PPF) format. Name the PPF file using the following naming convention:

YYYYMMDD\_TTCCRRR\_D\_L\_W\_S\_X\_PT.PPF

where:

YYYY = year

MM = Month, leading zero

DD = Day of month, leading zero

TT = District, leading zero

CCC = County, 2 or 3 letter abbreviation as shown in section 1-1.08

RRR = Route number, no leading zeros

D = Traffic direction as NB, SB, WB, or EB

L = Lane number from left to right in direction of travel

W = Wheel path as "L" for left, "R" for right, or "B" for both

S = Beginning station to the nearest foot (i.e., 10+20) or beginning post mile to the nearest hundredth (i.e., 25.06) no leading zero

X = Profile operation as "EXIST" for existing pavement, "INTER" for after prepaving smoothness correction, "PAVE" for after paving, and "CORR" for after final surface pavement correction

PT = Pavement type (i.e., HMA, RHMA, HMA-O, RHMA-O, RHMA-G, etc.)

Electronic PPF files that do not follow this standardized naming convention will be rejected.

Within 2 business days of performing straightedge measurements, submit areas requiring smoothness correction. Identify locations of smoothness correction by:

1. Location Number
2. District-County-Route
3. Beginning station or post mile to the nearest 0.01 mile
4. For correction areas within a lane:
  - 4.1. Lane direction as NB, SB, EB, or WB
  - 4.2. Lane number from left to right in direction of travel
  - 4.3. Wheel path as "L" for left, "R" for right, or "B" for both
5. For correction areas not within a lane:
  - 5.1. Identify pavement area (i.e., shoulder, weight station, turnout)
  - 5.2. Direction and distance from centerline as "L" for left or "R" for right
6. Estimated size of correction area

### 39-1.12C Inertial Profiler Calibration and Verification Tests

IP equipment must display a current certification decal with expiration date.

Operate the IP according to the manufacturer's recommendations and AASHTO R57-10 at 1-inch recording intervals.

Notify the Engineer 2 business days before performing IP calibration and verification testing.

Conduct the following IP calibration and verification tests in the Engineer's presence each day before performing inertial profiling:

1. Block test. Verify the height sensor accuracy under AASHTO R57-10, section 5.3.2.3.
2. Bounce test. Verify the combined height sensor and accelerometer accuracy under AASHTO R57-10, section 5.3.2.3.2.
3. DMI test. Calibrate the accuracy of the testing procedure under AASHTO R56-10, section 8.4.
4. Manufacturer's recommended tests.

Conduct cross correlation IP verification test in the Engineer's presence before performing initial profiling. Verify cross correlation IP verification test at least annually. Conduct 5 repeat runs of the IP on an authorized test section. The test section must be on an existing asphalt concrete pavement surface 0.1 mile long. Calculate a cross correlation to determine the repeatability of your device under Section 8.3.1.2 of AASHTO R56-10 using ProVAL profiler certification analysis with a 3 feet maximum offset. The cross correlation must be a minimum of 0.92.

For each 0.1 mile section, your IRI values must be within 10 percent of the Department's IRI values. The Engineer may order you to recalibrate your IP equipment and reprofile. If your results are inaccurate due to operator error, the Engineer may disqualify your IP operator.

### 39-1.12D Acceptance Criteria

For areas that require pavement smoothness determined using an IP, the pavement surface must:

1. Have no areas of localized roughness with an IRI greater than 120 in/mi
2. Comply with the MRI requirements shown in the following tables for a 0.1 mile section:

**HMA<sup>a</sup> Pavement Smoothness Acceptance Criteria**

HMA thickness	MRI requirement
> 0.20 foot	60 in/mi or less
≤0.20 foot	75 in/mi or less

<sup>a</sup> Except OGFC

### OGFC Pavement Smoothness Acceptance Criteria

OGFC placement on	MRI requirement
New construction, or HMA overlay	60 in/mi or less
Existing pavement	75 in/mi or less
Milled surface	75 in/mi or less

For areas that require pavement smoothness determined using a 12-foot straightedge, the HMA pavement surface must not vary from the lower edge of the straightedge by more than:

1. 0.01 foot when the straightedge is laid parallel with the centerline
2. 0.02 foot when the straightedge is laid perpendicular to the centerline and extends from edge to edge of a traffic lane
3. 0.02 foot when the straightedge is laid within 24 feet of a pavement conform

Pavement smoothness may be accepted based on your testing in the absence of the Department's testing.

#### 39-1.12E Smoothness Measurement

##### 39-1.12E(1) General

Notify the Engineer of start location by station and start time at least 2 business days before profiling.

Remove foreign objects on the pavement surface before profiling.

##### 39-1.12E(2) Inertial Profiler

Mark the beginning and ending station on the pavement shoulder before profiling. Stationing must be the same when profiling more than one surface.

While collecting the profile data to determine IRI, record the following locations in the raw profile data:

1. Begin and end of all bridge approach slabs
2. Begin and end of all bridges
3. Begin and end of all culverts visible on the roadway surface

Determine the MRI for 0.1-mile fixed sections using the ProVAL ride quality analysis with a 250 mm filter. Profile the left and right wheel paths of each lane. Calculate the MRI of each lane. A partial section less than 0.1 mile that is the result of an interruption to continuous pavement surface must comply with the MRI specifications for a full section. Adjust the MRI for a partial section to reflect a full section based on the proportion of a section paved.

Determine the areas of localized roughness using a continuous IRI for each wheel path with a 25-foot interval using a 250 mm filter. Localized roughness greater than 120 in/mi must be corrected regardless of the IRI values of a 0.1-mile section.

Determine the MRI of the HMA, except OGFC. If the MRI of the final pavement surface is greater than the MRI acceptance requirement in the table titled "HMA Pavement Smoothness Acceptance Criteria" in section 39-1.12D, correct to the MRI acceptance requirement in the table.

The final surface of HMA must meet MRI acceptance requirements in the table titled "HMA Pavement Smoothness Acceptance Criteria" in section 39-1.12D before placing OGFC.

Determine the MRI of the OGFC. If OGFC MRI is greater than the accepted value in the table titled "OGFC Pavement Smoothness Acceptance Criteria" in section 39-1.12D, correct to the MRI acceptance requirement in the table.

##### 39-1.12E(3) Straightedge

Measure areas that require 12-foot straightedge. If the straightedge measurement is greater than the accepted value in section 39-1.12D, correct to the acceptance requirement.

### **39-1.12F Smoothness Correction**

If the final surface of the pavement does not comply with section 39-1.12D, grind the pavement to within specified tolerances, remove and replace it, or place an overlay of HMA. Do not start corrective work until your method is authorized.

Smoothness correction of the final pavement surface must leave at least 75 percent of the specified HMA thickness. If ordered, core the pavement at the locations determined by the Engineer. Coring, including traffic control, is change order work. Remove and replace deficient pavement areas where the overlay thickness is less than 75 percent of the thickness specified as determined by the Engineer.

If you choose to correct OGFC, the Engineer determines if the corrective method causes raveling. OGFC that is raveling must be removed and replaced.

Corrected HMA pavement areas must be uniform rectangles with edges:

1. Parallel to the nearest HMA pavement edge or lane line
2. Perpendicular to the pavement centerline

On ground areas not to be overlaid with OGFC, apply fog seal coat under section 37-2.

Where corrections are made within areas requiring testing with IP, reprofile the entire lane length with the IP device.

Where corrections are made within areas requiring testing with a 12-foot straightedge, retest the corrected area with the straightedge.

### **39-1.12G Prepaving Inertial Profiler**

Section 39-1.12G applies to existing asphalt concrete areas receiving an HMA overlay. Comply with section 39-1.12A–39-1.12C and 39-1.12E.

Before starting paving operations, perform prepaving IP measurements. Prepaving IP includes taking profiles of the existing pavement, analyzing the data with ProVAL to determine existing pavement IRI, MRI, and areas of localized roughness.

Identify areas of localized roughness greater than 140 in/mi.

### **39-1.12H Prepaving Grinding**

Section 39-1.12H applies to existing asphalt concrete areas receiving an HMA overlay of less than or equal to 0.20 foot.

Correct areas of localized roughness greater than 140 in/mi.

Prepaving grinding day includes correcting areas of localized roughness, taking profiles of the corrected areas, and submitting profile data as specified in section 39-1.12B.

Notify the Engineer of those areas of localized roughness that cannot be corrected by prepaving grinding. The Engineer responds to your notification within 5 business days.

For those areas of localized roughness that cannot be corrected by grinding, the Engineer may order you to either (1) not correct the areas of localized roughness or (2) correct areas of localized roughness by a different method and take profiles of the corrected areas with an IP.

Corrective work not performed by prepaving grinding, including taking profiles of the corrected areas and associated traffic control, is change order work.

Correct prepaving areas of localized roughness that you predict will cause the final surface of HMA pavement to be noncompliant with the smoothness specifications. After correcting prepaving areas of localized roughness, take profiles of the corrected area and submit profile data as specified in section 39-1.12B.

Dispose of grinding residue.

Pave within 7 days of correcting areas.

The final pavement surface must comply with section 39-1.12D.

If ordered not to correct areas of localized roughness, the smoothness specifications do not apply to the final pavement surface placed in those areas.

**Replace section 39-1.16 with:**

**39-1.16 RUMBLE STRIPS**

**39-1.16A General**

Construct rumble strips in the top layer of HMA surfacing by ground-in methods.

**39-1.16B Materials**

Not Used

**39-1.16C Construction**

Select the method and equipment for constructing ground-in indentations.

Do not construct rumble strips on structures or approach slabs.

Construct rumble strips within 2 inches of the specified alignment. The grinding equipment must be equipped with a sighting device enabling the operator to maintain the rumble strip alignment.

Indentations must comply with the specified dimensions within 0.06 inch in depth and 10 percent in length and width.

The Engineer orders grinding or removal and replacement of noncompliant rumble strips to bring them within specified tolerances. Ground surface areas must be neat and uniform in appearance.

The grinding equipment must be equipped with a vacuum attachment to remove residue from the roadbed.

Dispose of removed material.

On ground areas, apply fog seal coat under section 37-2.

**39-1.16D Payment**

Rumble strips are measured by the station along the length of the rumble strips without deductions for gaps between indentations.

**Replace section 39-1.20 with:**

**39-1.20 LIQUID ANTISTRIP TREATMENT**

**39-1.20A General**

**39-1.20A(1) Summary**

Treat asphalt binder with liquid antistrip (LAS) treatment to bond the asphalt binder to aggregate in HMA.

**39-1.20A(2) Submittals**

For LAS, submit with the proposed JMF submittal:

1. MSDS
2. One 1-pint sample
3. Infrared analysis including copy of absorption spectra

Submit a certified copy of test results and an MSDS for each LAS lot.

Submit a certificate of compliance for each LAS shipment. With each certificate of compliance, submit:

1. Your signature and printed name
2. Shipment number
3. Material type
4. Material specific gravity
5. Refinery
6. Consignee
7. Destination
8. Quantity
9. Contact or purchase order number
10. Shipment date

Submit proportions for LAS as part of the JMF submittal. If you change the brand or type of LAS, submit a new JMF.

For each job site delivery of LAS, submit one 1/2-pint sample to METS. Submit shipping documents to the Engineer. Label each LAS sampling container with:

1. LAS type
2. Application rate
3. Sample date
4. Contract number

At the end of each day's production shift, submit production data in electronic and printed media. Present data on electronic media in tab delimited format. Use line feed carriage return with 1 separate record per line for each production data set. Allow sufficient fields for the specified data. Include data titles at least once per report. For each mixing operation type, submit in order:

1. Batch mixing:
  - 1.1. Production date
  - 1.2. Time of batch completion
  - 1.3. Mix size and type
  - 1.4. Each ingredient's weight
  - 1.5. Asphalt binder content as a percentage of the dry aggregate weight
  - 1.6. LAS content as a percentage of the asphalt binder weight
2. Continuous mixing:
  - 2.1. Production date
  - 2.2. Data capture time
  - 2.3. Mix size and type
  - 2.4. Flow rate of wet aggregate collected directly from the aggregate weigh belt
  - 2.5. Aggregate moisture content as percentage of the dry aggregate weight
  - 2.6. Flow rate of asphalt binder collected from the asphalt binder meter
  - 2.7. Flow rate of LAS collected from the LAS meter
  - 2.8. Asphalt binder content as percentage of total weight of mix calculated from:
    - 2.8.1. Aggregate weigh belt output
    - 2.8.2. Aggregate moisture input
    - 2.8.3. Asphalt binder meter output
  - 2.9. LAS content as percentage of the asphalt binder weight calculated from:
    - 2.9.1. Asphalt binder meter output
    - 2.9.2. LAS meter output

### **39-1.20A(3) Quality Control and Assurance**

For continuous mixing and batch mixing operations, sample asphalt binder before adding LAS. For continuous mixing operations, sample combined asphalt binder and LAS after the static mixer.

The Engineer orders proportioning operations stopped for any of the following if you:

1. Do not submit data
2. Submit incomplete, untimely, or incorrectly formatted data
3. Do not take corrective actions
4. Take late or unsuccessful corrective actions
5. Do not stop production when proportioning tolerances are exceeded
6. Use malfunctioning or failed proportioning devices

If you stop production, notify the Engineer of any corrective actions taken before resuming.

#### **39-1.20B Materials**

LAS-treated asphalt binder must comply with the specifications for asphalt binder in section 39-1.02C. Do not use LAS as a substitute for asphalt binder.

LAS total amine value must be 325 minimum when tested under ASTM D 2074.

Use only 1 LAS type or brand at a time. Do not mix LAS types or brands.

Store and mix LAS under the manufacturer's instruction.

#### **39-1.20C Construction**

LAS must be from 0.5 to 1.0 percent by weight of asphalt binder.

If 3 consecutive sets of recorded production data show actual delivered LAS weight is more than  $\pm 1$  percent of the authorized mix design LAS weight, stop production and take corrective action.

If a set of recorded production data shows actual delivered LAS weight is more than  $\pm 2$  percent of the authorized mix design LAS weight, stop production. If the LAS weight exceeds 1.2 percent of the asphalt binder weight, do not use the HMA represented by that data.

The continuous mixing plant controller proportioning the HMA must produce a production data log. The log consists of a series of data sets captured at 10-minute intervals throughout daily production. The data must be a production activity register and not a summation. The material represented by the data is the quantity produced 5 minutes before and 5 minutes after the capture time. For the duration of the Contract, collected data must be stored by the plant controller or a computer's memory at the plant.

#### **39-1.20D Payment**

Payment for treating asphalt binder with LAS is included in payment for the HMA involved.

### **Replace section 39-1.30 with:**

#### **39-1.30 EDGE TREATMENT, HOT MIX ASPHALT PAVEMENT**

##### **39-1.30A General**

Section 39-1.30 includes specifications for constructing the edges of HMA pavement as shown.

##### **39-1.30B Materials**

For the safety edge, use the same type of HMA used for the adjacent lane or shoulder.

##### **39-1.30C Construction**

The edge of roadway where the safety edge treatment is to be placed must have a solid base, free of debris such as loose material, grass, weeds, or mud. Grade areas to receive the safety edge as required.

The safety edge treatment must be placed monolithic with the adjacent lane or shoulder and shaped and compacted with a device attached to the paver.

The device must be capable of shaping and compacting HMA to the required cross section as shown. Compaction must be by constraining the HMA to reduce the cross sectional area by 10 to 15 percent. The device must produce a uniform surface texture without tearing, shoving, or gouging and must not leave marks such as ridges and indentations. The device must be capable of transition to cross roads, driveways, and obstructions.



**Loading Schedules**

Performance test		Proof test	
Load increment	Hold time (minutes)	Load increment	Hold time (minutes)
AL	Until stable	AL	Until stable
0.25T	1-2	0.25T	1-2
AL	Until stable	0.50T	1-2
0.25T	1-2	0.75T	1-2
0.50T	1-2	1.00T	1-2
AL	Until stable	1.25T	1-2
0.25T	1-2	1.50T <sup>a</sup>	10 or 60
0.50T	1-2	AL	Until stable
0.75T	1-2	--	--
AL	Until stable	--	--
0.25T	1-2	--	--
0.50T	1-2	--	--
0.75T	1-2	--	--
1.00T	1-2	--	--
AL	Until stable	--	--
0.25T	1-2	--	--
0.50T	1-2	--	--
0.75T	1-2	--	--
1.00T	1-2	--	--
1.25T	1-2	--	--
AL	Until stable	--	--
0.25T	1-2	--	--
0.50T	1-2	--	--
0.75T	1-2	--	--
1.00T	1-2	--	--
1.25T	1-2	--	--
1.50T <sup>a</sup>	10 or 60	--	--
AL	Until stable	--	--

NOTES: T = anchor design force shown. AL = alignment load, 0.10T.  
<sup>a</sup>Maximum test load

**Delete the 2nd paragraph of section 46-2.02B.**

**Replace the 2nd paragraph of section 46-2.02B with:**

The permanent bearing plate must effectively distribute the design force, T, uniformly to the concrete such that:

1. Concrete bearing stress does not exceed 1,600 psi
2. Bending stress of the plate does not exceed:
  - 2.1. 0.55 of the yield strength for steel
  - 2.2. 0.36 of the yield strength for cast steel or cast iron

**Add to section 46-2.03A:**

Expect difficult ground anchor installation at Middle Fork Smith River Bridge (Replace) due to the presence of the following conditions:



Calculations must show a summary of computed stresses in (1) temporary supports, (2) connections between temporary supports and the existing structure, and (3) existing load-supporting members. The computed stresses must include the effect of the jacking sequence. Calculations must include a lateral stiffness assessment of the temporary support system.

Shop drawings and calculations must be signed by an engineer who is registered as a civil engineer in the State.

Review time for temporary support shop drawings is shown in the following table:

Structure name	Review time, days
Middle Fork Smith River Bridge (Replace)	30

For falsework over railways, authorization of shop drawings is contingent upon the drawings being satisfactory to the railway company involved.

**48-3.01D Quality Control and Assurance**

**48-3.01D(1) General**

Welding, welder qualification, and welding inspection for temporary supports must comply with AWS D1.1.

Calibrate each jack within 6 months of use and after each repair. Each jack and its gage must (1) be calibrated as a unit with the cylinder extension in the approximate position that it will be at the final jacking force and (2) accompanied by a certified calibration chart. Each load cell must be calibrated. Calibration must be performed by an authorized laboratory.

Before starting bridge removal activities, an engineer who is registered as a civil engineer in the State must inspect and certify that (1) the temporary supports, jacking system, and displacement monitoring system comply with the authorized shop drawings and (2) the materials and workmanship are satisfactory for the work. A copy of this certification must be available at the job site at all times.

An engineer who is registered as a civil engineer in the State must:

1. Be present during jacking activities or adjustments and during bridge removal activities.
2. Inspect jacking and removal activities and report daily on the progress of the operation and the status of the remaining structure. The daily report must be available at the job site at all times.
3. Immediately submit proposed procedures to correct or remedy unplanned occurrences.

**48-3.01D(2) Displacement Monitoring**

Monitor and record vertical and horizontal displacements of the temporary supports and the existing structure. Use vandal-resistant displacement monitoring equipment. Perform monitoring continuously during jacking activities and at least weekly during removal and reconstruction activities. Make monitoring records available at the job site during normal work hours. Monitoring records must be signed by an engineer who is registered as a civil engineer in the State.

As a minimum, monitor the existing structure at the supported bent and at the midspan of both adjoining spans. Locate control points at each location near the center and at both edges of the superstructure. As a minimum, take elevations at the following times:

1. Before starting jacking activities
2. Immediately after completing jacking
3. After completing bridge removal
4. Before connecting the reconstructed or retrofitted superstructure to the substructure
5. After removing temporary supports

Perform an initial survey to record the location of the existing structure before starting work.

### **48-3.01D(3) Design Criteria**

The Engineer does not authorize temporary support designs based on allowable stresses greater than those specified in section 48-2.01D(3)(c).

If falsework loads are imposed on temporary supports, the temporary supports must also satisfy the deflection criteria in section 48-2.01D(3)(c).

The temporary support system must support the initial jacking loads and the minimum temporary support design loads and forces shown. Adjust vertical design loads for the weight of the temporary supports and jacking system, construction equipment loads, and additional loads imposed by jacking activities. Construction equipment loads must be at least 20 psf of deck surface area of the frame involved.

Temporary supports must resist the specified lateral design forces applied at the point where the column to be removed meets the superstructure. If the temporary support lateral stiffness exceeds the specified minimum stiffness, increase the lateral design forces to be compatible with the temporary support stiffness.

Place temporary supports resisting transverse lateral loads within 1/2 of the span length from the existing bent. Place temporary supports resisting longitudinal lateral loads within the frame where columns are to be removed.

You may use the permanent piles as part of the temporary support foundation. Do not move or adjust permanent piles from the locations shown. If you install permanent piles longer than described to support the temporary supports above the top of the footing and later cut off the piles at their final elevation, you must use shear devices adequate to transfer all pile reactions into the footing.

Design temporary support footings to carry the loads imposed without exceeding the estimated soil bearing values or anticipated settlements. You must determine soil bearing values.

Where temporary supports are placed on the deck of an existing structure:

1. Temporary supports must bear either (1) directly on girder stems or bent caps of the supporting structure or (2) on falsework sills that transmit the load to the stems or cap without overstressing any member of the new or existing structure.
2. Temporary supports must not induce permanent forces into the completed structure or produce cracking.
3. Place additional temporary supports beneath the existing structure where temporary support loads are imposed on the existing structure. Design and construct the additional temporary supports to support all loads from the upper structure and construction activities.

Provide additional bracing as required to withstand all imposed loads during each phase of temporary supports erection and removal. Include wind loads complying under section 48-2.01D(3)(b) in the design of additional bracing.

Mechanically connect (1) the existing structure to the temporary supports and (2) the temporary supports to their foundations. Mechanical connections must be capable of resisting the lateral design forces. Friction forces developed between the existing structure and temporary supports (1) are not considered an effective mechanical connection and (2) must not be used to reduce lateral forces.

Design mechanical connections to accommodate adjustments to the temporary support frame during use.

If the concrete is to be prestressed, design temporary supports to support changes to the loads caused by prestressing forces.

Temporary supports must comply with section 48-2.01D(3)(d).

### **48-3.02 MATERIALS**

Manufactured assemblies must comply with section 48-2.01D(3)(c)(iv).

### **48-3.03 CONSTRUCTION**

Where described, install temporary crash cushion modules under section 12-3.15 before starting temporary support activities. Remove crash cushion modules when authorized.

The construction sequence and application of temporary support jacking loads is described. You may submit proposed changes to the Engineer for authorization.

Construct temporary supports under section 48-2.03C.

Equip each jack with a pressure gage or load cell for determining the jacking force. Each pressure gage must have an accurately reading dial at least 6 inches in diameter. Each load cell must be provided with an indicator to determine the jacking force.

Provide a redundant system of supports during jacking activities. The redundant system must include stacks of steel plates added as necessary to maintain the redundant supports at each jack location within 1/4 inch of the jacking sill or corbels.

Before starting bridge removal work at a location being supported, the temporary support system must (1) apply a force to the structure that is equal to the initial jacking load or the dead load shown and (2) hold that load until all initial compression and settlement of the system is completed.

Apply jacking loads simultaneously. Control and monitor jacking operations to prevent distortion and stresses that would damage the structure. Maintain total vertical displacements at control points to less than 1/4 inch from elevations recorded before jacking or as authorized.

Stop jacking activities if unanticipated displacements, cracking, or other damage occurs. Apply corrective measures satisfactory to the Engineer before resuming jacking activities.

After reconstruction activities, the monitored control points must not deviate by more than 1/4 inch from the initial vertical survey elevations or other authorized elevations.

Remove temporary supports under section 48-2.03D. If traffic is carried on the structure on temporary supports, do not release temporary supports until the supported concrete has attained 100 percent of the specified strength.

Remove attachments from the existing structure. Restore concrete surfaces to original conditions except where permanent alterations are shown.

#### **48-3.04 PAYMENT**

Not Used

### **Replace section 48-6 with: 48-6 TEMPORARY RETAINING WALL**

#### **48-6.01 GENERAL**

Section 48-6 includes specifications for constructing a temporary retaining wall.

#### **48-6.02 SUBMITTALS**

Earthwork for the temporary retaining wall must comply with section 19

Submit shop drawings, calculations and geotechnical data for review by the Department.

Substantiate the values used in the design for the temporary retaining walls.

Shop drawings and calculations must be signed by an engineer who is registered as a civil engineer in the State of California.

Shop drawings must include a vertical debris protection barrier adjacent to the face of the wall for the length of the wall and extend from the lowest adjacent grade to a minimum of 4 feet above the wall construction at all times. The protection barrier must be designed to prevent debris and construction materials from dropping or falling on the traffic or the adjacent river below the wall.

Sufficient details for construction sequence, bracing and supports for the protection barrier must be shown on the shop drawings.

Submit four sets of shop drawings and two copies of design calculations.



1. Pile placement plan, dry and wet
2. Acceptance testing, including gamma-gamma logging, cross-hole sonic logging, and coring
3. *Pile Design Data Form*
4. Mitigation process
5. Timeline and critical path activities
6. Structural, geotechnical, and corrosion design requirements
7. Future meetings, if necessary, for pile mitigation and pile mitigation plan review
8. Safety requirements, including Cal/OSHA and Tunnel Safety Orders

**Add to section 49-3.02A(4)(d)(ii):**

If inspection pipes are not shown:

1. Include in the pile installation plan a plan view drawing of the pile showing reinforcement and inspection pipes.
2. Place inspection pipes radially around the pile, inside the outermost spiral or hoop reinforcement and no more than 1 inch clear of the outermost spiral or hoop reinforcement.
3. Place inspection pipes around the pile at a uniform spacing not exceeding 33 inches measured along the circle passing through the centers of inspection pipes. Use at least 2 inspection pipes per pile. Place inspection pipes to provide the maximum diameter circle that passes through the centers of the inspection pipes while maintaining the spacing required herein.
4. Place inspection pipes at least 3 inches clear of the vertical reinforcement. Where the vertical reinforcement configuration does not allow this clearance while achieving radial location requirements, maximize the distance to vertical rebar while still maintaining the requirement for radial location.

Where the dimensions of the pile reinforcement do not allow inspection pipes to be placed as specified above, submit a request for deviation before fabricating pile reinforcement.

**Add to section 49-3.02B(6)(c):**

The synthetic slurry must be one of the materials shown in the following table:

Material	Manufacturer
SlurryPro CDP	KB INTERNATIONAL LLC 735 BOARD ST STE 209 CHATTANOOGA TN 37402 (423) 266-6964
Super Mud	PDS CO INC 105 W SHARP ST EL DORADO AR 71731 (870) 863-5707
Shore Pac GCV	CETCO CONSTRUCTION DRILLING PRODUCTS 2870 FORBS AVE HOFFMAN ESTATES IL 60192 (800) 527-9948
Terragel or Novagel Polymer	GEO-TECH SERVICES LLC 220 N. ZAPATA HWY STE 11A-449A LAREDO TX 78043 (210) 259-6386

Use synthetic slurries in compliance with the manufacturer's instructions. Synthetic slurries shown in the above table may not be appropriate for a given job site.

Synthetic slurries must comply with the Department's requirements for synthetic slurries to be included in the above table. The requirements are available from the Offices of Structure Design, P.O. Box 168041, MS# 9-4/11G, Sacramento, CA 95816-8041.

SlurryPro CDP synthetic slurry must comply with the requirements shown in the following table:

**SLURRYPRO CDP**

Property	Test	Value
Density During drilling	Mud Weight (density), API 13B-1, section 1	≤ 67.0 pcf <sup>a</sup>
Before final cleaning and immediately before placing concrete		≤ 64.0 pcf <sup>a</sup>
Viscosity During drilling	Marsh Funnel and Cup. API 13B-1, section 2.2	50–120 sec/qt
Before final cleaning and immediately before placing concrete		≤ 70 sec/qt
pH	Glass electrode pH meter or pH paper	6.0–11.5
Sand content, percent by volume Before final cleaning and immediately before placing concrete	Sand, API 13B-1, section 5	≤ 0.5 percent

<sup>a</sup>If authorized, you may use slurry in salt water. The allowable density of slurry in salt water may be increased by 2 pcf.

Slurry temperature must be at least 40 degrees F when tested.

Super Mud synthetic slurry must comply with the requirements shown in the following table:

**SUPER MUD**

Property	Test	Value
Density During drilling	Mud Weight (Density), API 13B-1, section 1	≤ 64.0 pcf <sup>a</sup>
Before final cleaning and immediately before placing concrete		≤ 64.0 pcf <sup>a</sup>
Viscosity During drilling	Marsh Funnel and Cup. API 13B-1, section 2.2	32–60 sec/qt
Before final cleaning and immediately before placing concrete		≤ 60 sec/qt
pH	Glass electrode pH meter or pH paper	8.0–10.0
Sand content, percent by volume Before final cleaning and immediately before placing concrete	Sand, API 13B-1, section 5	≤ 0.5 percent

<sup>a</sup>If authorized, you may use slurry in salt water. The allowable density of slurry in salt water may be increased by 2 pcf.

Slurry temperature must be at least 40 degrees F when tested.

Shore Pac GCV synthetic slurry must comply with the requirements shown in the following table:

**SHORE PAC GCV**

Property	Test	Value
Density During drilling	Mud Weight (Density), API 13B-1, section 1	≤ 64.0 pcf <sup>a</sup>
Before final cleaning and immediately before placing concrete		≤ 64.0 pcf <sup>a</sup>
Viscosity During drilling	Marsh Funnel and Cup. API 13B-1, section 2.2	33–74 sec/qt
Before final cleaning and immediately before placing concrete		≤ 57 sec/qt
pH	Glass electrode pH meter or pH paper	8.0–11.0
Sand content, percent by volume Before final cleaning and immediately before placing concrete	Sand, API 13B-1, section 5	≤ 0.5 percent

<sup>a</sup>If authorized, you may use slurry in salt water. The allowable density of slurry in salt water may be increased by 2 pcf.

Slurry temperature must be at least 40 degrees F when tested.

Terragel or Novagel Polymer synthetic slurry must comply with the requirements shown in the following table:

**TERRAGEL OR NOVAGEL POLYMER**

Property	Test	Value
Density During drilling	Mud Weight (Density), API 13B-1, section 1	≤ 67.0 pcf <sup>a</sup>
Before final cleaning and immediately before placing concrete		≤ 64.0 pcf <sup>a</sup>
Viscosity During drilling	Marsh Funnel and Cup. API 13B-1, section 2.2	45–104 sec/qt
Before final cleaning and immediately before placing concrete		≤ 104 sec/qt
pH	Glass electrode pH meter or pH paper	6.0–11.5
Sand content, percent by volume Before final cleaning and immediately before placing concrete	Sand, API 13B-1, section 5	≤ 0.5 percent

<sup>a</sup>If authorized, you may use slurry in salt water. The allowable density of slurry in salt water may be increased by 2 pcf.

Slurry temperature must be at least 40 degrees F when tested.

**Replace the 9<sup>th</sup> paragraph of section 49-3.02C(2) with:**

Dispose of drill cuttings and materials cleaned from the bottom of the hole.

At Kelly Creek Barrier, disposal of drill cuttings and materials cleaned from the bottom of the hole must comply with section 14-11.

**Replace the 16<sup>th</sup> paragraph of section 49-3.02C(8) with:**

Dispose of material resulting from using slurry.

At Kelly Creek Barrier, disposal of material resulting from using slurry must comply with section 14-11.

**Replace "Reserved" in section 49-5 with:**

**49-5.01 GENERAL**

**49-5.01A Summary**

Section 49-5 includes specifications for constructing micropiles.

HS thread bars must comply with the specifications for HS steel prestressing bars in section 50.

Bar reinforcing steel must comply with section 52, except you may use deformed bar reinforcing steel that complies with ASTM A 615/A 615M, Grade 60.

**49-5.01B Definitions**

**micropile:** Small-diameter, bored, CIP composite pile, in which the applied load is resisted by steel reinforcing elements, grout, and frictional ground-grout bond.

**steel reinforcing element:** Steel element used to strengthen or stiffen a micropile, such as bar reinforcing steel, HS thread bar, pipe, hollow structural section (HSS), or casing.

**49-5.01C Submittals**

**49-5.01C(1) General**

Do not order materials nor install micropiles until the experience qualifications, shop drawings and calculations, and installation plan are authorized.

**49-5.01C(2) Experience Qualifications**

Submit the following experience qualification information:

1. Summary of the micropile subcontractor's experience that demonstrates compliance with section 49-5.01D(2).
2. Construction details and structural details from at least 3 completed micropile installations performed by the micropile subcontractor in the last 5 years. The installations must be from 3 separate projects of similar scope to this Contract. Include a project description and the owner's name and current phone number.
3. List of drill rig operators and on-site foremen who will perform the micropile work and a summary of each individual's experience that demonstrates compliance with section 49-5.01D(2).

Allow 10 days for the review.

**49-5.01C(3) Shop Drawings and Calculations**

Submit 5 copies of micropile shop drawings and calculations to OSD, Documents Unit. Notify the Engineer of the submittal. Include in the notification the date and contents of the submittal.

Allow 30 days for the review. After the review, submit from 6 to 12 copies, as requested, for final authorization and use during construction. Within 20 days after final authorization, submit 1 copy of final shop drawings and calculations.

The shop drawings and calculations must be sealed and signed by an engineer who is registered as a civil engineer in the State.

The shop drawings and calculations must include:

1. Name, address, and phone number of the micropile subcontractor
2. Plan view, including:
  - 2.1. Station and offset at each skewback footing
  - 2.2. Location of any existing utilities, adjacent existing structures, and other potential interferences
  - 2.3. Micropile layout and spacing
  - 2.4. Unique identification number for each micropile

3. Typical sections, including:
  - 3.1. Micropile inclination
  - 3.2. Drilled hole diameter
  - 3.3. Micropile length
  - 3.4. Micropile cutoff location
  - 3.5. Steel reinforcing element details, including sizes and lengths
  - 3.6. Centralizers and any spacers
  - 3.7. Micropile anchorage details
4. Material properties
5. General notes for constructing the micropiles, including overall construction sequencing

If you propose alternative steel reinforcing elements, include with the shop drawings and calculations:

1. Calculations for the structural capacity of the micropile with the alternative steel reinforcing elements
2. Calculations for the structural capacity of the micropile shown
3. Details and calculations for any anchorage changes needed to accommodate the alternative steel reinforcing elements

#### **49-5.01C(4) Installation Plan**

Submit a micropile installation plan, including:

1. Detailed construction procedures, including personnel, materials, testing, and equipment.
2. Layout drawing showing the micropile installation sequence.
3. Drilling or coring methods and equipment, including methods to provide drilled hole support, to drill a straight hole, to advance through boulders and other obstructions, and to prevent detrimental ground movements.
4. Methods for placing, positioning, and supporting steel reinforcing elements.
5. Grouting plan, including:
  - 5.1. Grout mix design. Include test results from an authorized laboratory for the compressive strength of the mix at 3, 7, 14, and 28 days and the density of the mix.
  - 5.2. Procedures for monitoring grout quality.
  - 5.3. Placement procedures and equipment.
  - 5.4. Methods and equipment for monitoring and recording grout depth and volume as the grout is placed.
  - 5.5. Grouting rate calculations, if requested. Base the calculations on the static head on the grout and losses throughout the placing system, including anticipated head of drilling fluid to be displaced, if applicable.
6. Plan for the control and disposal of surface and ground water, drill flush, and excess waste grout.

The installation plan must be sealed and signed by an engineer who is registered as a civil engineer in the State.

Allow 20 days for the review.

#### **49-5.01C(5) Certificates of Compliance**

Submit certificates of compliance for all steel materials.

#### **49-5.01C(6) Mill Test Reports**

Submit certified mill test reports for each heat number of each type of steel reinforcing element at least 7 days before using the materials in the work. The certified mill test reports must include ultimate strength, yield strength, elongation, and chemical composition.

#### **49-5.01C(7) Installation Log**

Submit each installation log as an informational submittal within 1 business day of the micropile installation.

#### **49-5.01C(8) Grout Test Results**

Submit grout test results for density, efflux time, and compressive strength within 1 business day of testing.

**49-5.01C(9) Load Test Data**

Not Used

**49-5.01D Quality Control and Assurance****49-5.01D(1) General**

Not Used

**49-5.01D(2) Experience Qualifications**

The micropile subcontractor must:

1. Be experienced in micropile construction
2. Have successfully constructed at least 5 projects in the last 5 years involving a combined total of at least 100 micropiles
3. Have previous micropile drilling and grouting experience in soil or rock similar to that for this Contract

The on-site foremen and drill rig operators must have experience installing micropiles on at least 3 projects completed in the last 5 years.

**49-5.01D(3) Preconstruction Meeting**

Schedule and hold a micropile preconstruction meeting at least 5 business days after submitting the micropile shop drawings and installation plan and at least 10 days before starting micropile construction. You must provide a facility for the meeting.

The meeting must include the Engineer, your representatives, representatives from the micropile subcontractor, and representatives from any other subcontractor to be involved in the micropile construction.

The Engineer conducts the meeting. Be prepared to discuss:

1. Contractual relationships and delineation of responsibilities among you and the subcontractors
2. Contacts and communication protocol between you and your representatives, the subcontractors, and the Engineer
3. Coordination of the construction schedule and activities
4. Anticipated subsurface conditions
5. Structural, geotechnical, and construction requirements
6. Materials testing
7. Load testing

**49-5.01D(4) Grout Testing****49-5.01D(4)(a) General**

Not Used

**49-5.01D(4)(b) Density**

Before placing grout into each micropile, test the density under API RP 13B-1 using the Baroid mud balance. Take the grout test samples directly from the grout plant.

**49-5.01D(4)(c) Efflux Time**

Before placing grout into each micropile, test the efflux time under California Test 541. Take the grout test samples at the point of placement.

**49-5.01D(4)(d) Compressive Strength**

Test the grout compressive strength under ASTM C 109/C 109M at an authorized laboratory.

Test at least 1 set of three 2-inch grout cubes from each grout plant each day of operation or for every 10 micropiles installed, whichever occurs more frequently. Take the grout test samples directly from the grout plant.

**49-5.01D(5) Load Testing****49-5.01D(5)(a) General**

Section 49-1.01D does not apply to micropile load testing.

Grout for load test micropiles must attain the compressive strength shown before load testing is performed.

The Department furnishes the load test beam, jacks, load cells, and instrumentation for load testing.

Furnish the labor, materials, tools, equipment, and incidentals as required to assist the Department in the installation, operation, and removal of the Department-furnished load test equipment. This is change order work.

The Department's load cell is the primary load measurement system.

**49-5.01D(5)(b) Verification Load Testing**

**49-5.01D(5)(b)(i) General**

Perform verification load testing on each verification test micropile installed.

The verification test micropiles at the listed locations represent the production micropiles at the support locations shown in the following table:

Bridge no.	Verification test micropile location	Support locations
01-0080	Adjacent to Skewback 6L	Piers 6 and 11

The Department performs verification load testing in tension.

If the total axial movement at the top of the pile measured relative to the top of the pile before the start of verification load testing exceeds 0.50 inch at the 1st test load applied that is equal to 1.00 times the tensile nominal resistance shown, the Department increases the length of the production micropiles and steel reinforcing elements. Lengthening the production micropiles and steel reinforcing elements is change order work.

After requesting testing and providing access to the verification test micropile and anchor piles, allow 20 days for the Department to perform the testing, provide the test report, notify you of any changes to the production micropiles, and authorize starting production micropile construction.

**49-5.01D(5)(b)(ii) Verification Load Test Procedure**

Not Used

**49-5.01D(5)(b)(iii) Verification Load Test Acceptance**

Not Used

**49-5.01D(5)(c) Proof Load Testing**

**49-5.01D(5)(c)(i) General**

The Department performs proof load testing in compression on 1 micropile at each of the 4 skewback footings. The Engineer selects each micropile to be proof load tested. The Engineer does not notify you of which micropile is to be proof load tested until after the micropile has been installed.

The micropile proof load test at each skewback footing must take place after at least 25 percent of the micropiles at the footing have been installed.

After requesting testing and providing access to the proof test micropile and anchor piles, allow 20 days for the Department to perform the testing and provide the test report.

**49-5.01D(5)(c)(ii) Proof Load Test Procedure**

Not Used

**49-5.01D(5)(c)(iii) Proof Load Test Acceptance**

Each proof load test must comply with the following acceptance criteria:

1. Total axial movement at the top of the pile measured relative to the top of the pile before the start of load testing must not exceed 0.50 inch at the 1st compression test load applied that is equal to 0.75 times the nominal resistance shown.
2. Slope of the applied test load versus the top of micropile movement must not exceed 0.025 inch per kip at the maximum test load.

If a proof load test fails to comply with the acceptance criteria, the micropile is rejected and you must suspend micropile construction and comply with one of the following procedures:

1. Post-grout the rejected micropile. If the post-grouted micropile complies with the acceptance criteria when the Department retests the rejected micropile, post-grout all of the micropiles in the footing using identical methods.
2. Request that the Department proof load test 2 additional micropiles selected by the Engineer in the same footing. After requesting testing and providing access to the proof test micropiles and anchor piles, allow 20 days per proof test micropile for the Department to perform the testing and provide the test report. Submit a plan for replacing rejected micropiles or for installing additional micropiles, including details for any micropile or footing changes required to provide the total micropiling support capacity shown. Suspend micropile construction until the plan is authorized.

The Department deducts the cost of any proof load testing performed as a result of a rejected proof-tested micropile.

## **49-5.02 MATERIALS**

### **49-5.02A General**

Welding must comply with AWS D1.1.

### **49-5.02B Steel Reinforcing Elements**

#### **49-5.02B(1) General**

Do not order steel reinforcing elements for production micropiles until the start of production micropile construction is authorized.

Mill secondary steel reinforcing elements must not be used.

Steel reinforcing elements must not contain splices or joints.

You may use alternative steel reinforcing elements if:

1. Structural capacity of the micropile with the alternative steel reinforcing elements is greater than or equal to the structural capacity of the micropile shown
2. Alternative elements comply with the specifications for steel reinforcing elements
3. Drilled hole diameter is adjusted to provide at least the same grout cover as the micropile section shown and does not exceed the smaller of 13 inches or 1/3 of the micropile center-to-center spacing

Alternative steel reinforcing elements must consist of pipe, round HSS, casing, bar reinforcing steel, HS thread bars, or a combination of these. You may use pipe, HSS, or casing complying with one of the following specifications or another authorized specification:

1. ASTM A 53/A 53M, Type E or S, Grade B
2. ASTM A 500/A 500M
3. ASTM A 501
4. ASTM A 618/A 618M
5. ASTM A 106/A 106M, Grade B
6. API N80
7. API P110
8. API 5L, minimum Grade B, PSL1

#### **49-5.02B(2) Pipe, Hollow Structural Sections, and Casing**

Casing must comply with API N80.

Pipe, HSS, and casing to be welded for structural purposes must have a carbon equivalency as defined in AWS D1.1, Annex I5.1, not exceeding 0.45 percent and a sulfur content not exceeding 0.05 percent.

Welded seams must be CJP welds.

#### **49-5.02B(3) Bar Reinforcing Steel**

For anchorages that require threading nuts and plates onto bar reinforcing, you may cut threads into the bar reinforcing steel if you provide the next larger bar number designation from that shown.

#### **49-5.02C Anchorage Components**

Steel plates for the micropile anchorage must comply with ASTM A 709/A 709M, Grade 50, or ASTM A 572/A 572M, Grade 50.

Nuts and washers for the micropile anchorage must be capable of holding the bar at a load producing a tensile stress of at least the specified minimum ultimate tensile strength of the bar.

#### **49-5.02D Grout**

Grout must be a stable, neat grout consisting of cement and water. Cement must comply with section 90-1.02B(2). Water must comply with section 90-1.02D.

If authorized, you may use an admixture in the grout. The admixture must comply with sections 90-1.01C(4), 90-1.01D(1), and 90-1.02E, except the admixture:

1. Must not contain chloride ions in excess of 0.25 percent by weight
2. May be dispensed in solid form
3. Must not be an accelerating admixture

If authorized, you may add fine aggregate to the grout. Fine aggregate must comply with section 90-1.02C(3). Grout with fine aggregate must:

1. Have a slump of at least 7 inches when measured under ASTM C 143/C 143M
2. Have an air content of no more than 2 percent when measured under California Test 504
3. Not contain air-entraining admixtures

Grout must comply with the following requirements:

1. Density must be greater than or equal to the density submitted with the authorized mix design.
2. Efflux time must be at least 11 seconds.
3. Compressive strength must be at least that shown at 28 days.

#### **49-5.02E Centralizers and Spacers**

Centralizers and spacers must be fabricated from plastic, steel, or other material that is not detrimental to the steel reinforcing elements. Do not use wood centralizers or spacers.

Centralizers and spacers must be strong enough to support the steel reinforcing elements during construction activities.

#### **49-5.02F Corrosion Protection**

Not Used

### **49-5.03 CONSTRUCTION**

#### **49-5.03A General**

Do not construct any production micropiles until authorized.

Do not drill a micropile that is within a center-to-center spacing of 5 feet from an open micropile hole or a micropile in which the initial grout has set for less than 12 hours.

#### **49-5.03B Verification Test Micropiles**

Install a verification test micropile and anchor piles at the location shown. Notify the Engineer at least 7 days before installing the verification test micropile and anchor piles.

Excavate the verification load test site and keep the test site free of water throughout the testing.

Construct the verification test micropile in the Engineer's presence, using similar equipment, methods, and inclination as those to be used for the production micropiles it represents.

When authorized, remove the verification test micropile and anchor piles as specified for removing portions of bridges in section 15-4.

#### **49-5.03C Proof Test Micropiles**

Throughout proof load testing, maintain the test site in a condition that is free of water.

The anchor piles for proof testing must be identical to the production micropiles.

Install 4 anchor piles to be used in performing the proof tests. The anchor pile locations are adjacent to the skewback footings and are selected by the Engineer. For each proof load test, the Engineer notifies you of the location of any required anchor piles at the same time as the Engineer notifies you of which micropile is to be proof load tested. Perform any excavation necessary to install the anchor piles.

If a proof-tested micropile is rejected and you choose the procedure in item 2 in the list in the 2nd paragraph of section 49-5.01D(5)(c)(iii), perform any excavation and install any anchor piles required for the Department to perform the additional proof load testing. Perform all work required in the authorized plan.

The Department does not pay for work resulting from a rejected proof-tested micropile.

#### **49-5.03D Drilling**

Use temporary casing or another authorized drilled hole support method in caving or unstable ground.

Each drilled hole must comply with the following tolerances:

1. Centerline of the drilled hole must not deviate from the micropile location shown by more than 3 inches.
2. Center-to-center spacing of the drilled holes must not deviate from the micropile spacing shown by more than 3 inches.
3. Axis of the drilled hole must not deviate from the alignment shown by more than 1-1/2 inches per 10 feet of length.

Select drilling equipment and methods that are suitable for drilling through the conditions to be encountered without causing damage to any overlying or adjacent structure or service and without causing detrimental ground movements.

The drilled hole must be open along its full length to the hole diameter shown before placing grout or any steel reinforcing elements not used to case the drilled hole.

Remove any material dislodged or drawn into the hole during micropile construction. Immediately before placing reinforcing steel elements or grout, remove any loose material at the bottom of the hole.

Dispose of drill cuttings under section 19-2.03B.

#### **49-5.03E Placing Steel Reinforcing Elements**

Place the steel reinforcing elements before withdrawing any temporary casing.

Use spacers to separate steel reinforcing elements if more than 1 type of steel reinforcing element is used. Place the spacers at 10-foot maximum intervals.

Use centralizers to support the steel reinforcing elements in the center of the hole and to provide at least the specified grout cover. Place the centralizers at 10-foot maximum intervals, with the uppermost centralizer a maximum of 5 feet from the top of the micropile and the lowermost centralizer from 2 to 5 feet from the bottom of the micropile.

Centralizers and spacers must (1) be attached to the steel reinforcing elements securely enough to withstand installation stresses and (2) allow the free flow of grout without misalignment of the steel reinforcing elements.

Before inserting each steel reinforcing element into a drilled hole, clean the surface of the element of deleterious substances, such as soil, mud, grease, and oil.

If you cannot insert a steel reinforcing element into the drilled hole to the required depth without difficulty, remove the reinforcing element, clean any grout from the surface of the reinforcing element, clean or redrill the hole, and reinsert the reinforcing element. Do not force or drive a reinforcing element into a drilled hole. Micropiles with partially inserted steel reinforcing elements are rejected.

#### **49-5.03F Grouting**

Grout each micropile the same day the hole is drilled.

You may place the grout before or after placing the steel reinforcing elements.

Continuously agitate the grout until it is placed. Place the grout within 1 hour of mixing.

Inject the grout at the lowest point of the drilled hole. Continue the injection until uncontaminated grout flows from the top of the micropile.

Grout each micropile in 1 continuous operation. Use grouting procedures that ensure complete continuity of the grout column. The grout must completely encase the centralized micropile casing such that no annular space remains between the outer surface of the casing and the drilled-hole sidewalls.

If temporary casing is used, remove the casing before or during grout placement. If you remove the casing during grout placement, extract the casing in stages. After you remove each length of casing, bring the grout level back up to ground level before removing the next length of casing. Maintain the grout at a level above the bottom of the temporary casing adequate to prevent displacement of the grout by material from outside the casing. The tremie pipe or casing must extend at least 10 feet below the grout level in the drilled hole at all times during grout placement.

Grout tubes may remain in the hole after the completion of grouting, but must be filled with grout.

Maintain the grout level at or above the micropile cutoff elevation until the grout has set.

Provide a positive means of support for maintaining the position of the steel reinforcing elements until the grout has set.

Load test micropiles must remain undisturbed until the grout is strong enough to provide anchorage during load testing.

Dispose of material resulting from grouting.

#### **49-5.03G Ground Heave and Subsidence**

If you observe signs of ground heave or subsidence, immediately notify the Engineer and suspend the drilling and grouting operations. If the Engineer determines that the movements require corrective action, take the actions necessary to stop the movement or perform repairs.

#### **49-5.03H Installation Log**

Prepare a separate installation log for each micropile.

The installation log must include:

1. Micropile identification number and location
2. Superintendent, drill rig operator, grout plant operator, and any other personnel involved in the micropile installation
3. Date, time, and duration of drilling, steel reinforcing element installation, and grout placement
4. Drilling or coring method and speed
5. Details of any hole stabilization method used
6. Description of soil or rock encountered
7. Quantity of ground water encountered
8. Description of any unusual installation behavior or conditions
9. Drilled hole diameter
10. Micropile length
11. Log of grout quantities, including the time and micropile depth

12. Lengths of steel reinforcing elements

**49-5.04 PAYMENT**

Section 49-1.04 does not apply.

Anchor piles, except those installed as a result of a rejected proof-tested micropile, and verification test micropiles are paid for as micropiles.

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**51 CONCRETE STRUCTURES**

**Add to section 51-1.01A:**

The portions of the bridges shown in the following table must be constructed of mass concrete under section 51-6:

Bridge name and number	Portions of bridge
Middle Fork Smith River Bridge - 01-0080	Skewbacks 6R, 6L, 11L, and 11R

**Add to the list in the 6th paragraph of the RSS for section 51-1.01A:**

**Add to section 51-1.02B:**

For the portions of structures shown in the following table, concrete must contain at least 675 pounds of cementitious material per cubic yard:

Bridge name	Portion of structure
Middle Fork Smith River Bridge (Replace)	All except bridge footings and CIDH piles
North Retaining Wall	All except retaining wall footings
Monkey Creek Retaining Wall	All except concrete backfill and lean concrete backfill
Kelly Creek Barrier	All except CIDH piles

**Add to section 51-1.03G(1):**

The random rock concrete surface texture must match the texture and pattern of the referee sample available for inspection by bidders at 1801 30th Street, Sacramento, CA 95816. Call 916-227-8005 to make an appointment for viewing.

Form liners are not required for the wood concrete surface texture on concrete barriers.

Random rock concrete texture must resemble rough surfaced randomly stacked fieldstones as shown. Rock pattern must vary in shape. Widths must vary between 2.5 to 18 inches and heights must vary between 2 to 12 inches. Relief must be between 1/4 to 1-1/2 inches. The texture must have random shadow patterns. The texture must not have repetitive secondary shadow patterns. The form liner pattern must match adjoining form liner patterns and appear seamless with no visible horizontal or vertical seams in the pattern.

The wood beam texture on concrete barriers must be an architectural texture simulating the appearance of a continuous timber beam with random shadow patterns. Surfaces between imprints that do not exhibit the wood concrete texture must be textured with a suitable tool.



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## DIVISION VIII MISCELLANEOUS CONSTRUCTION

### 72 SLOPE PROTECTION

Replace section 72-7 with:

#### 72-7 ROCKFALL PROTECTION SYSTEMS

##### 72-7.01 DRAPERY SYSTEMS

##### 72-7.01A DOUBLE TWISTED WIRE MESH DRAPERY SYSTEM

##### 72-7.01A(1) GENERAL

##### 72-7.01A(1)(a) Summary

Section 72-7.01(A) includes general specifications for fabricating and constructing double twisted wire mesh drapery system (DTWM) as shown.

Double twisted wire mesh drapery system includes drilled and grouted ground anchor assemblies, double twisted wire mesh fabric, cables and hardware.

In undisturbed areas to be covered with DTWM, clear objectionable materials under section 16-1.03B, except leave grass intact and prune brush to 2 inches above the ground.

In the area shown for DTWM, begin installing the system immediately after completing slope excavation.

##### 72-7.01A(1)(b) Submittals

##### 72-7.01A(b)(i) General

Submit a certified calibration chart for each jack and its gage as an informational submittal. Calibrate each jack and pressure gage as a unit, with the cylinder extension in the approximate position it will be at the final jacking force. Each jack and pressure gage unit must be calibrated within one year of being used on the project.

Submit a sample of double twisted wire mesh fabric to the Engineer. Do not order double twisted wire mesh fabric until the Engineer authorizes the color.

##### 72-7.01A(b)(ii) Test Data

Submit the following data for each ground anchor test:

1. Key personnel
2. Test loading equipment
3. Ground anchor location
4. Time and date of:
  - 4.1. Drilling
  - 4.2. Installation
  - 4.3. Grouting
  - 4.4. Testing
5. Hole diameter and depth
6. Drilling method
7. Soil or rock classification and description

Submit the test data when ground anchor testing is complete or when requested.

##### 72-7.01A(1)(c) Quality Control and Assurance

Submit certificates of compliance for materials used.

##### 72-7.01A(1)(c)(i) Load Testing

The minimum allowable design load for grouted ground anchor assemblies is 2 tons.

Proof test 20 percent of the grouted ground anchor assemblies to at least 1.1 times the minimum allowable design load. Anchors failing before reaching the allowable design load will be replaced and retested at your expense. The Engineer will determine which ground anchors to proof test.

Jacking equipment and the movement measuring system must be stable during all phases of loading.

No part of the yoke or load frame can bear within 3 feet of the anchor being tested.

Perform pullout testing in the presence of the Engineer.

Conduct pullout testing by measuring the test load applied to the anchor and the anchor end movement at each load.

Pullout test ground anchor assemblies by loading the anchor to the maximum test load or to failure, whichever occurs first. Failure is the point where the movement of the anchor continues without an increase in load or when the anchor has displaced 2 inches. Record the failure load corresponding to the failure load point as part of the test data.

During the pullout test, monitor and record displacement of the anchors relative to a stable reference point which is founded at least 3 feet from the anchor and test load reaction points.

Do not unload or reposition the test equipment during load testing.

#### **72-7.01A(1)(c)(ii) Jacking Equipment and Calibration**

Apply the test loads using a hydraulic jack supported by a reaction frame that can support the test equipment without excessive deformation.

Use a calibrated pressure gage or a load cell to determine the magnitude of applied test loads.

The pressure gage must have an accurately reading, clearly visible dial or display. Dial gages must be graduated in 100 psi increments or less.

Calibrate each jack and its gage as a unit, with the cylinder extension in the approximate position it will have at the final jacking force. Calibrate the jack and gage assembly under the specifications for jacks used to stress tendons permanently anchored at 25 percent or more of the specified ultimate tensile strength in section 50-1.01D(3).

The load cell must be calibrated and have an indicator capable of measuring the maximum test load. The load cell range must be such that the lower 10 percent of the manufacturer's rated capacity is not used in determining the jacking force.

#### **72-7.01A(1)(c)(iii) Grout**

Before using grout with fine aggregate, produce a test batch to verify the consistency. Produce and deliver the test batch under conditions and in time periods similar to those expected during the grouting of ground anchors. Dispose of the grout after testing.

#### **72-7.01A(2) MATERIALS**

##### **72-7.01A(2)(a) General**

Comply with ASTM A 36 for steel components.

Comply with ASTM A 325 for bolts, nuts and washers.

Galvanize steel materials including double twisted wire mesh, except wire ropes and cables, under section 75-1.05.

Galvanize wire ropes and cables under Federal Specification RR W 410D.

Top support cable must be 5/8-inch wire rope.

Tag lines must be 5/8-inch wire rope.

#### **Ground Anchor Assemblies**

You may choose either:

1. 3/4-inch diameter steel cable anchors with 3-inch diameter centralizers
2. 1-inch threaded hollow steel bar anchors with plain coupling

**Grout**

Comply with section 46-3.02E.

**Double Twisted Wire Mesh**

**Material**

Wire must comply with the following:

Property	Test Method	Specification
Tensile strength (psi) <sup>a</sup>	ASTM A 370	60,000 min

<sup>a</sup>Tensile area includes galvanization.

Double twisted wire mesh must form a uniform hexagonal pattern using a non-raveling twist. The major axis of any opening must not exceed 4.7 inches. Hexagonal openings, approximately 3.25 inches by 4.5 inches, must not exceed 11.4 square inches. Wire mesh must be continuous. Gaps at edges or joins must be smaller than the specified hexagonal openings. Use selvage wire on both edges of double twisted wire mesh panels.

Double twisted wire mesh drapery panels must be a minimum of 12 feet wide.

Double twisted wire mesh skirt panels must be 6 feet wide.

The allowable alternatives for double twisted wire mesh must consist of one of the following suppliers or a Department-authorized equal:

Maccaferri Gabions  
 3650 Seaport Boulevard  
 West Sacramento, CA 95691  
 (800) 328-5805  
 Website: <http://www.maccaferri-northamerica.com/>

Reed and Graham (Terra Aqua Gabions)  
 26 Sky Light Court  
 Sacramento, CA 95828  
 (916) 381-9900  
 Website: [www.rginc.com/geo](http://www.rginc.com/geo)

Chama Valley Production Company  
 State Road 95 #265  
 HC75 Box 1317  
 Rutherton, MN 87551  
 Phone: (575) 588-0332  
 Fax: (575) 588-0336  
 Email: [chamaproducts@gmail.com](mailto:chamaproducts@gmail.com)

You may submit a request for an alternative supplier under section 6-3.02.

**Coating**

Galvanize double twisted wire mesh panels before coating.

Coat double twisted wire mesh with brown polyvinyl chloride (PVC) coating conforming to Federal Standard No. 595B, Federal Color No. FS 31090. Apply PVC coating to minimum 0.038 millimeter film thickness.

**Wire Mesh Fasteners**

Fasten double twisted wire mesh folds and panels together using:

1. Ring fasteners,
2. Twist ties, or
3. 5/16-inch wire rope for lacing

Fasteners must be as strong as or stronger than the wire used in double twisted wire mesh fabric.

### **Miscellaneous Hardware**

All miscellaneous hardware used in double twisted wire mesh drapery system must conform to section 75.

### **72-7.01A(3) CONSTRUCTION**

#### **72-7.01A(3)(a) General**

Install ground anchors and top support cable a minimum of 10 feet beyond the top of new and existing cut slopes as shown.

Helicopters are not allowed for installation of DTWM.

Water or grout from ground anchor or construction must not:

1. Fall on traffic
2. Flow across shoulders or lanes occupied by traffic
3. Flow into vegetation, gutters, or other drainage facilities

Do not use an excessive quantity of water when drilling and installing ground anchors.

#### **72-7.01A(3)(b) Slopes and Foundation Conditions**

Difficult drilling and anchor installation is anticipated due to the presence of weathered and fractured rock and sandy gravel with cobbles.

The area above the slopes to be covered by DTWM is not accessible by standard motorized equipment.

An access road is not available. Construction of a temporary roadway to access top of cut areas is not allowed. You must install DTWM from the bottom of the slopes.

Slopes being draped generally are oriented 53 to 80 degrees from the horizontal, with small sections of the slope being as steep as vertical. Slopes being draped have vertical dimensions (bottom of catchment to hinge point at top of slope) ranging from about 45 to 55 feet, and slope lengths as long as 65 feet. These slopes are composed of weathered and fractured rock and sandy gravel with cobbles.

Anchors are installed in rock or soil as shown.

#### **72-7.01A(3)(c) Excavation and Drilling**

Drill minimum 3-inch diameter ground anchor holes to the depth shown.

Spread excavated material uniformly around the vicinity of the anchors.

Space drilled holes as shown, approximately 40 feet apart. Adjust spacing to conform to local topography.

Verify and record the length of each drilled hole before grouting.

#### **72-7.01A(3)(d) Ground Anchor Assembly Installation**

Install and grout ground anchors in the same work shift that the holes are drilled.

Fray the end of each cable anchor as shown.

Use centralizers to support cable anchors in the center of drilled holes. Space centralizers at 2.0 feet maximum along the cable length and 0.5 feet from the cable end.

Verify and record the drilled hole length before grouting anchors.

Attach each ground anchor assembly to the top support cable. Use tag lines if necessary.





**78-2.01C Quality Control and Assurance**

**78-2.01C(1) General**

Not Used

**78-2.01C(2) Stone Veneer**

Obtain manufactured stone veneers from a single source and from a single manufacturer.

**78-2.01C(3) Test Panels**

Test panels must be:

1. Constructed at an authorized location
2. At least 4 by 4 feet by 5 inches deep
3. Constructed and finished using the personnel, materials, equipment, and methods to be used in the work
4. Authorized before starting work

The Engineer may request that additional test panels be constructed until the specified patterns and colors are attained.

The Engineer uses the authorized test panel to determine acceptability of the work.

**78-2.02 Materials**

**78-2.02A General**

Not Used

**78-2.02B Stone Veneer**

Stone veneer must be manufactured from light weight concrete under manufacturer's recommendations.

**78-2.02C Mortar**

Mortar must comply with the stone veneer manufacturer's recommendations.

Mortar must match color no. 36492 of FED-STD-595 unless authorized.

**78-2.03 Construction**

**78-2.03A General**

Install stone veneers under manufacturer's recommendations.

Remove splashes, stains, and spots from exposed faces of the stone veneer.

**78-2.04 Payment**

Not Used

**Replace Reserved in section 78-3 of the RSS with:**

**78-3 BAT HABITATS**

**78-3.01 GENERAL**

**78-3.01A Summary**

Section 78-3 includes specifications for constructing and placing bat habitats.

**78-3.01B Submittals**

**78-3.01B(1) General**

Submit fiber reinforcement manufacturer's product data including application rate and mixing instructions.

**78-3.01C Quality Control and Assurance**

Not Used



1. TYPE SKT TERMINAL SYSTEM - Type SKT terminal system must be a SKT 350 sequential kinking terminal manufactured by Road Systems, Inc., located in Big Spring, Texas, and must include items detailed for Type SKT terminal system shown on the plans. The SKT 350 sequential kinking terminal can be obtained from the distributor, Universal Industrial Sales, P.O. Box 699, Pleasant Grove, UT 84062, telephone (801) 785-0505 or from the distributor, Gregory Highway Products, 4100 13th Street, S.W., Canton, OH 44708, telephone (330) 477-4800.
2. TYPE ET TERMINAL SYSTEM - Type ET terminal system must be an ET-2000 PLUS (4-tube system) extruder terminal as manufactured by Trinity Highway Products, LLC, and must include items detailed for Type ET terminal system shown on the plans. The ET-2000 PLUS (4-tube system) extruder terminal can be obtained from the manufacturer, Trinity Highway Products, LLC, P.O. Box 99, Centerville, UT 84012, telephone (800) 772-7976.

Submit a certificate of compliance for terminal systems.

Terminal systems must be installed under the manufacturer's installation instructions and these specifications. Each terminal system installed must be identified by painting the type of terminal system in neat black letters and figures 2 inches high on the backside of the rail element between system posts numbers 4 and 5.

For Type ET terminal system, the steel foundation tubes with soil plates attached must be, at the Contractor's option, either driven, with or without pilot holes, or placed in drilled holes. Space around the steel foundation tubes must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. The wood terminal posts must be inserted into the steel foundation tubes by hand and must not be driven. Before the wood terminal posts are inserted, the inside surfaces of the steel foundation tubes to receive the wood posts must be coated with a grease that will not melt or run at a temperature of 149 degrees F or less. The edges of the wood terminal posts may be slightly rounded to facilitate insertion of the post into the steel foundation tubes.

For Type SKT terminal system, the soil tubes must be, at the Contractor's option, driven with or without pilot holes, or placed in drilled holes. Space around the steel foundation tubes must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. Wood posts must be inserted into the steel foundation tubes by hand. Before the wood terminal posts are inserted, the inside surfaces of the steel foundation tubes to receive the wood posts must be coated with a grease that will not melt or run at a temperature of 149 degrees F or less. The edges of the wood posts may be slightly rounded to facilitate insertion of the post into the steel foundation tubes.

After installing the terminal system, dispose of surplus excavated material in a uniform manner along the adjacent roadway where designated by the Engineer.

**Replace section 83-1.02C(3) with:**

**83-1.02C(3) Alternative Flared Terminal System**

Alternative flared terminal system must be furnished and installed as shown on the plans and under these special provisions.

The allowable alternatives for a flared terminal system must consist of one of the following or a Department-authorized equal.

1. TYPE FLEAT TERMINAL SYSTEM - Type FLEAT terminal system must be a Flared Energy Absorbing Terminal 350 manufactured by Road Systems, Inc., located in Big Spring, Texas, and must include items detailed for Type FLEAT terminal system shown on the plans. The Flared Energy Absorbing Terminal 350 can be obtained from the distributor, Universal Industrial Sales, P.O. Box 699, Pleasant Grove, UT 84062, telephone (801) 785-0505 or from the distributor, Gregory Industries, Inc., 4100 13<sup>th</sup> Street, S.W., Canton, OH 44708, telephone (330) 477-4800.
2. TYPE SRT TERMINAL SYSTEM - Type SRT terminal system must be an SRT-350 Slotted Rail Terminal (8-post system) as manufactured by Trinity Highway Products, LLC, and must include items detailed for Type SRT terminal system shown on the plans. The SRT-350 Slotted Rail Terminal (8-post system) can be obtained from the manufacturer, Trinity Highway Products, LLC, P.O. Box 99, Centerville, UT 84012, telephone (800) 772-7976.

Submit a certificate of compliance for terminal systems.

Terminal systems must be installed under the manufacturer's installation instructions and these specifications. Each terminal system installed must be identified by painting the type of terminal system in neat black letters and figures 2 inches high on the backside of the rail element between system posts numbers 4 and 5.

For Type SRT terminal system, the steel foundation tubes with soil plates attached must be, at the Contractor's option, either driven, with or without pilot holes, or placed in drilled holes. Space around the steel foundation tubes must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. The wood terminal posts must be inserted into the steel foundation tubes by hand and must not be driven. Before the wood terminal posts are inserted, the inside surfaces of the steel foundation tubes to receive the wood posts must be coated with a grease that will not melt or run at a temperature of 149 degrees F or less. The edges of the wood terminal posts may be slightly rounded to facilitate insertion of the post into the steel foundation tubes.

For Type FLEAT terminal system, the soil tubes must be, at the Contractor's option, driven with or without pilot holes, or placed in drilled holes. Space around the steel foundation tubes must be backfilled with selected earth, free of rock, placed in layers approximately 4 inches thick and each layer must be moistened and thoroughly compacted. Wood posts must be inserted into the steel foundation tubes by hand. Before the wood terminal posts are inserted, the inside surfaces of the steel foundation tubes to receive the wood posts must be coated with a grease that will not melt or run at a temperature of 149 degrees F or less. The edges of the wood posts may be slightly rounded to facilitate insertion of the post into the steel foundation tubes.

After installing the terminal system, dispose of surplus excavated material in a uniform manner along the adjacent roadway where designated by the Engineer.

**Add to section 83-1.02E:**

Where shown, cables must be black vinyl-coated and post must be painted. The painted post must match color no. 37038 of FED-STD 595 unless authorized.

Painting posts must comply with section 59-3.

Eye bolts shown cast in concrete at the top of walls may drilled and bonded. Drilling and bonding eye bolts must comply with the requirements for drilling and bonding dowels in section 51-1.

**Add to section 83-2.02D(1):**

Manufactured stone veneer must comply with section 78-2.

Wood beam texture must comply with section 51-1.

**Add to section 83-2.03:**

Concrete barrier (Barrier transition) is paid for as concrete barrier (Type 80 Modified).

Payment for stone veneer and wood beam texture is included in the payment for the concrete barrier.



Bridge name	Portion of structure
Middle Fork Smith River Bridge (Replace)	Bridge decks, barrier slabs, and concrete barriers
North Retaining Wall	Concrete barriers
Monkey Creek Retaining Wall	Barrier slabs and concrete barriers
Kelly Creek Barrier	Concrete barriers

**Add to section 90-1.02I(2)(a):**

For concrete at Middle Fork Smith River Bridge (Replace), North Retaining Wall, Monkey Creek Retaining Wall, and Kelly Creek Barrier, the mortar strength of the fine aggregate relative to the mortar strength of Ottawa sand must be a minimum of 100 percent under California Test 515.

**Add to section 90-1.02I(2)(b):**

Concrete at the following locations is exposed to deicing chemicals:

Bridge name	Portion of structure
Middle Fork Smith River Bridge (Replace)	Bridge decks, barrier slabs, and concrete barriers
North Retaining Wall	Concrete barriers
Monkey Creek Retaining Wall	Barrier slabs and concrete barriers
Kelly Creek Barrier	Concrete barriers

**Add to section 90-2.02B:**

You may use rice hull ash as an SCM. Rice hull ash must comply with AASHTO M 321 and the chemical and physical requirements shown in the following tables:

Chemical property	Requirement (percent)
Silicon dioxide (SiO <sub>2</sub> ) <sup>a</sup>	90 min
Loss on ignition	5.0 max
Total alkalis as Na <sub>2</sub> O equivalent	3.0 max

Physical property	Requirement
Particle size distribution	
Less than 45 microns	95 percent
Less than 10 microns	50 percent
Strength activity index with portland cement <sup>b</sup>	
7 days	95 percent (min percent of control)
28 days	110 percent (min percent of control)
Expansion at 16 days when testing project materials under ASTM C 1567 <sup>c</sup>	0.10 percent max
Surface area when testing by nitrogen adsorption under ASTM D 5604	40.0 m <sup>2</sup> /g min

<sup>a</sup>SiO<sub>2</sub> in crystalline form must not exceed 1.0 percent.

<sup>b</sup>When tested under AASHTO M 307 for strength activity testing of silica fume.

<sup>c</sup>In the test mix, Type II or V portland cement must be replaced with at least 12 percent rice hull ash by weight.

For the purpose of calculating the equations for the cementitious material specifications, consider rice hull ash to be represented by the variable  $UF$ .

**REVISED STANDARD SPECIFICATIONS  
APPLICABLE TO THE 2010 EDITION  
OF THE STANDARD SPECIFICATIONS**

# REVISED STANDARD SPECIFICATIONS DATED 04-19-13

Revised standard specifications are under headings that correspond with the main-section headings of the *Standard Specifications*. A main-section heading is a heading shown in the table of contents of the *Standard Specifications*. A date under a main-section heading is the date of the latest revision to the section.

Each revision to the *Standard Specifications* begins with a revision clause that describes a revision to the *Standard Specifications* or introduces a revision to the *Standard Specifications*. For a revision clause that describes a revision, the date on the right above the clause is the publication date of the revision. For a revision clause that introduces a revision, the date on the right above a revised term, phrase, clause, paragraph, or section is the publication date of the revised term, phrase, clause, paragraph, or section. For a multiple-paragraph or multiple-section revision, the date on the right above a paragraph or section is the publication date of the paragraphs or sections that follow.

Any paragraph added or deleted by a revision clause does not change the paragraph numbering of the *Standard Specifications* for any other reference to a paragraph of the *Standard Specifications*.

## DIVISION I GENERAL PROVISIONS

### 1 GENERAL

04-19-13

**Replace "current" in the 2nd paragraph of section 1-1.05 with:**

most recent

04-20-12

**Add to the 4th paragraph of section 1-1.05:**

04-20-12

Any reference directly to a revised standard specification section is for convenience only. Lack of a direct reference to a revised standard specification section does not indicate a revised standard specification for the section does not exist.

**Add to the 1st table in section 1-1.06:**

04-19-13

LCS	Department's lane closure system
POC	pedestrian overcrossing
QSD	qualified SWPPP developer
QSP	qualified SWPPP practitioner
TRO	time-related overhead
WPC	water pollution control

**Delete the abbreviation and its meaning for *UDBE* in the 1st table of section 1-1.06.**

06-20-12

**Delete "Contract completion date" and its definition in section 1-1.07B.**

10-19-12

**Delete "critical delay" and its definition in section 1-1.07B.**

10-19-12

**Replace "day" and its definition in section 1-1.07B with:**

10-19-12

**day:** 24 consecutive hours running from midnight to midnight; calendar day.

1. **business day:** Day on the calendar except a Saturday and a holiday.
2. **working day:** Time measure unit for work progress. A working day is any 24-consecutive-hour period except:
  - 2.1. Saturday and holiday.
  - 2.2. Day during which you cannot perform work on the controlling activity for at least 50 percent of the scheduled work shift with at least 50 percent of the scheduled labor and equipment due to any of the following:
    - 2.2.1. Adverse weather-related conditions.
    - 2.2.2. Maintaining traffic under the Contract.
    - 2.2.3. Suspension of a controlling activity that you and the Engineer agree benefits both parties.
    - 2.2.4. Unanticipated event not caused by either party such as:
      - 2.2.4.1. Act of God.
      - 2.2.4.2. Act of a public enemy.
      - 2.2.4.3. Epidemic.
      - 2.2.4.4. Fire.
      - 2.2.4.5. Flood.
      - 2.2.4.6. Governor-declared state of emergency.
      - 2.2.4.7. Landslide.
      - 2.2.4.8. Quarantine restriction.
    - 2.2.5. Issue involving a third party, including:
      - 2.2.5.1. Industry or area-wide labor strike.
      - 2.2.5.2. Material shortage.
      - 2.2.5.3. Freight embargo.
      - 2.2.5.4. Jurisdictional requirement of a law enforcement agency.
      - 2.2.5.5. Workforce labor dispute of a utility or nonhighway facility owner resulting in a nonhighway facility rearrangement not described and not solely for the Contractor's convenience. Rearrangement of a nonhighway facility includes installation, relocation, alteration, or removal of the facility.
  - 2.3. Day during a concurrent delay.
3. **original working days:**
  - 3.1. Working days to complete the work shown on the *Notice to Bidders* for a non-cost plus time based bid.
  - 3.2. Working days bid to complete the work for a cost plus time based bid.

Where working days is specified without the modifier "original" in the context of the number of working days to complete the work, interpret the number as the number of original working days as adjusted by any time adjustment.

**Replace "Contract" in the definition of "early completion time" in section 1-1.07B with:**

10-19-12

work

**Replace "excusable delay" and its definition in section 1-1.07B with:**

10-19-12

**delay:** Event that extends the completion of an activity.

1. **excusable delay:** Delay caused by the Department and not reasonably foreseeable when the work began such as:
  - 1.1. Change in the work
  - 1.2. Department action that is not part of the Contract
  - 1.3. Presence of an underground utility main not described in the Contract or in a location substantially different from that specified
  - 1.4. Described facility rearrangement not rearranged as described, by the utility owner by the date specified, unless the rearrangement is solely for the Contractor's convenience
  - 1.5. Department's failure to obtain timely access to the right-of-way
  - 1.6. Department's failure to review a submittal or provide notification in the time specified
2. **critical delay:** Excusable delay that extends the scheduled completion date
3. **concurrent delay:** Occurrence of at least 2 of the following events in the same period of time, either partially or entirely:
  - 3.1. Critical delay
  - 3.2. Delay to a controlling activity caused by you
  - 3.3. Non-working day

**Replace "project" in the definition of "scheduled completion date" in section 1-1.07B with:**

10-19-12

work

**Add to section 1-1.07B:**

10-19-12

**Contract time:** Number of original working days as adjusted by any time adjustment.

06-20-12

**Disadvantaged Business Enterprise:** Disadvantaged Business Enterprise as defined in 49 CFR 26.5.

**Replace "PO BOX 911" in the District 3 mailing address in the table in section 1-1.08 with:**

04-20-12

703 B ST

**Add to the table in section 1-1.11:**

01-20-12

Office Engineer--All Projects Currently Advertised	<a href="http://www.dot.ca.gov/hq/esc/oe/weekly_ads/all_advertised.php">http://www.dot.ca.gov/hq/esc/oe/weekly_ads/all_advertised.php</a>	--	--
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**2 BIDDING**

10-19-12

**Replace the 3rd paragraph of section 2-1.06B with:**

01-20-12

If an *Information Handout* or cross sections are available:

1. You may view them at the Contract Plans and Special Provisions link at the Office Engineer–All Projects Currently Advertised Web site
2. For an informal-bid contract, you may obtain them at the Bidders' Exchange street address

01-20-12

**Add a paragraph break between the 1st and 2nd sentences of the 5th paragraph of section 2-1.06B.**

**Add between "and" and "are" in item 2 in the list in the 7th paragraph of section 2-1.06B:**

they

04-20-12

06-20-12

**Delete "Underutilized" in "Underutilized Disadvantaged Business Enterprises" in the heading of section 2-1.12B.**

**Delete *U* in *UDBE* at each occurrence in section 2-1.12B.**

06-20-12

**Replace the 2nd paragraph of section 2-1.12B(1) with:**

To ensure equal participation of DBEs provided in 49 CFR 26.5, the Department shows a goal for DBEs.

06-20-12

**Delete the 3rd paragraph of section 2-1.12B(1):**

06-20-12

**Replace the 7th paragraph of section 2-1.12B(1) with:**

All DBE participation will count toward the Department's federally-mandated statewide overall DBE goal.

06-20-12

**Replace "offered" at the end of the 2nd sentence of item 7 in the list of 2nd paragraph of section 2-1.12B(3) with:**

provided

06-20-12

**Delete the 2nd paragraph of section 2-1.33A.**

01-20-12

**Replace the 3rd paragraph of section 2-1.33A with:**

Except for each subcontracted bid item number and corresponding percentage and proof of each required SSPC QP certification, do not fax submittals.

01-20-12



## 5 CONTROL OF WORK

10-19-12

**Add between "million" and ", professionally" in the 3rd paragraph of section 5-1.09A:**

and 100 or more working days

10-19-12

**Add to the list in the 4th paragraph of section 5-1.09A:**

9. Considering discussing with and involving all stakeholders in evaluating potential VECPs

10-19-12

**Add to the end of item 1.1 in the list in the 7th paragraph of section 5-1.09A:**

, including VECPs

10-19-12

**Replace the 1st paragraph of section 5-1.09C with:**

For a contract with a total bid over \$10 million and 100 or more working days, training in partnering skills development is required.

10-19-12

**Delete the 2nd paragraph of section 5-1.09C.**

10-19-12

**Replace "at least 2 representatives" in the 5th paragraph of section 5-1.09C with:**

field supervisory personnel

10-19-12

**Replace the 1st and 2nd sentences in the 7th paragraph of section 5-1.13B(1) with:**

If a DBE is decertified before completing its work, the DBE must notify you in writing of the decertification date. If a business becomes a certified DBE before completing its work, the business must notify you in writing of the certification date.

06-20-12

**Replace "90" in the last sentence of the 7th paragraph of section 5-1.13B(1) with:**

30

06-20-12

**Replace "Underutilized" in "Underutilized Disadvantaged Business Enterprises" in the heading of section 5-1.13B(2) with:**

Performance of

06-20-12

**Delete *U* in *UDBE* at each occurrence in section 5-1.13B(2).**

06-20-12

**Replace the 3rd paragraph of section 5-1.13B(2) with:**

06-20-12

Do not terminate or substitute a listed DBE for convenience and perform the work with your own forces or obtain materials from other sources without authorization from the Department.

**Replace item 6 in the list in the 4th paragraph of section 5-1.13B(2) with:**

06-20-12

6. Listed DBE is ineligible to work on the project because of suspension or debarment.

**Add to the list in the 4th paragraph of section 5-1.13B(2):**

06-20-12

8. Listed DBE voluntarily withdraws with written notice from the Contract.
9. Listed DBE is ineligible to receive credit for the type of work required.
10. Listed DBE owner dies or becomes disabled resulting in the inability to perform the work on the Contract.
11. Department determines other documented good cause.

**Add between the 4th and 5th paragraphs of section 5-1.13B(2):**

07-20-12

Notify the original DBE of your intent to use other forces or material sources and provide the reasons. Provide the DBE with 5 days to respond to your notice and advise you and the Department of the reasons why the use of other forces or sources of materials should not occur. Your request to use other forces or material sources must include:

1. 1 or more of the reasons listed in the preceding paragraph
2. Notices from you to the DBE regarding the request
3. Notices from the DBE to you regarding the request

**Add between "terminated" and ", you" in the 5th paragraph of section 5-1.13B(2):**

07-20-12

or substituted

**Replace "Contract" in item 1 in the list in the 5th paragraph of section 5-1.13C with:**

10-19-12

work

**Replace "Reserved" in section 5-1.20C with:**

10-19-12

If the Contract includes an agreement with a railroad company, the Department makes the provisions of the agreement available in the *Information Handout* in the document titled "Railroad Relations and Insurance Requirements." Comply with the requirements in the document.

**Add between the 2nd and 3rd paragraphs of section 5-1.23A:**

10-19-12

Submit action and informational submittals to the Engineer.



## 7 LEGAL RELATIONS AND RESPONSIBILITY TO THE PUBLIC

07-27-12

Replace "20 days" in the 14th paragraph of section 7-1.04 with:

09-16-11

25 days

Replace "90 days" in the 14th paragraph of section 7-1.04 with:

09-16-11

125 days

Add between the 18th and 19th paragraphs of section 7-1.04:

09-16-11

Temporary facilities that could be a hazard to public safety if improperly designed must comply with design requirements described in the Contract for those facilities or, if none are described, with standard design criteria or codes appropriate for the facility involved. Submit shop drawings and design calculations for the temporary facilities and show the standard design criteria or codes used. Shop drawings and supplemental calculations must be sealed and signed by an engineer who is registered as a civil engineer in the State.

Replace the 2nd paragraph of section 7-1.11A with:

07-27-12

A copy of form FHWA-1273 is included in section 7-1.11B. The training and promotion section of section II refers to training provisions as if they were included in the special provisions. The Department specifies the provisions in section 7-1.11D of the *Standard Specifications*. If a number of trainees or apprentices is required, the Department shows the number on the *Notice to Bidders*. Interpret each FHWA-1273 clause shown in the following table as having the same meaning as the corresponding Department clause:

**FHWA-1273 Nondiscrimination Clauses**

FHWA-1273 section	FHWA-1273 clause	Department clause
Training and Promotion	In the event a special provision for training is provided under this contract, this subparagraph will be superseded as indicated in the special provision.	If section 7-1.11D applies, section 7-1.11D supersedes this subparagraph.
Records and Reports	If on-the-job training is being required by special provision, the contractor will be required to collect and report training data.	If the Contract requires on-the-job training, collect and report training data.

Replace the form in section 7-1.11B with:

07-20-12

**REQUIRED CONTRACT PROVISIONS  
FEDERAL-AID CONSTRUCTION CONTRACTS**

- I. General
- II. Nondiscrimination
- III. Nonsegregated Facilities
- IV. Davis-Bacon and Related Act Provisions
- V. Contract Work Hours and Safety Standards Act Provisions
- VI. Subletting or Assigning the Contract
- VII. Safety: Accident Prevention
- VIII. False Statements Concerning Highway Projects
- IX. Implementation of Clean Air Act and Federal Water Pollution Control Act
- X. Compliance with Governmentwide Suspension and Debarment Requirements
- XI. Certification Regarding Use of Contract Funds for Lobbying

3. A breach of any of the stipulations contained in these Required Contract Provisions may be sufficient grounds for withholding of progress payments, withholding of final payment, termination of the contract, suspension / debarment or any other action determined to be appropriate by the contracting agency and FHWA.

4. Selection of Labor: During the performance of this contract, the contractor shall not use convict labor for any purpose within the limits of a construction project on a Federal-aid highway unless it is labor performed by convicts who are on parole, supervised release, or probation. The term Federal-aid highway does not include roadways functionally classified as local roads or rural minor collectors.

**ATTACHMENTS**

A. Employment and Materials Preference for Appalachian Development Highway System or Appalachian Local Access Road Contracts (included in Appalachian contracts only)

**II. NONDISCRIMINATION**

The provisions of this section related to 23 CFR Part 230 are applicable to all Federal-aid construction contracts and to all related construction subcontracts of \$10,000 or more. The provisions of 23 CFR Part 230 are not applicable to material supply, engineering, or architectural service contracts.

**I. GENERAL**

1. Form FHWA-1273 must be physically incorporated in each construction contract funded under Title 23 (excluding emergency contracts solely intended for debris removal). The contractor (or subcontractor) must insert this form in each subcontract and further require its inclusion in all lower tier subcontracts (excluding purchase orders, rental agreements and other agreements for supplies or services).

In addition, the contractor and all subcontractors must comply with the following policies: Executive Order 11246, 41 CFR 60, 29 CFR 1625-1627, Title 23 USC Section 140, the Rehabilitation Act of 1973, as amended (29 USC 794), Title VI of the Civil Rights Act of 1964, as amended, and related regulations including 49 CFR Parts 21, 26 and 27; and 23 CFR Parts 200, 230, and 633.

The applicable requirements of Form FHWA-1273 are incorporated by reference for work done under any purchase order, rental agreement or agreement for other services. The prime contractor shall be responsible for compliance by any subcontractor, lower-tier subcontractor or service provider.

The contractor and all subcontractors must comply with: the requirements of the Equal Opportunity Clause in 41 CFR 60-1.4(b) and, for all construction contracts exceeding \$10,000, the Standard Federal Equal Employment Opportunity Construction Contract Specifications in 41 CFR 60-4.3.

Form FHWA-1273 must be included in all Federal-aid design-build contracts, in all subcontracts and in lower tier subcontracts (excluding subcontracts for design services, purchase orders, rental agreements and other agreements for supplies or services). The design-builder shall be responsible for compliance by any subcontractor, lower-tier subcontractor or service provider.

Note: The U.S. Department of Labor has exclusive authority to determine compliance with Executive Order 11246 and the policies of the Secretary of Labor including 41 CFR 60, and 29 CFR 1625-1627. The contracting agency and the FHWA have the authority and the responsibility to ensure compliance with Title 23 USC Section 140, the Rehabilitation Act of 1973, as amended (29 USC 794), and Title VI of the Civil Rights Act of 1964, as amended, and related regulations including 49 CFR Parts 21, 26 and 27; and 23 CFR Parts 200, 230, and 633.

Contracting agencies may reference Form FHWA-1273 in bid proposal or request for proposal documents, however, the Form FHWA-1273 must be physically incorporated (not referenced) in all contracts, subcontracts and lower-tier subcontracts (excluding purchase orders, rental agreements and other agreements for supplies or services related to a construction contract).

The following provision is adopted from 23 CFR 230, Appendix A, with appropriate revisions to conform to the U.S. Department of Labor (US DOL) and FHWA requirements.

2. Subject to the applicability criteria noted in the following sections, these contract provisions shall apply to all work performed on the contract by the contractor's own organization and with the assistance of workers under the contractor's immediate superintendence and to all work performed on the contract by piecework, station work, or by subcontract.

**1. Equal Employment Opportunity:** Equal employment opportunity (EEO) requirements not to discriminate and to take affirmative action to assure equal opportunity as set forth under laws, executive orders, rules, regulations (28 CFR 35, 29 CFR 1630, 29 CFR 1625-1627, 41 CFR 60 and 49 CFR 27) and orders of the Secretary of Labor as modified by the provisions prescribed herein, and imposed pursuant to 23 U.S.C. 140 shall constitute the EEO and specific affirmative action standards for the contractor's project activities under

this contract. The provisions of the Americans with Disabilities Act of 1990 (42 U.S.C. 12101 et seq.) set forth under 28 CFR 35 and 29 CFR 1630 are incorporated by reference in this contract. In the execution of this contract, the contractor agrees to comply with the following minimum specific requirement activities of EEO:

a. The contractor will work with the contracting agency and the Federal Government to ensure that it has made every good faith effort to provide equal opportunity with respect to all of its terms and conditions of employment and in their review of activities under the contract.

b. The contractor will accept as its operating policy the following statement:

"It is the policy of this Company to assure that applicants are employed, and that employees are treated during employment, without regard to their race, religion, sex, color, national origin, age or disability. Such action shall include: employment, upgrading, demotion, or transfer; recruitment or recruitment advertising; layoff or termination; rates of pay or other forms of compensation; and selection for training, including apprenticeship, pre-apprenticeship, and/or on-the-job training."

**2. EEO Officer:** The contractor will designate and make known to the contracting officers an EEO Officer who will have the responsibility for and must be capable of effectively administering and promoting an active EEO program and who must be assigned adequate authority and responsibility to do so.

**3. Dissemination of Policy:** All members of the contractor's staff who are authorized to hire, supervise, promote, and discharge employees, or who recommend such action, or who are substantially involved in such action, will be made fully cognizant of, and will implement, the contractor's EEO policy and contractual responsibilities to provide EEO in each grade and classification of employment. To ensure that the above agreement will be met, the following actions will be taken as a minimum:

a. Periodic meetings of supervisory and personnel office employees will be conducted before the start of work and then not less often than once every six months, at which time the contractor's EEO policy and its implementation will be reviewed and explained. The meetings will be conducted by the EEO Officer.

b. All new supervisory or personnel office employees will be given a thorough indoctrination by the EEO Officer, covering all major aspects of the contractor's EEO obligations within thirty days following their reporting for duty with the contractor.

c. All personnel who are engaged in direct recruitment for the project will be instructed by the EEO Officer in the contractor's procedures for locating and hiring minorities and women.

d. Notices and posters setting forth the contractor's EEO policy will be placed in areas readily accessible to employees, applicants for employment and potential employees.

e. The contractor's EEO policy and the procedures to implement such policy will be brought to the attention of employees by means of meetings, employee handbooks, or other appropriate means.

**4. Recruitment:** When advertising for employees, the contractor will include in all advertisements for employees the notation: "An Equal Opportunity Employer." All such advertisements will be placed in publications having a large circulation among minorities and women in the area from which the project work force would normally be derived.

a. The contractor will, unless precluded by a valid bargaining agreement, conduct systematic and direct recruitment through public and private employee referral sources likely to yield qualified minorities and women. To meet this requirement, the contractor will identify sources of potential minority group employees, and establish with such identified sources procedures whereby minority and women applicants may be referred to the contractor for employment consideration.

b. In the event the contractor has a valid bargaining agreement providing for exclusive hiring hall referrals, the contractor is expected to observe the provisions of that agreement to the extent that the system meets the contractor's compliance with EEO contract provisions. Where implementation of such an agreement has the effect of discriminating against minorities or women, or obligates the contractor to do the same, such implementation violates Federal nondiscrimination provisions.

c. The contractor will encourage its present employees to refer minorities and women as applicants for employment. Information and procedures with regard to referring such applicants will be discussed with employees.

**5. Personnel Actions:** Wages, working conditions, and employee benefits shall be established and administered, and personnel actions of every type, including hiring, upgrading, promotion, transfer, demotion, layoff, and termination, shall be taken without regard to race, color, religion, sex, national origin, age or disability. The following procedures shall be followed:

a. The contractor will conduct periodic inspections of project sites to insure that working conditions and employee facilities do not indicate discriminatory treatment of project site personnel.

b. The contractor will periodically evaluate the spread of wages paid within each classification to determine any evidence of discriminatory wage practices.

c. The contractor will periodically review selected personnel actions in depth to determine whether there is evidence of discrimination. Where evidence is found, the contractor will promptly take corrective action. If the review indicates that the discrimination may extend beyond the actions reviewed, such corrective action shall include all affected persons.

d. The contractor will promptly investigate all complaints of alleged discrimination made to the contractor in connection with its obligations under this contract, will attempt to resolve such complaints, and will take appropriate corrective action within a reasonable time. If the investigation indicates that the discrimination may affect persons other than the complainant, such corrective action shall include such other persons. Upon completion of each investigation, the contractor will inform every complainant of all of their avenues of appeal.

**6. Training and Promotion:**

a. The contractor will assist in locating, qualifying, and increasing the skills of minorities and women who are

applicants for employment or current employees. Such efforts should be aimed at developing full journey level status employees in the type of trade or job classification involved.

b. Consistent with the contractor's work force requirements and as permissible under Federal and State regulations, the contractor shall make full use of training programs, i.e., apprenticeship, and on-the-job training programs for the geographical area of contract performance. In the event a special provision for training is provided under this contract, this subparagraph will be superseded as indicated in the special provision. The contracting agency may reserve training positions for persons who receive welfare assistance in accordance with 23 U.S.C. 140(a).

c. The contractor will advise employees and applicants for employment of available training programs and entrance requirements for each.

d. The contractor will periodically review the training and promotion potential of employees who are minorities and women and will encourage eligible employees to apply for such training and promotion.

**7. Unions:** If the contractor relies in whole or in part upon unions as a source of employees, the contractor will use good faith efforts to obtain the cooperation of such unions to increase opportunities for minorities and women. Actions by the contractor, either directly or through a contractor's association acting as agent, will include the procedures set forth below:

a. The contractor will use good faith efforts to develop, in cooperation with the unions, joint training programs aimed toward qualifying more minorities and women for membership in the unions and increasing the skills of minorities and women so that they may qualify for higher paying employment.

b. The contractor will use good faith efforts to incorporate an EEO clause into each union agreement to the end that such union will be contractually bound to refer applicants without regard to their race, color, religion, sex, national origin, age or disability.

c. The contractor is to obtain information as to the referral practices and policies of the labor union except that to the extent such information is within the exclusive possession of the labor union and such labor union refuses to furnish such information to the contractor, the contractor shall so certify to the contracting agency and shall set forth what efforts have been made to obtain such information.

d. In the event the union is unable to provide the contractor with a reasonable flow of referrals within the time limit set forth in the collective bargaining agreement, the contractor will, through independent recruitment efforts, fill the employment vacancies without regard to race, color, religion, sex, national origin, age or disability; making full efforts to obtain qualified and/or qualifiable minorities and women. The failure of a union to provide sufficient referrals (even though it is obligated to provide exclusive referrals under the terms of a collective bargaining agreement) does not relieve the contractor from the requirements of this paragraph. In the event the union referral practice prevents the contractor from meeting the obligations pursuant to Executive Order 11246, as amended, and these special provisions, such contractor shall immediately notify the contracting agency.

**8. Reasonable Accommodation for Applicants / Employees with Disabilities:** The contractor must be familiar

with the requirements for and comply with the Americans with Disabilities Act and all rules and regulations established there under. Employers must provide reasonable accommodation in all employment activities unless to do so would cause an undue hardship.

**9. Selection of Subcontractors, Procurement of Materials and Leasing of Equipment:** The contractor shall not discriminate on the grounds of race, color, religion, sex, national origin, age or disability in the selection and retention of subcontractors, including procurement of materials and leases of equipment. The contractor shall take all necessary and reasonable steps to ensure nondiscrimination in the administration of this contract.

a. The contractor shall notify all potential subcontractors and suppliers and lessors of their EEO obligations under this contract.

b. The contractor will use good faith efforts to ensure subcontractor compliance with their EEO obligations.

**10. Assurance Required by 49 CFR 26.13(b):**

a. The requirements of 49 CFR Part 26 and the State DOT's U.S. DOT-approved DBE program are incorporated by reference.

b. The contractor or subcontractor shall not discriminate on the basis of race, color, national origin, or sex in the performance of this contract. The contractor shall carry out applicable requirements of 49 CFR Part 26 in the award and administration of DOT-assisted contracts. Failure by the contractor to carry out these requirements is a material breach of this contract, which may result in the termination of this contract or such other remedy as the contracting agency deems appropriate.

**11. Records and Reports:** The contractor shall keep such records as necessary to document compliance with the EEO requirements. Such records shall be retained for a period of three years following the date of the final payment to the contractor for all contract work and shall be available at reasonable times and places for inspection by authorized representatives of the contracting agency and the FHWA.

a. The records kept by the contractor shall document the following:

(1) The number and work hours of minority and non-minority group members and women employed in each work classification on the project;

(2) The progress and efforts being made in cooperation with unions, when applicable, to increase employment opportunities for minorities and women; and

(3) The progress and efforts being made in locating, hiring, training, qualifying, and upgrading minorities and women;

b. The contractors and subcontractors will submit an annual report to the contracting agency each July for the duration of the project, indicating the number of minority, women, and non-minority group employees currently engaged in each work classification required by the contract work. This information is to be reported on [Form FHWA-1391](#). The staffing data should represent the project work force on board in all or any part of the last payroll period preceding the end of July. If on-the-job training is being required by special provision, the contractor

will be required to collect and report training data. The employment data should reflect the work force on board during all or any part of the last payroll period preceding the end of July.

### III. NONSEGREGATED FACILITIES

This provision is applicable to all Federal-aid construction contracts and to all related construction subcontracts of \$10,000 or more.

The contractor must ensure that facilities provided for employees are provided in such a manner that segregation on the basis of race, color, religion, sex, or national origin cannot result. The contractor may neither require such segregated use by written or oral policies nor tolerate such use by employee custom. The contractor's obligation extends further to ensure that its employees are not assigned to perform their services at any location, under the contractor's control, where the facilities are segregated. The term "facilities" includes waiting rooms, work areas, restaurants and other eating areas, time clocks, restrooms, washrooms, locker rooms, and other storage or dressing areas, parking lots, drinking fountains, recreation or entertainment areas, transportation, and housing provided for employees. The contractor shall provide separate or single-user restrooms and necessary dressing or sleeping areas to assure privacy between sexes.

### IV. DAVIS-BACON AND RELATED ACT PROVISIONS

This section is applicable to all Federal-aid construction projects exceeding \$2,000 and to all related subcontracts and lower-tier subcontracts (regardless of subcontract size). The requirements apply to all projects located within the right-of-way of a roadway that is functionally classified as Federal-aid highway. This excludes roadways functionally classified as local roads or rural minor collectors, which are exempt. Contracting agencies may elect to apply these requirements to other projects.

The following provisions are from the U.S. Department of Labor regulations in 29 CFR 5.5 "Contract provisions and related matters" with minor revisions to conform to the FHWA-1273 format and FHWA program requirements.

#### 1. Minimum wages

a. All laborers and mechanics employed or working upon the site of the work, will be paid unconditionally and not less often than once a week, and without subsequent deduction or rebate on any account (except such payroll deductions as are permitted by regulations issued by the Secretary of Labor under the Copeland Act (29 CFR part 3)), the full amount of wages and bona fide fringe benefits (or cash equivalents thereof) due at time of payment computed at rates not less than those contained in the wage determination of the Secretary of Labor which is attached hereto and made a part hereof, regardless of any contractual relationship which may be alleged to exist between the contractor and such laborers and mechanics.

Contributions made or costs reasonably anticipated for bona fide fringe benefits under section 1(b)(2) of the Davis-Bacon Act on behalf of laborers or mechanics are considered wages paid to such laborers or mechanics, subject to the provisions

of paragraph 1.d. of this section; also, regular contributions made or costs incurred for more than a weekly period (but not less often than quarterly) under plans, funds, or programs which cover the particular weekly period, are deemed to be constructively made or incurred during such weekly period. Such laborers and mechanics shall be paid the appropriate wage rate and fringe benefits on the wage determination for the classification of work actually performed, without regard to skill, except as provided in 29 CFR 5.5(a)(4). Laborers or mechanics performing work in more than one classification may be compensated at the rate specified for each classification for the time actually worked therein: Provided, That the employer's payroll records accurately set forth the time spent in each classification in which work is performed. The wage determination (including any additional classification and wage rates conformed under paragraph 1.b. of this section) and the Davis-Bacon poster (WH-1321) shall be posted at all times by the contractor and its subcontractors at the site of the work in a prominent and accessible place where it can be easily seen by the workers.

b.(1) The contracting officer shall require that any class of laborers or mechanics, including helpers, which is not listed in the wage determination and which is to be employed under the contract shall be classified in conformance with the wage determination. The contracting officer shall approve an additional classification and wage rate and fringe benefits therefore only when the following criteria have been met:

(i) The work to be performed by the classification requested is not performed by a classification in the wage determination; and

(ii) The classification is utilized in the area by the construction industry; and

(iii) The proposed wage rate, including any bona fide fringe benefits, bears a reasonable relationship to the wage rates contained in the wage determination.

(2) If the contractor and the laborers and mechanics to be employed in the classification (if known), or their representatives, and the contracting officer agree on the classification and wage rate (including the amount designated for fringe benefits where appropriate), a report of the action taken shall be sent by the contracting officer to the Administrator of the Wage and Hour Division, Employment Standards Administration, U.S. Department of Labor, Washington, DC 20210. The Administrator, or an authorized representative, will approve, modify, or disapprove every additional classification action within 30 days of receipt and so advise the contracting officer or will notify the contracting officer within the 30-day period that additional time is necessary.

(3) In the event the contractor, the laborers or mechanics to be employed in the classification or their representatives, and the contracting officer do not agree on the proposed classification and wage rate (including the amount designated for fringe benefits, where appropriate), the contracting officer shall refer the questions, including the views of all interested parties and the recommendation of the contracting officer, to the Wage and Hour Administrator for determination. The Wage and Hour Administrator, or an authorized representative, will issue a determination within 30 days of receipt and so advise the contracting officer or

will notify the contracting officer within the 30-day period that additional time is necessary.

(4) The wage rate (including fringe benefits where appropriate) determined pursuant to paragraphs 1.b.(2) or 1.b.(3) of this section, shall be paid to all workers performing work in the classification under this contract from the first day on which work is performed in the classification.

c. Whenever the minimum wage rate prescribed in the contract for a class of laborers or mechanics includes a fringe benefit which is not expressed as an hourly rate, the contractor shall either pay the benefit as stated in the wage determination or shall pay another bona fide fringe benefit or an hourly cash equivalent thereof.

d. If the contractor does not make payments to a trustee or other third person, the contractor may consider as part of the wages of any laborer or mechanic the amount of any costs reasonably anticipated in providing bona fide fringe benefits under a plan or program. Provided, That the Secretary of Labor has found, upon the written request of the contractor, that the applicable standards of the Davis-Bacon Act have been met. The Secretary of Labor may require the contractor to set aside in a separate account assets for the meeting of obligations under the plan or program.

## 2. Withholding

The contracting agency shall upon its own action or upon written request of an authorized representative of the Department of Labor, withhold or cause to be withheld from the contractor under this contract, or any other Federal contract with the same prime contractor, or any other federally-assisted contract subject to Davis-Bacon prevailing wage requirements, which is held by the same prime contractor, so much of the accrued payments or advances as may be considered necessary to pay laborers and mechanics, including apprentices, trainees, and helpers, employed by the contractor or any subcontractor the full amount of wages required by the contract. In the event of failure to pay any laborer or mechanic, including any apprentice, trainee, or helper, employed or working on the site of the work, all or part of the wages required by the contract, the contracting agency may, after written notice to the contractor, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds until such violations have ceased.

## 3. Payrolls and basic records

a. Payrolls and basic records relating thereto shall be maintained by the contractor during the course of the work and preserved for a period of three years thereafter for all laborers and mechanics working at the site of the work. Such records shall contain the name, address, and social security number of each such worker, his or her correct classification, hourly rates of wages paid (including rates of contributions or costs anticipated for bona fide fringe benefits or cash equivalents thereof of the types described in section 1(b)(2)(B) of the Davis-Bacon Act), daily and weekly number of hours worked, deductions made and actual wages paid. Whenever the Secretary of Labor has found under 29 CFR 5.5(a)(1)(iv) that the wages of any laborer or mechanic include the amount of any costs reasonably anticipated in providing benefits under a plan or program described in section 1(b)(2)(B) of the Davis-

Bacon Act, the contractor shall maintain records which show that the commitment to provide such benefits is enforceable, that the plan or program is financially responsible, and that the plan or program has been communicated in writing to the laborers or mechanics affected, and records which show the costs anticipated or the actual cost incurred in providing such benefits. Contractors employing apprentices or trainees under approved programs shall maintain written evidence of the registration of apprenticeship programs and certification of trainee programs, the registration of the apprentices and trainees, and the ratios and wage rates prescribed in the applicable programs.

b.(1) The contractor shall submit weekly for each week in which any contract work is performed a copy of all payrolls to the contracting agency. The payrolls submitted shall set out accurately and completely all of the information required to be maintained under 29 CFR 5.5(a)(3)(i), except that full social security numbers and home addresses shall not be included on weekly transmittals. Instead the payrolls shall only need to include an individually identifying number for each employee (e.g., the last four digits of the employee's social security number). The required weekly payroll information may be submitted in any form desired. Optional Form WH-347 is available for this purpose from the Wage and Hour Division Web site at <http://www.dol.gov/esa/whd/forms/wh347instr.htm> or its successor site. The prime contractor is responsible for the submission of copies of payrolls by all subcontractors. Contractors and subcontractors shall maintain the full social security number and current address of each covered worker, and shall provide them upon request to the contracting agency for transmission to the State DOT, the FHWA or the Wage and Hour Division of the Department of Labor for purposes of an investigation or audit of compliance with prevailing wage requirements. It is not a violation of this section for a prime contractor to require a subcontractor to provide addresses and social security numbers to the prime contractor for its own records, without weekly submission to the contracting agency..

(2) Each payroll submitted shall be accompanied by a "Statement of Compliance," signed by the contractor or subcontractor or his or her agent who pays or supervises the payment of the persons employed under the contract and shall certify the following:

(i) That the payroll for the payroll period contains the information required to be provided under §5.5 (a)(3)(ii) of Regulations, 29 CFR part 5, the appropriate information is being maintained under §5.5 (a)(3)(i) of Regulations, 29 CFR part 5, and that such information is correct and complete;

(ii) That each laborer or mechanic (including each helper, apprentice, and trainee) employed on the contract during the payroll period has been paid the full weekly wages earned, without rebate, either directly or indirectly, and that no deductions have been made either directly or indirectly from the full wages earned, other than permissible deductions as set forth in Regulations, 29 CFR part 3;

(iii) That each laborer or mechanic has been paid not less than the applicable wage rates and fringe benefits or cash equivalents for the classification of work performed, as specified in the applicable wage determination incorporated into the contract.

(3) The weekly submission of a properly executed certification set forth on the reverse side of Optional Form WH-347 shall satisfy the requirement for submission of the "Statement of Compliance" required by paragraph 3.b.(2) of this section.

(4) The falsification of any of the above certifications may subject the contractor or subcontractor to civil or criminal prosecution under section 1001 of title 18 and section 231 of title 31 of the United States Code.

c. The contractor or subcontractor shall make the records required under paragraph 3.a. of this section available for inspection, copying, or transcription by authorized representatives of the contracting agency, the State DOT, the FHWA, or the Department of Labor, and shall permit such representatives to interview employees during working hours on the job. If the contractor or subcontractor fails to submit the required records or to make them available, the FHWA may, after written notice to the contractor, the contracting agency or the State DOT, take such action as may be necessary to cause the suspension of any further payment, advance, or guarantee of funds. Furthermore, failure to submit the required records upon request or to make such records available may be grounds for debarment action pursuant to 29 CFR 5.12.

#### 4. Apprentices and trainees

##### a. Apprentices (programs of the USDOL).

Apprentices will be permitted to work at less than the predetermined rate for the work they performed when they are employed pursuant to and individually registered in a bona fide apprenticeship program registered with the U.S. Department of Labor, Employment and Training Administration, Office of Apprenticeship Training, Employer and Labor Services, or with a State Apprenticeship Agency recognized by the Office, or if a person is employed in his or her first 90 days of probationary employment as an apprentice in such an apprenticeship program, who is not individually registered in the program, but who has been certified by the Office of Apprenticeship Training, Employer and Labor Services or a State Apprenticeship Agency (where appropriate) to be eligible for probationary employment as an apprentice.

The allowable ratio of apprentices to journeymen on the job site in any craft classification shall not be greater than the ratio permitted to the contractor as to the entire work force under the registered program. Any worker listed on a payroll at an apprentice wage rate, who is not registered or otherwise employed as stated above, shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed. In addition, any apprentice performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed. Where a contractor is performing construction on a project in a locality other than that in which its program is registered, the ratios and wage rates (expressed in percentages of the journeyman's hourly rate) specified in the contractor's or subcontractor's registered program shall be observed.

Every apprentice must be paid at not less than the rate specified in the registered program for the apprentice's level of progress, expressed as a percentage of the journeymen hourly

rate specified in the applicable wage determination. Apprentices shall be paid fringe benefits in accordance with the provisions of the apprenticeship program. If the apprenticeship program does not specify fringe benefits, apprentices must be paid the full amount of fringe benefits listed on the wage determination for the applicable classification. If the Administrator determines that a different practice prevails for the applicable apprentice classification, fringes shall be paid in accordance with that determination.

In the event the Office of Apprenticeship Training, Employer and Labor Services, or a State Apprenticeship Agency recognized by the Office, withdraws approval of an apprenticeship program, the contractor will no longer be permitted to utilize apprentices at less than the applicable predetermined rate for the work performed until an acceptable program is approved.

##### b. Trainees (programs of the USDOL).

Except as provided in 29 CFR 5.16, trainees will not be permitted to work at less than the predetermined rate for the work performed unless they are employed pursuant to and individually registered in a program which has received prior approval, evidenced by formal certification by the U.S. Department of Labor, Employment and Training Administration.

The ratio of trainees to journeymen on the job site shall not be greater than permitted under the plan approved by the Employment and Training Administration.

Every trainee must be paid at not less than the rate specified in the approved program for the trainee's level of progress, expressed as a percentage of the journeyman hourly rate specified in the applicable wage determination. Trainees shall be paid fringe benefits in accordance with the provisions of the trainee program. If the trainee program does not mention fringe benefits, trainees shall be paid the full amount of fringe benefits listed on the wage determination unless the Administrator of the Wage and Hour Division determines that there is an apprenticeship program associated with the corresponding journeyman wage rate on the wage determination which provides for less than full fringe benefits for apprentices. Any employee listed on the payroll at a trainee rate who is not registered and participating in a training plan approved by the Employment and Training Administration shall be paid not less than the applicable wage rate on the wage determination for the classification of work actually performed. In addition, any trainee performing work on the job site in excess of the ratio permitted under the registered program shall be paid not less than the applicable wage rate on the wage determination for the work actually performed.

In the event the Employment and Training Administration withdraws approval of a training program, the contractor will no longer be permitted to utilize trainees at less than the applicable predetermined rate for the work performed until an acceptable program is approved.

c. Equal employment opportunity. The utilization of apprentices, trainees and journeymen under this part shall be in conformity with the equal employment opportunity requirements of Executive Order 11246, as amended, and 29 CFR part 30.

d. Apprentices and Trainees (programs of the U.S. DOT).

Apprentices and trainees working under apprenticeship and skill training programs which have been certified by the Secretary of Transportation as promoting EEO in connection with Federal-aid highway construction programs are not subject to the requirements of paragraph 4 of this Section IV. The straight time hourly wage rates for apprentices and trainees under such programs will be established by the particular programs. The ratio of apprentices and trainees to journeymen shall not be greater than permitted by the terms of the particular program.

**5. Compliance with Copeland Act requirements.** The contractor shall comply with the requirements of 29 CFR part 3, which are incorporated by reference in this contract.

**6. Subcontracts.** The contractor or subcontractor shall insert Form FHWA-1273 in any subcontracts and also require the subcontractors to include Form FHWA-1273 in any lower tier subcontracts. The prime contractor shall be responsible for the compliance by any subcontractor or lower tier subcontractor with all the contract clauses in 29 CFR 5.5.

**7. Contract termination; debarment.** A breach of the contract clauses in 29 CFR 5.5 may be grounds for termination of the contract, and for debarment as a contractor and a subcontractor as provided in 29 CFR 5.12.

**8. Compliance with Davis-Bacon and Related Act requirements.** All rulings and interpretations of the Davis-Bacon and Related Acts contained in 29 CFR parts 1, 3, and 5 are herein incorporated by reference in this contract.

**9. Disputes concerning labor standards.** Disputes arising out of the labor standards provisions of this contract shall not be subject to the general disputes clause of this contract. Such disputes shall be resolved in accordance with the procedures of the Department of Labor set forth in 29 CFR parts 5, 6, and 7. Disputes within the meaning of this clause include disputes between the contractor (or any of its subcontractors) and the contracting agency, the U.S. Department of Labor, or the employees or their representatives.

**10. Certification of eligibility.**

a. By entering into this contract, the contractor certifies that neither it (nor he or she) nor any person or firm who has an interest in the contractor's firm is a person or firm ineligible to be awarded Government contracts by virtue of section 3(a) of the Davis-Bacon Act or 29 CFR 5.12(a)(1).

b. No part of this contract shall be subcontracted to any person or firm ineligible for award of a Government contract by virtue of section 3(a) of the Davis-Bacon Act or 29 CFR 5.12(a)(1).

c. The penalty for making false statements is prescribed in the U.S. Criminal Code, 18 U.S.C. 1001.

**V. CONTRACT WORK HOURS AND SAFETY STANDARDS ACT**

The following clauses apply to any Federal-aid construction contract in an amount in excess of \$100,000 and subject to the overtime provisions of the Contract Work Hours and Safety Standards Act. These clauses shall be inserted in addition to the clauses required by 29 CFR 5.5(a) or 29 CFR 4.6. As used in this paragraph, the terms laborers and mechanics include watchmen and guards.

**1. Overtime requirements.** No contractor or subcontractor contracting for any part of the contract work which may require or involve the employment of laborers or mechanics shall require or permit any such laborer or mechanic in any workweek in which he or she is employed on such work to work in excess of forty hours in such workweek unless such laborer or mechanic receives compensation at a rate not less than one and one-half times the basic rate of pay for all hours worked in excess of forty hours in such workweek.

**2. Violation; liability for unpaid wages; liquidated damages.** In the event of any violation of the clause set forth in paragraph (1.) of this section, the contractor and any subcontractor responsible therefor shall be liable for the unpaid wages. In addition, such contractor and subcontractor shall be liable to the United States (in the case of work done under contract for the District of Columbia or a territory, to such District or to such territory), for liquidated damages. Such liquidated damages shall be computed with respect to each individual laborer or mechanic, including watchmen and guards, employed in violation of the clause set forth in paragraph (1.) of this section, in the sum of \$10 for each calendar day on which such individual was required or permitted to work in excess of the standard workweek of forty hours without payment of the overtime wages required by the clause set forth in paragraph (1.) of this section.

**3. Withholding for unpaid wages and liquidated damages.** The FHWA or the contracting agency shall upon its own action or upon written request of an authorized representative of the Department of Labor withhold or cause to be withheld, from any moneys payable on account of work performed by the contractor or subcontractor under any such contract or any other Federal contract with the same prime contractor, or any other federally-assisted contract subject to the Contract Work Hours and Safety Standards Act, which is held by the same prime contractor, such sums as may be determined to be necessary to satisfy any liabilities of such contractor or subcontractor for unpaid wages and liquidated damages as provided in the clause set forth in paragraph (2.) of this section.

**4. Subcontracts.** The contractor or subcontractor shall insert in any subcontracts the clauses set forth in paragraph (1.) through (4.) of this section and also a clause requiring the subcontractors to include these clauses in any lower tier subcontracts. The prime contractor shall be responsible for compliance by any subcontractor or lower tier subcontractor with the clauses set forth in paragraphs (1.) through (4.) of this section.

## VI. SUBLETTING OR ASSIGNING THE CONTRACT

This provision is applicable to all Federal-aid construction contracts on the National Highway System.

1. The contractor shall perform with its own organization contract work amounting to not less than 30 percent (or a greater percentage if specified elsewhere in the contract) of the total original contract price, excluding any specialty items designated by the contracting agency. Specialty items may be performed by subcontract and the amount of any such specialty items performed may be deducted from the total original contract price before computing the amount of work required to be performed by the contractor's own organization (23 CFR 635.116).

a. The term "perform work with its own organization" refers to workers employed or leased by the prime contractor, and equipment owned or rented by the prime contractor, with or without operators. Such term does not include employees or equipment of a subcontractor or lower tier subcontractor, agents of the prime contractor, or any other assignees. The term may include payments for the costs of hiring leased employees from an employee leasing firm meeting all relevant Federal and State regulatory requirements. Leased employees may only be included in this term if the prime contractor meets all of the following conditions:

(1) the prime contractor maintains control over the supervision of the day-to-day activities of the leased employees;

(2) the prime contractor remains responsible for the quality of the work of the leased employees;

(3) the prime contractor retains all power to accept or exclude individual employees from work on the project; and

(4) the prime contractor remains ultimately responsible for the payment of predetermined minimum wages, the submission of payrolls, statements of compliance and all other Federal regulatory requirements.

b. "Specialty Items" shall be construed to be limited to work that requires highly specialized knowledge, abilities, or equipment not ordinarily available in the type of contracting organizations qualified and expected to bid or propose on the contract as a whole and in general are to be limited to minor components of the overall contract.

2. The contract amount upon which the requirements set forth in paragraph (1) of Section VI is computed includes the cost of material and manufactured products which are to be purchased or produced by the contractor under the contract provisions.

3. The contractor shall furnish (a) a competent superintendent or supervisor who is employed by the firm, has full authority to direct performance of the work in accordance with the contract requirements, and is in charge of all construction operations (regardless of who performs the work) and (b) such other of its own organizational resources (supervision, management, and engineering services) as the contracting officer determines is necessary to assure the performance of the contract.

4. No portion of the contract shall be sublet, assigned or otherwise disposed of except with the written consent of the contracting officer, or authorized representative, and such consent when given shall not be construed to relieve the contractor of any responsibility for the fulfillment of the contract. Written consent will be given only after the contracting agency has assured that each subcontract is

evidenced in writing and that it contains all pertinent provisions and requirements of the prime contract.

5. The 30% self-performance requirement of paragraph (1) is not applicable to design-build contracts; however, contracting agencies may establish their own self-performance requirements.

## VII. SAFETY: ACCIDENT PREVENTION

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

1. In the performance of this contract the contractor shall comply with all applicable Federal, State, and local laws governing safety, health, and sanitation (23 CFR 635). The contractor shall provide all safeguards, safety devices and protective equipment and take any other needed actions as it determines, or as the contracting officer may determine, to be reasonably necessary to protect the life and health of employees on the job and the safety of the public and to protect property in connection with the performance of the work covered by the contract.

2. It is a condition of this contract, and shall be made a condition of each subcontract, which the contractor enters into pursuant to this contract, that the contractor and any subcontractor shall not permit any employee, in performance of the contract, to work in surroundings or under conditions which are unsanitary, hazardous or dangerous to his/her health or safety, as determined under construction safety and health standards (29 CFR 1926) promulgated by the Secretary of Labor, in accordance with Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C. 3704).

3. Pursuant to 29 CFR 1926.3, it is a condition of this contract that the Secretary of Labor or authorized representative thereof, shall have right of entry to any site of contract performance to inspect or investigate the matter of compliance with the construction safety and health standards and to carry out the duties of the Secretary under Section 107 of the Contract Work Hours and Safety Standards Act (40 U.S.C.3704).

## VIII. FALSE STATEMENTS CONCERNING HIGHWAY PROJECTS

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

In order to assure high quality and durable construction in conformity with approved plans and specifications and a high degree of reliability on statements and representations made by engineers, contractors, suppliers, and workers on Federal-aid highway projects, it is essential that all persons concerned with the project perform their functions as carefully, thoroughly, and honestly as possible. Willful falsification, distortion, or misrepresentation with respect to any facts related to the project is a violation of Federal law. To prevent any misunderstanding regarding the seriousness of these and similar acts, Form FHWA-1022 shall be posted on each Federal-aid highway project (23 CFR 635) in one or more places where it is readily available to all persons concerned with the project:

18 U.S.C. 1020 reads as follows:

"Whoever, being an officer, agent, or employee of the United States, or of any State or Territory, or whoever, whether a person, association, firm, or corporation, knowingly makes any false statement, false representation, or false report as to the character, quality, quantity, or cost of the material used or to be used, or the quantity or quality of the work performed or to be performed, or the cost thereof in connection with the submission of plans, maps, specifications, contracts, or costs of construction on any highway or related project submitted for approval to the Secretary of Transportation; or

Whoever knowingly makes any false statement, false representation, false report or false claim with respect to the character, quality, quantity, or cost of any work performed or to be performed, or materials furnished or to be furnished, in connection with the construction of any highway or related project approved by the Secretary of Transportation; or

Whoever knowingly makes any false statement or false representation as to material fact in any statement, certificate, or report submitted pursuant to provisions of the Federal-aid Roads Act approved July 1, 1916, (39 Stat. 355), as amended and supplemented;

Shall be fined under this title or imprisoned not more than 5 years or both."

#### **IX. IMPLEMENTATION OF CLEAN AIR ACT AND FEDERAL WATER POLLUTION CONTROL ACT**

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts.

By submission of this bid/proposal or the execution of this contract, or subcontract, as appropriate, the bidder, proposer, Federal-aid construction contractor, or subcontractor, as appropriate, will be deemed to have stipulated as follows:

1. That any person who is or will be utilized in the performance of this contract is not prohibited from receiving an award due to a violation of Section 508 of the Clean Water Act or Section 306 of the Clean Air Act.
2. That the contractor agrees to include or cause to be included the requirements of paragraph (1) of this Section X in every subcontract, and further agrees to take such action as the contracting agency may direct as a means of enforcing such requirements.

#### **X. CERTIFICATION REGARDING DEBARMENT, SUSPENSION, INELIGIBILITY AND VOLUNTARY EXCLUSION**

This provision is applicable to all Federal-aid construction contracts, design-build contracts, subcontracts, lower-tier subcontracts, purchase orders, lease agreements, consultant contracts or any other covered transaction requiring FHWA approval or that is estimated to cost \$25,000 or more – as defined in 2 CFR Parts 180 and 1200.

##### **1. Instructions for Certification – First Tier Participants:**

- a. By signing and submitting this proposal, the prospective first tier participant is providing the certification set out below.
- b. The inability of a person to provide the certification set out below will not necessarily result in denial of participation in this

covered transaction. The prospective first tier participant shall submit an explanation of why it cannot provide the certification set out below. The certification or explanation will be considered in connection with the department or agency's determination whether to enter into this transaction. However, failure of the prospective first tier participant to furnish a certification or an explanation shall disqualify such a person from participation in this transaction.

c. The certification in this clause is a material representation of fact upon which reliance was placed when the contracting agency determined to enter into this transaction. If it is later determined that the prospective participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the contracting agency may terminate this transaction for cause of default.

d. The prospective first tier participant shall provide immediate written notice to the contracting agency to whom this proposal is submitted if any time the prospective first tier participant learns that its certification was erroneous when submitted or has become erroneous by reason of changed circumstances.

e. The terms "covered transaction," "debarred," "suspended," "ineligible," "participant," "person," "principal," and "voluntarily excluded," as used in this clause, are defined in 2 CFR Parts 180 and 1200. "First Tier Covered Transactions" refers to any covered transaction between a grantee or subgrantee of Federal funds and a participant (such as the prime or general contract). "Lower Tier Covered Transactions" refers to any covered transaction under a First Tier Covered Transaction (such as subcontracts). "First Tier Participant" refers to the participant who has entered into a covered transaction with a grantee or subgrantee of Federal funds (such as the prime or general contractor). "Lower Tier Participant" refers any participant who has entered into a covered transaction with a First Tier Participant or other Lower Tier Participants (such as subcontractors and suppliers).

f. The prospective first tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency entering into this transaction.

g. The prospective first tier participant further agrees by submitting this proposal that it will include the clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transactions," provided by the department or contracting agency, entering into this covered transaction, without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions exceeding the \$25,000 threshold.

h. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant is responsible for ensuring that its principals are not suspended, debarred, or otherwise ineligible to participate in covered transactions. To verify the eligibility of its principals, as well as the eligibility of any lower tier prospective participants, each participant may, but is not required to, check the Excluded Parties List System website (<https://www.epls.gov/>), which is compiled by the General Services Administration.

i. Nothing contained in the foregoing shall be construed to require the establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of the prospective participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

j. Except for transactions authorized under paragraph (f) of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the department or agency may terminate this transaction for cause or default.

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## **2. Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion – First Tier Participants:**

a. The prospective first tier participant certifies to the best of its knowledge and belief, that it and its principals:

(1) Are not presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participating in covered transactions by any Federal department or agency;

(2) Have not within a three-year period preceding this proposal been convicted of or had a civil judgment rendered against them for commission of fraud or a criminal offense in connection with obtaining, attempting to obtain, or performing a public (Federal, State or local) transaction or contract under a public transaction; violation of Federal or State antitrust statutes or commission of embezzlement, theft, forgery, bribery, falsification or destruction of records, making false statements, or receiving stolen property;

(3) Are not presently indicted for or otherwise criminally or civilly charged by a governmental entity (Federal, State or local) with commission of any of the offenses enumerated in paragraph (a)(2) of this certification; and

(4) Have not within a three-year period preceding this application/proposal had one or more public transactions (Federal, State or local) terminated for cause or default.

b. Where the prospective participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

## **2. Instructions for Certification - Lower Tier Participants:**

(Applicable to all subcontracts, purchase orders and other lower tier transactions requiring prior FHWA approval or estimated to cost \$25,000 or more - 2 CFR Parts 180 and 1200)

a. By signing and submitting this proposal, the prospective lower tier is providing the certification set out below.

b. The certification in this clause is a material representation of fact upon which reliance was placed when this transaction was entered into. If it is later determined that the prospective lower tier participant knowingly rendered an erroneous certification, in addition to other remedies available to the Federal Government, the department, or agency with which

this transaction originated may pursue available remedies, including suspension and/or debarment.

c. The prospective lower tier participant shall provide immediate written notice to the person to which this proposal is submitted if at any time the prospective lower tier participant learns that its certification was erroneous by reason of changed circumstances.

d. The terms "covered transaction," "debarred," "suspended," "ineligible," "participant," "person," "principal," and "voluntarily excluded," as used in this clause, are defined in 2 CFR Parts 180 and 1200. You may contact the person to which this proposal is submitted for assistance in obtaining a copy of those regulations. "First Tier Covered Transactions" refers to any covered transaction between a grantee or subgrantee of Federal funds and a participant (such as the prime or general contract). "Lower Tier Covered Transactions" refers to any covered transaction under a First Tier Covered Transaction (such as subcontracts). "First Tier Participant" refers to the participant who has entered into a covered transaction with a grantee or subgrantee of Federal funds (such as the prime or general contractor). "Lower Tier Participant" refers to any participant who has entered into a covered transaction with a First Tier Participant or other Lower Tier Participants (such as subcontractors and suppliers).

e. The prospective lower tier participant agrees by submitting this proposal that, should the proposed covered transaction be entered into, it shall not knowingly enter into any lower tier covered transaction with a person who is debarred, suspended, declared ineligible, or voluntarily excluded from participation in this covered transaction, unless authorized by the department or agency with which this transaction originated.

f. The prospective lower tier participant further agrees by submitting this proposal that it will include this clause titled "Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion-Lower Tier Covered Transaction," without modification, in all lower tier covered transactions and in all solicitations for lower tier covered transactions exceeding the \$25,000 threshold.

g. A participant in a covered transaction may rely upon a certification of a prospective participant in a lower tier covered transaction that is not debarred, suspended, ineligible, or voluntarily excluded from the covered transaction, unless it knows that the certification is erroneous. A participant is responsible for ensuring that its principals are not suspended, debarred, or otherwise ineligible to participate in covered transactions. To verify the eligibility of its principals, as well as the eligibility of any lower tier prospective participants, each participant may, but is not required to, check the Excluded Parties List System website (<https://www.epls.gov/>), which is compiled by the General Services Administration.

h. Nothing contained in the foregoing shall be construed to require establishment of a system of records in order to render in good faith the certification required by this clause. The knowledge and information of participant is not required to exceed that which is normally possessed by a prudent person in the ordinary course of business dealings.

i. Except for transactions authorized under paragraph e of these instructions, if a participant in a covered transaction knowingly enters into a lower tier covered transaction with a person who is suspended, debarred, ineligible, or voluntarily excluded from participation in this transaction, in addition to other remedies available to the Federal Government, the

department or agency with which this transaction originated may pursue available remedies, including suspension and/or debarment.

\*\*\*\*\*

**Certification Regarding Debarment, Suspension, Ineligibility and Voluntary Exclusion--Lower Tier Participants:**

1. The prospective lower tier participant certifies, by submission of this proposal, that neither it nor its principals is presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participating in covered transactions by any Federal department or agency.

2. Where the prospective lower tier participant is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

\*\*\*\*\*

**XI. CERTIFICATION REGARDING USE OF CONTRACT FUNDS FOR LOBBYING**

This provision is applicable to all Federal-aid construction contracts and to all related subcontracts which exceed \$100,000 (49 CFR 20).

1. The prospective participant certifies, by signing and submitting this bid or proposal, to the best of his or her knowledge and belief, that:

a. No Federal appropriated funds have been paid or will be paid, by or on behalf of the undersigned, to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with the awarding of any Federal contract, the making of any Federal grant, the making of any Federal loan, the entering into of any cooperative agreement, and the extension, continuation, renewal, amendment, or modification of any Federal contract, grant, loan, or cooperative agreement.

b. If any funds other than Federal appropriated funds have been paid or will be paid to any person for influencing or attempting to influence an officer or employee of any Federal agency, a Member of Congress, an officer or employee of Congress, or an employee of a Member of Congress in connection with this Federal contract, grant, loan, or cooperative agreement, the undersigned shall complete and submit Standard Form-LLL, "Disclosure Form to Report Lobbying," in accordance with its instructions.

2. This certification is a material representation of fact upon which reliance was placed when this transaction was made or entered into. Submission of this certification is a prerequisite for making or entering into this transaction imposed by 31 U.S.C. 1352. Any person who fails to file the required certification shall be subject to a civil penalty of not less than \$10,000 and not more than \$100,000 for each such failure.

3. The prospective participant also agrees by submitting its bid or proposal that the participant shall require that the language of this certification be included in all lower tier subcontracts, which exceed \$100,000 and that all such recipients shall certify and disclose accordingly.



**Replace "Contract" in the 3rd paragraph of section 8-1.02D(2) with:**

10-19-12

work

**Replace "Contract" in item 9 in the list in the 4th paragraph of section 8-1.02D(4) with:**

10-19-12

work

**Replace "Contract completion" in the 4th paragraph of section 8-1.02D(6) with:**

10-19-12

work completion

**Replace "Contract working days" in the 4th paragraph of section 8-1.02D(6) with:**

10-19-12

original working days

**Delete items 1.3 and 1.4 in the list in the 1st paragraph of section 8-1.02D(10).**

04-20-12

**Replace the last paragraph of section 8-1.04B with:**

10-19-12

The Department does not adjust time for starting before receiving notice of Contract approval.

**Replace the 1st paragraph of section 8-1.05 with:**

10-19-12

Contract time starts on the last day specified to start job site activities in section 8-1.04 or on the day you start job site activities, whichever occurs first.

**Replace the 2nd paragraph of section 8-1.05 with:**

10-19-12

Complete the work within the Contract time.

**Delete "unless the Contract is suspended for reasons unrelated to your performance" in the 4th paragraph of section 8-1.05.**

10-19-12

**Replace the headings and paragraphs in section 8-1.06 with:**

10-19-12

The Engineer may suspend work wholly or in part due to conditions unsuitable for work progress. Provide for public safety and a smooth and unobstructed passageway through the work zone during the suspension as specified under sections 7-1.03 and 7-1.04. Providing the passageway is force account work. The Department makes a time adjustment for the suspension due to a critical delay.

The Engineer may suspend work wholly or in part due to your failure to (1) fulfill the Engineer's orders, (2) fulfill a Contract part, or (3) perform weather-dependent work when conditions are favorable so that weather-related unsuitable conditions are avoided or do not occur. The Department may provide for a



Cost	Percent markup
Labor	30
Materials	10
Equipment rental	10

Delete ", Huntington Beach," in the 3rd paragraph of section 9-1.07A.

04-20-12

Replace the formula in section 9-1.07B(2) with:

$$Q_h = HMATT \times X_a$$

04-20-12

Replace "weight of dry aggregate" in the definition of the variable  $X_a$  in section 9-1.07B(2) with:

total weight of HMA

04-20-12

Replace the formula in section 9-1.07B(3) with:

$$Q_{rh} = RHMATT \times 0.80 \times X_{arb}$$

04-20-12

Replace "weight of dry aggregate" in the definition of the variable  $X_{arb}$  in section 9-1.07B(3) with:

total weight of rubberized HMA

04-20-12

Replace the heading of section 9-1.07B(4) with:

Hot Mix Asphalt with Modified Asphalt Binder

04-20-12

Add between "in" and "modified" in the introductory clause of section 9-1.07B(4):

HMA with

04-20-12

Replace the formula in section 9-1.07B(4) with:

$$Q_{mh} = MHMATT \times [(100 - X_{am}) / 100] \times X_{mab}$$

04-20-12

Replace "weight of dry aggregate" in the definition of the variable  $X_{mab}$  in section 9-1.07B(4) with:

total weight of HMA

04-20-12

Replace the formula in section 9-1.07B(5) with:

$$Q_{rap} = HMATT \times X_{aa}$$

04-20-12

**Replace "weight of dry aggregate" in the definitions of the variables  $X_{aa}$  and  $X_{ta}$  in section 9-1.07B(5) with:**

04-20-12

total weight of HMA

**Add after the variable definitions in section 9-1.07B(9):**

04-20-12

The quantity of extender oil is included in the quantity of asphalt.

**Replace the headings and paragraphs in section 9-1.11 with:**

10-19-12

**9-1.11A General**

Section 9-1.11 applies if a bid item for time-related overhead is included in the Contract. If a bid item for time-related overhead is included, you must exclude the time-related overhead from every other bid item price.

**9-1.11B Payment Quantity**

The TRO quantity does not include the number of working days to complete plant establishment work.

For a contract with a TRO lump sum quantity on the Bid Item List, the Department pays you based on the following conversions:

1. LS unit of measure is replaced with WDAY
2. Lump sum quantity is replaced with the number of working days bid
3. Lump sum unit price is replaced with the item total divided by the number of working days bid

**9-1.11C Payment Inclusions**

Payment for the TRO bid item includes payment for time-related field- and home-office overhead for the time required to complete the work.

The field office overhead includes time-related expenses associated with the normal and recurring construction activities not directly attributed to the work, including:

1. Salaries, benefits, and equipment costs of:
  - 1.1. Project managers
  - 1.2. General superintendents
  - 1.3. Field office managers
  - 1.4. Field office staff assigned to the project
2. Rent
3. Utilities
4. Maintenance
5. Security
6. Supplies
7. Office equipment costs for the project's field office

The home-office overhead includes the fixed general and administrative expenses for operating your business, including:

1. General administration
2. Insurance
3. Personnel and subcontract administration
4. Purchasing
5. Accounting
6. Project engineering and estimating

Payment for the TRO bid item does not include payment for:

1. The home-office overhead expenses specifically related to:
  - 1.1. Your other contracts or other businesses
  - 1.2. Equipment coordination
  - 1.3. Material deliveries
  - 1.4. Consultant and legal fees
2. Non-time-related costs and expenses such as mobilization, licenses, permits, and other charges incurred once during the Contract
3. Additional overhead involved in incentive/disincentive provisions to satisfy an internal milestone or multiple calendar requirements
4. Additional overhead involved in performing additional work that is not a controlling activity
5. Overhead costs incurred by your subcontractors of any tier or suppliers

#### **9-1.11D Payment Schedule**

For progress payments, the total work completed for the TRO bid item is the number of working days shown for the pay period on the *Weekly Statement of Working Days*.

For progress payments, the Department pays a unit price equal to the lesser of the following amounts:

1. Price per working day as bid or as converted under section 9-1.11B.
2. 20 percent of the total bid divided by the number of original working days

For a contract without plant establishment work, the Department pays you the balance due of the TRO item total as specified in section 9-1.17B.

For a contract with plant establishment work, the Department pays you the balance due of the TRO item total in the 1st progress payment after all non-plant establishment work is completed.

#### **9-1.11E Payment Adjustments**

The 3rd paragraph of section 9-1.17C does not apply.

The Department does not adjust the unit price for an increase or decrease in the TRO quantity except as specified in section 9-1.11E.

Section 9-1.17D(2)(b) does not apply except as specified for the audit report below.

If the TRO bid item quantity exceeds 149 percent of the quantity shown on the Bid Item List or as converted under section 9-1.11B, the Engineer may adjust or you may request an adjustment of the unit price for the excess quantity. For the adjustment, submit an audit report within 60 days of the Engineer's request. The report must be prepared as specified for an audit report for an overhead claim in section 9-1.17D(2)(b).

Within 20 days of the Engineer's request, make your financial records available for an audit by the State for the purpose of verifying the actual rate of TRO described in your audit. The actual rate of TRO described is subject to the Engineer's authorization.

The Department pays the authorized actual rate for TRO in excess of 149 percent of the quantity shown on the Bid Item List or as converted under section 9-1.11B.

The Department pays for 1/2 the cost of the report; the Contractor pays for the other 1/2. The cost is determined under section 9-1.05.

**Delete "revised Contract" in item 1 of the 1st paragraph of section 9-1.16E(2).**

10-19-12

**Replace "2014" in the 1st paragraph of section 9-1.16F with:**

10-19-12

2020







**Replace "NEL violation" in item 3.6.2 in the list in the 1st paragraph of section 13-1.01D(3)(c) with:**

04-19-13

receiving water monitoring trigger

**Replace the 1st paragraph in section 13-2.01B with:**

04-19-13

Within 7 days after Contract approval, submit 2 copies of your WPCP for review. Allow 5 business days for review.

After the Engineer authorizes the WPCP, submit an electronic copy and 3 printed copies of the authorized WPCP.

If the RWQCB requires review of the authorized WPCP, the Engineer submits the authorized WPCP to the RWQCB for its review and comment. If the Engineer orders changes to the WPCP based on the RWQCB's comments, amend the WPCP within 3 business days.

**Replace the 1st paragraph in section 13-3.01B(2)(a) with:**

04-19-13

Within 15 days of Contract approval, submit 3 copies of your SWPPP for review. The Engineer provides comments and specifies the date when the review stopped if revisions are required. Change and resubmit a revised SWPPP within 15 days of receiving the Engineer's comments. The Department's review resumes when a complete SWPPP has been resubmitted.

When the Engineer authorizes the SWPPP, submit an electronic copy and 4 printed copies of the authorized SWPPP.

If the RWQCB requires review of the authorized SWPPP, the Engineer submits the authorized SWPPP to the RWQCB for its review and comment. If the Engineer requests changes to the SWPPP based on the RWQCB's comments, amend the SWPPP within 10 days.

**Replace "NELs" in item 3.1 in the 3rd paragraph of section 13-3.01B(2)(a) with:**

04-19-13

receiving water monitoring triggers

**Replace section 13-3.01B(6)(c) with:**

04-19-13

**13-3.01B(6)(c) Receiving Water Monitoring Trigger Report**

Whenever a receiving water monitoring trigger is exceeded, notify the Engineer and submit a receiving water monitoring trigger report within 48 hours after conclusion of a storm event. The report must include:

1. Field sampling results and inspections, including:
  - 1.1. Analytical methods, reporting units, and detection limits
  - 1.2. Date, location, time of sampling, visual observation and measurements
  - 1.3. Quantity of precipitation from the storm event
2. Description of BMPs and corrective actions

**Replace "NEL" in the 6th paragraph of section 13-3.01C(1) with:**

04-19-13

receiving water monitoring trigger

Replace section 13-3.01C(3) with:

04-19-13

**13-3.01C(3) Receiving Water Monitoring Trigger**

For a risk level 3 project, receiving water monitoring triggers must comply with the values shown in the following table:

**Receiving Water Monitoring Trigger**

Parameter	Test method	Detection limit (min)	Unit	Value
pH	Field test with calibrated portable instrument	0.2	pH	Lower limit = 6.0 Upper limit = 9.0
Turbidity	Field test with calibrated portable instrument	1	NTU	500 NTU max

The storm event daily average for storms up to the 5-year, 24-hour storm must not exceed the receiving water monitoring trigger for turbidity.

The daily average sampling results must not exceed the receiving water monitoring trigger for pH.

04-19-13

**Delete "and NELs are violated" in the 3rd paragraph of section 13-3.03C.**

**Replace "working days" at each occurrence in section 13-3.04 with.**

original working days

10-19-12

04-19-13

**Delete the 1st sentence in the 2nd paragraph of section 13-4.03C(3).**

**Add between the 2nd and 3rd paragraphs of section 13-4.03C(3):**

Manage stockpiles by implementing water pollution control practices on:

1. Active stockpiles before a forecasted storm event
2. Inactive stockpiles according to the WPCP or SWPPP schedule

04-19-13

**Replace the paragraph in section 13-4.04 with:**

Not Used

04-20-12

10-19-12

**Delete "or stockpile" in the 3rd paragraph of section 13-5.02F.**



5. Be fastened securely to the existing frame without projections above the surface of the road or into the clear opening

**Add to the end of section 15-4.01A(2):**

Allow 20 days for review of the bridge removal work plan.

04-19-13

**Replace the 1st paragraph of section 15-5.01C(1) with:**

Before starting deck rehabilitation activities, complete the removal of any traffic stripes, pavement markings, and pavement markers.

10-19-12

**Replace the 2nd and 3rd paragraphs of section 15-5.01C(2) with:**

Perform the following activities in the order listed:

10-19-12

1. Abrasive blast the deck surface with steel shot. Perform abrasive blasting after the removal of any unsound concrete and placement of any rapid setting concrete patches.
2. Sweep the deck surface.
3. Blow the deck surface clean using high-pressure air.

**Replace the 2nd paragraph of section 15-5.01C(4) with:**

Before removing asphalt concrete surfacing, verify the depth of the surfacing at the supports and midspans of each structure (1) in each shoulder, (2) in the traveled way, and (3) at the roadway crown, if a crown is present.

10-19-12

**Delete "and concrete expansion dams" in the 3rd paragraph of section 15-5.01C(4).**

04-19-13

**Replace the 2nd paragraph of section 15-5.03A(2) with:**

For a contract with less than 60 original working days, submit certificates of compliance for the filler material and bonding agents.

10-19-12

**Replace "51-1.02C" in the 1st paragraph of section 15-5.03B with:**

51-1.02F

04-19-13

**Replace the 4th paragraph of section 15-5.03B with:**

For a contract with less than 60 original working days, alternative materials must be authorized before use.

10-19-12

**Add between the 5th and 6th paragraphs of section 15-5.03C:**

The final surface finish of the patched concrete surface must comply with section 51-1.03F.

10-19-12

**Delete the 4th paragraph of section 15-5.05C.**

10-19-12

**Replace "51-1.03F(5)" in the 3rd paragraph of section 15-5.06C(1) with:**

51-1.01D(4)

10-19-12

**Replace "51-1.03E(5)" in the 5th paragraph of section 15-5.06C(1) with:**

51-1.03F(5)

10-19-12

**Delete the 9th paragraph of section 15-5.06C(1).**

10-19-12

**Delete the 15th paragraph of section 15-5.06C(1).**

04-19-13

**Add to section 15-5.06C(1):**

Texture the polyester concrete surface before gelling occurs by longitudinal tining under 51-1.03F(5)(b)(iii), except do not perform initial texturing.

10-19-12

**Replace section 15-5.06C(2) with:**

**15-5.06C(2) Reserved**

04-19-13

**Delete the 3rd paragraph of section 15-5.06D.**

04-19-13

**Replace the 1st paragraph in section 15-5.07B(4) with:**

Payment for furnishing dowels is not included in the payment for core and pressure grout dowel.

10-19-12

**Replace section 15-5.09 with:**

**15-5.09 POLYESTER CONCRETE EXPANSION DAMS**

04-19-13

**15-5.09A General**

Section 15-5.09 includes specifications for constructing polyester concrete expansion dams.

Polyester concrete expansion dams must comply with the specifications for polyester concrete overlays in section 15-5.06, except a trial slab is not required.



**Replace "sets" in the 3rd and 4th paragraphs of section 19-3.01A(2)(d) with:**

copies

04-19-13

**Add to section 19-3.01A(3)(b):**

For soil nail walls, wall zones are specified in the special provisions.

01-20-12

For ground anchor walls, a wall zone is the entire wall unless otherwise specified in the special provisions.

**Delete the 2nd sentence in the 4th paragraph of section 19-3.01A(3)(b).**

01-20-12

**Replace "90" in the paragraph of section 19-3.02G with:**

90-1

01-18-13

**Replace the heading of section 19-3.03C with:**

**19-3.03B(4) Cofferdams**

04-19-13

**Replace the heading of section 19-3.03D with:**

**19-3.03B(5) Water Control and Foundation Treatment**

04-19-13

**Replace the 1st paragraph of section 19-3.03E(3) with:**

Compact structure backfill behind lagging of soldier pile walls by hand tamping, mechanical compaction, or other authorized means.

01-20-12

**Replace the 2nd paragraph of section 19-3.03F with:**

Do not backfill over or place material over slurry cement backfill until 4 hours after placement. When concrete sand is used as aggregate and the in-place material is free draining, you may start backfilling as soon as the surface water is gone.

01-20-12

**Add between the 2nd and 3rd paragraphs of section 19-3.03K:**

Before you excavate for the installation of ground anchors in a wall zone:

01-20-12

1. Complete stability testing
2. Obtain authorization of test data











**binder replacement:** Amount of RAP binder in OBC in percent.

**surface course:** Upper 0.2 feet of HMA exclusive of OGFC.

**Add to the end of the paragraph in section 39-1.02A:**

10-19-12

as shown

**Replace the paragraphs in section 39-1.02F with:**

02-22-13

**39-1.02F(1) General**

You may produce HMA Type A or B using RAP. HMA produced using RAP must comply with the specifications for HMA, except aggregate quality specifications do not apply to RAP. You may substitute RAP at a substitution rate not exceeding 25 percent of the aggregate blend. Do not use RAP in OGFC and RHMA-G.

Assign the substitution rate of RAP aggregate for virgin aggregate with the JMF submittal. The JMF must include the percent of RAP used.

Provide enough space for meeting RAP handling requirements at your facility. Provide a clean, graded, well-drained area for stockpiles. Prevent material contamination and segregation.

If RAP is from multiple sources, blend the RAP thoroughly and completely. RAP stockpiles must be homogeneous.

Isolate the processed RAP stockpiles from other materials. Store processed RAP in conical or longitudinal stockpiles. Processed RAP must not be agglomerated or be allowed to congeal in large stockpiles.

AASHTO T 324 (Modified) is AASHTO T 324, "Hamburg Wheel-Track Testing of Compacted Hot Mix Asphalt (HMA)," with the following parameters:

1. Target air voids must equal  $7 \pm 1$  percent
2. Number of test specimens must be 4
3. Test specimen must be a 6-inch gyratory compacted specimen
4. Test temperature must be set at  $140 \pm 2$  degrees F
5. Measurements for impression must be taken at every 100 passes
6. Inflection point defined as the number of wheel passes at the intersection of the creep slope and the stripping slope
7. Testing shut off must be set at 25,000 passes

**39-1.02F(2) Substitution Rate of 15 Percent or Less**

For a RAP substitution rate of 15 percent or less, you may stockpile RAP during the entire project.

**39-1.02F(3) Substitution Rate Greater than 15 Percent**

For a RAP substitution rate greater than 15 percent, fractionate RAP into 2 sizes, a coarse fraction RAP retained on 1/4-inch screen and a fine fraction RAP passing 1/4-inch screen.

Sample and test processed RAP at a minimum frequency of 1 sample per 1000 tons with a minimum of 6 samples for each processed RAP stockpile. The asphalt binder content and specific gravity must meet the processed RAP quality characteristics. If a processed RAP stockpile is augmented, sample and test processed RAP quality characteristics at a minimum frequency of 1 sample per 500 tons of augmented RAP.

The processed RAP asphalt binder content must be within  $\pm 2.0$  percent of the average processed RAP stockpile asphalt binder content when tested under ASTM D 2172, Method B. If a new processed RAP stockpile is required, the average binder content of the new processed RAP stockpile must be within  $\pm 2.0$  percent of the average binder content of the original processed RAP stockpile.

The maximum specific gravity for processed RAP must be within  $\pm 0.06$  when tested under California Test 309 of the average maximum specific gravity reported on page 4 of your *Contractor Hot Mix Asphalt Design Data* form.

**Replace "less than 10 percent" in note "b" in the table in the 5th paragraph of section 39-1.02E with:**

01-20-12

10 percent or less

**Replace items 7 and 8 in the 5th paragraph of section 39-1.03A with:**

02-22-13

7. Substitution rate by more than 5 percent if your assigned RAP substitution rate is 15 percent or less
8. Substitution rate by more than 3 percent if your assigned RAP substitution rate is greater than 15 percent
9. Average binder content by more than 2 percent from the average binder content of the original processed RAP stockpile used in the mix design
10. Maximum specific gravity of processed RAP by more than  $\pm 0.060$  from the average maximum specific gravity of processed RAP reported on page 4 of your *Contractor Hot Mix Asphalt Design Data* form
11. Any material in the JMF

**Replace the 1st paragraph of section 39-1.03B with:**

02-22-13

Perform a mix design that produces HMA with the values for the quality characteristics shown in the following table:

**HMA Mix Design Requirements**

Quality characteristic	Test method	HMA type		
		A	B	RHMA-G
Air void content (%)	California Test 367	4.0	4.0	Section 39-1.03B
Voids in mineral aggregate (% min.) No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367	17.0	17.0	--
		15.0	15.0	--
		14.0	14.0	18.0–23.0
		13.0	13.0	18.0–23.0
Voids filled with asphalt (%) No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367	65.0–75.0	65.0–75.0	Note a
		65.0–75.0	65.0–75.0	
		65.0–75.0	65.0–75.0	
		65.0–75.0	65.0–75.0	
Dust proportion No. 4 and 3/8" gradings 1/2" and 3/4" gradings	California Test 367	0.6–1.2	0.6–1.2	Note a
		0.6–1.2	0.6–1.2	
Stabilometer value (min.) No. 4 and 3/8" gradings 1/2" and 3/4" gradings	California Test 366	30	30	--
		37	35	23

<sup>a</sup> Report this value in the JMF submittal.

For RAP substitution rate greater than 15 percent, the mix design must comply with the additional quality characteristics shown in the following table:

**Additional HMA Mix Design Requirements  
for RAP Substitution Rate Greater Than 15 Percent**

Quality characteristic	Test method	HMA type		
		A	B	RHMA-G
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth)	AASHTO T 324 (Modified) <sup>a</sup>			
PG-58		10,000	10,000	--
PG-64		15,000	15,000	
PG-70		20,000	20,000	
PG-76 or higher		25,000	25,000	
Hamburg wheel track (inflection point minimum number of passes)	AASHTO T 324 (Modified) <sup>a</sup>			
PG-58		10,000	10,000	--
PG-64		10,000	10,000	
PG-70		12,500	12,500	
PG-76 or higher		15000	15000	
Moisture susceptibility (minimum dry strength, psi)	California Test 371 <sup>a</sup>	120	120	--
Moisture susceptibility (tensile strength ration, %)	California Test 371 <sup>a</sup>	70	70	--

<sup>a</sup>Test plant produced HMA.

For HMA with RAP, the maximum binder replacement must be 25.0 percent of OBC for surface course and 40.0 percent of OBC for lower courses.

For HMA with a binder replacement less than or equal to 25 percent of OBC, you may request that the PG asphalt binder grade with upper and lower temperature classifications be reduced by 6 degrees C from the specified grade.

For HMA with a binder replacement greater than 25 percent but less than or equal to 40 percent of OBC, you must use a PG asphalt binder grade with upper and lower temperature classifications reduced by 6 degrees C from the specified grade.

**Replace item 4 in the list in the 1st paragraph of section 39-1.03C with:**

4. JMF renewal on a *Caltrans Job Mix Formula Renewal* form, if applicable

01-20-12

**Add after the last paragraph of section 39-1.03C:**

For RAP substitution rate greater than 15 percent, submit with the JMF submittal:

1. California Test 371 tensile strength ratio and minimum dry strength test results
2. AASHTO T 324 (Modified) test results

02-22-13

For RAP substitution rate greater than 15 percent, submit California Test 371 and AASHTO T 324 (Modified) test results to the Engineer and to:

Moisture\_Tests@dot.ca.gov

**Replace the 2nd paragraph of section 39-1.03E with:**

04-20-12

Use the OBC specified on your *Contractor Hot Mix Asphalt Design Data* form. No adjustments to asphalt binder content are allowed. Based on your testing and production experience, you may submit an adjusted aggregate gradation TV on a *Contractor Job Mix Formula Proposal* form before verification testing. Aggregate gradation TV must be within the TV limits specified in the aggregate gradation tables.

**Add between the 3rd and 4th paragraphs of section 39-1.03E:**

04-20-12

Asphalt binder set point for HMA must be the OBC specified on your *Contractor Hot Mix Asphalt Design Data* form. When RAP is used, asphalt binder set point for HMA must be:

$$\text{Asphalt Binder Set Point} = \frac{\frac{BC_{OBC}}{\left(1 - \frac{BC_{OBC}}{100}\right)} - R_{RAP} \left[ \frac{BC_{RAP}}{\left(1 - \frac{BC_{RAP}}{100}\right)} \right]}{100 + \frac{BC_{OBC}}{\left(1 - \frac{BC_{OBC}}{100}\right)}}$$

Where:

$BC_{OBC}$  = optimum asphalt binder content, percent based on total weight of mix

$R_{RAP}$  = RAP ratio by weight of aggregate

$BC_{RAP}$  = asphalt binder content of RAP, percent based on total weight of RAP mix

**Replace item 4 in the list in the 8th paragraph of section 39-1.03E with:**

04-20-12

4. HMA quality specified in the table titled "HMA Mix Design Requirements" except:
  - 4.1. Air void content, design value  $\pm 2.0$  percent
  - 4.2. Voids filled with asphalt, report only
  - 4.3. Dust proportion, report only

**Replace the 12th paragraph of section 39-1.03E with:**

04-20-12

If tests on plant-produced samples do not verify the JMF, the Engineer notifies you and you must submit a new JMF or submit an adjusted JMF based on your testing. JMF adjustments may include a change in aggregate gradation TV within the TV limits specified in the aggregate gradation tables.

**Replace the 14th paragraph of section 39-1.03E with:**

01-20-12

A verified JMF is valid for 12 months.

**Replace the last sentence in the 15th paragraph of section 39-1.03E with:**

01-20-12

This deduction does not apply to verifications initiated by the Engineer or JMF renewal.

**Replace the 16th paragraph of section 39-1.03E with:**

02-22-13

Except for RAP substitution rate greater than 15 percent, for any HMA produced under the QC/QA process the Department does not use California Test 371 test results for verification.

**Add between the 1st and 2nd paragraphs of section 39-1.03F:**

04-20-12

Target asphalt binder content on your Contractor *Job Mix Formula Proposal* form and the OBC specified on your *Contractor Hot Mix Asphalt Design Data* form must be the same.

**Delete the 4th paragraph of section 39-1.03F.**

01-20-12

**Replace items 3 and 5 in the list in the 6th paragraph of section 39-1.03F with:**

01-20-12

3. Engineer verifies each proposed JMF renewal within 20 days of receiving verification samples.
5. For each HMA type and aggregate gradation specified, the Engineer verifies at the Department's expense 1 proposed JMF renewal within a 12-month period.

**Add between the 6th and 7th paragraphs of section 39-1.03F:**

01-20-12

The most recent aggregate quality test results within the past 12 months may be used for verification of JMF renewal or the Engineer may perform aggregate quality tests for verification of JMF renewal.

**Replace section 39-1.03G with:**

04-20-12

**39-1.03G Job Mix Formula Modification**

For an accepted JMF, you may change asphalt binder source one time during production.

Submit your modified JMF request a minimum of 3 business days before production. Each modified JMF submittal must consist of:

1. Proposed modified JMF on *Contractor Job Mix Formula Proposal* form
2. Mix design records on *Contractor Hot Mix Asphalt Design Data* form for the accepted JMF to be modified
3. JMF verification on *Hot Mix Asphalt Verification* form for the accepted JMF to be modified
4. Quality characteristics test results for the modified JMF as specified in section 39-1.03B. Perform tests at the mix design OBC as shown on the *Contractor Asphalt Mix Design Data* form
5. If required, California Test 371 test results for the modified JMF.

With an accepted modified JMF submittal, the Engineer verifies each modified JMF within 5 business days of receiving all verification samples. If California Test 371 is required, the Engineer tests for California Test 371 within 10 days of receiving verification samples.

The Engineer verifies the modified JMF after the modified JMF HMA is placed on the project and verification samples are taken within the first 750 tons following sampling requirements in section 39-1.03E, "Job Mix Formula Verification." The Engineer tests verification samples for compliance with:

1. Stability as shown in the table titled "HMA Mix Design Requirements"
2. Air void content at design value  $\pm 2.0$  percent
3. Voids in mineral aggregate as shown in the table titled "HMA Mix Design Requirements"
4. Voids filled with asphalt, report only

5. Dust proportion, report only

If the modified JMF is verified, the Engineer revises your *Hot Mix Asphalt Verification* form to include the new asphalt binder source. Your revised form will have the same expiration date as the original form.

If a modified JMF is not verified, stop production and any HMA placed using the modified JMF is rejected.

The Engineer deducts \$2,000 from payments for each modified JMF verification. The Engineer deducts an additional \$2,000 for each modified JMF verification that requires California Test 371.

**Add to section 39-1.03:**

01-20-12

**39-1.03H Job Mix Formula Acceptance**

You may start HMA production if:

1. The Engineer's review of the JMF shows compliance with the specifications.
2. The Department has verified the JMF within 12 months before HMA production.
3. The Engineer accepts the verified JMF.

**Replace "3 days" in the 1st paragraph of section 39-1.04A with:**

01-20-12

3 business days

**Replace the 2nd sentence in the 2nd paragraph of section 39-1.04A with:**

01-20-12

During production, take samples under California Test 125. You may sample HMA from:

**Replace the 2nd paragraph of section 39-1.04E with:**

02-22-13

For RAP substitution rate of 15 percent or less, sample RAP once daily.

For RAP substitution rate of greater than 15percent, sample processed RAP twice daily.

Perform QC testing for processed RAP aggregate gradation under California Test 367, appendix B, and submit the results with the combined aggregate gradation.

**Replace "5 days" in the 1st paragraph of section 39-1.06 with:**

01-20-12

5 business days

**Replace the 3rd paragraph of section 39-1.08A with:**

04-20-12

During production, you may adjust hot or cold feed proportion controls for virgin aggregate and RAP.

**Add to section 39-1.08A:**

04-20-12

During production, asphalt binder set point for HMA Type A, HMA Type B, HMA Type C, and RHMA-G must be the OBC shown in *Contractor Hot Mix Asphalt Design Data* form. For OGFC, asphalt binder set

point must be the OBC shown on *Caltrans Hot Mix Asphalt Verification* form. If RAP is used, asphalt binder set point for HMA must be calculated as specified in section 39-1.03E.

02-22-13

For RAP substitution rate of 15 percent or less, you may adjust the RAP by  $\pm 5$  percent.

For RAP substitution greater than 15, you may adjust the RAP by  $\pm 3$  percent.

04-20-12

You must request adjustments to the plant asphalt binder set point based on new RAP stockpiles average asphalt binder content. Do not adjust the HMA plant asphalt binder set point until authorized.

**Replace the 3rd paragraph of section 39-1.08B with:**

09-16-11

Asphalt rubber binder must be from 375 to 425 degrees F when mixed with aggregate.

**Replace section 39-1.11 with:**

01-18-13

**39-1.11 CONSTRUCTION**

**39-1.11A General**

Do not place HMA on wet pavement or a frozen surface.

You may deposit HMA in a windrow and load it in the paver if:

1. Paver is equipped with a hopper that automatically feeds the screed
2. Loading equipment can pick up the windrowed material and deposit it in the paver hopper without damaging base material
3. Activities for deposit, pickup, loading, and paving are continuous
4. HMA temperature in the windrow does not fall below 260 degrees F

You may place HMA in 1 or more layers on areas less than 5 feet wide and outside the traveled way, including shoulders. You may use mechanical equipment other than a paver for these areas. The equipment must produce uniform smoothness and texture.

HMA handled, spread, or windrowed must not stain the finished surface of any improvement, including pavement.

Do not use petroleum products such as kerosene or diesel fuel to release HMA from trucks, spreaders, or compactors.

HMA must be free of:

1. Segregation
2. Coarse or fine aggregate pockets
3. Hardened lumps

**39-1.11B Longitudinal Joints**

**39-1.11B(1) General**

Longitudinal joints in the top layer must match specified lane edges. Alternate the longitudinal joint offsets in the lower layers at least 0.5 foot from each side of the specified lane edges. You may request other longitudinal joint placement patterns.

A vertical longitudinal joint of more than 0.15 ft is not allowed at any time between adjacent lanes open to traffic.

For HMA thickness of 0.15 ft or less, the distance between the ends of the adjacent surfaced lanes at the end of each day's work must not be greater than can be completed in the following day of normal paving.

For HMA thickness greater than 0.15 ft, you must place HMA on adjacent traveled way lanes so that at the end of each work shift the distance between the ends of HMA layers on adjacent lanes is from 5 to 10 feet. Place additional HMA along the transverse edge at each lane's end and along the exposed longitudinal edges between adjacent lanes. Hand rake and compact the additional HMA to form temporary conforms. You may place Kraft paper or another authorized bond breaker under the conform tapers to facilitate the taper removal when paving operations resume.

### **39-1.11B(2) Tapered Notched Wedge**

For divided highways with an HMA lift thickness greater than 0.15 foot, you may construct a 1-foot wide tapered notched wedge joint as a longitudinal joint between adjacent lanes open to traffic. A vertical notch of 0.75 inch maximum must be placed at the top and bottom of the tapered wedge.

The tapered notched wedge must retain its shape while exposed to traffic. Pave the adjacent lane within 1 day.

Construct the tapered portion of the tapered notched wedge with an authorized strike-off device. The strike-off device must provide a uniform slope and must not restrict the main screed of the paver.

You may use a device attached to the screed to construct longitudinal joints that will form a tapered notched wedge in a single pass. The tapered notched wedge must be compacted to a minimum of 91 percent compaction.

Perform QC testing on the completed tapered notch wedge joint as follows:

1. Perform field compaction tests at the rate of 1 test for each 750-foot section along the joint. Select random locations for testing within each 750-foot section.
2. Perform field compaction tests at the centerline of the joint, 6 inches from the upper vertical notch, after the adjacent lane is placed and before opening the pavement to traffic.
3. Determine maximum density test results.
4. Determine percent compaction of the longitudinal joint as the ratio of the average of the field compaction values and the maximum density test results.

For HMA under QC/QA construction process, the additional quality control compaction results associated with the tapered notch wedge will not be included in the computation of any quality factor and process control.

For acceptance of the completed tapered notch wedge joint, take two 4- or 6-inch diameter cores 6 inches from the upper vertical notch of the completed longitudinal joint for every 3,000 feet at locations designated by the Engineer. Take cores after the adjacent lane is placed and before opening the pavement to traffic. Cores must be taken in the presence of the Engineer and must be marked to identify the test sites. Submit the cores. One core will be used for determination of the field density and 1 core will be used for dispute resolution. The Engineer determines:

1. Field compaction by measuring the bulk specific gravity of the cores under California Test 308, Method A
2. Percent compaction as the ratio of the average of the bulk specific gravity of the core for each day's production to the maximum density test value

For HMA under QC/QA construction process, the additional quality assurance testing by the Engineer to determine field compaction associated with the tapered notch wedge will not be included in the Engineer's verification testing and in the computation of any quality factor and process control.

Determine percent compaction values each day the joint is completed and submit values within 24 hours of testing. If the percent compaction of 1 day's production is less than 91 percent, that day's notched wedge joint is rejected. Discontinue placement of the tapered notched wedge and notify the Engineer of changes you will make to your construction process in order to meet the specifications.

For HMA under QC/QA construction process, quantities of HMA placed in the completed longitudinal joint will have a quality factor  $QF_{QC5}$  of 1.0.

### **39-1.11C Widening Existing Pavement**

If widening existing pavement, construct new pavement structure to match the elevation of the existing pavement's edge before placing HMA over the existing pavement.

### **39-1.11D Shoulders, Medians, and Other Road Connections**

Until the adjoining through lane's top layer has been paved, do not pave the top layer of:

1. Shoulders
2. Tapers
3. Transitions
4. Road connections
5. Driveways
6. Curve widenings
7. Chain control lanes
8. Turnouts
9. Turn pockets

If the number of lanes changes, pave each through lane's top layer before paving a tapering lane's top layer. Simultaneous to paving a through lane's top layer, you may pave an adjoining area's top layer, including shoulders. Do not operate spreading equipment on any area's top layer until completing final compaction.

### **39-1.11E Leveling**

If leveling with HMA is specified, fill and level irregularities and ruts with HMA before spreading HMA over the base, existing surfaces, or bridge decks. You may use mechanical equipment other than a paver for these areas. The equipment must produce uniform smoothness and texture. HMA used to change an existing surface's cross slope or profile is not paid for as HMA (leveling).

If placing HMA against the edge of existing pavement, sawcut or grind the pavement straight and vertical along the joint and remove extraneous material.

### **39-1.11F Compaction**

Rolling must leave the completed surface compacted and smooth without tearing, cracking, or shoving. Complete finish rolling activities before the pavement surface temperature is:

1. Below 150 degrees F for HMA with unmodified binder
2. Below 140 degrees F for HMA with modified binder
3. Below 200 degrees F for RHMA-G

If a vibratory roller is used as a finish roller, turn the vibrator off.

Do not use a pneumatic-tired roller to compact RHMA-G.

For Standard and QC/QA construction processes, if 3/4-inch aggregate grading is specified, you may use a 1/2-inch aggregate grading if the specified total paved thickness is at least 0.15 foot and less than 0.20 foot thick.

Spread and compact HMA under sections 39-3.03 and 39-3.04 if any of the following applies:

1. Specified paved thickness is less than 0.15 foot.
2. Specified paved thickness is less than 0.20 foot and 3/4-inch aggregate grading is specified and used.
3. You spread and compact at:
  - 3.1. Asphalt concrete surfacing replacement areas
  - 3.2. Leveling courses
  - 3.3. Areas for which the Engineer determines conventional compaction and compaction measurement methods are impeded

Do not open new HMA pavement to public traffic until its mid-depth temperature is below 160 degrees F.

If you request and if authorized, you may cool HMA Type A and Type B with water when rolling activities are complete. Apply water under section 17-3.

Spread sand at a rate from 1 to 2 lb/sq yd on new RHMA-G, RHMA-O, and RHMA-O-HB pavement when finish rolling is complete. Sand must be free of clay or organic matter. Sand must comply with section 90-1.02C(4)(c). Keep traffic off the pavement until spreading sand is complete.

**Replace the 5th and 6th paragraphs of section 39-1.12C with:**

07-20-12

On tangents and horizontal curves with a centerline radius of curvature 2,000 feet or more, the  $PI_0$  must be at most 2.5 inches per 0.1-mile section.

On horizontal curves with a centerline radius of curvature between 1,000 feet and 2,000 feet including pavement within the superelevation transitions, the  $PI_0$  must be at most 5 inches per 0.1-mile section.

**Add to section 39-1.12:**

01-20-12

**39-1.12E Reserved**

**Add to section 39-1.14:**

01-20-12

Prepare the area to receive HMA for miscellaneous areas and dikes, including any excavation and backfill as needed.

**Replace "6.8" in item 3 in the list in the 4th paragraph of section 39-1.14 with:**

04-20-12

6.4

**Replace "6.0" in item 3 in the list in the 4th paragraph of section 39-1.14 with:**

04-20-12

5.7

**Replace "6.8" in the 1st paragraph of section 39-1.15B with:**

04-20-12

6.4

**Replace "6.0" in the 1st paragraph of section 39-1.15B with:**

04-20-12

5.7

**Replace the 1st paragraph of section 39-2.02B with:**

02-22-13

Perform sampling and testing at the specified frequency for the quality characteristics shown in the following table:

**Minimum Quality Control—Standard Construction Process**

Quality characteristic	Test method	Minimum sampling and testing frequency	HMA type			
			A	B	RHMA-G	OGFC
Aggregate gradation <sup>a</sup>	California Test 202	1 per 750 tons and any remaining part at the end of the project	JMF ± Tolerance <sup>b</sup>			
Sand equivalent (min) <sup>c</sup>	California Test 217		47	42	47	--
Asphalt binder content (%)	California Test 379 or 382		JMF±0.40	JMF±0.40	JMF ± 0.40	JMF ± 0.40
HMA moisture content (% max)	California Test 226 or 370	1 per 2,500 tons but not less than 1 per paving day	1.0	1.0	1.0	1.0
Field compaction (% max. theoretical density) <sup>d,e</sup>	QC plan	2 per business day (min.)	91–97	91–97	91–97	--
Stabilometer value (min) <sup>c</sup> No. 4 and 3/8" gradings 1/2" and 3/4" gradings	California Test 366	1 per 4,000 tons or 2 per 5 business days, whichever is greater	30	30	--	--
			37	35	23	--
Air void content (%) <sup>c,f</sup>	California Test 367		4 ± 2	4 ± 2	TV ± 2	--
Aggregate moisture content at continuous mixing plants and RAP moisture content at continuous mixing plants and batch mixing plants <sup>g</sup>	California Test 226 or 370	2 per day during production	--	--	--	--
Percent of crushed particles coarse aggregate (% min) One fractured face Two fractured faces Fine aggregate (% min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face	California Test 205	As designated in the QC plan. At least once per project	90	25	--	90
			75	--	90	75
Los Angeles Rattler (% max) Loss at 100 rev.	California Test 211		70	20	70	90
			12	--	12	12

Loss at 500 rev.			45	50	40	40
Flat and elongated particles (% max by weight @ 5:1)	California Test 235		Report only	Report only	Report only	Report only
Fine aggregate angularity (% min) <sup>h</sup>	California Test 234		45	45	45	--
Voids filled with asphalt (%) <sup>i</sup> No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367		65.0-75.0 65.0-75.0 65.0-75.0 65.0-75.0	65.0-75.0 65.0-75.0 65.0-75.0 65.0-75.0	Report only	--
Voids in mineral aggregate (% min) <sup>i</sup> No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367		17.0 15.0 14.0 13.0	17.0 15.0 14.0 13.0	-- -- 18.0-23.0 18.0-23.0	--
Dust proportion <sup>l</sup> No. 4 and 3/8" gradings 1/2" and 3/4" gradings	California Test 367		0.6-1.2 0.6-1.2	0.6-1.2 0.6-1.2	Report only	--
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth) <sup>j</sup> PG-58 PG-64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)	1 per 10,000 tons or 1 per project whichever is more	10,000 15,000 20,000 25,000	10,000 15,000 20,000 25,000	--	--
Hamburg wheel track (inflection point minimum number of passes) <sup>j</sup> PG-58 PG-64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)	1 per 10,000 tons or 1 per project whichever is more	10,000 10,000 12,500 15000	10,000 10,000 12,500 15000	--	--
Moisture susceptibility (minimum dry strength, psi) <sup>j</sup>	California Test 371	For RAP ≥15% 1 per 10,000 tons or 1 per project whichever is greater	120	120	--	--
Moisture susceptibility (tensile strength ratio, %) <sup>j</sup>	California Test 371	For RAP ≥15% 1 per 10,000 tons or 1	70	70	--	--

		per project whichever is greater				
Smoothness	Section 39-1.12	--	12-foot straight- edge, must grind, and PI <sub>0</sub>			
Asphalt rubber binder viscosity @ 375 °F, centipoises	Section 39-1.02D	Section 39-1.04C	--	--	1,500– 4,000	1,500– 4,000
Asphalt modifier	Section 39-1.02D	Section 39-1.04C	--	--	Section 39-1.02D	Section 39-1.02D
CRM	Section 39-1.02D	Section 39-1.04C	--	--	Section 39-1.02D	Section 39-1.02D

<sup>a</sup> Determine combined aggregate gradation containing RAP under California Test 367.

<sup>b</sup> The tolerances must comply with the allowable tolerances in section 39-1.02E.

<sup>c</sup> Report the average of 3 tests from a single split sample.

<sup>d</sup> Determine field compaction for any of the following conditions:

1. 1/2-inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot.
2. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot.

<sup>e</sup> To determine field compaction use:

1. In-place density measurements using the method specified in your QC plan.
2. California Test 309 to determine the maximum theoretical density at the frequency specified in California Test 375, Part 5C.

<sup>f</sup> Determine the bulk specific gravity of each lab-compacted briquette under California Test 308, Method A, and theoretical maximum specific gravity under California Test 309.

<sup>g</sup> For adjusting the plant controller at the HMA plant.

<sup>h</sup> The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

<sup>i</sup> Report only.

<sup>j</sup> Applies to RAP substitution rate greater than 15 percent.

**Replace the 1st paragraph of section 39-2.03A with:**

02-22-13

The Department samples for acceptance testing and tests for the quality characteristics shown in the following table:

**HMA Acceptance—Standard Construction Process**

Quality characteristic	Test method	HMA type							
		A	B	RHMA-G	OGFC				
Aggregate gradation <sup>a</sup>	California Test 202	JMF ± tolerance <sup>c</sup>							
Sieve						3/4"	1/2"	3/8"	
1/2"						X <sup>b</sup>			
3/8"							X		
No. 4								X	
No. 8						X	X	X	
No. 200	X	X	X						
Sand equivalent (min) <sup>d</sup>	California Test 217	47	42	47	--				
Asphalt binder content (%)	California Test 379 or 382	JMF±0.40	JMF±0.40	JMF ± 0.40	JMF ± 0.40				
HMA moisture content (% max)	California Test 226 or 370	1.0	1.0	1.0	1.0				
Field compaction (% max. theoretical density) <sup>e, f</sup>	California Test 375	91–97	91–97	91–97	--				
Stabilometer value (min) <sup>g</sup>	California Test 366	30	30	--	--				
No. 4 and 3/8" gradings									
1/2" and 3/4" gradings						37	35	23	--
Air void content (%) <sup>d, g</sup>	California Test 367	4 ± 2	4 ± 2	TV ± 2	--				
Percent of crushed particles	California Test 205								
Coarse aggregate (% min)									
One fractured face						90	25	--	90
Two fractured faces						75	--	90	75
Fine aggregate (% min)									
(Passing no. 4 sieve and retained on no. 8 sieve.)									
One fractured face	70	20	70	90					
Los Angeles Rattler (% max)	California Test 211	12	--	12	12				
Loss at 100 rev.									
Loss at 500 rev.		45	50	40	40				
Fine aggregate angularity (% min) <sup>h</sup>	California Test 234	45	45	45	--				
Flat and elongated particles (% max by weight @ 5:1)	California Test 235	Report only	Report only	Report only	Report only				
Voids filled with asphalt (%) <sup>i</sup>	California Test 367	65.0–75.0	65.0–75.0	Report only	--				
No. 4 grading									
3/8" grading						65.0–75.0	65.0–75.0		
1/2" grading						65.0–75.0	65.0–75.0		
3/4" grading						65.0–75.0	65.0–75.0		
Voids in mineral aggregate (% min) <sup>i</sup>	California Test 367	17.0	17.0	--	--				
No. 4 grading									
3/8" grading						15.0	15.0		
1/2" grading						14.0	14.0	18.0–23.0	
3/4" grading						13.0	13.0	18.0–23.0	
Dust proportion <sup>i</sup>	California			Report only	--				

No. 4 and 3/8" gradings 1/2" and 3/4" gradings	Test 367	0.6-1.2 0.6-1.2	0.6-1.2 0.6-1.2		
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth) <sup>j</sup> PG-58 PG-64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)	10,000 15,000 20,000 25,000	10,000 15,000 20,000 25,000	--	--
Hamburg wheel track (inflection point minimum number of passes) <sup>j</sup> PG-58 PG-64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)	10,000 10,000 12,500 15000	10,000 10,000 12,500 15000	--	--
Moisture susceptibility (minimum dry strength, psi) <sup>j</sup>	California Test 371	120	120	--	--
Moisture susceptibility (tensile strength ration, %) <sup>j</sup>	California Test 371	70	70	--	--
Smoothness	Section 39-1.12	12-foot straight- edge, must grind, and PI <sub>0</sub>	12-foot straight- edge, must grind, and PI <sub>0</sub>	12-foot straight- edge, must grind, and PI <sub>0</sub>	12-foot straight- edge and must grind
Asphalt binder	Various	Section 92	Section 92	Section 92	Section 92
Asphalt rubber binder	Various	--	--	Section 92- 1.01D(2) and section 39-1.02D	Section 92-1.01D(2) and section 39-1.02D
Asphalt modifier	Various	--	--	Section 39-1.02D	Section 39-1.02D
CRM	Various	--	--	Section 39-1.02D	Section 39-1.02D

<sup>a</sup> The Engineer determines combined aggregate gradations containing RAP under California Test 367.

<sup>b</sup> "X" denotes the sieves the Engineer tests for the specified aggregate gradation.

<sup>c</sup> The tolerances must comply with the allowable tolerances in section 39-1.02E.

<sup>d</sup> The Engineer reports the average of 3 tests from a single split sample.

<sup>e</sup> The Engineer determines field compaction for any of the following conditions:

1. 1/2-inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot.
2. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot.

<sup>f</sup> To determine field compaction, the Engineer uses:

1. California Test 308, Method A, to determine in-place density of each density core.
2. California Test 309 to determine the maximum theoretical density at the frequency specified in California Test 375, Part 5C.

<sup>g</sup> The Engineer determines the bulk specific gravity of each lab-compacted briquette under California Test 308, Method A, and theoretical maximum specific gravity under California Test 309.

<sup>h</sup> The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

<sup>i</sup> Report only.

<sup>j</sup> Applies to RAP substitution rate greater than 15 percent.

**Replace the 5th paragraph of section 39-2.03A with:**

01-20-12

The Engineer determines the percent of maximum theoretical density from density cores taken from the final layer measured the full depth of the total paved HMA thickness if any of the following applies:

1. 1/2-inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot and any layer is less than 0.15 foot.
2. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.2 foot and any layer is less than 0.20 foot.

**Replace the 1st paragraph of section 39-3.02A with:**

02-22-13

The Department samples for acceptance testing and tests for the quality characteristics shown in the following table:

**HMA Acceptance—Method Construction Process**

Quality characteristic	Test method	HMA type			
		A	B	RHMA-G	OGFC
Aggregate gradation <sup>a</sup>	California Test 202	JMF ± tolerance <sup>b</sup>	JMF ± tolerance <sup>b</sup>	JMF ± tolerance <sup>b</sup>	JMF ± tolerance <sup>b</sup>
Sand equivalent (min) <sup>c</sup>	California Test 217	47	42	47	--
Asphalt binder content (%)	California Test 379 or 382	JMF±0.40	JMF±0.40	JMF ± 0.40	JMF ± 0.40
HMA moisture content (% max)	California Test 226 or 370	1.0	1.0	1.0	1.0
Stabilometer value (min) <sup>c</sup> No. 4 and 3/8" gradings 1/2" and 3/4" gradings	California Test 366	30 37	30 35	-- 23	-- --
Percent of crushed particles Coarse aggregate (% min) One fractured face Two fractured faces Fine aggregate (% min) (Passing no. 4 sieve and retained on no. 8 sieve.) One fractured face	California Test 205	90 75  70	25 -- 20	-- 90 70	90 75 90
Los Angeles Rattler (% max) Loss at 100 rev. Loss at 500 rev.	California Test 211	12 45	-- 50	12 40	12 40
Air void content (%) <sup>c, d</sup>	California Test 367	4 ± 2	4 ± 2	TV ± 2	--
Fine aggregate angularity (% min) <sup>e</sup>	California Test 234	45	45	45	--
Flat and elongated particles (% max by weight @ 5:1)	California Test 235	Report only	Report only	Report only	Report only
Voids filled with asphalt (%) <sup>f</sup> No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367	65.0–75.0 65.0–75.0 65.0–75.0 65.0–75.0	65.0–75.0 65.0–75.0 65.0–75.0 65.0–75.0	Report only	--
Voids in mineral aggregate (% min) <sup>f</sup> No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367	17.0 15.0 14.0 13.0	17.0 15.0 14.0 13.0	-- -- 18.0–23.0 18.0–23.0	--
Dust proportion <sup>f</sup> No. 4 and 3/8" gradings 1/2" and 3/4" gradings	California Test 367	0.6–1.2 0.6–1.2	0.6–1.2 0.6–1.2	Report only	--
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth) <sup>g</sup> PG-58 PG-64	AASHTO T 324 (Modified)	10,000 15,000	10,000 15,000	--	--

PG-70 PG-76 or higher		20,000 25,000	20,000 25,000		
Hamburg wheel track (inflection point minimum number of passes) <sup>g</sup>	AASHTO T 324 (Modified)			--	--
PG-58		10,000	10,000		
PG-64		10,000	10,000		
PG-70		12,500	12,500		
PG-76 or higher		15000	15000		
Moisture susceptibility (minimum dry strength, psi) <sup>g</sup>	California Test 371	120	120	--	--
Moisture susceptibility (tensile strength ration, %) <sup>g</sup>	California Test 371	70	70	--	--
Smoothness	Section 39-1.12	12-foot straight- edge and must-grind	12-foot straight- edge and must-grind	12-foot straight- edge and must-grind	12-foot straight- edge and must-grind
Asphalt binder	Various	Section 92	Section 92	Section 92	Section 92
Asphalt rubber binder	Various	--	--	Section 92- 1.01D(2) and section 39-1.02D	Section 92- 1.01D(2) and section 39-1.02D
Asphalt modifier	Various	--	--	Section 39-1.02D	Section 39-1.02D
CRM	Various	--	--	Section 39-1.02D	Section 39-1.02D

<sup>a</sup> The Engineer determines combined aggregate gradations containing RAP under California Test 367.

<sup>b</sup> The tolerances must comply with the allowable tolerances in section 39-1.02E.

<sup>c</sup> The Engineer reports the average of 3 tests from a single split sample.

<sup>d</sup> The Engineer determines the bulk specific gravity of each lab-compacted briquette under California Test 308, Method A, and theoretical maximum specific gravity under California Test 309.

<sup>e</sup> The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.

<sup>f</sup> Report only.

<sup>g</sup> Applies to RAP substitution rate greater than 15 percent.

**Replace "280 degrees F" in item 2 in the list in the 6th paragraph of section 39-3.04 with:**

285 degrees F

01-20-12

**Replace "5,000" in the 5th paragraph of section 39-4.02C with:**

10,000

02-22-13

**Replace the 7th paragraph of section 39-4.02C with:**

Except for RAP substitution rate of greater than 15 percent, the Department does not use results from California Test 371 to determine specification compliance.

02-22-13

**Replace the 8th paragraph of section 39-4.02C with:**

02-22-13

Comply with the values for the HMA quality characteristics and minimum random sampling and testing for quality control shown in the following table:

**Minimum Quality Control—QC/QA Construction Process**

Quality characteristic	Test method	Minimum sampling and testing frequency	HMA Type			Location of sampling	Maximum report-ing time allow-ance
			A	B	RHMA-G		
Aggregate gradation <sup>a</sup>	California Test 202	1 per 750 tons	JMF ± tolerance <sup>b</sup>	JMF ± tolerance <sup>b</sup>	JMF ± tolerance <sup>b</sup>	California Test 125	24 hours
Asphalt binder content (%)	California Test 379 or 382		JMF±0.40	JMF±0.40	JMF ±0.40	Loose mix behind paver See California Test 125	
Field compaction (% max. theoretical density) <sup>c,d</sup>	QC plan		92–96	92–96	91–96	QC plan	
Aggregate moisture content at continuous mixing plants and RAP moisture content at continuous mixing plants and batch mixing plants <sup>e</sup>	California Test 226 or 370	2 per day during production	--	--	--	Stock-piles or cold feed belts	--
Sand equivalent (min) <sup>f</sup>	California Test 217	1 per 750 tons	47	42	47	California Test 125	24 hours
HMA moisture content (% max)	California Test 226 or 370	1 per 2,500 tons but not less than 1 per paving day	1.0	1.0	1.0	Loose Mix Behind Paver See California Test 125	24 hours
Stabilometer value (min) <sup>f</sup>	California Test 366	1 per 4,000 tons or 2 per 5 business days, whichever is greater	30	30	--		48 hours
No. 4 and 3/8" gradings 1/2" and 3/4" gradings			37	35	23		
Air void content (%) <sup>f,g</sup>	California Test 367		4 ± 2	4 ± 2	TV ± 2		

Percent of crushed particles coarse aggregate (% min.): One fractured face Two fractured faces	California Test 205	As designated in QC plan.  At least once per project.	90	25	--	California Test 125	48 hours
			75	--	90		
Fine aggregate (% min) (Passing no. 4 sieve and retained on no. 8 sieve): One fractured face			70	20	70		
Los Angeles Rattler (% max): Loss at 100 rev. Loss at 500 rev.	California Test 211		12	--	12	California Test 125	
			45	50	40		
Fine aggregate angularity (% min) <sup>n</sup>	California Test 234		45	45	45	California Test 125	
Flat and elongated particle (% max by weight @ 5:1)	California Test 235		Report only	Report only	Report only	California Test 125	
Voids filled with asphalt (%) <sup>i</sup>  No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367				Report only		
			65.0–75.0	65.0–75.0			
		65.0–75.0	65.0–75.0				
		65.0–75.0	65.0–75.0				
		65.0–75.0	65.0–75.0				
Voids in mineral aggregate (% min.) <sup>i</sup>  No. 4 grading 3/8" grading 1/2" grading 3/4" grading	California Test 367						
		17.0	17.0	--			
		15.0	15.0	--			
		14.0	14.0	18.0–23.0			
		13.0	13.0	18.0–23.0			

Dust proportion <sup>i</sup>  No. 4 and 3/8" gradings 1/2" and 3/4" gradings	California Test 367		0.6–1.2 0.6–1.2	0.6–1.2 0.6–1.2	Report only		
Hamburg wheel track (minimum number of passes at 0.5 inch average rut depth) <sup>i</sup> PG-58 PG-64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)	1 per 10,000 tons or 1 per project whichever is greater	10,000 15,000 20,000 25,000	10,000 15,000 20,000 25,000	--	--	
Hamburg wheel track (inflection point minimum number of passes) <sup>i</sup> PG-58 PG-64 PG-70 PG-76 or higher	AASHTO T 324 (Modified)	1 per 10,000 tons or 1 per project whichever is greater	10,000 10,000 12,500 15000	10,000 10,000 12,500 15000	--	--	
Moisture susceptibility (minimum dry strength, psi) <sup>j</sup>	California Test 371	1 per 10,000 tons or 1 per project whichever is greater	120	120	--	--	
Moisture susceptibility (tensile strength ratio, %) <sup>j</sup>	California Test 371	1 per 10,000 tons or 1 per project whichever is greater	70	70	70	--	
Smoothness	Section 39-1.12	--	12-foot straight-edge, must-grind, and PI <sub>0</sub>	12-foot straight-edge, must-grind, and PI <sub>0</sub>	12-foot straight-edge, must-grind, and PI <sub>0</sub>	--	
Asphalt rubber binder viscosity @ 375 °F, centipoises	Section 39-1.02D	--	--	--	1,500–4,000	Section 39-1.02D	24 hours
CRM	Section 39-1.02D	--	--	--	Section 39-1.02D	Section 39-1.02D	48 hours

- <sup>a</sup> Determine combined aggregate gradation containing RAP under California Test 367.
- <sup>b</sup> The tolerances must comply with the allowable tolerances in section 39-1.02E.
- <sup>c</sup> Determines field compaction for any of the following conditions:
  1. 1/2-inch, 3/8-inch, or no. 4 aggregate grading is used and the specified total paved thickness is at least 0.15 foot.
  2. 3/4-inch aggregate grading is used and the specified total paved thickness is at least 0.20 foot.
- <sup>d</sup> To determine field compaction use:
  1. In-place density measurements using the method specified in your QC plan.
  2. California Test 309 to determine the maximum theoretical density at the frequency specified in California Test 375, Part 5C.
- <sup>e</sup> For adjusting the plant controller at the HMA plant.
- <sup>f</sup> Report the average of 3 tests from a single split sample.
- <sup>g</sup> Determine the bulk specific gravity of each lab-compacted briquette under California Test 308, Method A, and theoretical maximum specific gravity under California Test 309.
- <sup>h</sup> The Engineer waives this specification if HMA contains 10 percent or less of nonmanufactured sand by weight of total aggregate. Manufactured sand is fine aggregate produced by crushing rock or gravel.
- <sup>i</sup> Report only.
- <sup>j</sup> Applies to RAP substitution rate greater than 15 percent.

**Replace the 1st sentence in the 1st paragraph of section 39-4.03B(2) with:**

01-20-12

For aggregate gradation and asphalt binder content, the minimum ratio of verification testing frequency to quality control testing frequency is 1:5.

**Replace the 2nd "and" in the 7th paragraph of section 39-4.03B(2) with:**

01-20-12

or

**Replace the 1st paragraph of section 39-4.04A with:**

02-22-13

The Engineer samples for acceptance testing and tests for the following quality characteristics:

**HMA Acceptance—QC/QA Construction Process**

Index (i)	Quality characteristic				Weight -ing factor (w)	Test method	HMA type		
							A	B	RHMA-G
		Aggregate gradation <sup>a</sup>				California Test 202	JMF ± Tolerance <sup>c</sup>		
	Sieve	3/4"	1/2"	3/8"					
1	1/2"	X <sup>b</sup>	--	--	0.05				
1	3/8"	--	X	--	0.05				
1	No. 4	--	--	X	0.05				
2	No. 8	X	X	X	0.10				
3	No. 200	X	X	X	0.15				
4	Asphalt binder content (%)				0.30	California Test 379 or 382	JMF±0.40	JMF±0.40	JMF ± 0.40
5	Field compaction (% max. theoretical density) <sup>d, e</sup>				0.40	California Test 375	92–96	92–96	91–96
	Sand equivalent (min) <sup>f</sup>					California Test 217	47	42	47
	Stabilometer value (min) <sup>f</sup> No. 4 and 3/8" gradings 1/2" and 3/4" gradings					California Test 366	30 37	30 35	-- 23
	Air void content (%) <sup>f, g</sup>					California Test 367	4 ± 2	4 ± 2	TV ± 2
	Percent of crushed particles coarse aggregate (% min) One fractured face Two fractured faces Fine aggregate (% min) (Passing no. 4 sieve and retained on No. 8 sieve.) One fractured face					California Test 205	90 75	25 --	-- 90
	HMA moisture content (% max)					California Test 226 or 370	1.0	1.0	1.0
	Los Angeles Rattler (% max) Loss at 100 rev. Loss at 500 rev.					California Test 211	12 45	-- 50	12 40
	Fine aggregate angularity (% min) <sup>h</sup>					California Test 234	45	45	45
	Flat and elongated particle (% max by weight @ 5:1)					California Test 235	Report only	Report only	Report only
	Voids in mineral aggregate (% min) <sup>i</sup> No. 4 grading 3/8" grading 1/2" grading 3/4" grading					California Test 367	17.0 15.0 14.0 13.0	17.0 15.0 14.0 13.0	-- -- 18.0–23.0 18.0–23.0

	Voids filled with asphalt (%) <sup>i</sup> No. 4 grading 3/8" grading 1/2" grading 3/4" grading		California Test 367	65.0–75.0 65.0–75.0 65.0–75.0 65.0–75.0	65.0–75.0 65.0–75.0 65.0–75.0 65.0–75.0	Report only
	Dust proportion <sup>1</sup> No. 4 and 3/8" gradings 1/2" and 3/4" gradings		California Test 367	0.6–1.2 0.6–1.2	0.6–1.2 0.6–1.2	Report only
	Hamburg Wheel Tracker (minimum number of passes at 0.5 inch average rut depth) <sup>j</sup> PG-58 PG-64 PG-70 PG-76 or higher		AASHTO T 324 (Modified)	10,000 15,000 20,000 25,000	10,000 15,000 20,000 25,000	--
	Hamburg Wheel Tracker (inflection point minimum number of passes) <sup>j</sup> PG-58 PG-64 PG-70 PG-76 or higher		AASHTO T 324 (Modified)	10,000 15,000 20,000 25,000	10,000 15,000 20,000 25,000	--
	Moisture susceptibility (minimum dry strength, psi) <sup>j</sup>		California Test 371	120	120	--
	Moisture susceptibility (tensile strength ratio %) <sup>j</sup>		California Test 371	70	70	70
	Smoothness		Section 39-1.12	12-foot straight- edge, must grind, and PI <sub>0</sub>	12-foot straight- edge, must grind, and PI <sub>0</sub>	12-foot straight- edge, must grind, and PI <sub>0</sub>
	Asphalt binder		Various	Section 92	Section 92	Section 92
	Asphalt rubber binder		Various	--	--	Section 92-1.01D(2) and section 39-1.02D
	Asphalt modifier		Various	--	--	Section 39-1.02D
	CRM		Various	--	--	Section 39-1.02D



**Replace the 2nd and 3rd paragraphs in section 40-1.01D(4) with:**

01-20-12

The QC plan must include details of corrective action to be taken if any process is out of control. As a minimum, a process is out of control if any of the following occurs:

1. For fine and coarse aggregate gradation, 2 consecutive running averages of 4 tests are outside the specification limits
2. For individual penetration or air content measurements:
  - 2.1. One point falls outside the suspension limit line
  - 2.2. Two points in a row fall outside the action limit line

Stop production and take corrective action for out of control processes or the Engineer rejects subsequent material.

**Replace the 1st paragraph in section 40-1.01D(5) with:**

01-20-12

Determine the minimum cementitious materials content. Use your value for minimum cementitious material content for *MC* in equation 1 and equation 2 of section 90-1.02B(3).

**Replace the 1st sentence of the 3rd paragraph of section 40-1.01D(9) with:**

01-20-12

Use a California profilograph to determine the concrete pavement profile.

**Replace the title of the table in section 40-1.01D(13)(a) with:**

01-20-12

**Concrete Pavement Acceptance Testing**

**Replace the 2nd and 3rd paragraphs in section 40-1.01D(13)(a) with:**

01-20-12

Pavement smoothness may be accepted based on the Department's testing. A single test represents no more than 0.1 mile.

Acceptance of modulus of rupture, thickness, dowel bar and tie bar placement, coefficient of friction, smoothness, and air content, does not constitute final concrete pavement acceptance.

**Delete item 4 in the list in the 2nd paragraph in section 40-1.01D(13)(c)(2).**

01-20-12

**Replace items 1 and 2 in the list in the 2nd paragraph in 40-1.01D(13)(d) with:**

01-20-12

1. For tangents and horizontal curves having a centerline radius of curvature 2,000 feet or more, the  $PI_0$  must be at most 2-1/2 inches per 0.1-mile section.
2. For horizontal curves having a centerline radius of curvature from 1,000 to 2,000 feet including concrete pavement within the superelevation transitions of those curves, the  $PI_0$  must be at most 5 inches per 0.1-mile section.

**Replace the 1st and 2nd variables in the equation in section 40-1.01D(13)(f) with:**

01-20-12

$n_c$  = Number of your quality control tests (minimum of 6 required)

$n_v$  = Number of verification tests (minimum of 2 required)

**Replace "Your approved third party independent testing laboratory" in the 4th paragraph of section 40-1.01D(13)(f) with:**

01-20-12

The authorized laboratory

**Replace item 2 in the list in the 2nd paragraph of section 40-1.01D(13)(g):**

01-20-12

2. One test for every 4,000 square yards of concrete pavement with tie bars or remaining fraction of that area. Each tie bar test consists of 2 cores with 1 on each tie-bar-end to expose both ends and allow measurement.

**Replace section 40-1.01D(13)(h) with:**

01-20-12

**40-1.01D(13)(h) Bar Reinforcement**

Bar reinforcement is accepted based on inspection before concrete placement.

**Replace the paragraph in section 40-1.02B(2) with:**

01-20-12

PCC for concrete pavement must comply with section 90-1 except as otherwise specified.

**Replace the paragraphs in section 40-1.02D with:**

01-20-12

Bar reinforcement must be deformed bars.

If the project is not shown to be in high desert or any mountain climate region, bar reinforcement must comply with section 52.

If the project is shown to be in high desert or any mountain climate regions, bar reinforcement must be one of the following:

1. Epoxy-coated bar reinforcement under section 52-2.03B except bars must comply with either ASTM A 706/A 706M; ASTM A 996/A 996M; or ASTM A 615/A 615M, Grade 40 or 60. Bars must be handled under ASTM D 3963/D 3963M and section 52-2.02C.
2. Low carbon, chromium steel bar complying with ASTM A 1035/A 1035M

**Replace the paragraphs in section 40-1.02E with:**

01-20-12

Tie bars must be deformed bars.

If the project is not shown to be in high desert or any mountain climate region, tie bars must be one of the following:

1. Epoxy-coated bar reinforcement. Bars must comply with either section 52-2.02B or 52-2.03B except bars must comply with either ASTM A 706/A 706M; ASTM A 996/A 996M; or ASTM A 615/A 615M, Grade 40 or 60.
2. Stainless-steel bars. Bars must be descaled, pickled, polished, and solid stainless-steel bars under ASTM A 955/A 955M, Grade 60, UNS Designation S31603 or S31803.
3. Low carbon, chromium-steel bars under ASTM A 1035/A 1035M.

If the project is shown to be in high desert or any mountain climate region, tie bars must be one of the following:

1. Epoxy-coated bar reinforcement. Bars must comply with section 52-2.03B except bars must comply with either ASTM A 706/A 706M; ASTM A 996/A 996M; or ASTM A 615/A 615M, Grade 40 or 60.
2. Stainless-steel bars. Bars must be descaled, pickled, polished, and solid stainless-steel bars under ASTM A 955/A 955M, Grade 60, UNS Designation S31603 or S31803.

Fabricate, sample, and handle epoxy-coated tie bars under ASTM D 3963/D 3963M, section 52-2.02C, or section 52-2.03C.

Do not bend tie bars.

**Replace the 1st, 2nd, and 3rd paragraphs in section 40-1.02F with:**

01-20-12

Dowel bars must be plain bars. Fabricate, sample, and handle epoxy-coated dowel bars under ASTM D 3963/D 3963M and section 52-2.03C except each sample must be 18 inches long.

If the project is not shown to be in high desert or any mountain climate region, dowel bars must be one of the following:

1. Epoxy-coated bars. Bars must comply with ASTM A 615/A 615M, Grade 40 or 60. Epoxy coating must comply with either section 52-2.02B or 52-2.03B.
2. Stainless-steel bars. Bars must be descaled, pickled, polished, and solid stainless-steel bars under ASTM A 955/A 955M, Grade 60, UNS Designation S31603 or S31803.
3. Low carbon, chromium-steel bars under ASTM A 1035/A 1035M.

If the project is shown to be in high desert or any mountain climate region, dowel bars must be one of the following:

1. Epoxy-coated bars. Bars must comply with ASTM A 615/A 615M, Grade 40 or 60. Epoxy coating must comply with section 52-2.03B.
2. Stainless-steel bars. Bars must be descaled, pickled, polished, and solid stainless-steel bars under ASTM A 955/A 955M, Grade 60, UNS Designation S31603 or S31803.

**Replace the paragraphs in section 40-1.02G with:**

01-20-12

For dowel and tie bar baskets, wire must comply with ASTM A 82/A 82M and be welded under ASTM A 185/A 185M, Section 7.4. The minimum wire-size no. is W10. Use either U-frame or A-frame shaped assemblies.

If the project is not shown to be in high desert or any mountain climate region. Baskets may be epoxy-coated, and the epoxy coating must comply with either section 52-2.02B or 52-2.03B.

If the project is shown to be in high desert or any mountain climate region, wire for dowel bar and tie bar baskets must be one of the following:

1. Epoxy-coated wire complying with section 52-2.03B
2. Stainless-steel wire. Wire must be descaled, pickled, and polished solid stainless-steel. Wire must comply with (1) the chemical requirements in ASTM A 276/A 276M, UNS Designation S31603 or S31803 and (2) the tension requirements in ASTM A 1022/ A 1022M.

Handle epoxy-coated tie bar and dowel bar baskets under ASTM D 3963/D 3963M and either section 52-2.02B or 52-2.03B.

Fasteners must be driven fasteners under ASTM F 1667. Fasteners on lean concrete base or HMA must have a minimum shank diameter of 3/16 inch and a minimum shank length of 2-1/2 inches. For asphalt

treated permeable base or cement treated permeable base, the shank diameter must be at least 3/16 inch and the shank length must be at least 5 inches.

Fasteners, clips, and washers must have a minimum 0.2-mil thick zinc coating applied by either electroplating or galvanizing.

**Replace the 1st paragraph in section 40-1.02H with:**

01-20-12

Chemical adhesive for drilling and bonding dowels and tie bars must be on the Authorized Material List. The Authorized Material List indicates the appropriate chemical adhesive system for the concrete temperature and installation conditions.

**Replace section 40-1.02I(2) with:**

01-20-12

**40-1.02I(2) Silicone Joint Sealant**

Silicone joint sealant must be on the Authorized Material List.

**Replace the last sentence in section 40-1.02I(4) with:**

01-20-12

Show evidence that the seals are compressed from 30 to 50 percent for the joint width at time of installation.

**Replace the paragraph in section 40-1.02L with:**

01-20-12

Water for core drilling may be obtained from a potable water source, or submit proof that it does not contain:

1. More than 1,000 parts per million of chlorides as Cl
2. More than 1,300 parts per million of sulfates as  $SO_4$
3. Impurities that cause pavement discoloration or surface etching

**Replace the paragraph in section 40-1.03B with:**

01-20-12

Before placing concrete pavement, develop enough water supply for the work under section 17.

**Replace the last paragraph in section 40-1.03D(1) with:**

01-20-12

Removal of grinding residue must comply with section 42-1.03B.

**Replace the 1st and 2nd paragraphs in section 40-1.03E(6)(c) with:**

01-20-12

Install preformed compressions seals in isolation joints if specified in the special provisions.

Install longitudinal seals before transverse seals. Longitudinal seals must be continuous except splicing is allowed at intersections with transverse seals. Transverse seals must be continuous for the entire transverse length of concrete pavement except splices are allowed for widenings and staged construction. With a sharp instrument, cut across the longitudinal seal at the intersection with transverse

construction joints. If the longitudinal seal does not relax enough to properly install the transverse seal, trim the longitudinal seal to form a tight seal between the 2 joints.

If splicing is authorized, splicing must comply with the manufacturer's written instructions.

**Replace the 12th and 13th paragraphs in section 40-1.03G with:**

01-20-12

Construct additional test strips if you:

1. Propose different paving equipment including:
  - 1.1. Paver
  - 1.2. Dowel bar inserter
  - 1.3. Tie bar inserter
  - 1.4. Tining
  - 1.5. Curing equipment
2. Change concrete mix proportions

You may request authorization to eliminate the test strip if you use paving equipment and personnel from a Department project (1) for the same type of pavement and (2) completed within the past 12 months. Submit supporting documents and previous project information with your request.

**Replace the 1st paragraph in section 40-1.03I with:**

01-20-12

Place tie bars in compliance with the tolerances shown in the following table:

<b>Tie Bar Tolerance</b>	
Dimension	Tolerance
Horizontal and vertical skew	10 degrees maximum
Longitudinal translation	± 2 inch maximum
Horizontal offset (embedment)	± 2 inch maximum
Vertical depth	1. Not less than 1/2 inch below the saw cut depth of joints 2. When measured at any point along the bar, not less than 2 inches clear of the pavement's surface and bottom

**Replace item 4 in the list in the 2nd paragraph in section 40-1.03I with:**

01-20-12

4. Use tie bar baskets. Anchor baskets at least 200 feet in advance of pavement placement activity. If you request a waiver, describe the construction limitations or restricted access preventing the advanced anchoring. After the baskets are anchored and before paving, demonstrate the tie bars do not move from their specified depth and alignment during paving. Use fasteners to anchor tie bar baskets.

**Replace "The maximum distance below the depth shown must be 0.05 foot." in the table in section 40-1.03J with:**

01-20-12

The maximum distance below the depth shown must be 5/8 inch.

**Replace sections 40-1.03L and 40-1.03M with:**

01-20-12

**40-1.03L Finishing**

**40-1.03L(1) General**

Reserved

**40-1.03L(2) Preliminary Finishing**

**40-1.03L(2)(a) General**

Preliminary finishing must produce a smooth and true-to-grade finish. After preliminary finishing, mark each day's paving with a stamp. The stamp must be authorized before paving starts. The stamp must be approximately 1 by 2 feet in size. The stamp must form a uniform mark from 1/8 to 1/4 inch deep. Locate the mark  $20 \pm 5$  feet from the transverse construction joint formed at each day's start of paving and  $1 \pm 0.25$  foot from the pavement's outside edge. The stamp mark must show the month, day, and year of placement and the station of the transverse construction joint. Orient the stamp mark so it can be read from the pavement's outside edge.

Do not apply more water to the pavement surface than can evaporate before float finishing and texturing are completed.

**40-1.03L(2)(b) Stationary Side Form Finishing**

If stationary side form construction is used, give the pavement a preliminary finish by the machine float method or the hand method.

If using the machine float method:

1. Use self-propelled machine floats.
2. Determine the number of machine floats required to perform the work at a rate equal to the pavement delivery rate. If the time from paving to machine float finishing exceeds 30 minutes, stop pavement delivery. When machine floats are in proper position, you may resume pavement delivery and paving.
3. Run machine floats on side forms or adjacent pavement lanes. If running on adjacent pavement, protect the adjacent pavement surface under section 40-1.03P. Floats must be hardwood, steel, or steel-shod wood. Floats must be equipped with devices that adjust the underside to a true flat surface.

If using the hand method, finish pavement smooth and true to grade with manually operated floats or powered finishing machines.

**40-1.03L(2)(c) Slip-Form Finishing**

If slip-form construction is used, the slip-form paver must give the pavement a preliminary finish. You may supplement the slip-form paver with machine floats.

Before the pavement hardens, correct pavement edge slump in excess of 0.02 foot exclusive of edge rounding.

**40-1.03L(3) Final Finishing**

After completing preliminary finishing, round the edges of the initial paving widths to a 0.04-foot radius. Round transverse and longitudinal construction joints to a 0.02-foot radius.

Before curing, texture the pavement. Perform initial texturing with a burlap drag or broom device that produces striations parallel to the centerline. Perform final texturing with a steel-tined device that produces grooves parallel with the centerline.

Construct longitudinal grooves with a self-propelled machine designed specifically for grooving and texturing pavement. The machine must have tracks to maintain constant speed, provide traction, and maintain accurate tracking along the pavement surface. The machine must have a single row of rectangular spring steel tines. The tines must be from 3/32 to 1/8 inch wide, on 3/4-inch centers, and must have enough length, thickness, and resilience to form grooves approximately 3/16 inch deep. The machine must have horizontal and vertical controls. The machine must apply constant down pressure on the pavement surface during texturing. The machines must not cause ravels.

Construct grooves over the entire pavement width in a single pass except do not construct grooves 3 inches from the pavement edges and longitudinal joints. Final texture must be uniform and smooth. Use a guide to properly align the grooves. Grooves must be parallel and aligned to the pavement edge across the pavement width. Grooves must be from 1/8 to 3/16 inch deep after the pavement has hardened.

For irregular areas and areas inaccessible to the grooving machine, you may hand-construct grooves under section 40-1.03L(2) using the hand method. Hand-constructed grooves must comply with the specifications for machine-constructed grooves.

Initial and final texturing must produce a coefficient of friction of at least 0.30 when tested under California Test 342. Notify the Engineer when the pavement is scheduled to be opened to traffic to allow at least 25 days for the Department to schedule testing for coefficient of friction. Notify the Engineer when the pavement is ready for testing which is the latter of:

1. Seven days after paving
2. When the pavement has attained a modulus of rupture of 550 psi

The Department tests for coefficient of friction within 7 days of receiving notification that the pavement is ready for testing.

Do not open the pavement to traffic unless the coefficient of friction is at least 0.30.

#### **40-1.03M Reserved**

#### **Replace the 4th paragraph of 40-1.03P with:**

01-20-12

Construct crossings for traffic convenience. If authorized, you may use RSC for crossings. Do not open crossings until the Department determines that the pavement's modulus of rupture is at least 550 psi under California Test 523 or California Test 524.

#### **Replace the 1st paragraph of section 40-6.01A with:**

01-20-12

Section 40-6 includes specifications for applying a high molecular weight methacrylate resin system to pavement surface cracks that do not extend the full slab depth.

#### **Replace the 4th paragraph of section 40-6.01C(2) with:**

01-20-12

If the project is in an urban area adjacent to a school or residence, the public safety plan must also include an airborne emissions monitoring plan prepared by a CIH certified in comprehensive practice by the American Board of Industrial Hygiene. Submit a copy of the CIH's certification. The CIH must monitor the emissions at a minimum of 4 points including the mixing point, the application point, and the point of nearest public contact. At work completion, submit a report by the industrial hygienist with results of the airborne emissions monitoring plan.

#### **Delete the 1st sentence of the 2nd paragraph in section 40-6.02B.**

01-20-12

#### **Replace item 4 in the list in the last paragraph in section 40-6.03A with:**

01-20-12

4. Coefficient of friction is at least 0.30 under California Test 342









**Replace the 2nd paragraph of section 49-2.01D with:**

01-20-12

Furnish piling is measured along the longest side of the pile from the specified tip elevation shown to the plane of pile cutoff.

**Replace "sets" in the 1st paragraph of section 49-2.04A(3) with:**

04-19-13

copies

**Replace the 3rd and 4th paragraphs of section 49-2.04B(2) with:**

10-19-12

Piles in a corrosive environment must be steam or water cured under section 90-4.03.

If piles in a corrosive environment are steam cured, either:

1. Keep the piles continuously wet for at least 3 days. The 3 days includes the holding and steam curing periods.
2. Apply curing compound under section 90-1.03B(3) after steam curing.

**Add to section 49-3.01A:**

01-20-12

Concrete must comply with section 51.

**Replace the 1st paragraph of section 49-3.01C with:**

01-20-12

Except for CIDH concrete piles constructed under slurry, construct CIP concrete piles such that the excavation methods and the concrete placement procedures provide for placing the concrete against undisturbed material in a dry or dewatered hole.

**Replace "Reserved" in section 49-3.02A(2) with:**

01-20-12

**dry hole:**

1. Except for CIDH concrete piles specified as end bearing, a drilled hole that:
  - 1.1. Accumulates no more than 12 inches of water in the bottom of the drilled hole during a period of 1 hour without any pumping from the hole during the hour.
  - 1.2. Has no more than 3 inches of water in the bottom of the drilled hole immediately before placing concrete.
2. For CIDH concrete piles specified as end bearing, a drilled hole free of water without the use of pumps.

**Replace "Reserved" in section 49-3.02A(3)(a) with:**

01-20-12

If plastic spacers are proposed for use, submit the manufacturer's data and a sample of the plastic spacer. Allow 10 days for review.

**Replace item 5 in the list in the 1st paragraph of section 49-3.02A(3)(b) with:**

10-19-12

5. Methods and equipment for determining:
  - 5.1. Depth of concrete
  - 5.2. Theoretical volume of concrete to be placed, including the effects on volume if casings are withdrawn
  - 5.3. Actual volume of concrete placed

**Add to the list in the 1st paragraph of section 49-3.02A(3)(b):**

01-18-13

8. Drilling sequence and concrete placement plan.

**Replace item 2 in the list in the 1st paragraph of section 49-3.02A(3)(g) with:**

01-20-12

2. Be sealed and signed by an engineer who is registered as a civil engineer in the State. This requirement is waived for either of the following conditions:
  - 2.1. The proposed mitigation will be performed under the current Department-published version of *ADSC Standard Mitigation Plan 'A' - Basic Repair* without exception or modification.
  - 2.2. The Engineer determines that the rejected pile does not require mitigation due to structural, geotechnical, or corrosion concerns, and you elect to repair the pile using the current Department-published version of *ADSC Standard Mitigation Plan 'B' - Grouting Repair* without exception or modification.

**Replace item 1 in the list in the 1st paragraph of section 49-3.02A(4)(d)(ii) with:**

01-20-12

1. Inspection pipes must be schedule 40 PVC pipe complying with ASTM D 1785 with a nominal pipe size of 2 inches. Watertight PVC couplers complying with ASTM D 2466 are allowed to facilitate pipe lengths in excess of those commercially available. Log the location of the inspection pipe couplers with respect to the plane of pile cutoff.

**Add to section 49-3.02A(4)(d)(iv):**

01-20-12

If the Engineer determines it is not feasible to use one of ADSC's standard mitigation plans to mitigate the pile, schedule a meeting and meet with the Engineer before submitting a nonstandard mitigation plan.

The meeting attendees must include your representatives and the Engineer's representatives involved in the pile mitigation. The purpose of the meeting is to discuss the type of pile mitigation acceptable to the Department.

Provide the meeting facility. The Engineer conducts the meeting.

**Replace the 1st paragraph of section 49-3.02B(5) with:**

01-20-12

Grout used to backfill casings must comply with section 50-1.02C, except:

1. Grout must consist of cementitious material and water, and may contain an admixture if authorized. Cementitious material must comply with section 90-1.02B, except SCMs are not required. The minimum cementitious material content of the grout must not be less than 845 lb/cu yd of grout.
2. Aggregate must be used to extend the grout as follows:



2. Each jack used to tension prestressing steel permanently anchored at 25 percent or more of its specified minimum ultimate tensile strength must be calibrated by METS within 1 year of use and after each repair. You must:
  - 2.1. Schedule the calibration of the jacking equipment with METS
  - 2.2. Verify that the jack and supporting systems are complete, with proper components, and are in good operating condition
  - 2.3. Mechanically calibrate the gages with a dead weight tester or other authorized means before calibration of the jacking equipment by METS
  - 2.4. Provide enough labor, equipment, and material to (1) install and support the jacking and calibration equipment and (2) remove the equipment after the calibration is complete
  - 2.5. Plot the calibration results
3. Each jack used to tension prestressing steel permanently anchored at less than 25 percent of its specified minimum ultimate tensile strength must be calibrated by an authorized laboratory within 6 months of use and after each repair.

**Replace "diameter" in item 9 in the list in the 1st paragraph of section 50-1.02D with:**

cross-sectional area

04-20-12

**Add to section 50-1.02:**

**50-1.02G Sheathing**

09-16-11

Sheathing for debonding prestressing strand must:

1. Be split or un-split flexible polymer plastic tubing
2. Have a minimum wall thickness of 0.025 inch
3. Have an inside diameter exceeding the maximum outside diameter of the strand by 0.025 to 0.14 inch

Split sheathing must overlap at least 3/8 inch.

Waterproofing tape used to seal the ends of the sheathing must be flexible adhesive tape.

The sheathing and waterproof tape must not react with the concrete, coating, or steel.

**Add to section 50-1.03B(1):**

01-20-12

After seating, the maximum tensile stress in the prestressing steel must not exceed 75 percent of the minimum ultimate tensile strength shown.

**Add to section 50-1.03B(2):**

09-16-11

**50-1.03B(2)(e) Debonding Prestressing Strands**

Where shown, debond prestressing strands by encasing the strands in plastic sheathing along the entire length shown and sealing the ends of the sheathing with waterproof tape.

Distribute the debonded strands symmetrically about the vertical centerline of the girder. The debonded lengths of pairs of strands must be equal.

Do not terminate debonding at any one cross section of the member for more than 40 percent of the debonded strands or 4 strands, whichever is greater.

Thoroughly seal the ends with waterproof tape to prevent the intrusion of water or cement paste before placing the concrete.

AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA

## 51 CONCRETE STRUCTURES

04-19-13

**Replace the paragraphs of section 51-1.01A with:**

10-19-12

Section 51-1 includes general specifications for constructing concrete structures.

Earthwork for the following concrete structures must comply with section 19-3:

1. Sound wall footings
2. Sound wall pile caps
3. Culverts
4. Barrier slabs
5. Junction structures
6. Minor structures
7. Pipe culvert headwalls, endwalls, and wingwalls for a pipe with a diameter of 5 feet or greater

Falsework must comply with section 48-2.

Joints must comply with section 51-2.

Elastomeric bearing pads must comply with section 51-3.

Reinforcement for the following concrete structures must comply with section 52:

1. Sound wall footings
2. Sound wall pile caps
3. Barrier slabs
4. Junction structures
5. Minor structures
6. PC concrete members

You may use RSC for a concrete structure only where the specifications allow the use of RSC.

**Replace the heading of section 51-1.01D(4) with:**

04-19-13

### Testing Concrete Surfaces

**Add to section 51-1.01D(4)(a):**

04-19-13

The Engineer tests POC deck surfaces for smoothness and crack intensity.

**Add to the list in the 1st paragraph of section 51-1.01D(4)(b):**

04-19-13

3. Completed deck surfaces, including ramps and landings of POCs

**Replace the 4th paragraph in section 51-1.01D(4)(b) with:**

04-19-13

Except for POCs, surface smoothness is tested using a bridge profilograph under California Test 547. Two profiles are obtained in each lane approximately 3 feet from the lane lines and 1 profile is obtained in

each shoulder approximately 3 feet from the curb or rail face. Profiles are taken parallel to the direction of traffic.

**Add between the 5th and 6th paragraphs of section 51-1.01D(4)(b):**

04-19-13

POC deck surfaces must comply with the following smoothness requirements:

1. Surfaces between grade changes must not vary more than 0.02 foot from the lower edge of a 12-foot-long straightedge placed parallel to the centerline of the POC
2. Surface must not vary more than 0.01 foot from the lower edge of a 6-foot-long straightedge placed perpendicular to the centerline of the POC

**Add to section 51-1.01D(4)(d):**

04-19-13

The Engineer measures crack intensity of POC deck surfaces after curing, before prestressing, and before falsework release. Clean the surface for the Engineer to measure surface crack intensity.

In any 100 sq ft portion of a new POC deck surface, if there are more than 10 feet of cracks having a width at any point of over 0.02 inch, treat the deck with methacrylate resin under section 15-5.05. Treat the entire deck width between the curbs to 5 feet beyond where the furthest continuous crack emanating from the 100 sq ft section is 0.02 inch wide. Treat the deck surface before grinding.

**Add to section 51-1.03C(2)(c)(i):**

04-20-12

Permanent steel deck forms are only allowed where shown or if specified as an option in the special provisions.

**Replace the 3rd paragraph of section 51-1.03C(2)(c)(ii) with:**

04-20-12

Compute the physical design properties under AISI's *North American Specification for the Design of Cold-Formed Steel Structural Members*.

**Replace the 8th paragraph of section 51-1.03D(1) with:**

10-19-12

Except for concrete placed as pipe culvert headwalls and endwalls, slope paving and aprons, and concrete placed under water, consolidate concrete using high-frequency internal vibrators within 15 minutes of placing concrete in the forms. Do not attach vibrators to or hold them against forms or reinforcing steel. Do not displace reinforcement, ducts, or prestressing steel during vibrating.

**Add to section 51-1.03E(5):**

08-05-11

Drill the holes without damaging the adjacent concrete. If reinforcement is encountered during drilling before the specified depth is attained, notify the Engineer. Unless coring through the reinforcement is authorized, drill a new hole adjacent to the rejected hole to the depth shown.

**Add to section 51-1.03F(5)(a):**

04-19-13

For approach slabs, sleeper slabs, and other roadway surfaces of concrete structures, texture the roadway surface as specified for bridge deck surfaces in section 51-1.03F(5)(b).

**Replace "Reserved" in section 51-1.03F(5)(b) with:**

04-20-12

**51-1.03F(5)(b)(i) General**

Except for bridge widenings, texture the bridge deck surfaces longitudinally by grinding and grooving or by longitudinal tining.

10-19-12

For bridge widenings, texture the deck surface longitudinally by longitudinal tining.

04-20-12

In freeze-thaw areas, do not texture PCC surfaces of bridge decks.

**51-1.03F(5)(b)(ii) Grinding and Grooving**

When texturing the deck surface by grinding and grooving, place a 1/4 inch of sacrificial concrete cover on the bridge deck above the finished grade shown. Place items to be embedded in the concrete based on the final profile grade elevations shown. Construct joint seals after completing the grinding and grooving.

Before grinding and grooving, deck surfaces must comply with the smoothness and deck crack treatment requirements.

Grind and groove the deck surface as follows:

1. Grind the surface to within 18 inches of the toe of the barrier under section 42-3. Grinding must not reduce the concrete cover on reinforcing steel to less than 1-3/4 inches.
2. Groove the ground surfaces longitudinally under section 42-2. The grooves must be parallel to the centerline.

**51-1.03F(5)(b)(iii) Longitudinal Tining**

When texturing the deck surface by longitudinal tining, perform initial texturing with a burlap drag or broom device that produces striations parallel to the centerline. Perform final texturing with spring steel tines that produce grooves parallel with the centerline.

The tines must:

1. Be rectangular in cross section
2. Be from 3/32 to 1/8 inch wide on 3/4-inch centers
3. Have enough length, thickness, and resilience to form grooves approximately 3/16 inch deep

Construct grooves to within 6 inches of the layout line of the concrete barrier toe. Grooves must be from 1/8 to 3/16 inch deep and 3/16 inch wide after concrete has hardened.

For irregular areas and areas inaccessible to the grooving machine, you may hand construct grooves. Hand-constructed grooves must comply with the specifications for machine-constructed grooves.

Tining must not cause tearing of the deck surface or visible separation of coarse aggregate at the surface.

**Add to section 51-1.03F:**

04-19-13

**51-1.03F(6) Finishing Pedestrian Overcrossing Surfaces**

Construct deck surfaces, including ramps and landings of POCs to the grade and cross section shown. Surfaces must comply with the specified smoothness, surface texture, and surface crack requirements.

The Engineer sets deck elevation control points for your use in establishing the grade and cross section of the deck surface. The grade established by the deck elevation control points includes all camber allowances. Except for landings, elevation control points include the beginning and end of the ramp and will not be closer together than approximately 8 feet longitudinally and 4 feet transversely to the POC centerline. Landing elevation control points are at the beginning and the end of the landing.

Broom finish the deck surfaces of POCs. Apply the broom finish perpendicular to the path of travel. You may apply water mist to the surface immediately before brooming.

Clean any discolored concrete by abrasive blast cleaning or other authorized methods.

**Replace the paragraphs of section 51-1.04 with:**

10-19-12

If concrete involved in bridge work is not designated by type and is not otherwise paid for under a separate bid item, the concrete is paid for as structural concrete, bridge.

The payment quantity for structural concrete includes the volume in the concrete occupied by bar reinforcing steel, structural steel, prestressing steel materials, and piling.

The payment quantity for seal course concrete is the actual volume of seal course concrete placed except the payment quantity must not exceed the volume of concrete contained between vertical planes 1 foot outside the neat lines of the seal course shown. The Department does not adjust the unit price for an increase or decrease in the seal course concrete quantity.

Structural concrete for pier columns is measured as follows:

1. Horizontal limits are vertical planes at the neat lines of the pier column shown.
2. Bottom limit is the bottom of the foundation excavation in the completed work.
3. Upper limit is the top of the pier column concrete shown.

The payment quantity for drill and bond dowel is determined from the number and depths of the holes shown.

**Replace section 51-2.01B(2) with:**

04-19-13

**51-2.01B(2) Reserved**

04-19-13

**Delete the 4th paragraph of section 51-2.01C.**

**Replace "SSPC-QP 3" in the 1st paragraph of section 51-2.02A(2) with:**

10-19-12

AISC-420-10/SSPC-QP 3

**Replace the 2nd and 3rd paragraphs of section 51-2.02B(3)(b) with:**

04-20-12

Concrete saws for cutting grooves in the concrete must have diamond blades with a minimum thickness of 3/16 inch. Cut both sides of the groove simultaneously for a minimum 1st pass depth of 2 inches. The completed groove must have:

1. Top width within 1/8 inch of the width shown or ordered
2. Bottom width not varying from the top width by more than 1/16 inch for each 2 inches of depth
3. Uniform width and depth

Cutting grooves in existing decks includes cutting any conflicting reinforcing steel.

**Replace "sets" in the 1st and 2nd paragraphs of section 51-2.02D(1)(c)(ii) with:**

copies

04-19-13

**Replace "set" in the 7th paragraph of section 51-2.02D(1)(c)(ii) with:**

copy

04-19-13

**Add to the 1st paragraph of section 51-2.02D(3):**

POC deck surfaces must comply with section 51-1.03F(6) before placing and anchoring joint seal assemblies.

04-19-13

**Replace "sets" in the 2nd paragraph of section 51-2.02E(1)(c) with:**

copies

04-19-13

**Replace "set" in the 6th paragraph of section 51-2.02E(1)(c) with:**

copy

04-19-13

**Replace the 2nd paragraph of section 51-2.02E(1)(e) with:**

Except for components in contact with the tires, the design loading must be the AASHTO LRFD Bridge Design Specifications Design Truck with 100 percent dynamic load allowance. Each component in contact with the tires must support a minimum of 80 percent of the AASHTO LRFD Bridge Design Specifications Design Truck with 100 percent dynamic load allowance. The tire contact area must be 10 inches measured normal to the longitudinal assembly axis by 20 inches wide. The assembly must provide a smooth-riding joint without slapping of components or tire rumble.

08-05-11

**Replace "sets" in the 1st and 2nd paragraphs of section 51-2.02F(1)(c) with:**

copies

04-19-13

**Add between the 1st and 2nd paragraphs of section 51-4.01A:**

Prestressing concrete members must comply with section 50.

10-19-12

**Delete the 2nd paragraph of section 51-4.01A.**

04-20-12

**Replace the 3rd paragraph of section 51-4.01C(2) with:**

04-20-12

For segmental or spliced-girder construction, shop drawings must include the following additional information:

1. Details showing construction joints or closure joints
2. Arrangement of bar reinforcing steel, prestressing tendons, and pressure-grouting pipe
3. Materials and methods for making closures
4. Construction joint keys and surface treatment
5. Other requested information

For segmental girder construction, shop drawings must include concrete form and casting details.

**Replace "sets" in the 1st paragraph of section 51-4.01C(3) with:**

04-19-13

copies

**Delete the 1st and 2nd paragraphs of section 51-4.02A.**

10-19-12

**Replace the 3rd paragraph of section 51-4.02B(2) with:**

04-20-12

For segmental or spliced-girder construction, materials for construction joints or closure joints at exterior girders must match the color and texture of the adjoining concrete.

**Add to section 51-4.02B(2):**

04-20-12

At spliced-girder closure joints:

1. If shear keys are not shown, the vertical surfaces of the girder segment ends must be given a coarse texture as specified for the top surface of PC members.
2. Post-tensioning ducts must extend out of the vertical surface of the girder segment closure end sufficiently to facilitate splicing of the duct.

For spliced girders, pretension strand extending from the closure end of the girder segment to be embedded in the closure joint must be free of mortar, oil, dirt, excessive mill scale and scabby rust, and other coatings that would destroy or reduce the bond.

**Add to section 51-4.03B:**

04-20-12

The specifications for prestressing force distribution and sequencing of stressing in the post-tensioning activity in 50-1.03B(2)(a) do not apply if post-tensioning of spliced girders before starting deck construction is described. The composite deck-girder structure must be post-tensioned in a subsequent stage.

Temporary spliced-girder supports must comply with the specifications for falsework in section 48-2.

Before post-tensioning of spliced girders, remove the forms at CIP concrete closures and intermediate diaphragms to allow inspection for concrete consolidation.









You must provide enclosures for cleaning and painting structural steel. Cleaning and painting of new structural steel must be performed in an Enclosed Shop as defined in AISC-420-10/SSPC-QP 3. Maintain atmospheric conditions inside enclosures within specified limits.

Except for blast cleaning within closed buildings, perform blast cleaning and painting during daylight hours.

**Replace item 1 in the list in the 2nd paragraph of section 59-2.03C(1) with:**

10-19-12

1. Apply a stripe coat of undercoat paint on all edges, corners, seams, crevices, interior angles, junctions of joining members, weld lines, and similar surface irregularities. The stripe coat must completely hide the surface being covered. If spot blast cleaning portions of the bridge, apply the stripe coat of undercoat paint before each undercoat and follow with the undercoat as soon as practical. If removing all existing paint from the bridge, apply the undercoat first as soon as practical and follow with the stripe coat of undercoat paint for each undercoat.

**Replace the heading of section 59-2.03C(2) with:**

04-19-13

**Zinc Coating System**

**Add to section 59-2.03C(2)(a):**

04-19-13

Coatings for new structural steel and connections between new and existing structural steel must comply with the requirements shown in the following table:

<b>Zinc Coating System</b>		
Description	Coating	Dry film thickness (mils)
<b>All new surfaces:</b>		
Undercoat	Inorganic zinc primer, AASHTO M 300 Type I or II	4–8
Finish coat <sup>a</sup>	Exterior grade latex <sup>b</sup> , 2 coats	2 minimum each coat, 4–8 total
Total thickness, all coats		8–14
<b>Connections to existing structural steel:<sup>c</sup></b>		
Undercoat	Inorganic zinc primer, AASHTO M 300 Type I or II	4–8
Finish coat <sup>a</sup>	Exterior grade latex <sup>b</sup> , 2 coats	2 minimum each coat, 4–8 total
Total thickness, all coats		8–14

<sup>a</sup>If no finish coats are described, a final coat of inorganic zinc primer is required.

<sup>b</sup>Exterior grade latex must comply with section 91-2.02 unless otherwise specified.

<sup>c</sup>Includes the following locations:

1. New and existing contact surfaces
2. Existing member surfaces under new HS bolt heads, nuts, or washers
3. Bare surfaces of existing steel after trimming, cutting, drilling, or reaming
4. Areas within a 4-inch radius from the point of application of heat for welding or flame cutting

**Add to section 59-2.03C:**

04-19-13

**59-2.03C(3) Moisture-Cured Polyurethane Coating System**

Reserved

**59-2.03C(4) State Specification Paint Waterborne Coating System**

**59-2.03C(4)(a) General**

The State Specification PWB coating system for existing structural steel must comply with the requirements shown in the following table:

**State Specification PWB Coating System**

Surface	Description	State Specification PWB Coating	Dry film thickness (mils)
Surfaces cleaned to bare metal <sup>a</sup> :	1st undercoat	145	2-3
	2nd undercoat	146	2-3
	1st finish coat	171	1.5-3
	2nd finish coat	172	1.5-3
	Total thickness, all coats	--	7-12
Existing painted surfaces to be topcoated:	Undercoat	146	2-3
	1st finish coat	171	1.5-3
	2nd finish coat	172	1.5-3
	Total thickness, new coats	--	5-9

<sup>a</sup>Includes locations of spot blast cleaning

**59-2.03C(4)(b) Finish Coats**

Pressure rinse undercoated surfaces to receive finish coats. Perform pressure rinsing no sooner than 72 hours after the final application of undercoat.

The 1st finish coat must be applied within 48 hours of pressure rinsing.

Apply the 1st finish coat in 2 applications. The 1st application consists of a spray-applied mist application. Apply the 2nd application after the mist application has dried to a set-to-touch condition as determined using the procedure in section 7 of ASTM D 1640.

Apply the 2nd finish coat after the 1st finish coat has dried 12 hours unless authorized. You may apply the 2nd finish coat in a single application.

**Add to section 59-5.01:**

04-19-13

Where specified, prepare and paint sign structures under sections 59-2 and 59-3.

Instead of submitting proof of the certification complying with SSPC-QP 1, you may submit documentation with the painting quality work plan showing compliance with the requirements in section 3 of SSPC-QP 1.

Instead of submitting proof of the certification complying with SSPC-QP 2, you may submit documentation with the painting quality work plan showing compliance with the requirements in sections 4.2 through 4.4 of SSPC-QP 2, Category A.

Instead of submitting proof of the certification complying with AISC-420-10/SSPC-QP 3 (Enclosed Shop), you may submit documentation with the painting quality work plan showing compliance with the requirements in sections 5 through 18 of AISC-420-10/SSPC-QP3.













## 86 ELECTRICAL SYSTEMS

10-19-12

Replace section 86-2.06 with:

01-20-12

### 86-2.06 PULL BOXES

#### 86-2.06A General

##### 86-2.06A(1) Cover Marking

Marking must be clearly defined, uniform in depth, and parallel to either the long or short sides of the cover.

Marking letters must be 1 to 3 inches high.

Before galvanizing steel or cast iron cover, apply marking by one of the following methods:

1. Use cast iron strip at least 1/4 inch thick with letters raised a minimum of 1/16 inch. Fasten strip to cover with 1/4-inch flathead stainless steel machine bolts and nuts. Peen bolts after tightening.
2. Use sheet steel strip at least 0.027 inch thick with letters raised a minimum of 1/16 inch. Fasten strip to cover by spot welding, tack welding, or brazing, with 1/4-inch stainless steel rivets or 1/4-inch roundhead stainless steel machine bolts and nuts. Peen bolts after tightening.
3. Bead weld the letters on cover such that the letters are raised a minimum of 3/32 inch.

##### 86-2.06A(2) Installation and Use

Space pull boxes no more than 200 feet apart. You may install additional pull boxes to facilitate the work.

You may use a larger standard size pull box than that shown on the plans or specified.

A pull box in ground or sidewalk area must be installed as follows:

1. Embed bottom of the pull box in crushed rock.
2. Place a layer of roofing paper on the crushed rock.
3. Place grout over the layer of roofing paper. Grout must be 0.50 to 1 inch thick and sloped toward the drain hole.
4. Make a 1-inch drain hole in the center of the pull box through the grout and roofing paper.
5. Place grout between the pull box and the pull box extension, and around conduits.

The top of the pull box must be flush with the surrounding grade or the top of an adjacent curb, except in unpaved areas where the pull box is not immediately adjacent to and protected by a concrete foundation, pole, or other protective construction. Place the pull box 1-1/4 inches above the surrounding grade. Where practical, place a pull box shown in the vicinity of curbs or adjacent to a standard on the side of the foundation facing away from traffic. If a pull box is installed in a sidewalk area, adjust the depth of the pull box so that the top of the pull box is flush with the sidewalk.

Reconstruct the sump of an existing pull box if disturbed by your activities. Remove old grout and replace with new if the sump was grouted.

#### 86-2.06B Non-Traffic-Rated Pull Boxes

Reserved

#### 86-2.06C Traffic Pull Boxes

Traffic pull box and cover must comply with ASTM C857, "Standard Practice for Minimum Structural Design Loading for Underground Precast Concrete Utility Structures," for HS20-44 loading. You must be able to place the load anywhere on the box and cover for 1 minute without causing cracks or permanent deformations.

Frame must be anchored to the box with 1/4 by 2-1/4 inch concrete anchors. Four concrete anchors must be included for No. 3-1/2(T) pull box; one placed in each corner. Six concrete anchors must be included for No. 5(T) and No. 6(T) pull boxes; one placed in each corner and one near the middle of each of the longer sides.

Nuts must be zinc-plated carbon steel, vibration resistant, and have a wedge ramp at the root of the thread.

After installation of traffic pull box, install the steel cover and keep it bolted down when your activities are not in progress at the pull box. When the steel cover is placed for the final time, the cover and Z bar frame must be cleaned of debris and tightened securely.

Steel cover must be countersunk approximately 1/4 inch to accommodate the bolt head. When tightened, the bolt head must not exceed more than 1/8 inch above the top of the cover.

Concrete placed around and under traffic pull boxes must be minor concrete.

**Replace "project" in the 3rd paragraph of section 86-2.11A with:**

10-19-12

work

**Replace "Contract" in item 2 in the list in the 11th paragraph of section 86-2.11A with:**

10-19-12

work

AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA

### 88 GEOSYNTHETICS

01-18-13

**Replace the row for hydraulic bursting strength in the table in the 2nd paragraph of section 88-1.02B with:**

10-19-12

Puncture strength, lb min	ASTM D 6241	310
Trapezoid tearing strength, lb min	ASTM D 4533	56

**Replace the 3rd paragraph in section 88-1.02C with:**

10-19-12

Geocomposite wall drain must be from 0.25 to 2 inches thick.

**Replace the value for permittivity of woven fabric in the table in the 1st paragraph of section 88-1.02E with:**

01-20-12

0.05

**Replace the value for apparent size opening of nonwoven fabric in the table in the 1st paragraph of section 88-1.02E with:**

01-20-12

0.012

Replace the table in the 1st paragraph of section 88-1.02G with:

01-20-12

**Sediment Filter Bag**

Property	Test	Values	
		Woven	Nonwoven
Grab breaking load, lb, 1-inch grip min, in each direction	ASTM D 4632	200	250
Apparent elongation, percent min, in each direction	ASTM D 4632	10	50
Water flow rate, gal per minute/sq ft min and max average roll value	ASTM D 4491	100-200	75-200
Permittivity, sec <sup>-1</sup> min	ASTM D 4491	1.0	1.0
Apparent opening size, inches max average roll value	ASTM D 4751	0.023	0.012
Ultraviolet resistance, % min retained grab breaking load, 500 hr.	ASTM D 4355	70	70

Replace the table in the 1st paragraph of section 88-1.02H with:

01-20-12

**Temporary Cover**

Property	Test	Values	
		Woven	Nonwoven
Grab breaking load, lb, 1-inch grip min, in each direction	ASTM D 4632	200	200
Apparent elongation, percent min, in each direction	ASTM D 4632	15	50
Water flow rate, gal per minute/sq ft min and max average roll value	ASTM D 4491	4-10	80-120
Permittivity, sec <sup>-1</sup> min	ASTM D 4491	0.05	1.0
Apparent opening size, inches max average roll value	ASTM D 4751	0.023	0.012
Ultraviolet resistance, % min retained grab breaking load, 500 hr.	ASTM D 4355	70	70

Replace section 88-1.02P with:

01-18-13

**88-1.02P Biaxial Geogrid**

Geosynthetics used for biaxial geogrid must be a punched and drawn polypropylene material formed into an integrally formed biaxial grid. When tested under the referenced test methods, properties of biaxial geogrid must have the values shown in the following table:

**Biaxial Geogrid**

Property	Test	Value
Aperture size, inch <sup>a</sup> min and max	Calipered	0.8-1.3 x 1.0-1.6
Rib thickness, inch min	Calipered	0.04
Junction thickness, inch min	Calipered	0.150
Tensile strength, 2% strain, lb/ft <sup>a</sup> min	ASTM D 6637	410 x 620
Tensile strength at ultimate, lb/ft <sup>a</sup> min	ASTM D 6637	1,310 x 1,970
Ultraviolet resistance, percent min retained tensile strength, 500 hours	ASTM D 4355	100
Junction strength, lb/ft <sup>a</sup> min	ASTM D 7737	1,220 x 1,830
Overall flexural rigidity, mg-cm min	ASTM D 7748	750,000
Torsional rigidity at 20 cm-kg, mm-kg/deg <sup>b</sup> min	GRI:GG9	0.65

<sup>a</sup>Machine direction x cross direction

<sup>b</sup>Geosynthetic Research Institute, Test Method GG9, *Torsional Behavior of Bidirectional Geogrids When Subjected to In-Plane Rotation*

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**DIVISION X MATERIALS**

**90 CONCRETE**

08-05-11

**Replace the 3rd paragraph of section 90-1.01C(7) with:**

08-05-11

Submit weighmaster certificates in printed form or, if authorized, in electronic media. Present electronic media in a tab-delimited format on a CD or DVD. Captured data for the ingredients represented by each batch must be line feed carriage return and one line separate record with sufficient fields for the specified data.

**Replace the 3rd paragraph of section 90-3.01C(5) with:**

08-05-11

Production data must be input by hand into a pre-printed form or captured and printed by the proportioning device. Present electronic media containing recorded production data in a tab-delimited format on a CD or DVD. Each capture of production data must be followed by a line feed carriage return with sufficient fields for the specified data.

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